

Hawai'i Convention Center

Final Documents

DESIGN BUILDER :

NORDIC / PCL
A Joint Venture
Honolulu, HI

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LIFE SAFETY :

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VERTICAL TRANSPORTATION :

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San Francisco, CA

Structural

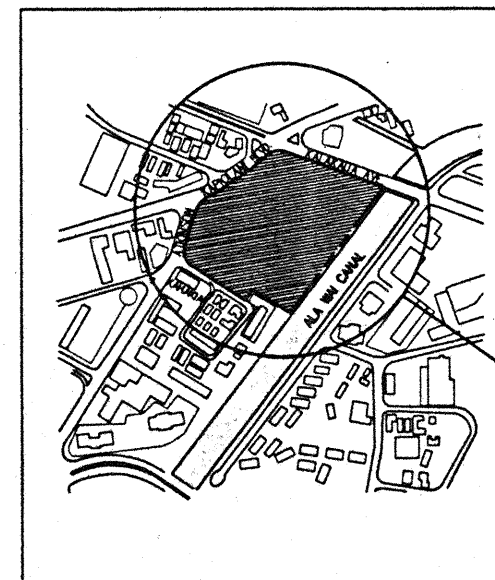
CODE INFORMATION

SITE INFORMATION

TAX MAP KEY: 2-3-35:1
LAND AREA OF PROJECT: 9.7 ACRES
CITY AND COUNTY OF HONOLULU ZONING DESIGNATION: BMX-3

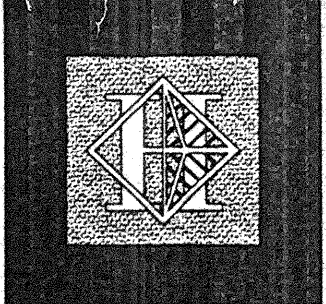
BUILDING INFORMATION

BUILDING CODE: UNIFORM BUILDING CODE
1991 EDITION AND CHAPTER 5 (AMENDMENTS)
TYPE OF CONSTRUCTION: TYPE 1 - FIRE-RESISTIVE
BUILDING HEIGHT: TOP FLOOR HEIGHT = 70'-6"
NUMBER OF STORIES: 4
PARKING: 800 SPACES
OCCUPANCY TYPES: A-21, A-3, B-2, & B-3



VICINITY MAP

PROJECT LOCATION



Hawai'i Convention Center

Convention Center Authority
Honolulu, HI

Wimberly Allison Tong & Goo, Inc.
Honolulu, HI

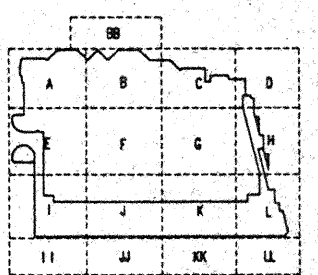
In association with

Loschky Marquardt & Nesholm
Seattle, WA

NORDIC / PCL
A Joint Venture
Honolulu, HI

Consultants

This work was prepared by me or under my supervision and construction of this project will be under my observation.



Revisions

No.	Date	By	Description
12/15/95			FINAL DOCUMENTS
12/17/95			OCTOBER COORDINATION
9/15/95			BUILDING PERMIT SET

Sheet Title

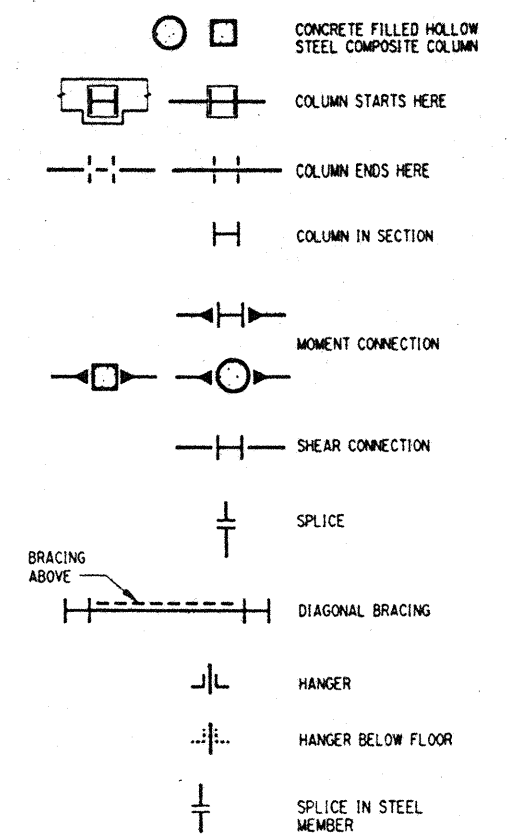
COVER SHEET:
STRUCTURAL

Designed	GL / KNPC
Drawn	
Checked	SRK / EW
Date	8 / 14 / 95
Job No.	93214
Sheet No.	

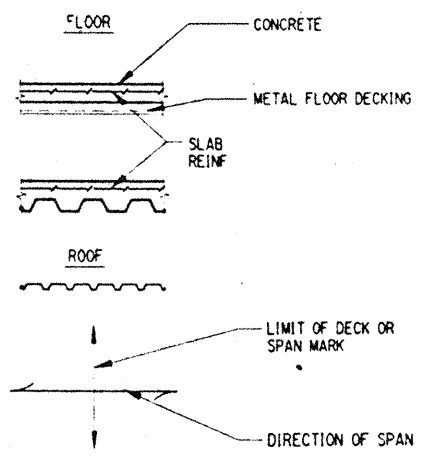
ABBREVIATIONS

AB ANCHOR BOLT	KIP or K 1,000 POUNDS
ACI AMERICAN CONCRETE INSTITUTE	KO KNOCK-OUT
ADD ADDITIONAL	KSI KIPS PER SQUARE INCH
ADJ ADJACENT	LAB LABORATORY
AGGR AGGREGATE	LB POUND
AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LF LINEAL FOOT
ALT ALTERNATE	LIN LINEAL: LINEAR
ALUM ALUMINUM	LLBB LONG LEG(S) BACK-TO-BACK
ANSI AMERICAN NATIONAL STANDARDS INSTITUTE	LLV LONG LEG(S) VERTICAL
APA AMERICAN PLYWOOD ASSOCIATION	LP LOW POINT
APPD APPROVED	LSL LONG SLOTTED HOLES
APPROX APPROXIMATE	LT WT LIGHTWEIGHT
ARCH ARCHITECTURAL: ARCHITECT	M MISC SHAPE
ASPT ASPHALT	MACH MACHINE
ASSY ASSEMBLY	MAS MASONRY
ASTM AMERICAN SOCIETY FOR TESTING AND MATERIALS	MATL MATERIAL
AWG AMERICAN WELDING SOCIETY	MAX MAXIMUM
B AND	MB MACHINE BOLT
L ANGLE	MC MISCELLANEOUS CHANNEL
BD BOARD	MECH MECHANICAL
BLDG BUILDING	MEMB MEMBRANE
BLK BLOCK: BLOCKING	MFR MANUFACTURE
BM BEAM	MIN MINIMUM: MINUTE
BOT BOTTOM	MISC MISCELLANEOUS
BRG BEARING	N NORTH
BRKT BRACKET	NF NEAR FACE
BT BENT	NFPA NATIONAL FOREST PRODUCTS ASSOC.
BTW BETWEEN	NIC NOT IN CONTRACT
C STANDARD CHANNEL	NO. or * NUMBER
CC CENTER TO CENTER	NS NEAR SIDE
CG CENTER OF GRAVITY	NTS NOT TO SCALE
CIP CAST-IN-PLACE	OC ON CENTER
CJ CONSTRUCTION JOINT: CONTROL JOINT	OD OUTSIDE DIAMETER
CL CENTERLINE	OF OUTSIDE FACE
CLR CLEARANCE: CLEAR	OPNG OPENING
CMU CONCRETE MASONRY UNIT	OVS OVER SIZED HOLES
COL COLUMN	PC PRECAST
COMP COMPRESSION	PERP PERPENDICULAR
CONC CONCRETE	PIPEX PIPE - EXTRA STRONG
CONN CONNECTION: CONNECT	PIPEXX PIPE - DOUBLE EXTRA STRONG
CONST CONSTRUCTION	PL PLATE
CONT CONTINUE: CONTINUOUS	PLND PLYWOOD
CONTR CONTRACTOR	PP PARTIAL PENETRATION
CORR CORRUGATED	PREFAB PREFABRICATED
CP COMPLETE PENETRATION WELD-ULTRASONIC TEST	PS PRESTRESSED
CTR CENTER	PSF POUNDS PER SQUARE FOOT
CTSK COUNTERSINK: COUNTERSUNK	PSI POUNDS PER SQUARE INCH
CU FT CUBIC FOOT	PVC POLYVINYL CHLORIDE
CU IN CUBIC INCH	* POUND
CY CUBIC YARD	
d PENNY (NAILS)	
DBL DOUBLE	
DEPT DEPARTMENT	
DET DETAIL	
DIA OR # DIAMETER	
DIAG DIAGONAL	
DIAPH DIAPHRAGM	
DICA DRILLED-IN CONCRETE ANCHOR	
DIM DIMENSION	
DISC DISCONTINUED	
DN DOWN	
DO DITTO	
DWG DRAWING	
DWL DWELS	
EA EACH	
EE EACH END	
EF EACH FACE	
EJ EXPANSION JOINT	
EL ELEVATION	
ELEC ELECTRICAL	
ELEV ELEVATOR	
ENGR ENGINEER	
EQ EARTHQUAKE	
EQUIP EQUIPMENT	
ETC ET CETERA	
EW EACH WAY	
EXIST EXISTING	
EXT EXTERIOR	
EXTD EXTEND or EXTENDED	
FB FLAT BAR	
FD FLOOR DRAIN	
FDN FOUNDATION	
FF FAR FACE	
FIN FINISH	
FL FLOOR: FLOOR LINE	
FLG FLANGE	
FOS FACE OF STUD	
FP FIREPROOF: FULL PENETRATION	
FT FOOT: FEET	
FTG FOOTING	
GA GAUGE	
GALV GALVANIZED	
GL GLUE LAMINATED	
GRND GROUND	
H or HORIZ HORIZONTAL	
HOSP HOSPITAL	
HP HP SHAPES: HIGH POINT	
HS HIGH STRENGTH	
HT HEIGHT	
ID INSIDE DIAMETER	
I.E. THAT IS	
IF INSIDE FACE	
IN INCH	
INCL INCLUDE	
INFO INFORMATION	
J JOIST	
JT JOINT	

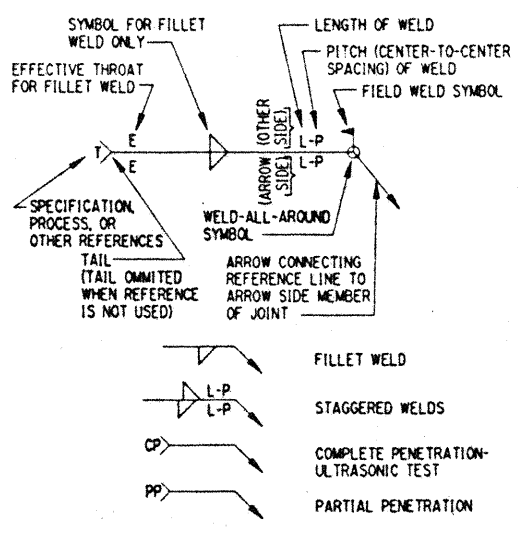
SYMBOLS



STEEL COLUMNS, BEAMS, HANGERS & BRACING



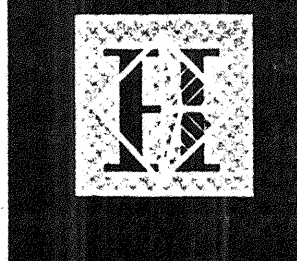
RIBBED FLOOR



WELDING SYMBOLS

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S2.2.B	INTERMEDIATE LEVEL FRAMING PLAN SECTOR B		
S2.2.C	INTERMEDIATE LEVEL FRAMING PLAN SECTOR C		
S2.2.D	INTERMEDIATE LEVEL FRAMING PLAN SECTOR D		
S2.2.E	INTERMEDIATE LEVEL FRAMING PLAN SECTOR E		
S2.2.F	INTERMEDIATE LEVEL FRAMING PLAN SECTOR F		
S2.2.G	INTERMEDIATE LEVEL FRAMING PLAN SECTOR G		
S2.2.H	INTERMEDIATE LEVEL FRAMING PLAN SECTOR H		
S2.2.I	INTERMEDIATE LEVEL FRAMING PLAN SECTOR I		
S2.2.J	INTERMEDIATE LEVEL FRAMING PLAN SECTOR J		
S2.2.K	INTERMEDIATE LEVEL FRAMING PLAN SECTOR K		
S2.2.L	INTERMEDIATE LEVEL FRAMING PLAN SECTOR L		
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S2.3.G	PARKING FRAMING PLAN SECTOR G		
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S2.3.I	PARKING FRAMING PLAN SECTOR I		
S2.3.J	PARKING FRAMING PLAN SECTOR J		
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S2.4.G	MEETING FRAMING PLAN SECTOR G		
S2.4.H	MEETING FRAMING PLAN SECTOR H		
S2.4.I	MEETING FRAMING PLAN SECTOR I		
S2.4.J	MEETING FRAMING PLAN SECTOR J		
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S2.5.F	BALLROOM/TERRACE FRAMING PLAN SECTOR F		
S2.5.G	BALLROOM/TERRACE FRAMING PLAN SECTOR G		
S2.5.H	BALLROOM/TERRACE FRAMING PLAN SECTOR H		
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S2.5.J	BALLROOM/TERRACE FRAMING PLAN SECTOR J		
S2.5.K	BALLROOM/TERRACE FRAMING PLAN SECTOR K		
S2.5.L	BALLROOM/TERRACE FRAMING PLAN SECTOR L		
S2.5.BB	BALLROOM/TERRACE FRAMING PLAN SECTOR BB		



Hawaii Convention Center

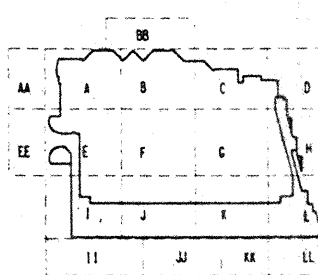
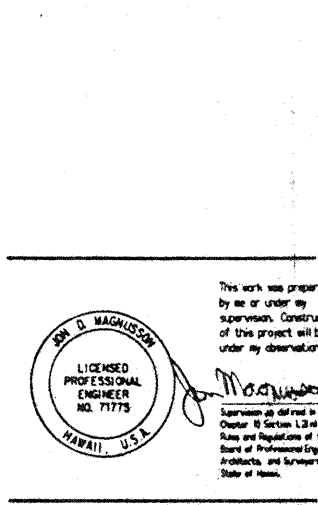
Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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Revisions

12-15-95	FINAL DOCUMENT
11-7-95	
9-15-95	BUILDING PERMIT SET
8-31-95	
8-4-95	
6-19-95	
5-8-95	ADDENDUM #3
4-17-95	ISSUED FOR FOUNDATION PERMIT

No. Date By Description

SYMBOLS, INDEX & ABBREVIATIONS

Designed: BJM
Drawn: DMS
Checked: JDM
Date: 4 / 12 / 95
Job No. 93036, Q2, 9
Sheet No.

Mon Dec 4 13:45:28 1995

GENERAL NOTES

GENERAL:

ALL TYPICAL DETAILS AND NOTES SHOWN ON DRAWINGS SHALL APPLY, UNLESS NOTED OTHERWISE.

BUILDING CODE:

ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. THE PUBLICATIONS LISTED BELOW ARE THE GOVERNING CODES AND STANDARDS AND ARE REFERENCED BY THE BASIC DESIGNATION IN THE CASE OF CONFLICTING REQUIREMENTS, THE BUILDING CODE SHALL GOVERN.

APPLICABLE CODES AND STANDARDS:

Table listing applicable codes and standards including Building Code, ACI, AISI, AWS D1.1, AWS D1.3, AWS A2.4, AISI, and ASTM.

CONCRETE:

MIXING AND PLACING OF ALL CONCRETE AND SELECTION OF MATERIALS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. PROPORTIONS OF AGGREGATE TO CEMENT SHALL BE SUCH AS TO PRODUCE A DENSE, WORKABLE MIX WHICH CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFACE WATER.

MAXIMUM SIZE OF AGGREGATE SHALL BE 1-1/2 INCHES, BUT MAXIMUM SIZE OF AGGREGATE SHALL NOT BE MORE THAN THREE QUARTERS OF THE CLEAR DISTANCE BETWEEN REINFORCING BARS.

MIX DESIGNS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR ACCEPTANCE PRIOR TO USE. MAXIMUM WATER-CEMENT RATIO AND SLUMP SHALL BE AS FOLLOWS FOR VARIOUS CONCRETE STRENGTHS (f'c) BASED ON STANDARD 28-DAY CYLINDER TESTS.

Table showing water-cement ratio by weight and maximum slump for various concrete strengths (f'c).

FORM CAMBER:

CONCRETE FORMWORK SHALL HAVE BUILT-IN CAMBER TO COMPENSATE FOR FORM SAG UNDER WET CONCRETE LOAD. IN ADDITION TO STRUCTURAL CAMBER, ONE-WAY SLABS SHALL BE CAMBERED 1/1000 OF THE SPAN, UNLESS OTHERWISE NOTED ON THE DRAWINGS.

CONSTRUCTION JOINTS:

ALL CONSTRUCTION JOINTS IN WALLS SHALL BE KEVED IN ACCORDANCE WITH THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS OR, AT THE CONTRACTOR'S OPTION, SHALL BE INTENTIONALLY ROUGHENED CONSTRUCTION JOINTS IN ACCORDANCE WITH THE FOLLOWING:

ALL CONSTRUCTION JOINTS FOR SLABS ON DECK SHALL BE IN ACCORDANCE WITH THE TYPICAL SLAB ON DECK CONSTRUCTION JOINT DETAIL SHOWN ON THE STRUCTURAL DRAWINGS.

SLEEVES:

EXCEPT AS DETAILED ON STRUCTURAL DRAWINGS, NO BEAMS, OR GIRDERS SHALL BE SLEEVED FOR PIPING OR DUCTS, UNLESS APPROVED BY THE ARCHITECT/ENGINEER.

ELECTRICAL CONDUIT:

IF CONDUIT IS TO RUN IN SLABS, THE CONDUIT SHALL BE RUN AT MID-DEPTH OF SLAB AND CONDUIT SIZE SHALL NOT EXCEED 33 PERCENT OF THE SLAB DEPTH. NO CONDUIT SHALL BE PLACED IN SLABS WITH ACTUAL CONCRETE THICKNESS LESS THAN 3 INCHES.

REINFORCING STEEL:

ALL REINFORCING SHALL BE NEW BILLET STOCK ASTM A 615, GRADE 60. REINFORCING STEEL TO BE WELDED TO STRUCT STEEL SHALL BE ASTM A706. BARS SHALL BE SECURELY TIED IN PLACE WITH #16 DOUBLE-ANNEALED IRON WIRE.

NO BENDING OR STRAIGHTENING OF REINFORCEMENT WILL BE PERMITTED AFTER PARTIAL EMBEDMENT IN CONCRETE. FOOTING AND PIER DOWELS MUST BE PROTECTED FROM DAMAGE BY EQUIPMENT.

NO REINFORCING BARS SHALL BE WELDED, EXCEPT AS FOLLOWS: CERTAIN VERTICAL COLUMN BARS AND ALL #14 AND #18 BARS SHALL HAVE FULL-PENETRATION BUTT WELDS (AS NOTED ON THE STRUCTURAL DRAWINGS).

CAST-IN-PLACE CONCRETE COVER OVER REINFORCING STEEL, UNLESS OTHERWISE NOTED, SHALL BE AS FOLLOWS:

- List of concrete cover requirements for different bar sizes and locations (interior face, slabs, joists, beams, columns, etc.).

WELDED WIRE FABRIC:

WELDED WIRE FABRIC (WWF) SHALL BE ELECTRICALLY WELDED AND CONFORM TO ASTM A 185. AN 8-INCH MINIMUM LAP SHALL BE PROVIDED FOR SIDE AND END LAPS.

CURTAIN WALL INSTALLATION

THE CURTAIN WALL CONNECTIONS SHALL ACCOUNT FOR CONSTRUCTION TOLERANCES, COLUMN SHORTENING, AND BEAM DEFLECTIONS. IN ADDITION, THE CURTAIN WALL DESIGN SHALL ACCOMMODATE A TYPICAL VERTICAL MOVEMENT AT EACH FLOOR OF 3/4 INCH DUE TO VARIABLE LIVE LOADING.

THE EXTERIOR BEAMS HAVE BEEN DESIGNED FOR UNIFORM LOADS REPRESENTING CURTAIN WALL REACTIONS. CANTILEVERED SLABS OVER EDGE BEAMS WERE DESIGNED WITH THE SAME WALL LOAD APPLIED AT THE FREE EDGE OF THE SLAB.

WALL CLADDING ATTACHMENTS SHALL NOT APPLY ANY LATERAL LOADS TO THE BOTTOM FLANGE OF THE BEAMS. IF ATTACHMENT IS MADE TO THE BOTTOM OF BEAMS, ADDITIONAL INCLINED STRUTS BRACING THE BOTTOM FLANGE OR OTHER EQUIVALENT MEANS TO COUNTERACT THIS FORCE SHALL BE PROVIDED BY THE WALL CLADDING SUPPLIER.

DRILLED-IN CONCRETE ANCHORS (DICA):

ACCEPTABLE DRILLED-IN CONCRETE ANCHORS, OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE DRAWINGS, SHALL BE AS FOLLOWS: HILTI "KWIK-BOLT-TIT" (ICBO #4627), "WEJ-TIT ANCHOR BOLT" (ICBO #1821) OR "ITW RAMSEY/RED HEAD TRUBOLT WEDGE ANCHORS" (ICBO #3372) OR APPROVED EQUAL.

NONSHRINK GROUT FOR BASE PLATES:

GROUT SHALL BE AN APPROVED NONSHRINK CEMENTITIOUS GROUT CONTAINING NATURAL AGGREGATES DELIVERED TO THE JOB SITE IN FACTORY PREPACKAGED CONTAINERS REQUIRING ONLY THE ADDITION OF WATER.

EPOXY GROUT:

"EPOXY GROUT" SHALL BE A 100 PERCENT SOLIDS PASTE DESIGNED FOR STRUCTURAL BONDING OF DOWELS IN HARDENED CONCRETE. WHEN MIXED AND CURED ACCORDING TO THE MANUFACTURER'S WRITTEN INSTRUCTIONS, EPOXY MATERIALS SHALL PRODUCE THE FOLLOWING PROPERTIES:

"EPOXY GROUT" SHALL BE USED FOR ALL DRILLED AND GROUTED REINFORCING BARS OR BOLTS UNLESS NOTED OTHERWISE. EPOXY GROUT SHALL BE "CONCRETE 1438" AS MANUFACTURED BY ADHESIVE ENGINEERING COMPANY, "SIKADUR 31 HI MOD GEL" AS MANUFACTURED BY SIKA CORPORATION, OR APPROVED EQUAL.

REINFORCING MATERIAL SHALL BE MIXED, APPLIED AND CURED IN STRICT ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. ALL PLACEMENT AND CURING SHALL BE CONDUCTED WITH CONCRETE AND AIR TEMPERATURES ABOVE 50 DEGREES. APPLY EPOXY MATERIAL ONLY TO CLEAN, DRY CONCRETE.

GROUTING REINFORCING BARS AND BOLTS:

REINFORCING BARS AND BOLTS EMBEDDED IN EXISTING CONCRETE SHALL BE GROUTED INTO HOLES DRILLED INTO THE EXISTING CONCRETE. HILTI HIT ADHESIVE ANCHOR (ICBO #4419) OR EQUAL SHALL BE USED. PROVIDE POSITIVE PROTECTION SO DOWELS ARE NOT DISTURBED DURING CURING.

STRUCTURAL STEEL:

ALL STEEL SHALL CONFORM TO THE FOLLOWING:

ALL STEEL, UNLESS OTHERWISE NOTED * ASTM A572, GRADE 50, A441, FY * 50 KSI OR A588 FY * 50 KSI

MATERIAL CALLED OUT ON PLANS AS (FY * 36) ASTM A36, FY * 36 KSI

ALL PLATE AND ANGLES, UNLESS OTHERWISE NOTED, ARE ASTM A36, FY * 36 KSI

SQUARE OR RECTANGULAR STRUCTURAL TUBE * ASTM A500, GRADE B, FY * 46 KSI

STEEL PIPE DIAMETER LESS THAN OR EQUAL TO 12 INCHES * ASTM A53, TYPE E OR S, GRADE B, FY * 35 KSI

ALL WORK SHALL BE IN ACCORDANCE WITH THE AISC SPECIFICATION. SHOP DRAWINGS SHALL BE SUBMITTED AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE COMMENCING FABRICATION.

ASTM A6 GROUP A AND S ROLLED SHAPES AND PLATES EXCEEDING 2-INCH THICKNESS USED FOR BUILT-UP MEMBERS REQUIRE SHARP V-NOTCH TESTING IN ACCORDANCE WITH ASTM A6 SUPPLEMENTARY REQUIREMENTS WHERE FULL-PENETRATION WELDS ARE CALLED OUT AT EITHER END OF THE MEMBER ON THE DRAWINGS.

SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. ALL WELDING SHALL BE DONE BY AWS CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS D1.1. WELDS SHOWN ON THE DRAWINGS ARE THE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES, BASED ON PLATE THICKNESS.

STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 7/8-INCH DIAMETER A325 BOLTS IN SINGLE SHEAR. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED AND INSPECTED IN ACCORDANCE WITH THE AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF THE SELECTION OF OPTIONAL DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO: ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS; WELDING PROCEDURES; REQUIRED ROOT OPENINGS; ROOT FACE DIMENSIONS; GROOVE ANGLES; BACKING BARS; COPELS; SURFACE ROUGHNESS VALUES; AND TAPERS OF UNEQUAL PARTS.

SHEAR CONNECTOR STUDS:

ALL SHEAR CONNECTOR STUDS SHALL BE 3/4 INCH IN DIAMETER UNLESS OTHERWISE NOTED. ACCEPTABLE TYPES SHALL BE "TRI-WELD" (ICBO #3741) OR NELSON (ICBO #2614). SHEAR CONNECTOR STUDS SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH WELDED EQUIPMENT RECOMMENDED BY MANUFACTURER OF STUDS.

STEEL COMPOSITE DECK:

THE STEEL DECK SHALL BE OF DEPTH SHOWN ON THE STRUCTURAL DRAWINGS. GAGE OF DECK SHALL BE DETERMINED BY THE CONTRACTOR BASED ON THE SPAN CONDITIONS, SHORING REQUIREMENTS, AND LOADS SHOWN ON THE DRAWINGS, INCLUDING LOAD DIAGRAMS, AND NOTES.

CONCRETE BONDING-TYPE UNITS SHALL BE FORMED WITH DEFORMATIONS TO PROVIDE AN INTERLOCK BETWEEN THE CONCRETE AND STEEL, UNLESS SHOWN OTHERWISE.

STEEL DECK TYPES SHALL BE VERCO TYPE W, ASC PACIFIC TYPE W, H.H. ROBERTSON TYPE QL-99 OR APPROVED EQUAL.

STEEL ROOF DECK:

THE STEEL DECK SHALL BE OF DEPTH AND GAGE SHOWN ON THE STRUCTURAL DRAWINGS. THE AISC SPECIFICATIONS SHALL GOVERN THE DESIGN OF ALL DECK UNITS.

STEEL DECK TYPES SHALL BE VERCO TYPE B, ASC PACIFIC TYPE B VERCO TYPE N24, ASC PACIFIC TYPE N24 OR APPROVED EQUAL. SEE DRAWINGS FOR LOCATIONS.

STEEL STAIR:

REFER TO PROJECT SPECIFICATIONS, ARCHITECTURAL DRAWINGS AND STRUCTURAL DRAWINGS TO DETERMINE EXTENT OF STAIRS AND WHAT SHALL BE PROVIDED BY THE STAIR MANUFACTURER.

COMPOSITE FLOOR SYSTEM:

FLOOR SLABS SHALL BE CONSTRUCTED TO THE ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE SPECIFICATIONS FOR FLOOR TOLERANCES.

MASONRY:

CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE BUILDING CODE. ALL HOLLOW LOAD BEARING CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C 90, GRADE M-1.

REQUIRED MORTAR PROPORTIONS BY VOLUME

Table showing required mortar proportions by volume for different masonry types.

ANCHOR BOLTS:

ANCHOR BOLTS SHALL BE ASTM A307 GRADE A STANDARD HEX HEAD FURNISHED WITH HEAVY HEX NUTS AND LOCK WASHERS. SIZES EXCEEDING A307 SHALL BE ASTM A36 ROD THREADED UNC-2A FURNISHED WITH DOUBLE HEAVY HEX NUTS.

FIREPROOFING STRUCTURAL STEEL:

REFER TO ARCHITECTURAL PLANS AND SPECIFICATIONS FOR MINIMUM HOURLY VALUES OF STEEL FIRE PROTECTION FOR DETERMINING THE THICKNESS OF SPRAY APPLIED FIREPROOFING.

STRUCTURAL DESIGN DATA:

LIVE LOADS:

Table listing live loads for various areas: Lobby, Exhibition Hall, Parking, Meeting Rooms, etc.

WIND LOADS: WIND PRESSURE SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. BASIC WIND SPEED * 80 MPH, EXPOSURE C, IMPORTANCE FACTOR OF 1.0.

SEISMIC LOADS: SEISMIC LOADING SHALL BE IN ACCORDANCE WITH THE BUILDING CODE (ZONE 2A).

V = 0.048 W, Z = 0.15, I = 1.0, S = 1.5, C = 2.55, R = 8.0

MOMENTS, SHEARS, AND ROTATIONAL FORCES ARE DELIVERED TO THE FOUNDATION BY THE STEEL BRACED FRAMES IN PROPORTION TO THEIR ABILITY TO RESIST LATERAL DEFORMATION.

PILING:

ALL PILES SHALL BE 16-1/2 INCH OCTAGONAL OR 12 INCH SQUARE (OPTIONAL 55 TON PILE) PRESTRESSED CONCRETE. DESIGN LOADS ON EACH PILE EQUALS 55 OR 150 TONS (SEE DRAWINGS).

MISCELLANEOUS:

REFER TO ALL DISCIPLINES' PLANS FOR SIZE AND LOCATION OF CURBS, EQUIPMENT PADS, WALL AND FLOOR OPENINGS, ARCHITECTURAL TREATMENT, AND DIMENSIONS NOT SHOWN.

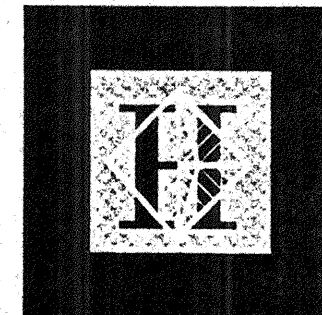
THE STEEL BY ITSELF IS A NON-Self-SUPPORTING STEEL FRAME.

LATERAL FORCES DUE TO WIND, EARTHQUAKE, SOIL, OR FOR STABILITY ARE CARRIED BY THE ROOF AND FLOOR DIAPHRAGMS TO THE LATERAL RESISTING SYSTEM.

SPECIAL INSPECTION:

THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION PER UBC SECTION 306. THESE INSPECTIONS SHALL BE PERFORMED BY A SPECIAL INSPECTOR CERTIFIED BY THE CITY OF HONOLULU TO PERFORM THE TYPES OF INSPECTIONS SPECIFIED.

Table with 3 columns: NO., ITEM, and DESCRIPTION. Lists 14 items requiring special inspection.



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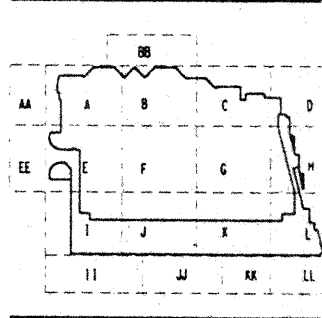
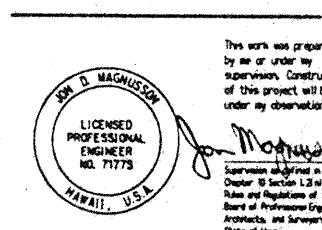
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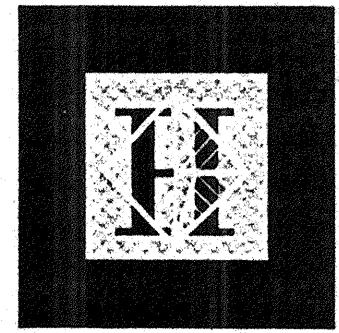
Revisions table with columns for Date, Description, and initials.

GENERAL NOTES

Designed BJM Drawn DMS Checked JDM Date 4 / 12 / 95 Job No. 93036.02.9 Sheet No.

DESIGN UNIFORM
SUPERIMPOSED LOADS

MARK	SUPER-IMPOSED DEAD LOAD (PSF)	LIVE LOAD (PSF)	REMARKS
A	17	100	LOBBY/EXIT/CORRIDOR
B	13	100	LOBBY/EXIT/CORRIDOR
C	17	50	RESTROOMS
D	33	50	OFFICE
E	13	150	MTG RMS/BALLROOM/STORAGE
F	17	125	KITCHEN/KITCHEN CORRIDOR/LIGHT STORAGE
G	13	125	MECHANICAL/ELECTRICAL/SHOPS
H	13	50	PARKING
J	50.5	50	PARKING(W/ TOPPING SLAB)
K	59.5	150	STORAGE(W/ TOPPING SLAB)
L	13	50	RESTROOMS
M	0	350 OR HS20-44	EXHIBITION HALL
N	253	20	PLANTER PLUS ACTUAL TREE WEIGHTS
P	63	100	TERRACE
Q	36	100	PREFUNCTION
R	63	50	TERRACE RESTROOMS
S	11	20	ROOF
T	50.5	150	BALLROOM(W/ TOPPING SLAB)
V	20	20	TRELLIS/SKYLIGHT
W	50.5	125	KITCHEN/KITCHEN CORRIDOR/LIGHT STRG(W/TOPPING SLAB)
X	34	20	BALLROOM ROOF
Y	15	20	CANOPY SKYLIGHT
Z	0	20	CANOPY
AA	50.5	100	LOBBY/EXIT/CORRIDOR(W/ TOPPING SLAB)
BB	99(STAIRS) 268(PLANTER) 100(STAIRS) 20(PLANTER)		GRAND STAIRWAY
CC	75	21.4	MAIN TRELLIS
DD	23	20	ROOF
EE	113	20	POOL
FF	97	100	NORTHWEST THEATRE
GG	121	100	SOUTHEAST THEATRE



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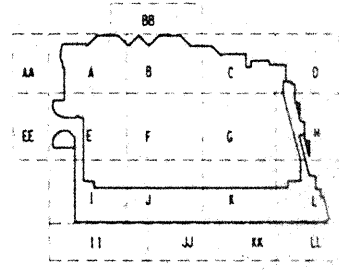
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This work was prepared by me or under my supervision. Construction of this project will be under my observation.

SEAL OF A LICENSED PROFESSIONAL ENGINEER
NO. 71775
HAWAII, U.S.A.



Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENT
9-15-95			BUILDING PERMIT SET
8-31-95			
8-4-95			
4-17-95			ISSUED FOR FOUNDATION PERMIT

LOAD MAPS

Designed_BJM
Drawn_DMS
Checked_JDM
Date 4 / 12 / 95
Job No. 93036_Q2_9
Sheet No.

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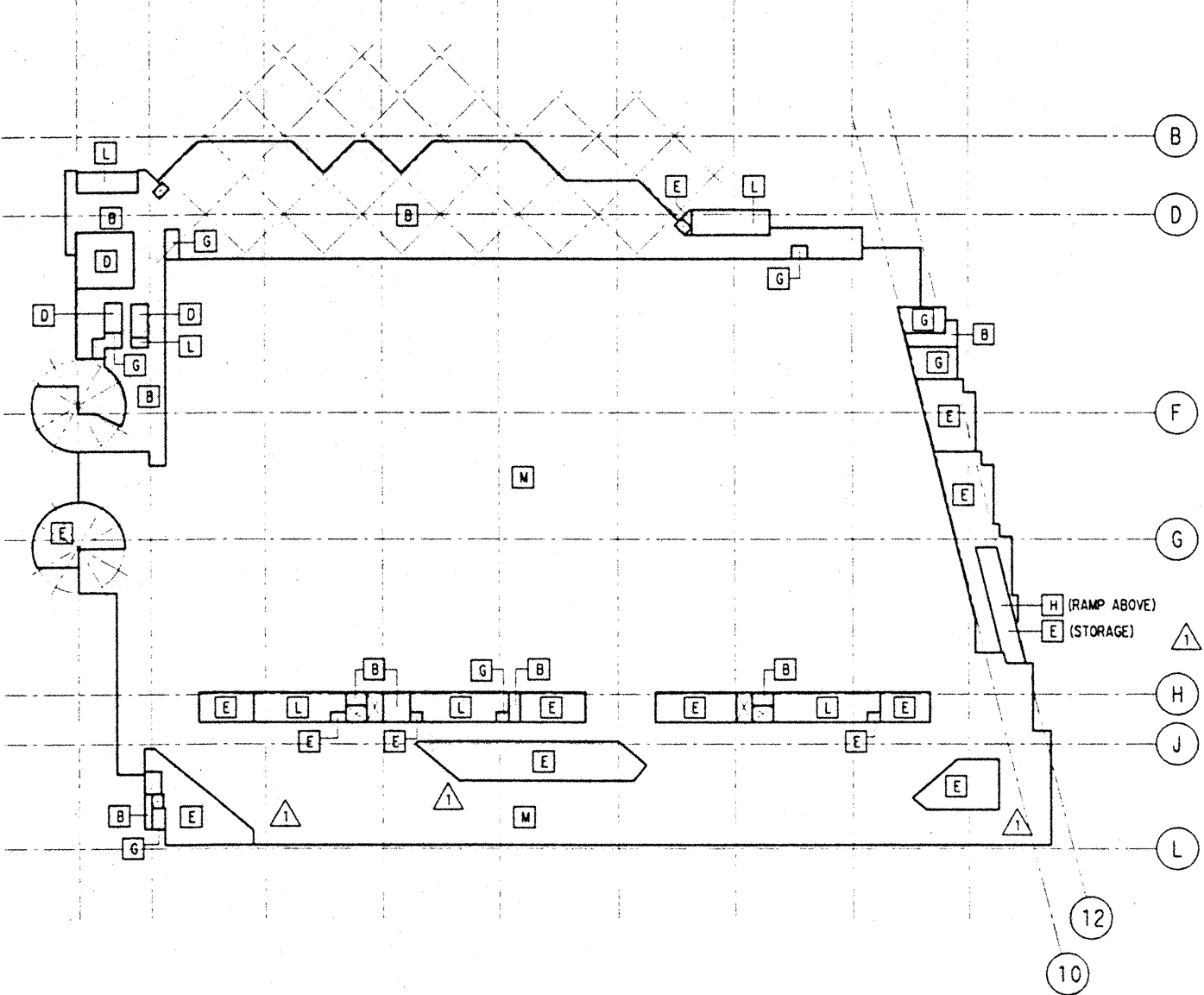
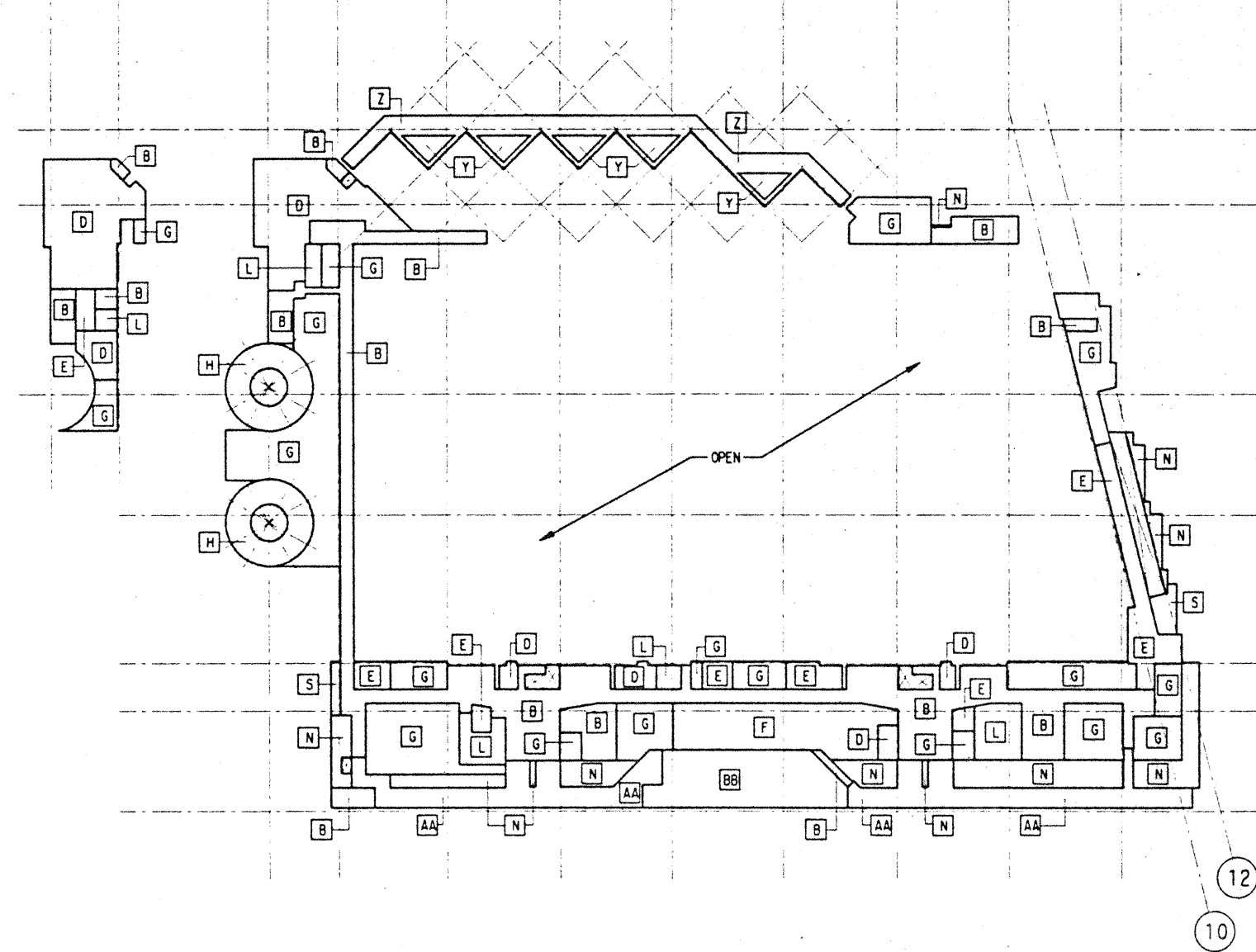


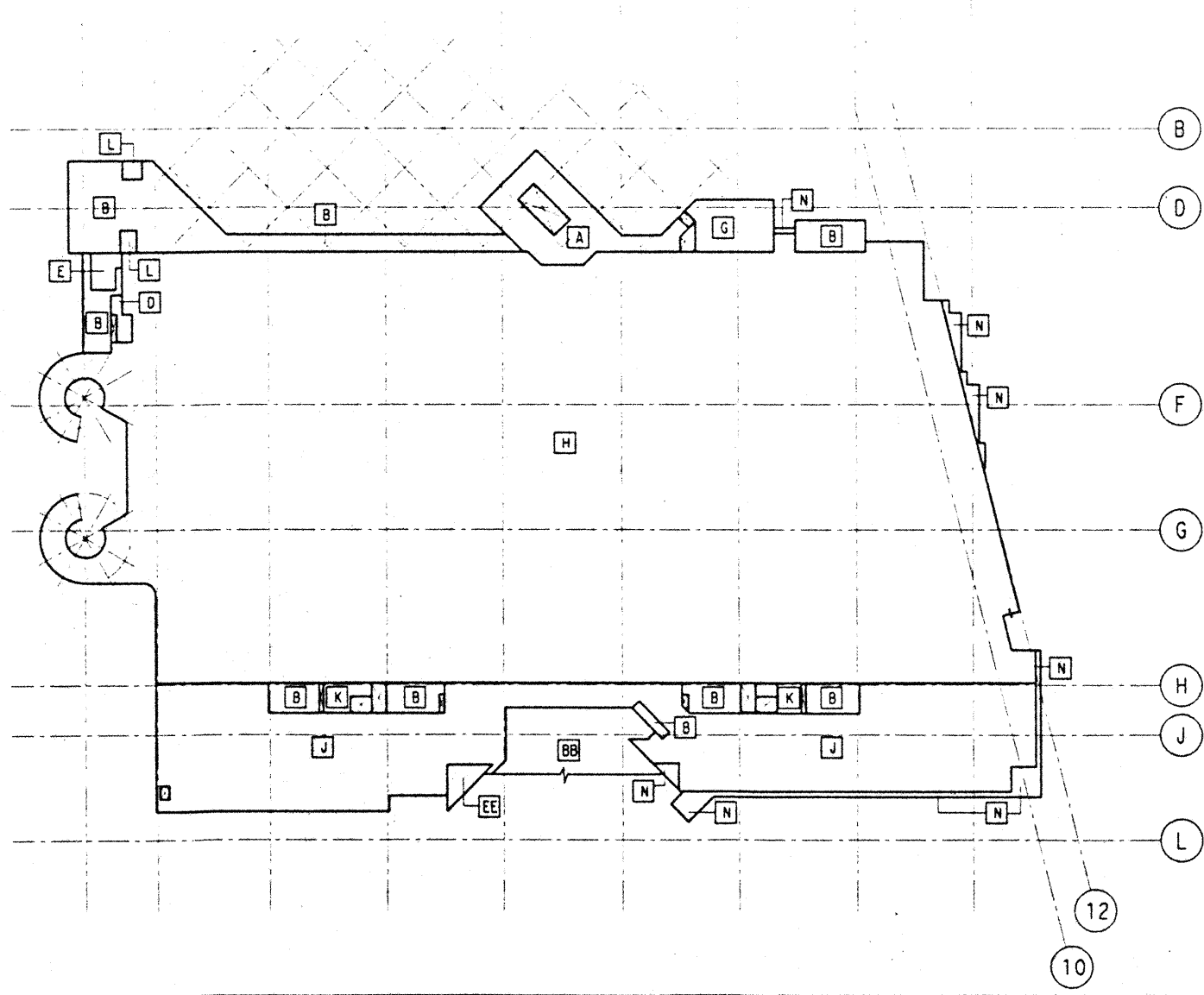
EXHIBIT HALL

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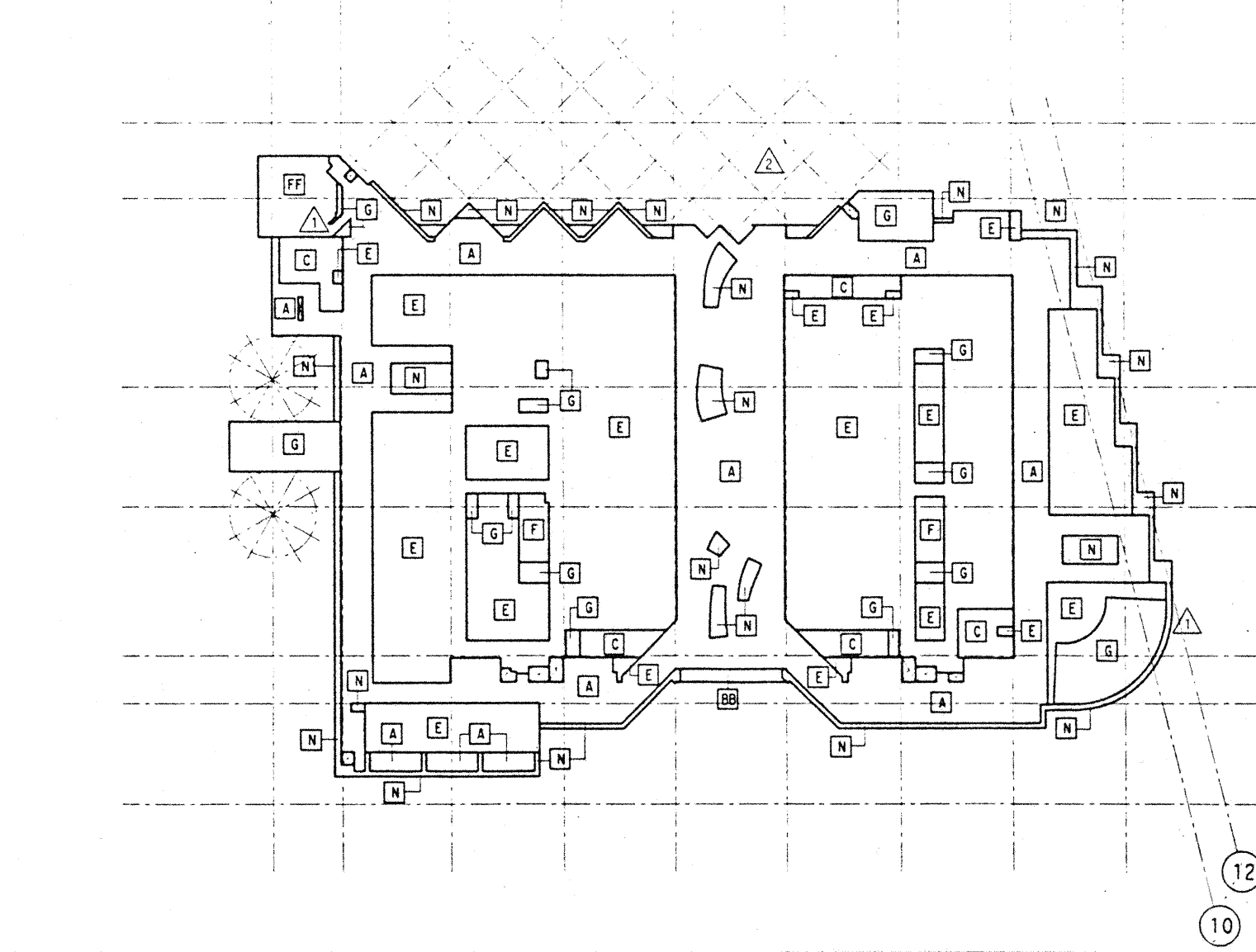
INTERMEDIATE

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PARKING

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MEETING

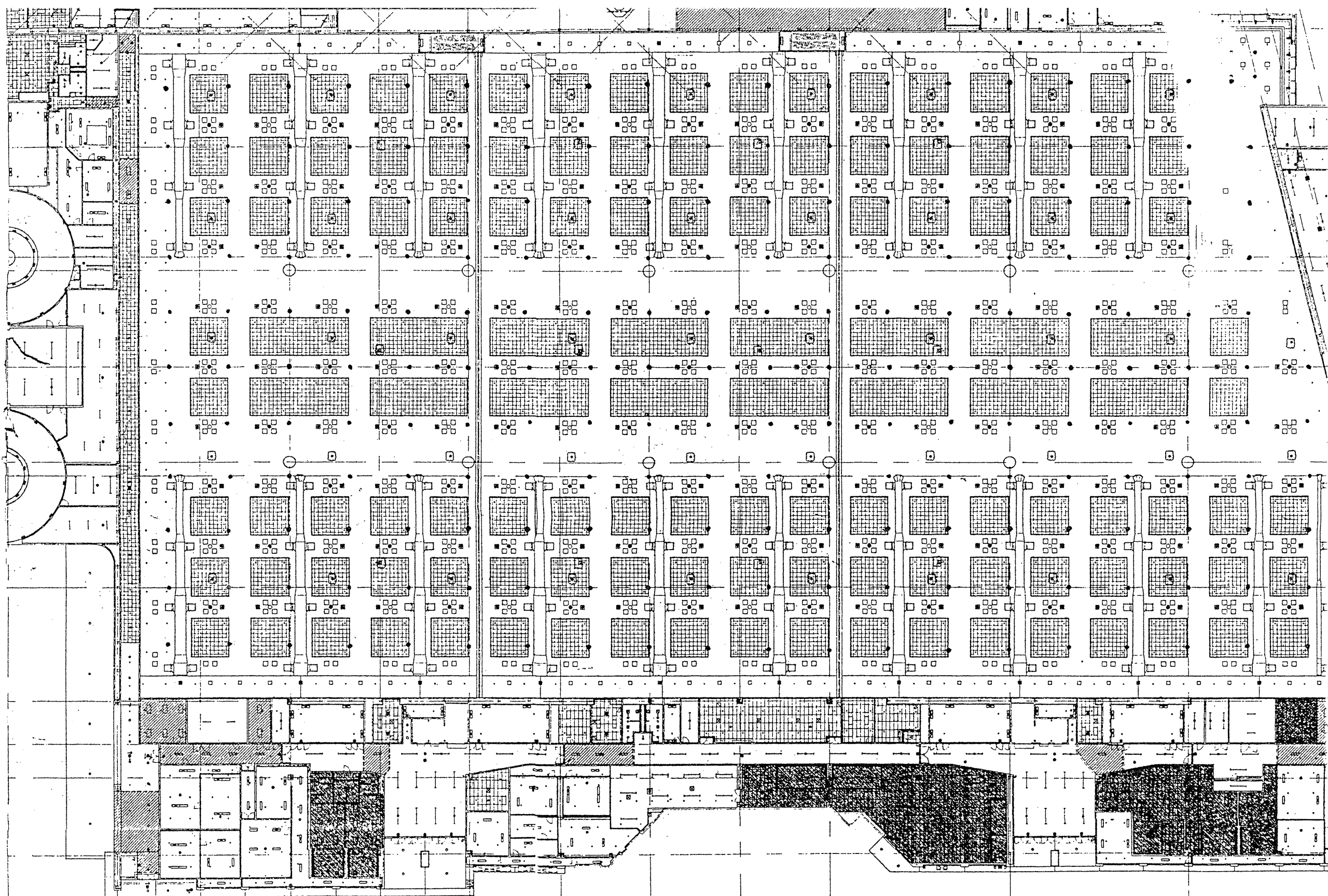
NATURAL FREQUENCY OF FLOOR STRUCTURE

AREA	MINIMUM NATURAL FREQUENCY (Hz)
EXHIBIT HALL	70.0 Hz
MEETING ROOMS	4.0 Hz
BALLROOM	9.2 Hz

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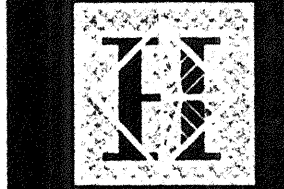


LOCATION OF EXHALL
RIGGING POINTS

● = 2 KIP MAX LOAD @ VERT - 600 lbs @ 45° AN
○ = 1 KIP MAX LOAD @ VERT - 300 lbs @ 45° AN

Inserted 11" x 17" Sheet

Insert before SO.4



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in association with

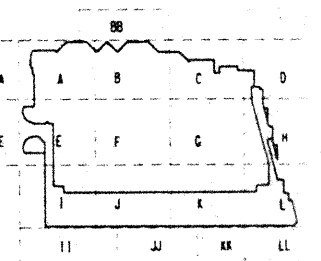
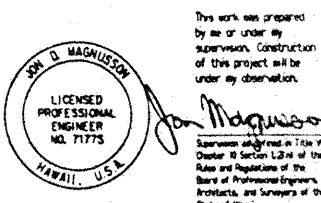
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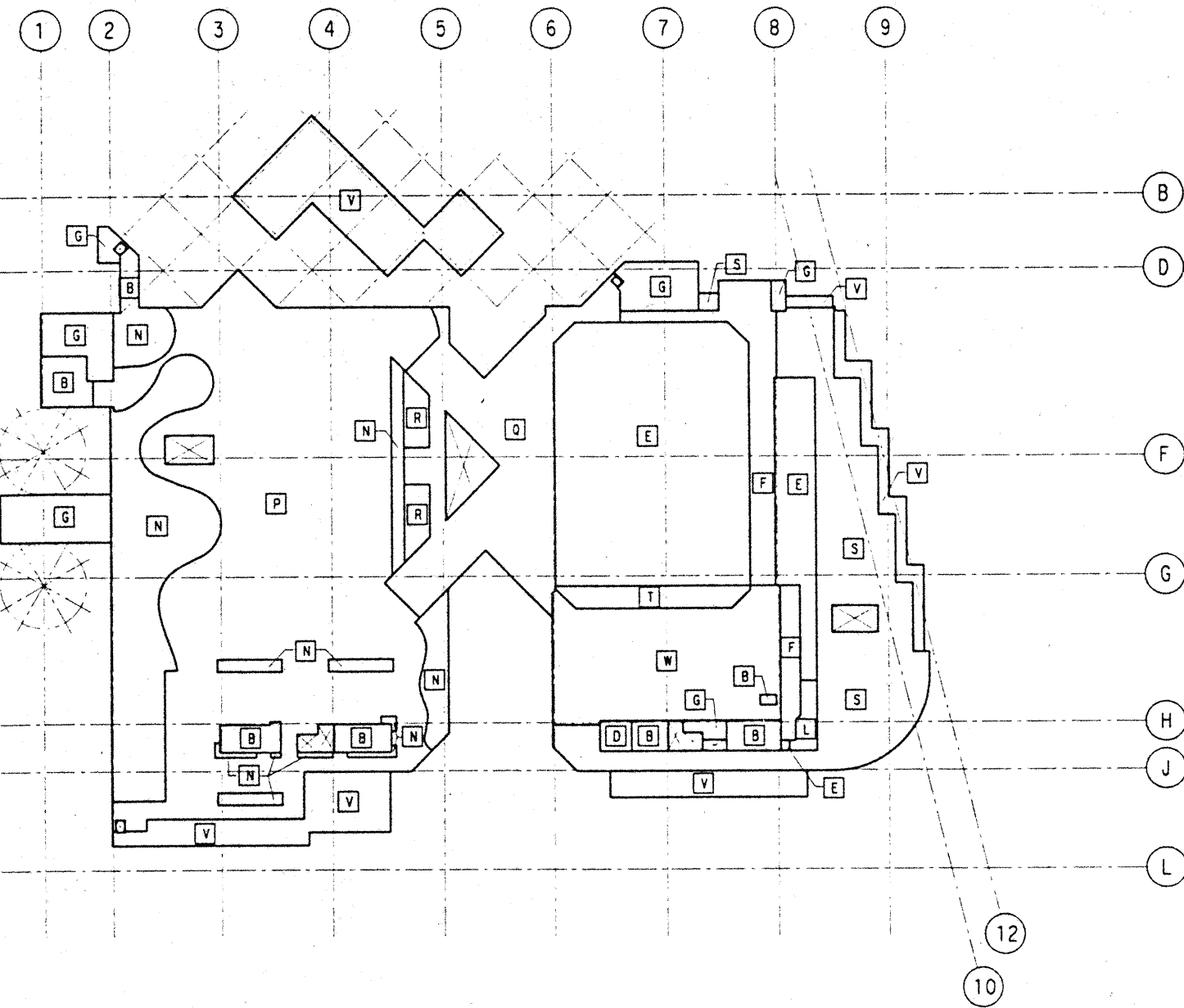
Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENT
9-15-95			BUILDING PERMIT SET
5-8-95			ADDENDUM #3
4-17-95			ISSUED FOR FOUNDATION PERMIT

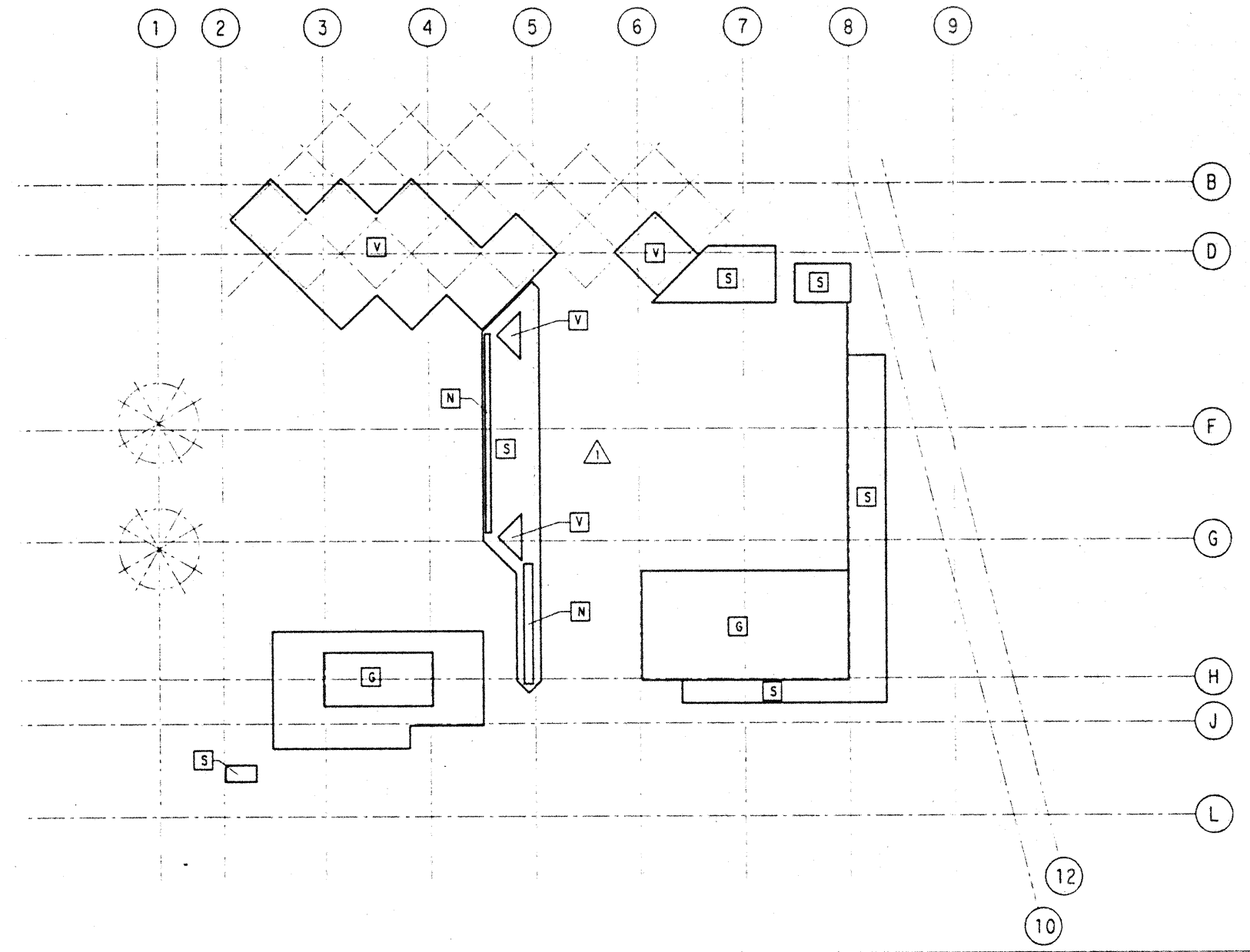
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LOAD MAPS

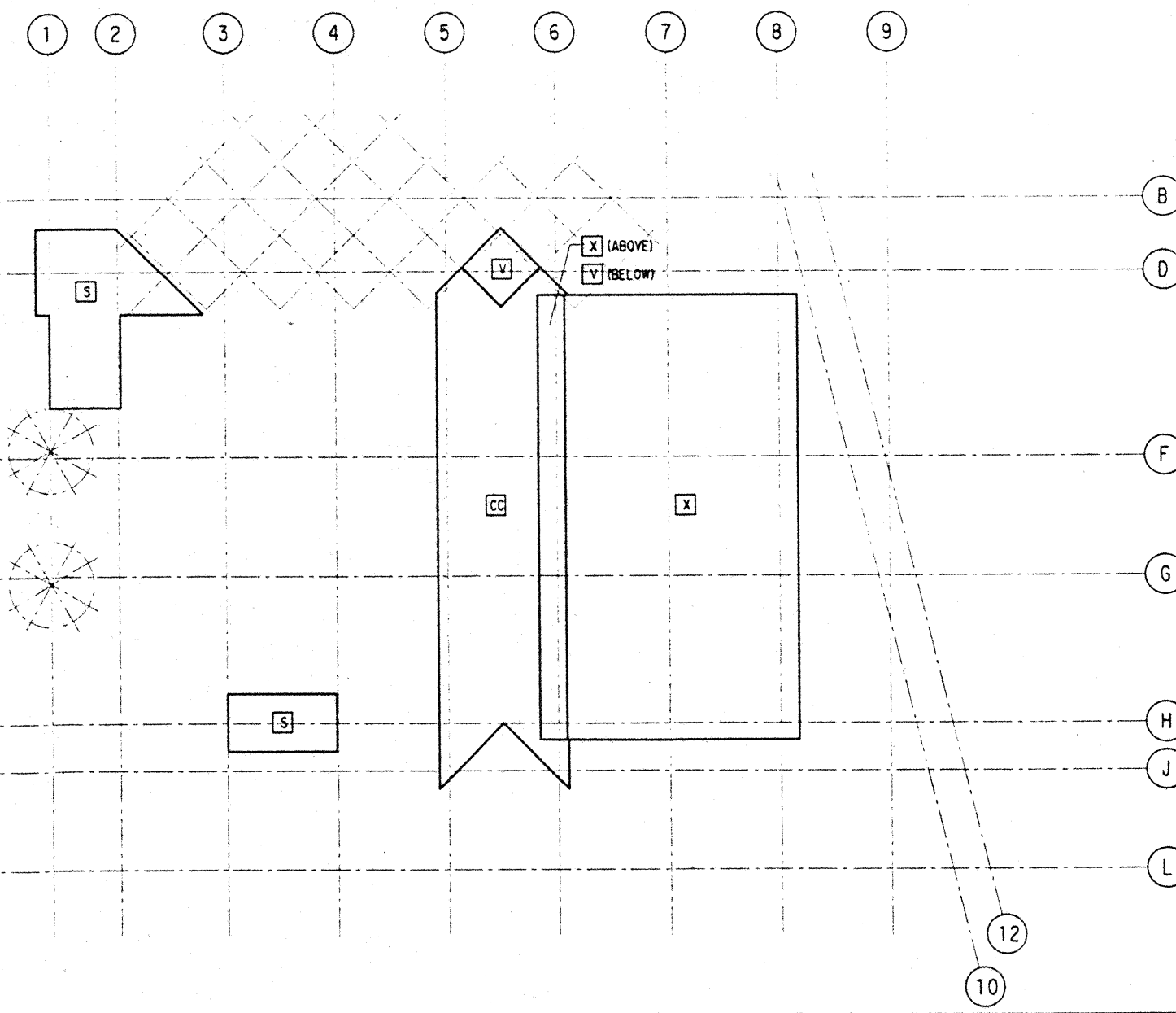
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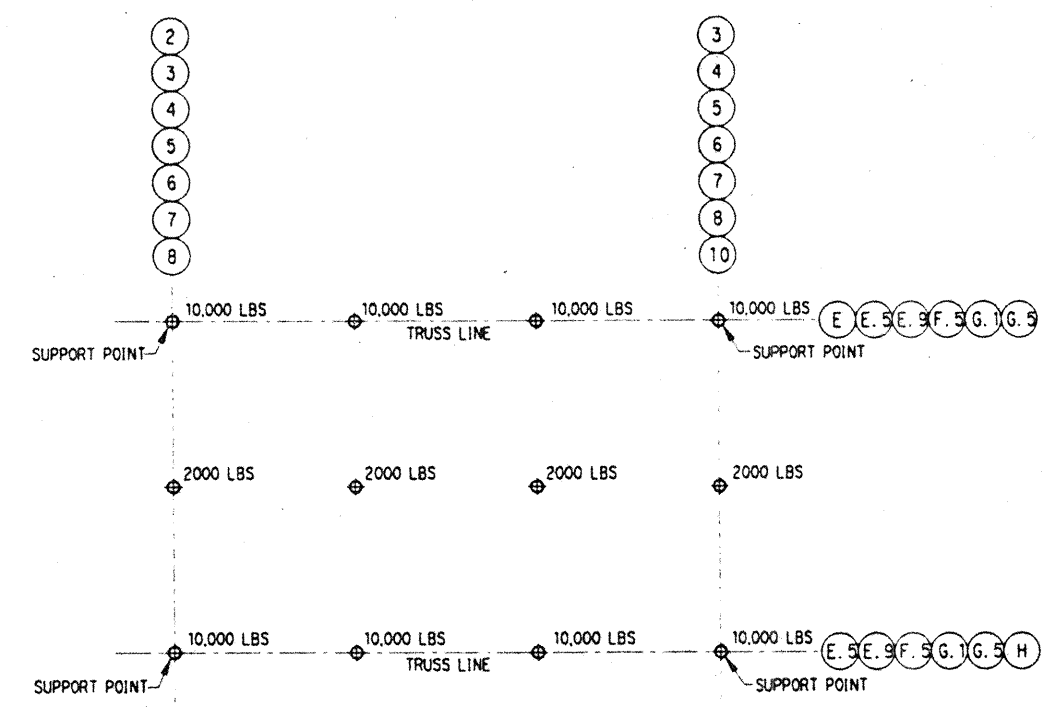
BALLROOM



MECHANICAL PENTHOUSE

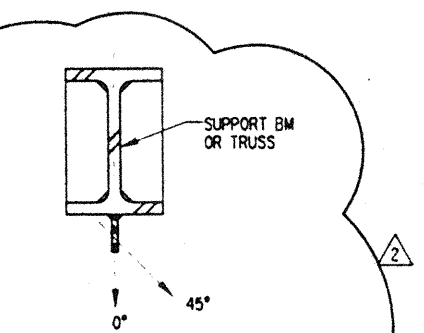


ROOF



- NOTES:
- LOADS SHOWN ARE MAXIMUM LOADS AT SPECIFIC LOCATIONS. TOTAL LOADS TRIBUTARY TO SUPPORT POINTS SHALL NOT EXCEED 26,000 LBS.
 - FOR ANGULAR LOADING, SEE WORKING LOADS-TABLE A.

WORKING LOADS - TABLE A	
	45°
0°	600 LBS
2000 LBS	3000 LBS
10,000 LBS	

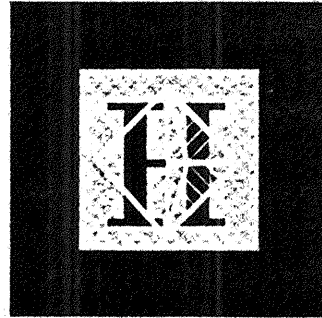


EXHIBITION HALL RIGGING LOADS

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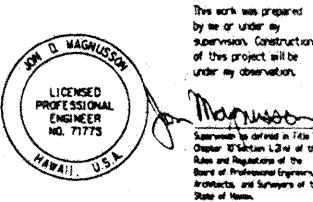
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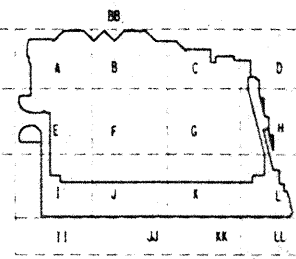
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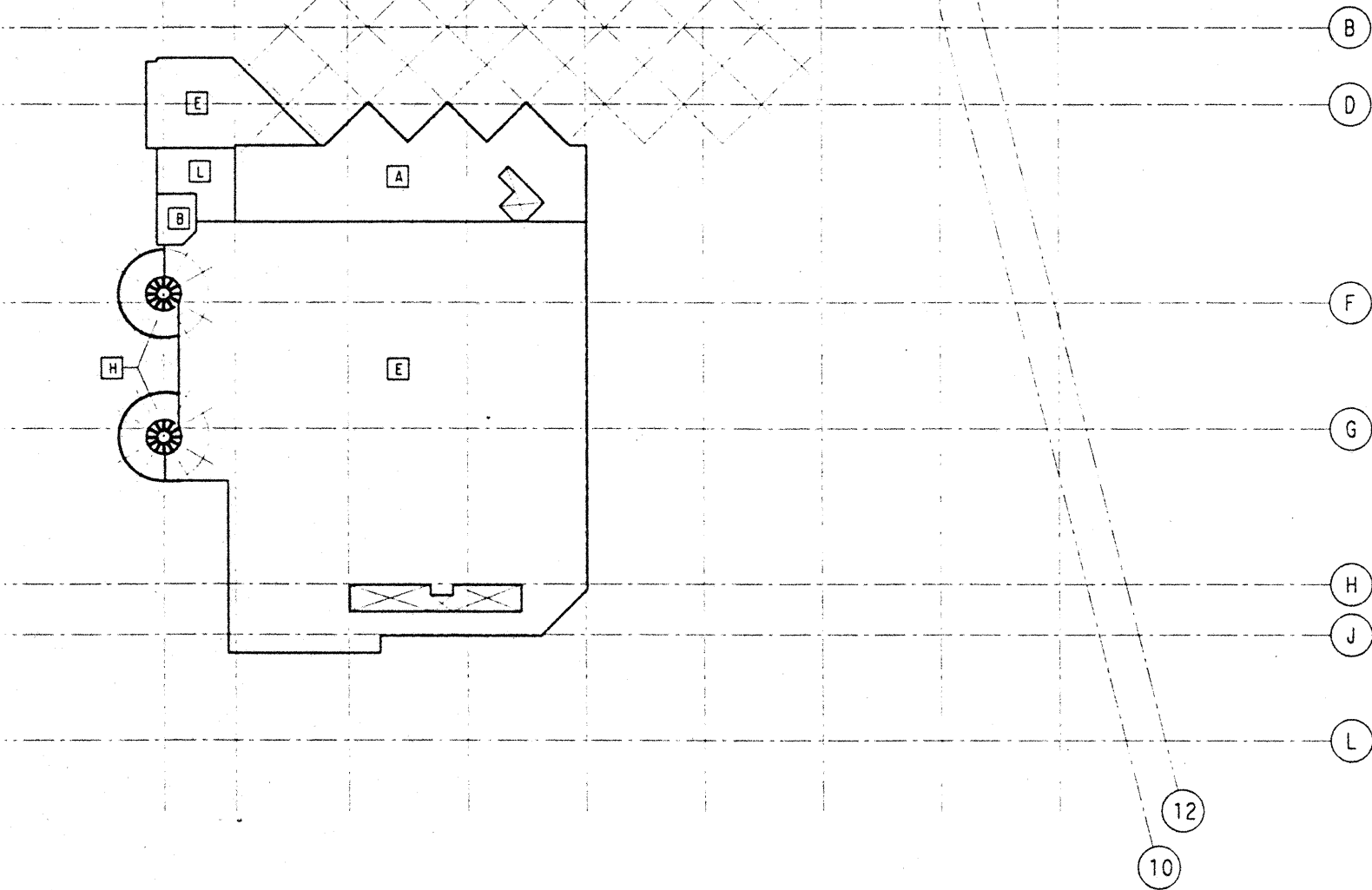


Revisions	
No.	Date By Description
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9-15-95	BUILDING PERMIT SET
5-8-95	ADDENDUM #3
4-17-95	ISSUED FOR FOUNDATION PERMIT

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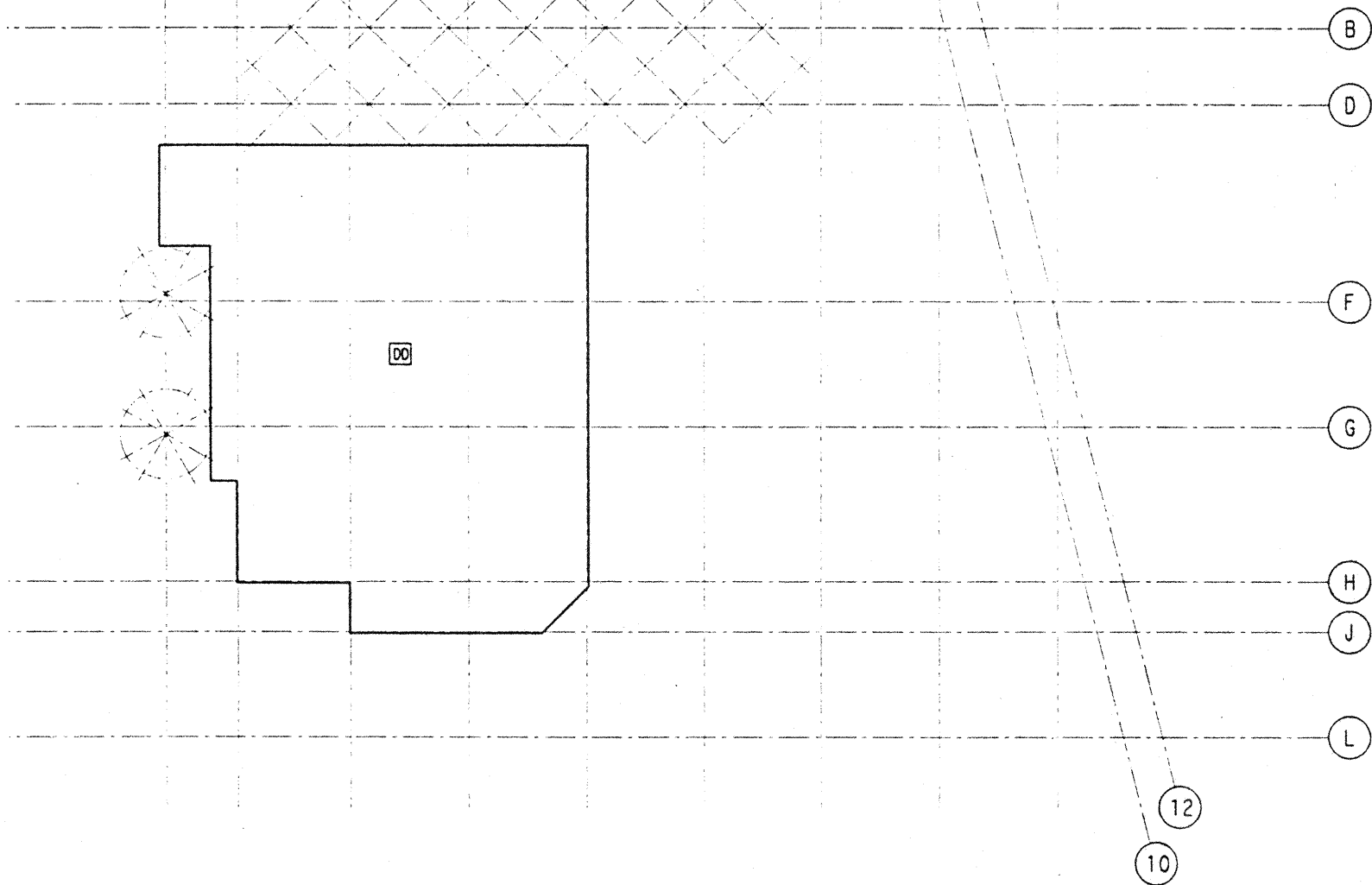
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FUTURE EXHIBITION HALL

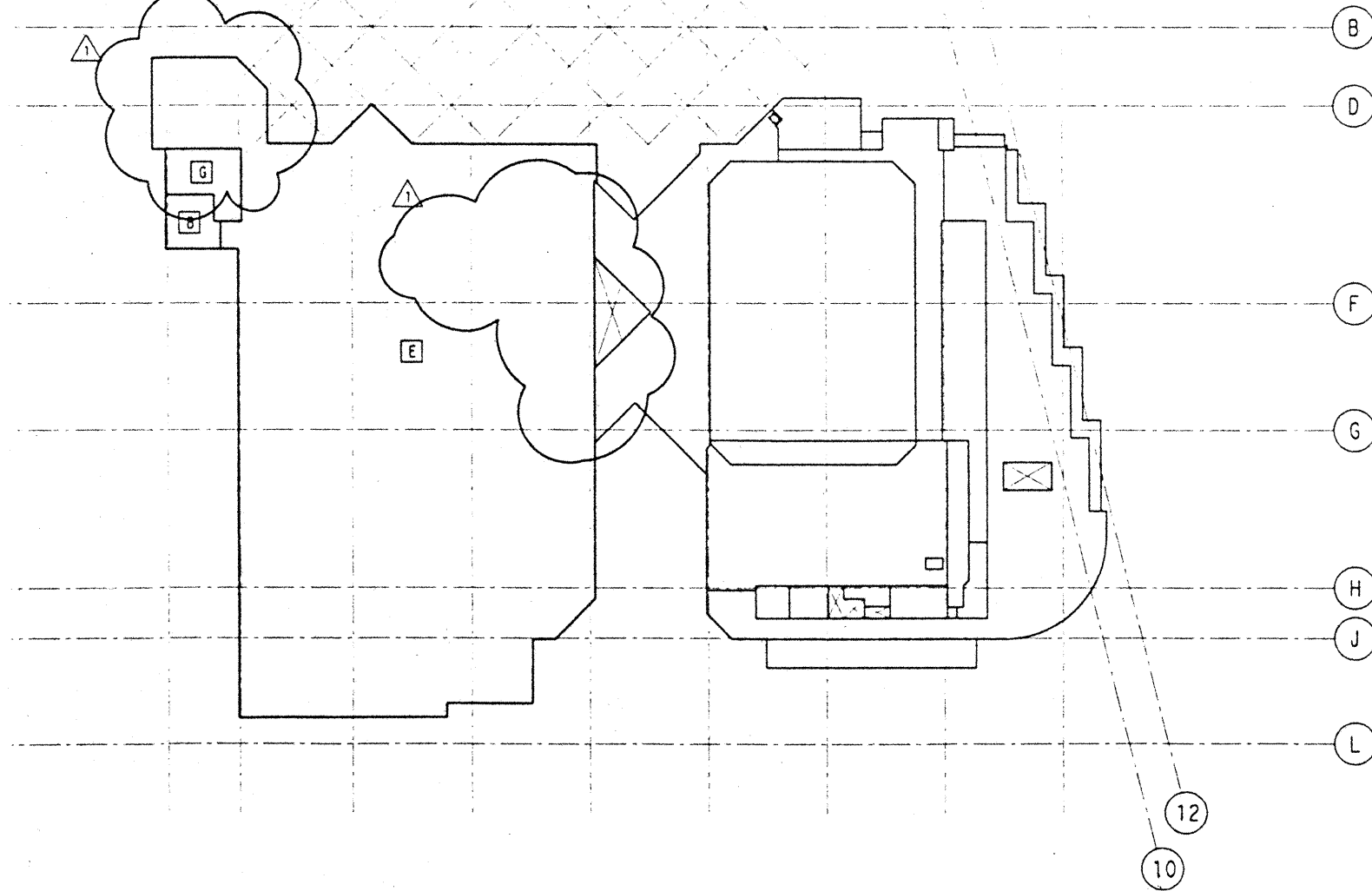
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FUTURE ROOF

1 2 3 4 5 6 7 8 9

REFER TO BALLROOM
PLAN FOR LOADS

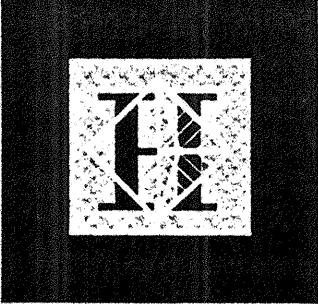


FUTURE MEETING

Fri Dec 1 13:24:30 1995

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dwg: ip14



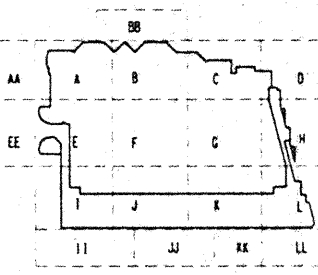
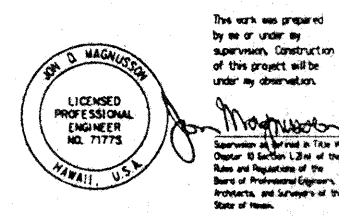
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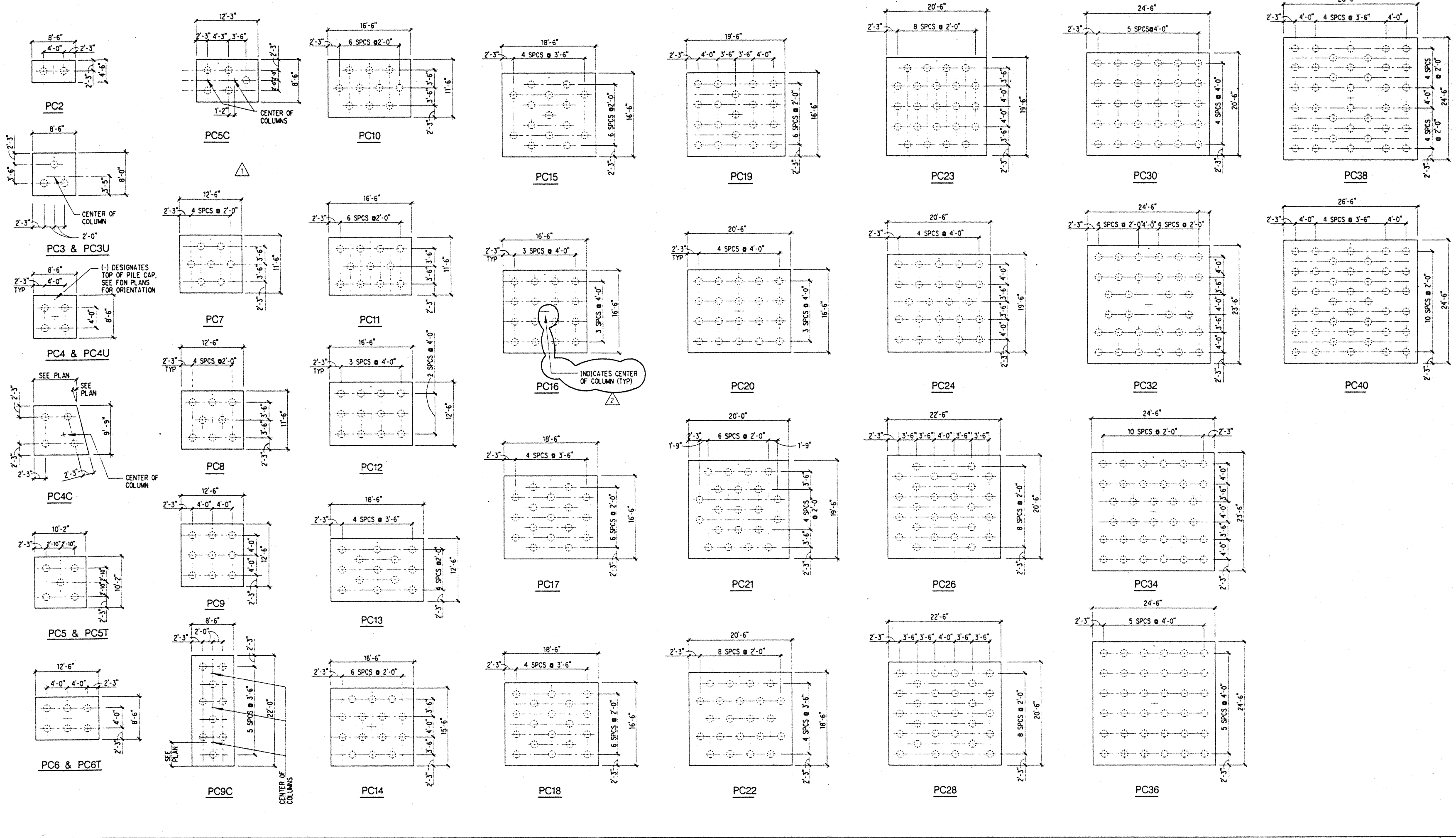
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TYPICAL FOUNDATION DETAILS

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PILE CAP PATTERNS & DIMENSIONS

15

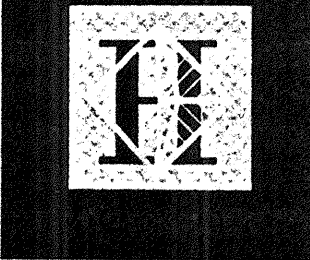
PILE MARK	THICKNESS	BOTTOM REINFORCEMENT		TOP REINFORCEMENT		PILE CAPACITY	REMARKS	PILE MARK	THICKNESS	BOTTOM REINFORCEMENT		TOP REINFORCEMENT		PILE CAPACITY	REMARKS
		LONG DIRECTION	SHORT DIRECTION	LONG DIRECTION	SHORT DIRECTION					LONG DIRECTION	SHORT DIRECTION	LONG DIRECTION	SHORT DIRECTION		
PC1	---	---	---	---	---	55T	SEE 18/S1.2 AND/OR INDIVIDUAL DETAILS	PC10	4'-7"	10*14	14*11H	---	---	150T	
PC2	3'-4"	6*9H	10*8H	---	---	150T		PC11	4'-10"	12*14	18*11H	---	---	150T	
PC2R	3'-4"	6*9H	10*8H	---	---	150T	LENGTH 9'-6"	PC12	5'-2"	13*14	19*11H	---	---	150T	
PC3	2'-8"	8*8H	11*8H	---	---	150T		PC13	5'-9"	18*14	18*11H	---	---	150T	
PC3U	2'-8"	8*8H	11*8H	8*8H	8*8H	50T UPLIFT	SEE 16/S1.2	PC14	5'-3"	16*14	16*14	---	---	150T	
PC4	3'-5"	10*9H	10*9H	---	---	150T		PC15	5'-3"	17*14	18*14	---	---	150T	
PC4C	3'-4"	E-W BARS 9*11H	N-S BARS 11*11H	8*8@12"OC H	8*8@12"OC H	150T	SEE 3 & 4/S1.2	PC16	5'-9"	18*14	18*14	---	---	150T	
PC4U	3'-5"	10*9H	10*9H	10*8H	10*8H	50T UPLIFT	SEE 16/S1.2	PC17	5'-6"	18*14	17*14	---	---	150T	
PC5	3'-11"	9*11H	9*11H	---	---	150T		PC18	5'-7"	20*14	20*14	---	---	150T	
PC5T	3'-11"	9*11H	9*11H	N-S BARS 10*11H	E-W BARS 12*8H	150T	SEE 5/S1.2 FOR ADDITIONAL REINF	PC19	6'-0"	21*14	21*14	---	---	150T	
PC5C	2'-8"	11*8H	18*8H	8*8H	---	50T UPLIFT		PC20	6'-4"	13*18	22*14	---	---	150T	
PC6	3'-8"	11*11H	13*9H	8*8H	13*11H	150T	SEE 5/S1.2 FOR ADDITIONAL REINF	PC21	5'-11"	22*14	22*14	---	---	150T	
PC6T	3'-8"	11*11H	13*9H	8*8H	13*11H	150T		PC22	6'-1"	22*14	24*14	---	---	150T	
PC7	4'-3"	10*11H	10*11H	---	---	150T		PC23	6'-1"	23*14	24*14	---	---	150T	
PC8	4'-2"	13*11H	14*11H	---	---	150T		PC24	6'-5"	25*14	25*14	---	---	150T	
PC9	5'-4"	13*11H	13*11H	---	---	150T		PC26	7'-2"	16*18	16*18	---	---	150T	
PC9C	2'-9"	11*8H	21*9H	8*8H	---	150T		PC28	7'-2"	16*18	18*18	---	---	150T	
								PC30	7'-7"	17*18	20*18	---	---	150T	
								PC32	7'-9"	21*18	21*18	---	---	150T	
								PC34	7'-8"	20*18	20*18	---	---	150T	
								PC36	8'-9"	23*18	23*18	---	---	150T	
								PC38	8'-7"	24*18	25*18	---	---	150T	
								PC40	8'-6"	23*18	25*18	---	---	150T	

NOTES:
1. LONG BARS ON BOTTOM FOR BOTTOM PILE CAP REINFORCEMENT.
2. LONG BARS ON TOP FOR TOP PILE CAP REINFORCEMENT.
3. PROVIDE TOP REINFORCEMENT IN ADDITION TO BOTTOM REINFORCEMENT
4. UPLIFT CONDITIONS ONLY, INDICATED BY PC#U ON PLANS. SEE 16/S1.2
5. PROVIDE STANDARD HOOKS @ EA END OF REINFORCEMENT WHEN "H" IS CALLED OUT IN SCHEDULE.

PILE CAP SCHEDULE

20

S1.1



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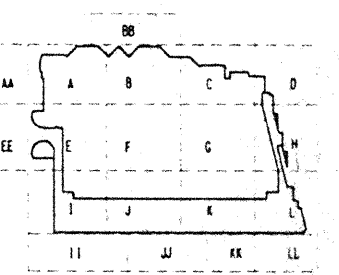
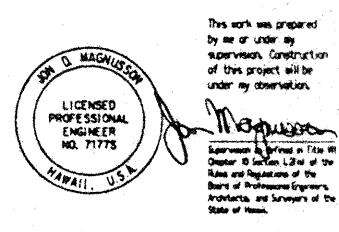
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SKILLING WARD MAGNUSON BAKERBEE INC.
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1321 First Avenue, Suite 3200 Seattle, WA 98101-1998
(206) 292-1200 FAX: (206) 292-1201

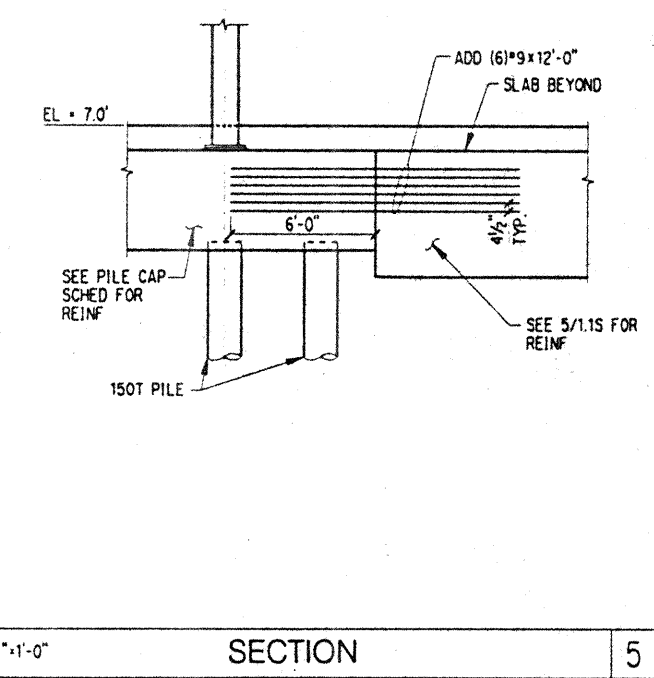


No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
6-19-95			
4-28-95			ADDENDUM #11
4-17-95			ISSUED FOR FOUNDATION PERMIT

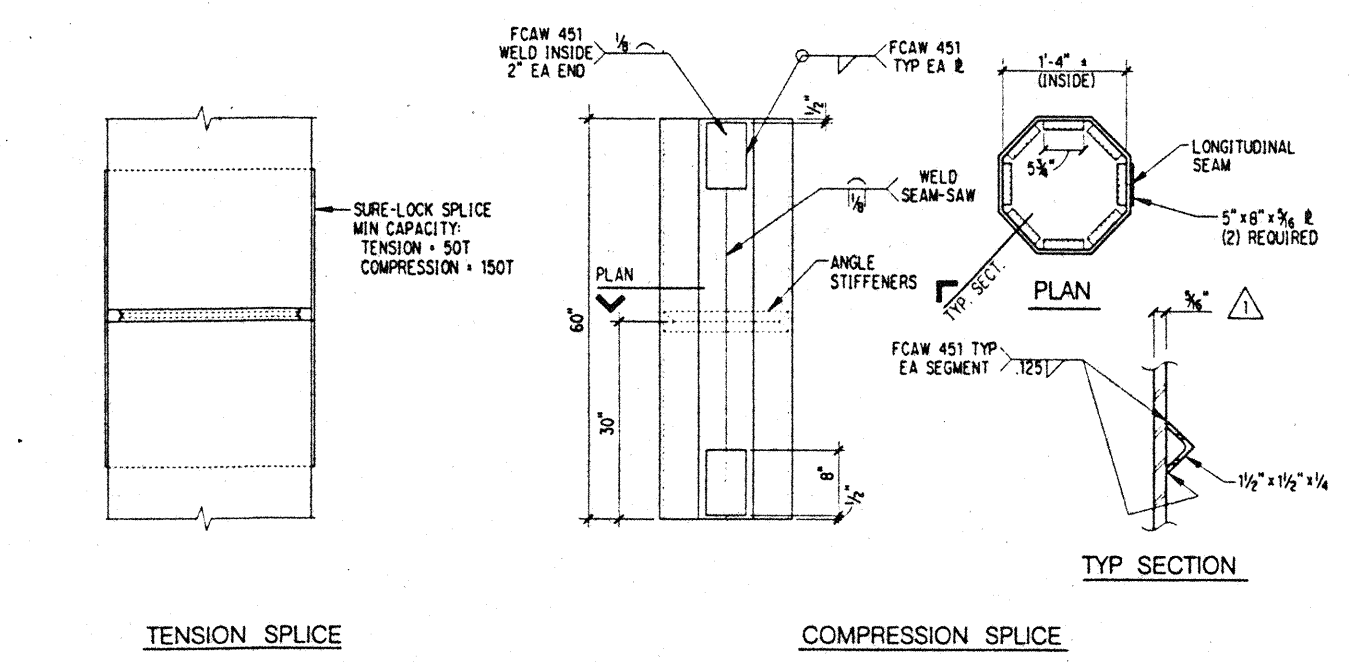
Sheet Title

TYPICAL CONCRETE AND FOUNDATION DETAILS

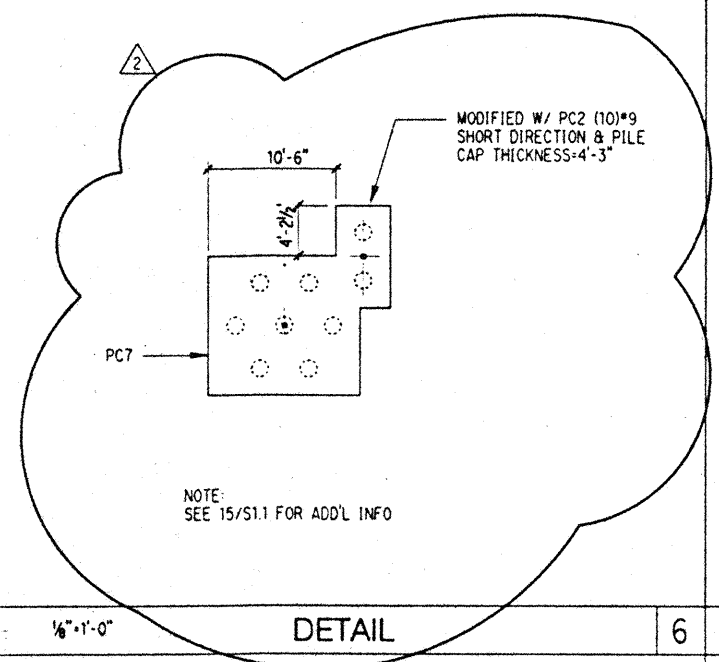
Designed BJM
 Drawn DMS
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 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



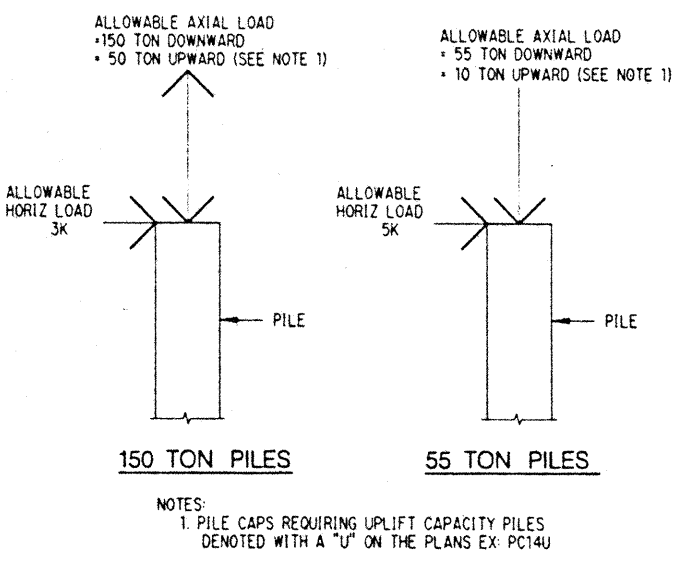
SECTION 5



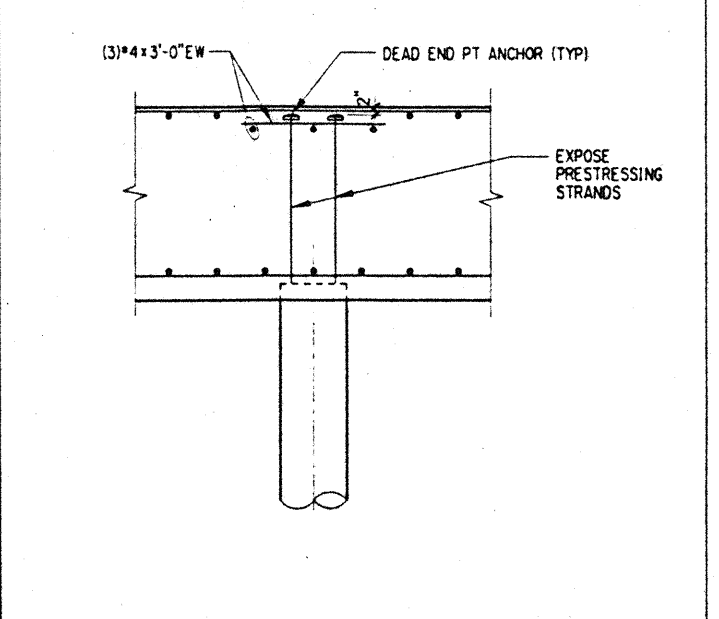
PILE SPLICE DETAILS 2



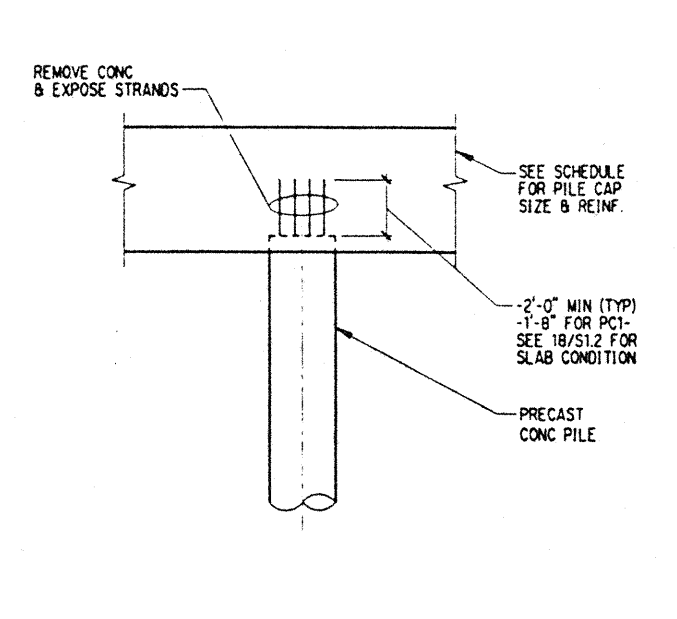
DETAIL 6



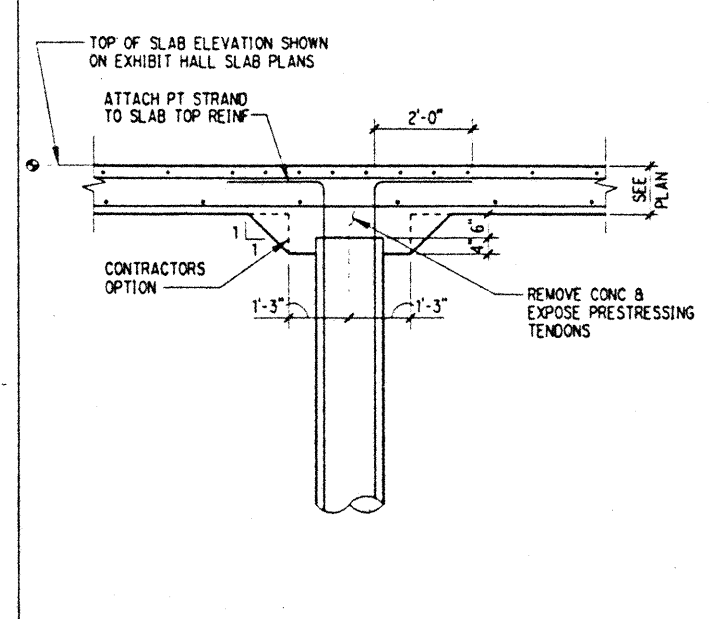
PILE DESIGN CAPACITY 11



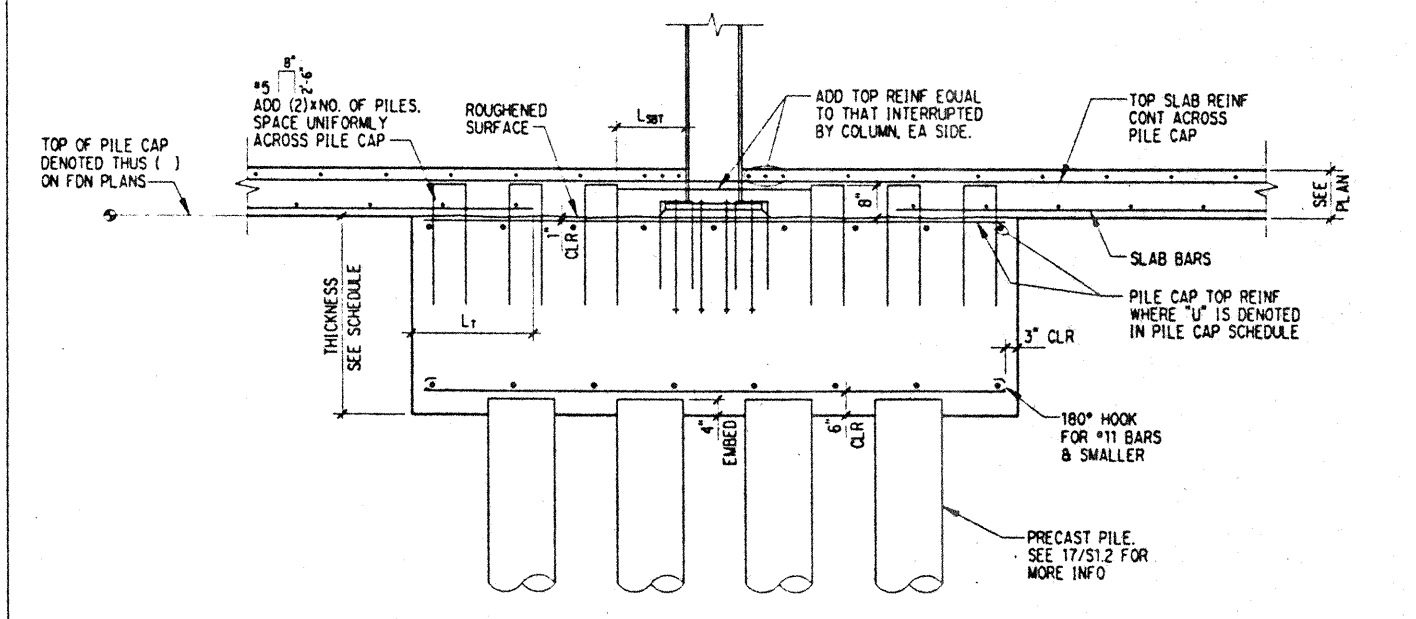
TYPICAL UPLIFT PILE DETAIL 16



TYPICAL PILE DOWEL DETAIL 17

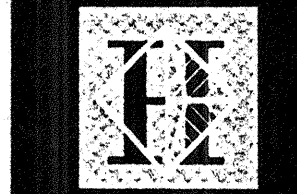


TYP PILE CAP SECT FOR PC1 @ SLAB 18



TYPICAL PILE CAP SECTION FOR PC2-PC32 20

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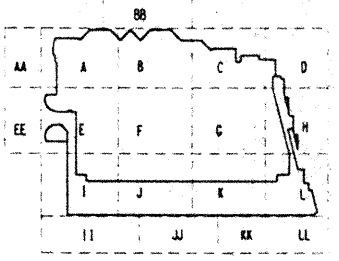
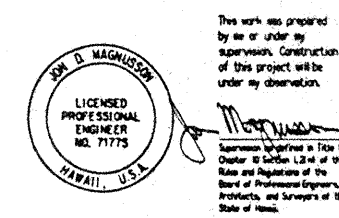
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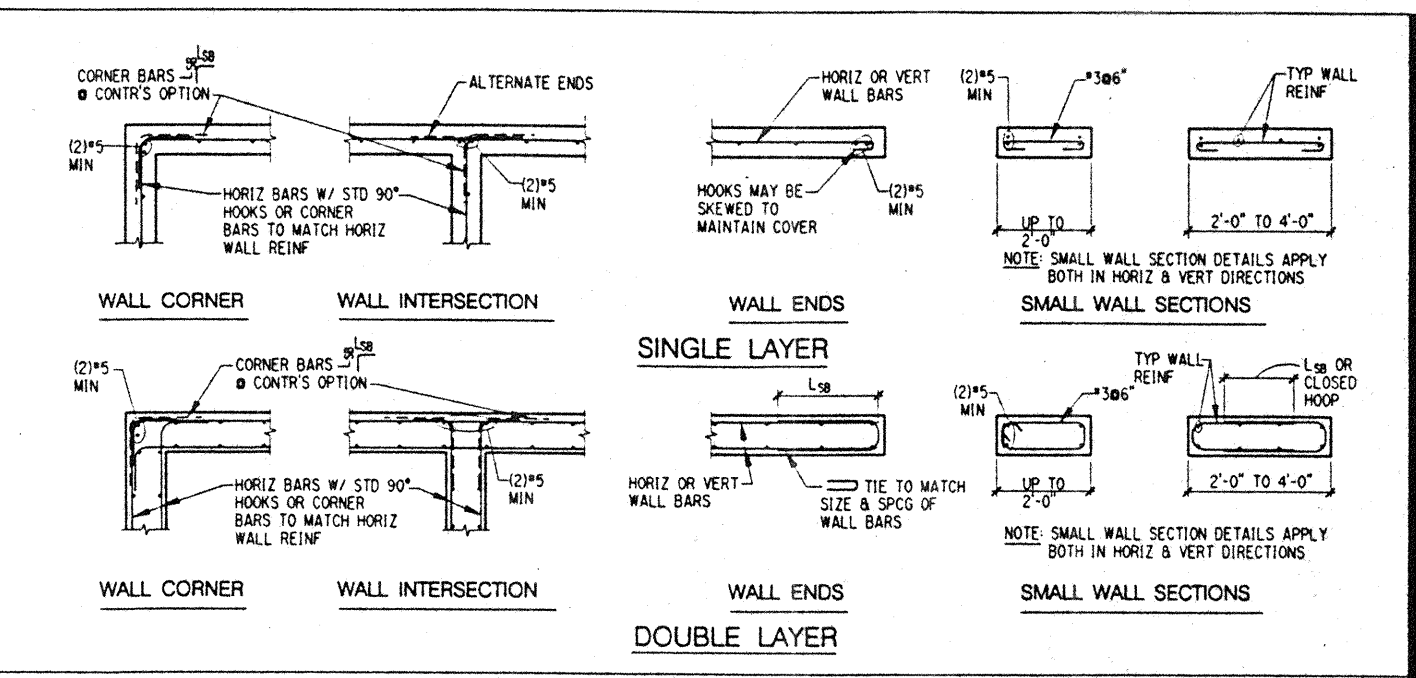
Revisions

No.	Date	By	Description

Sheet Title

TYPICAL ARCHITECTURAL CONCRETE DETAILS

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 Drawn DMS
 Checked JDM
 Date 12 / 15 / 95
 Job No. 93036.02.9
 Sheet No.

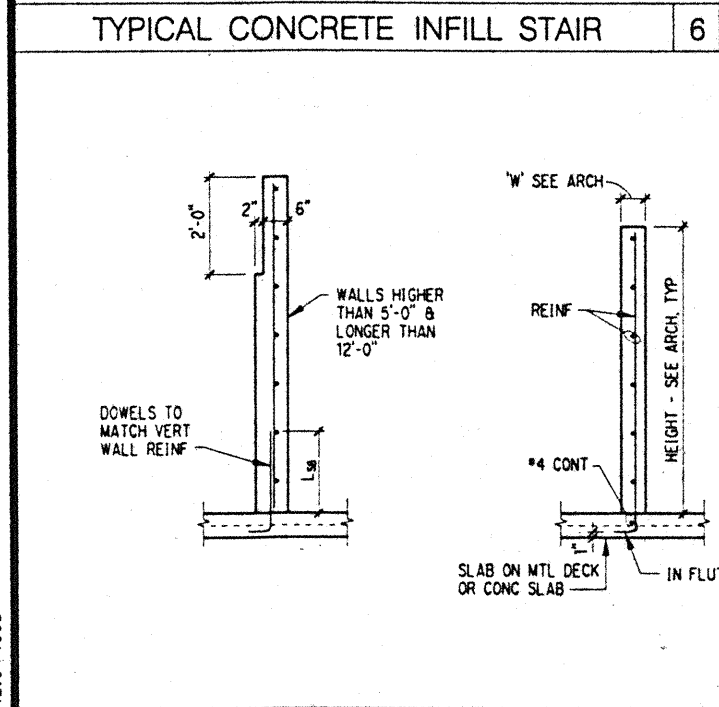
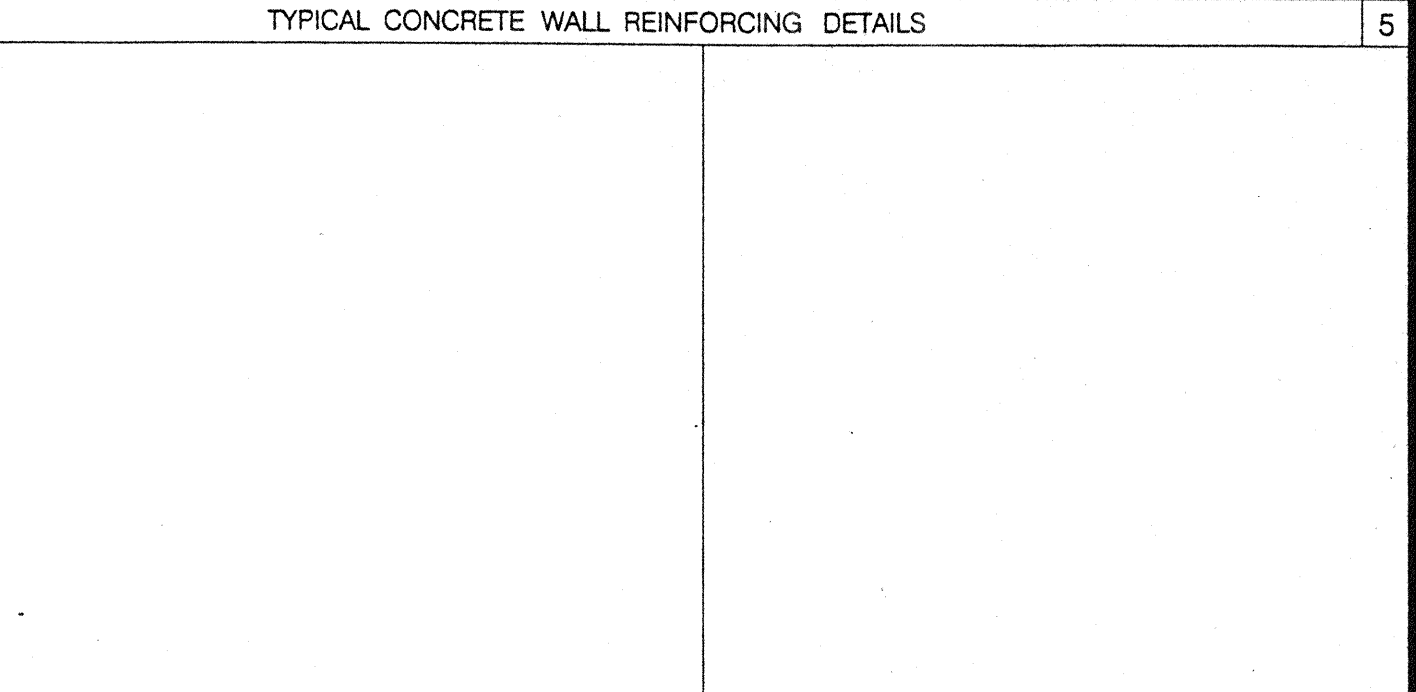
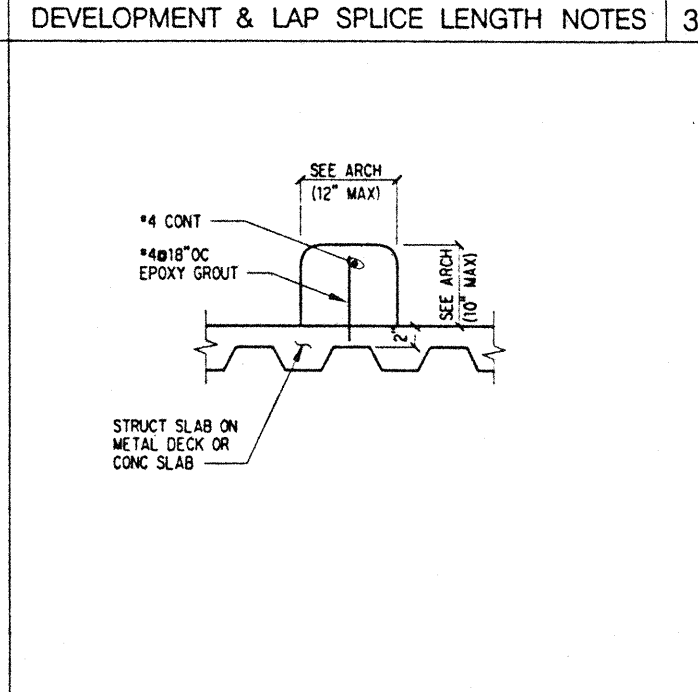
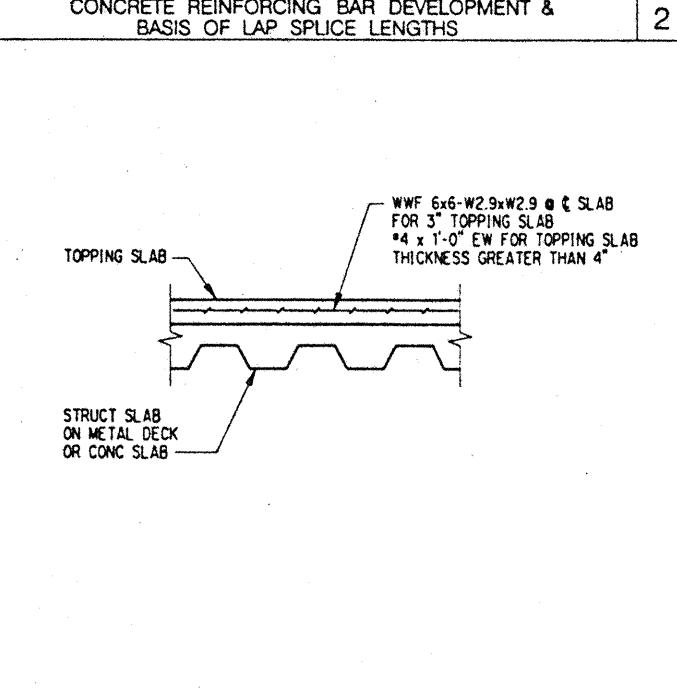
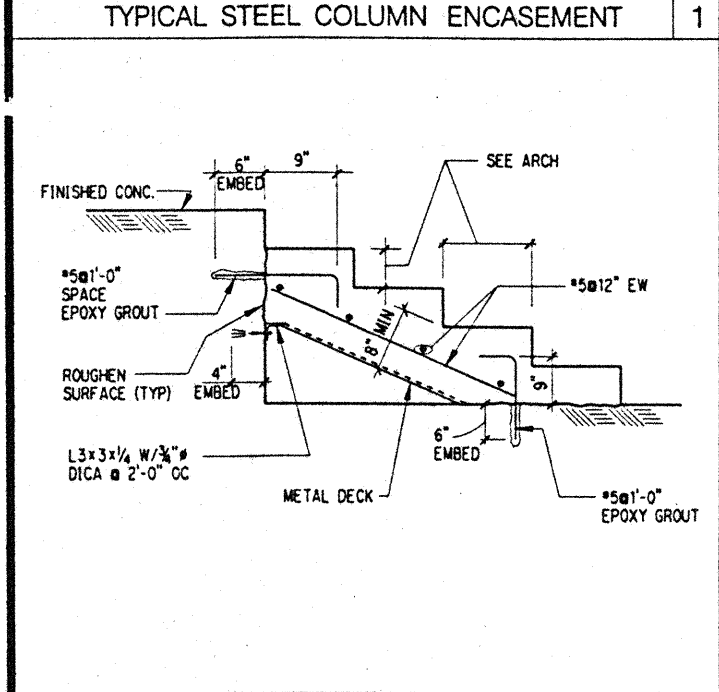
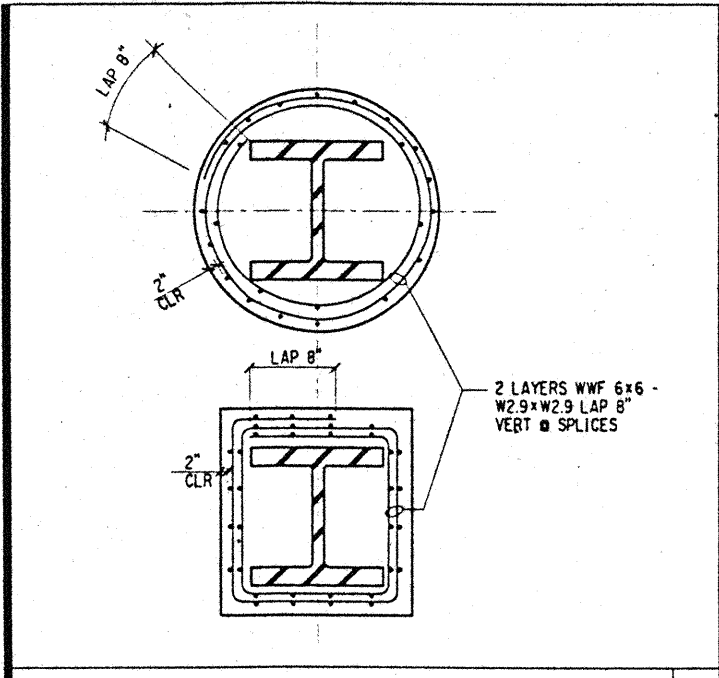


NOTES:

- NOTATIONS:
 d_b : NOMINAL BAR DIAMETER
 L_D : TENSION DEVELOPMENT LENGTH FOR OTHER THAN TOP BARS
 L_T : DEVELOPMENT LENGTH OF TOP BARS IN TENSION = $1.3 \times L_D$
 L_B : DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = $19 \times d_b$
 L_C : TIED COLUMN LAP SPLICE IN COMPRESSION = $30 \times d_b$
 L_{CS} : SPIRAL COLUMN LAP SPLICE IN COMPRESSION = $22.5 \times d_b$
 L_{SB} : TENSION LAP SPLICE LENGTH FOR OTHER THAN TOP BARS = $1.3 \times L_D$
 L_{SBT} : TENSION LAP SPLICE LENGTH OF TOP BARS = $1.69 \times L_D$
 L_{SBT} SHALL BE USED FOR ALL TOP BAR CONDITIONS EVEN WHEN SHOWN AS L_{SB} ON DRAWINGS.
 L_{SBT} : HORIZONTAL REINFORCING WITH MORE THAN 12" OF CONCRETE CAST BELOW.
- TOP BARS: HORIZONTAL REINFORCING WITH MORE THAN 12" OF CONCRETE CAST BELOW.
- REINFORCING $f_y = 60,000$ psi
- FOR $f'_c = 3000$ psi $L_D = 22 \times d_b$
- *14 & *18 BARS SHALL NOT BE LAP SPICED. SEE GENERAL NOTES ALSO.
- THESE TABLES ARE SIMPLIFIED VERSIONS OF ACI CODE CHAPTER 12 REQUIREMENTS AND MAY NOT MATCH OTHER PUBLISHED TABLES.
- IN SLABS AND WALLS, FOR #11 OR SMALLER BARS WITH $s > 6d_b$, DEVELOPMENT LENGTH MAY BE MULTIPLIED BY 0.80 EXCEPT IT SHALL NOT BE TAKEN LESS THAN THE LENGTH SHOWN UNDER $s \leq 6d_b$.

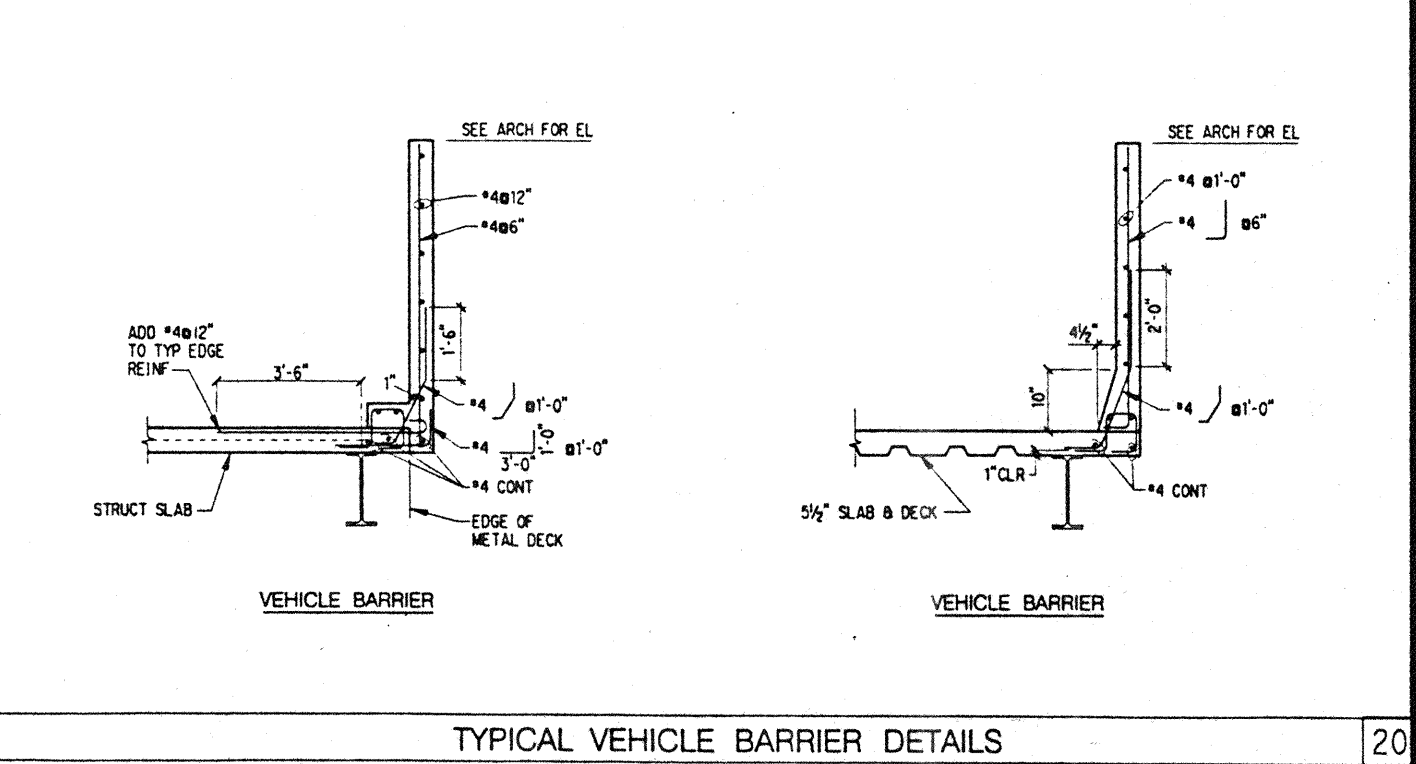
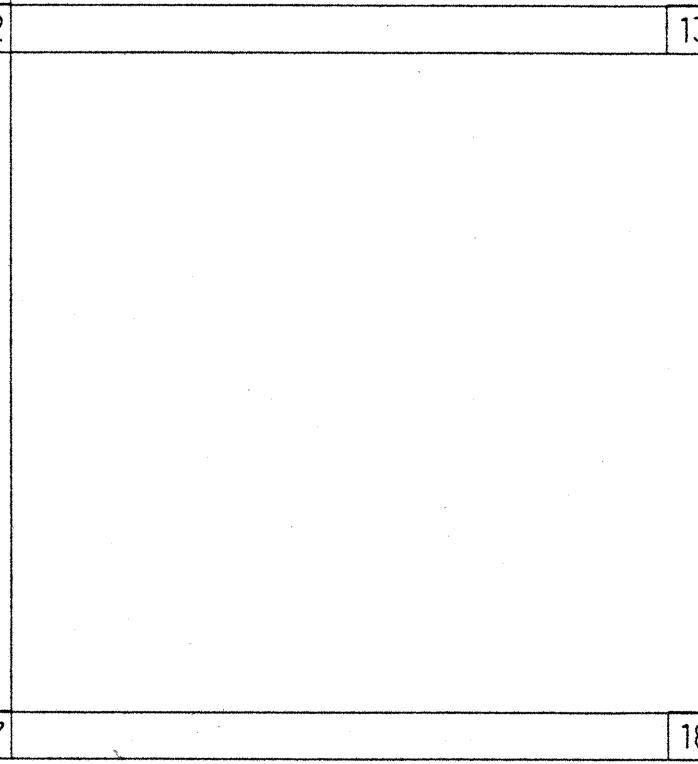
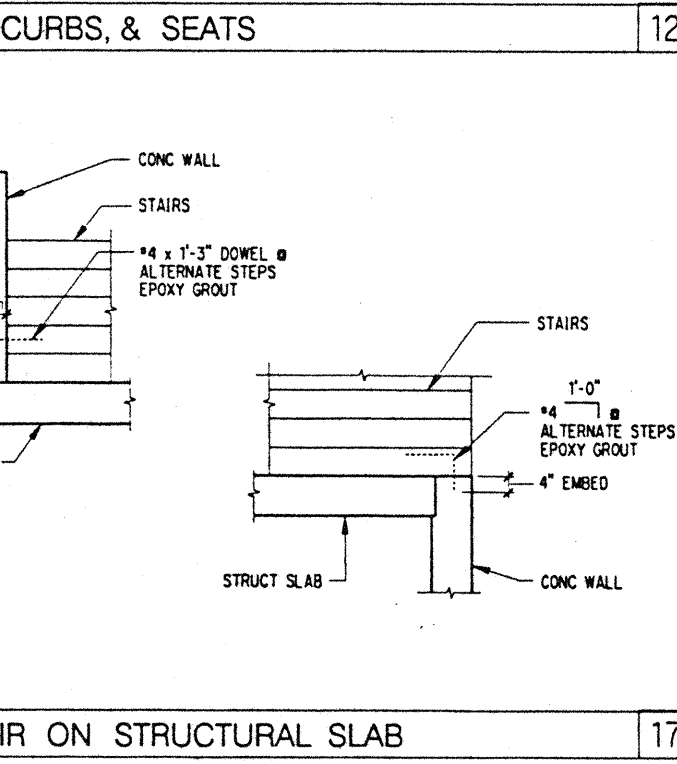
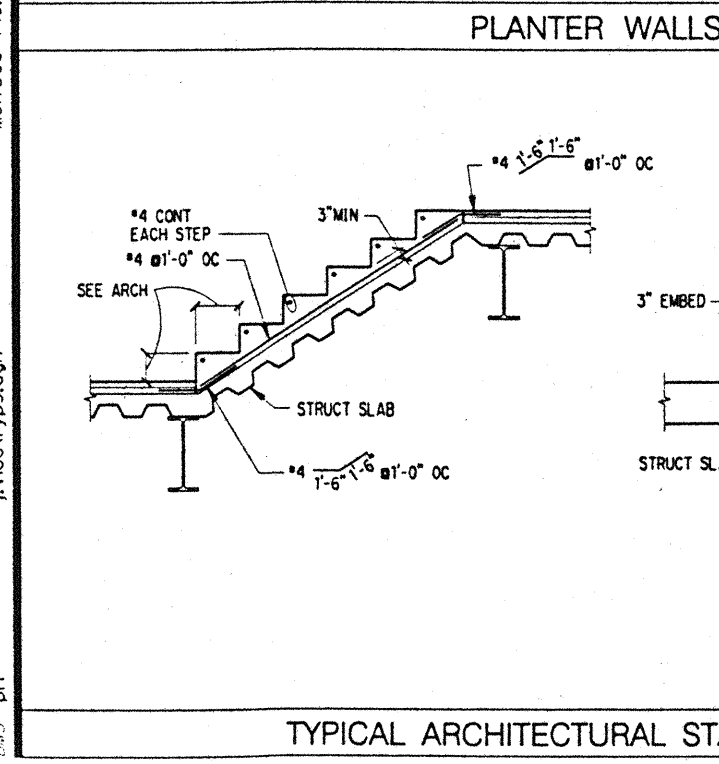
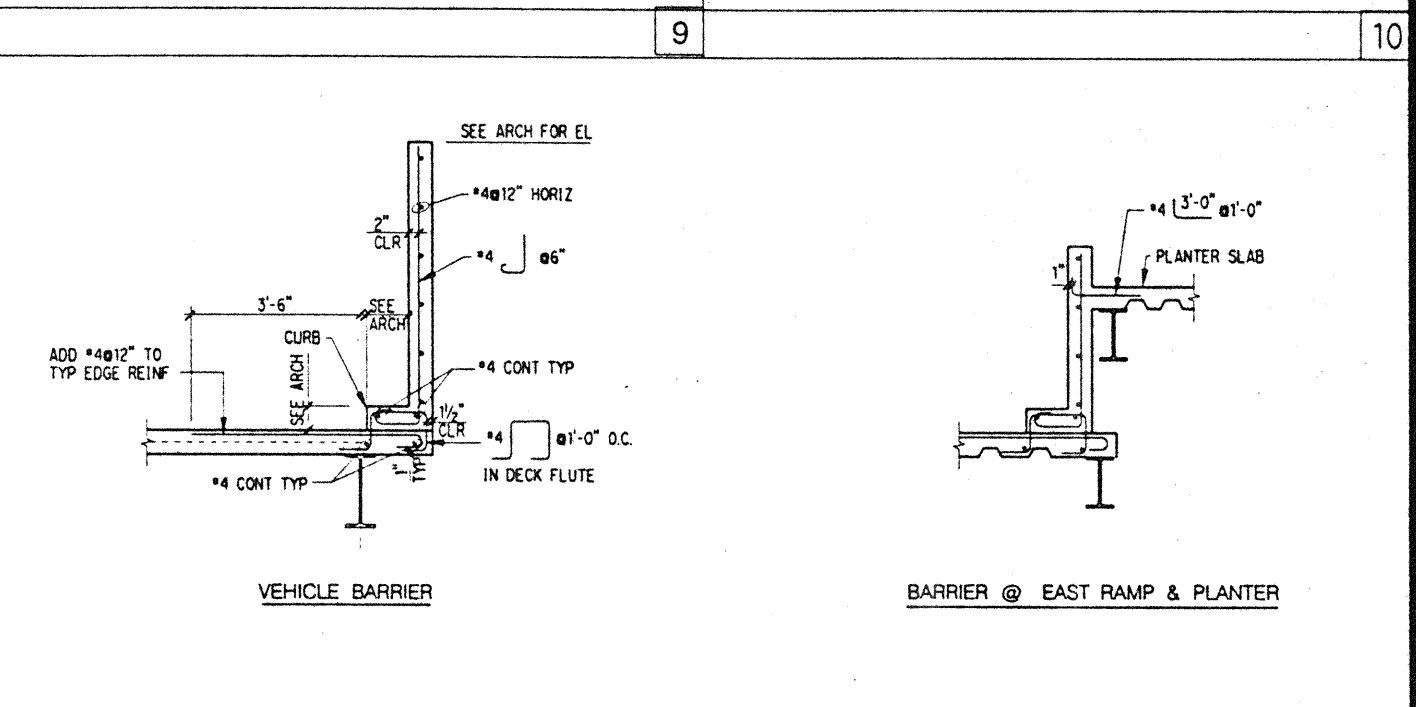
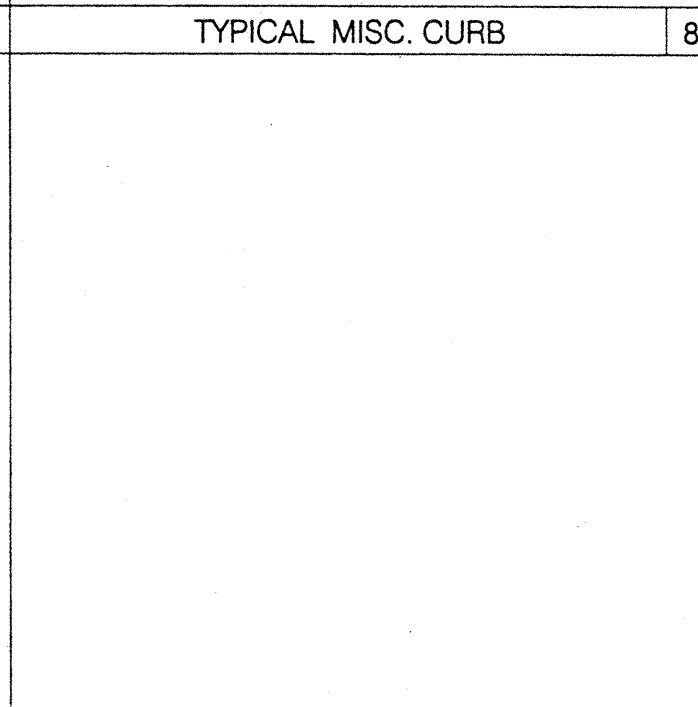
L_D $f'_c = 4000$ psi

BAR SIZE	BEAMS & COLUMNS				ALL OTHERS (SLABS, WALLS, ETC)	
	CTR TO CTR SPACING (S)	3d < S < 4d	5d	6d	CONC COVER (C)	C > d_b
#3	12	12	12	12	12	12
#4	15	15	15	15	15	15
#5	24	18	18	24	24	18
#6	34	24	22	34	24	24
#7	46	32	25	46	32	32
#8	60	42	30	60	42	42
#9	76	53	38	76	53	53
#10	97	68	48	97	68	68
#11	119	83	59	119	83	83
#14	162	113	81	162	113	113
#18	237	166	119	237	166	166

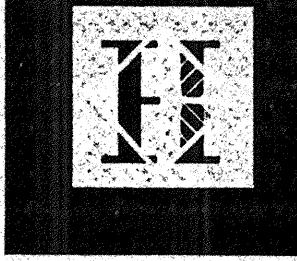


H	WALL THICK W	REINF
0'-3"	6"	#4 @ 12" @ EW
0'-3"	8"	#4 @ 12" @ EW
0'-3"	> 8"	#4 @ 12" @ EF, EW
3'-4"	6"	#4 @ 12" @ VERT #5 @ 12" @ HORIZ *
4'-5"	6"	#5 @ 12" @ VERT #5 @ 12" @ HORIZ *
> 5"	6"	#5 @ 12" @ VERT #5 @ 6" @ HORIZ *

* FOR WALLS LONGER THAN 12'-0" HORIZ REINF = #5 @ 6"



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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 2200 Seattle, WA 98101-5599
(206) 292-1200 FAX: (206) 292-1001

RECOMMENDED END HOOKS
ALL GRADES
D= FINISHED BEND DIAMETER

BAR SIZE	180° HOOKS		90° HOOKS	
	D	A OR G	D	A OR G
#3	2 1/4	5	3	6
#4	3	6	4	8
#5	3 3/4	7	5	10
#6	4 1/4	8	6	12
#7	5 1/4	10	7	14
#8	6 1/4	11	8	16
#9	7 1/4	13	10	18
#10	8 1/4	15	11	20
#11	9 1/4	17	13	22
#14	12 1/4	23	17	27
#18	16 1/4	30	22	35

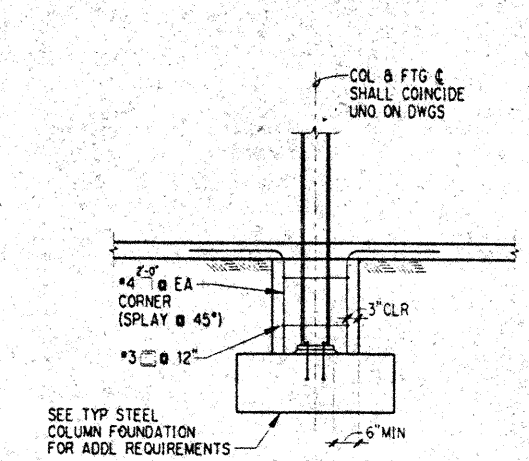
TABLE A
MINIMUM TENSION EMBEDMENT LENGTHS,
L_{eh}, FOR STANDARD END HOOKS ON GRADE 60 BARS

BAR SIZE	NORMAL WEIGHT CONCRETE, f _c (psi)						
	3,000	4,000	5,000	6,000	7,000 & 7,500	8,000	
#3	6"	6"	6"	6"	6"	6"	
#4	8"	7"	6"	6"	6"	6"	
#5	10"	9"	8"	7"	7"	6"	
#6	12"	10"	9"	8"	8"	7"	
#7	14"	12"	11"	10"	9"	9"	
#8	16"	14"	12"	11"	10"	10"	
#9	18"	15"	14"	13"	12"	11"	
#10	20"	17"	15"	14"	13"	12"	
#11	22"	19"	17"	16"	14"	14"	
#14	37"	32"	29"	27"	25"	23"	
#18	50"	43"	39"	35"	33"	31"	

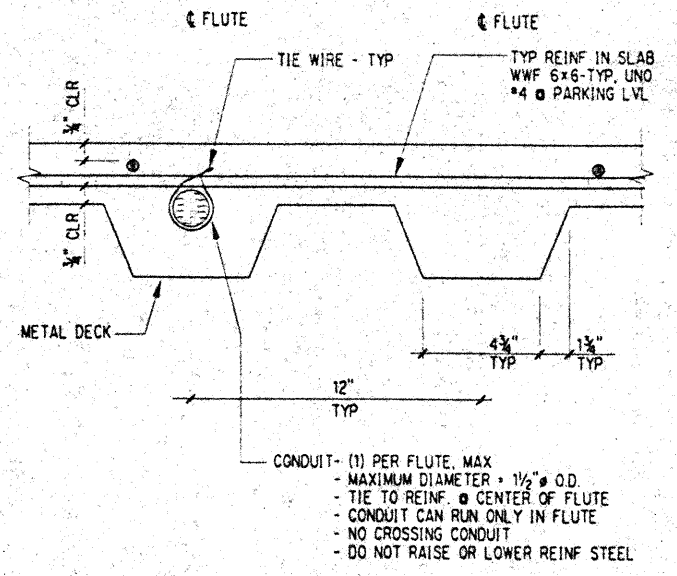
- NOTES:
- SIDE COVER ≥ 2 1/2" IN
 - END COVER (90° HOOKS) ≥ 2" IN
 - FOR 180° HOOKS AT RIGHT ANGLES TO EXPOSED SURFACES, SEE TABLE 'B' FOR MINIMUM LENGTHS TO PROVIDE 2" IN MINIMUM COVER TO TAIL

TABLE B
MINIMUM EMBEDMENT LENGTHS TO PROVIDE 2" IN COVER TO TAIL OF STANDARD 180° END HOOK

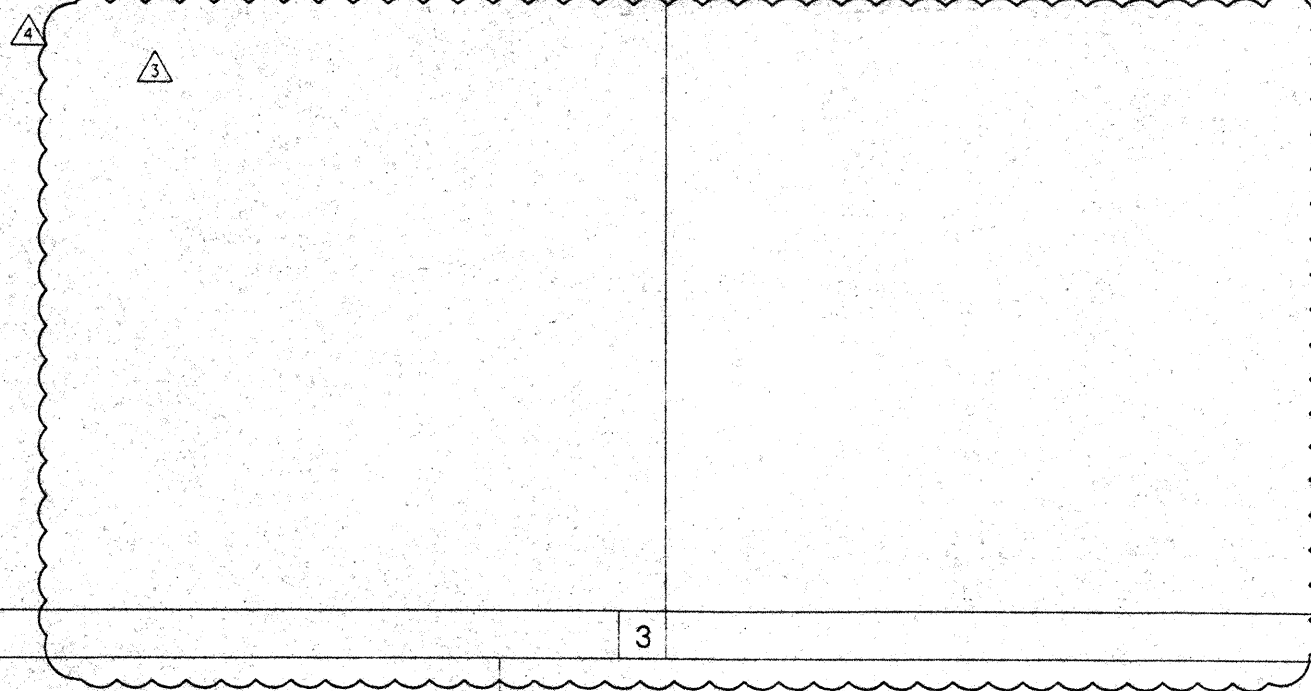
BAR SIZE	#3	#4	#5	#6	#7	#8	#9	#10	#11	#14	#18
6"	7"	7"	8"	9"	10"	12"	14"	15"	20"	25"	



TYP BELOW GRADE STL COL ENCASEMENT DET 2



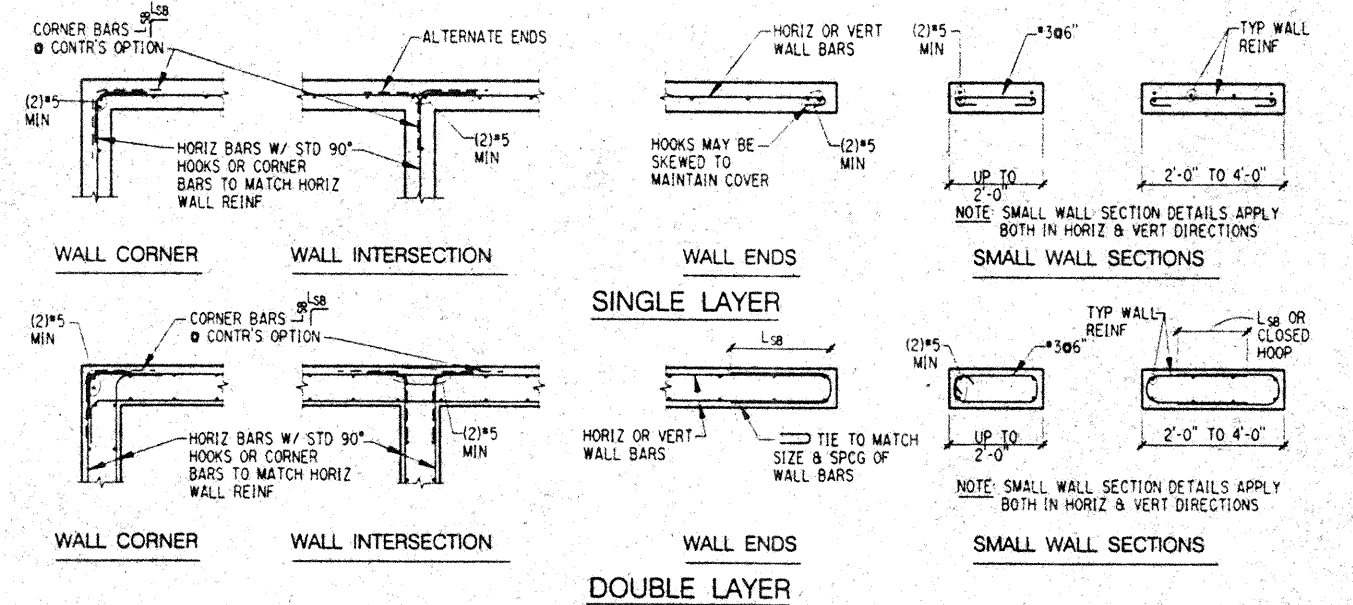
TYP STANDARD HOOKS & EMBEDMENT 6 TYP CONDUIT IN SLAB ON METAL DECK DETAIL 7



MINIMUM WALL REINFORCEMENT

WALL THICKNESS	HORIZ BARS	VERT. BARS	LOCATION
6" & UNDER	#4 @ 13"	#4 @ 13"	CENTERLINE
OVER 6-8"	#5 @ 15"	#5 @ 15"	CENTERLINE
OVER 8-10"	#5 @ 12"	#5 @ 12"	CENTERLINE
OVER 10-12"	#4 @ 12"	#4 @ 12"	EACH FACE
OVER 12-14"	#5 @ 18"	#5 @ 18"	EACH FACE
OVER 14-16"	#5 @ 15"	#5 @ 15"	EACH FACE
OVER 16-18"	#5 @ 14"	#5 @ 14"	EACH FACE
OVER 18-20"	#5 @ 12"	#5 @ 12"	EACH FACE
OVER 20-24"	#5 @ 10"	#5 @ 10"	EACH FACE

TYPICAL CONCRETE WALL REINFORCING DETAILS 10

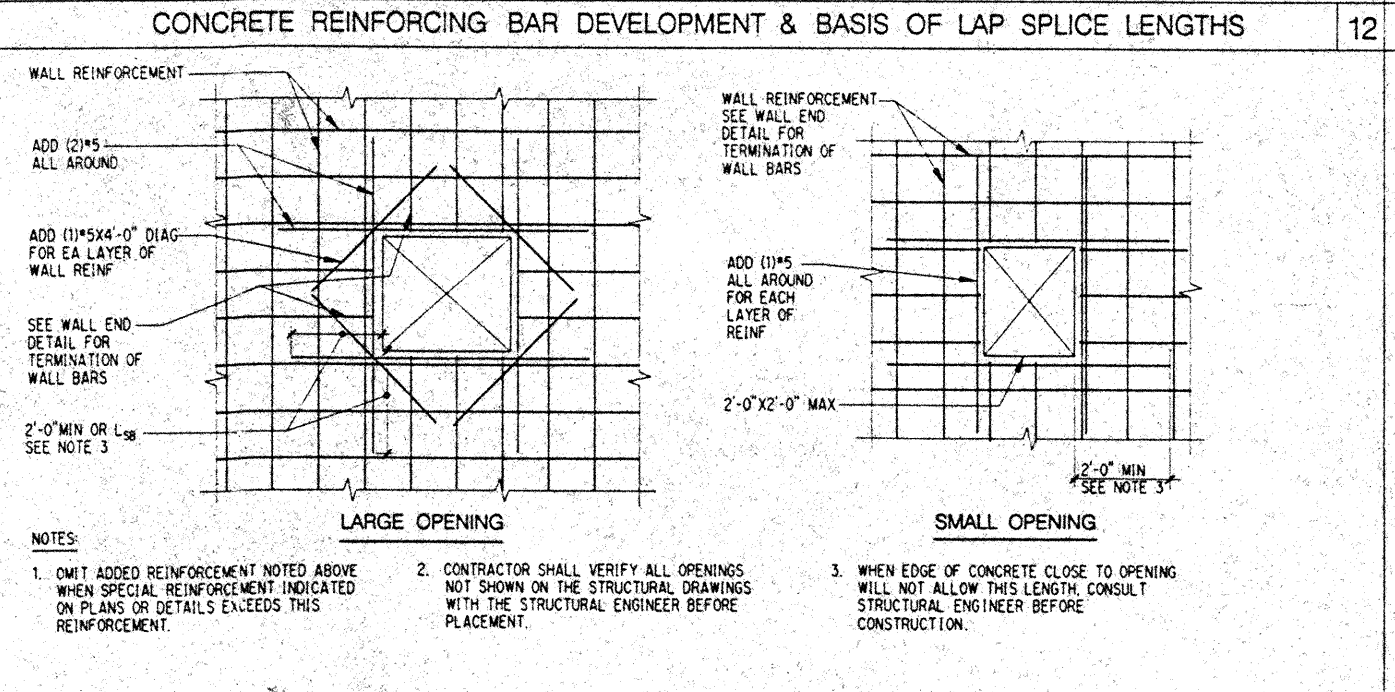


TYPICAL CONCRETE WALL REINFORCING DETAILS 15

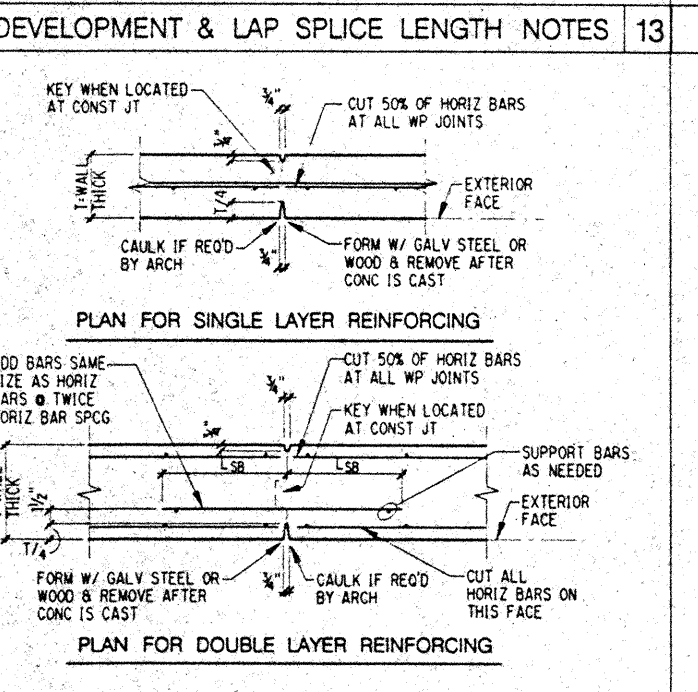
CONCRETE REINFORCING BAR DEVELOPMENT & BASIS OF LAP SPICE LENGTHS

L _d f _c = 3000 psi			L _d f _c = 4000 psi			L _d f _c = 5000 psi		
BAR SIZE	CTR TO CTR SPACING (S)	CONC COVER (C)	BAR SIZE	CTR TO CTR SPACING (S)	CONC COVER (C)	BAR SIZE	CTR TO CTR SPACING (S)	CONC COVER (C)
#3	13	13	#3	12	12	#3	12	12
#4	17	17	#4	15	15	#4	14	13
#5	21	21	#5	18	18	#5	16	16
#6	25	25	#6	22	22	#6	20	21
#7	29	29	#7	26	26	#7	24	23
#8	33	33	#8	30	30	#8	28	27
#9	37	37	#9	34	34	#9	32	31
#10	41	41	#10	38	38	#10	36	35
#11	45	45	#11	42	42	#11	40	39
#14	137	137	#14	119	119	#14	106	106
#18	274	274	#18	237	237	#18	212	212

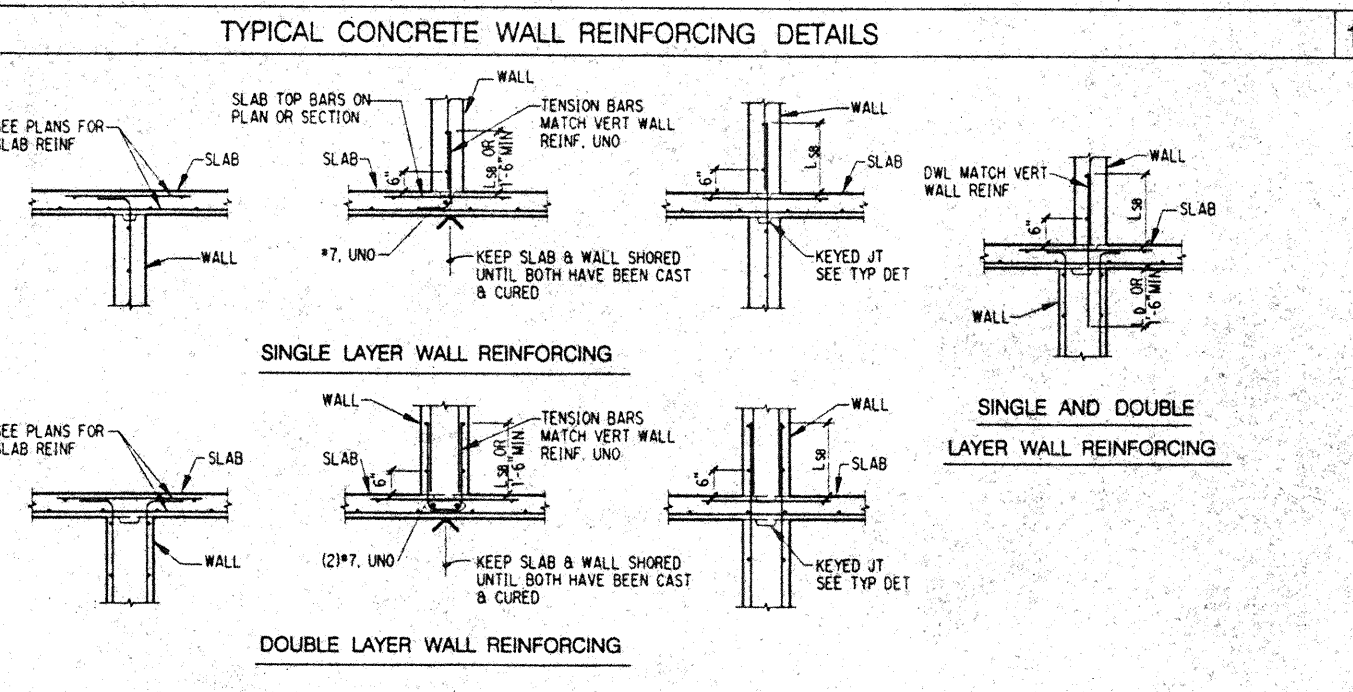
- NOTES:
- NOTATIONS:
d_s: NOMINAL BAR DIAMETER
L_d: TENSION DEVELOPMENT LENGTH FOR OTHER THAN TOP BARS
L_t: DEVELOPMENT LENGTH OF TOP BARS IN TENSION = 1.3 x L_d
L_b: DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = 19 x d_s
L_c: TIED COLUMN LAP SPICE IN COMPRESSION = 30 x d_s
L_{cs}: SPIRAL COLUMN LAP SPICE IN COMPRESSION = 22.5 x d_s
L_{sb}: TENSION LAP SPICE LENGTH FOR OTHER THAN TOP BARS = 1.3 x L_d
L_{sbt}: TENSION LAP SPICE LENGTH OF TOP BARS = 1.69 x L_d
 - TOP BARS: HORIZONTAL REINF WITH MORE THAN 12" OF CONCRETE CAST BELOW.
 - REINFORCING f_y = 60,000 psi
 - FOR f_c = 3000 psi: L_b = 22x d_s
 - #14 & #18 BARS SHALL NOT BE LAP SPICED. SEE GENERAL NOTES ALSO.
 - IN SLABS AND WALLS, FOR #11 OR SMALLER BARS WITH S₆₀, DEVELOPMENT LENGTH MAY BE MULTIPLIED BY 0.80 EXCEPT IT SHALL NOT BE TAKEN LESS THAN THE LENGTH SHOWN UNDER S₂₄.



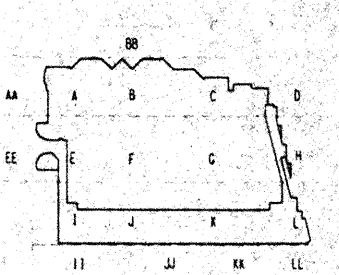
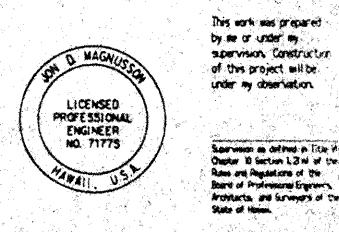
TYPICAL CONCRETE WALL OPENING REINFORCEMENT 17



TYP WEAKENED PLANE JT FOR CONC WALL 18



TYPICAL CONCRETE REINFORCING FOR INTERSECTIONS OF SLABS AND WALLS 20

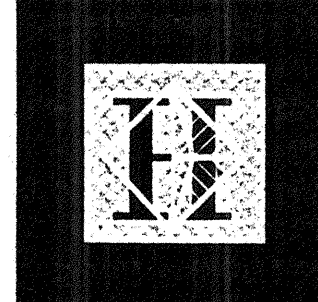


Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
11-7-95			BUILDING PERMIT SET
9-15-95			BUILDING PERMIT SET
8-4-95			BUILDING PERMIT SET
6-19-95			ISSUED FOR FOUNDATION PERMIT
4-17-95			ISSUED FOR FOUNDATION PERMIT

TYPICAL CONCRETE DETAILS

Designed: BJM
 Drawn: DMS
 Checked: JDM
 Date: 4 / 12 / 95
 Job No.: 93036.02.9
 Sheet No.



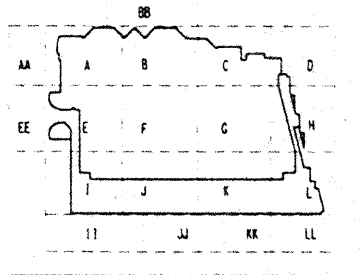
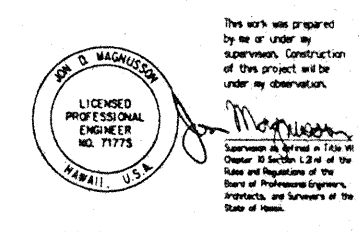
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Convention Center Authority
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1301 Fifth Avenue, Suite 5200 Seattle WA 98101-2699
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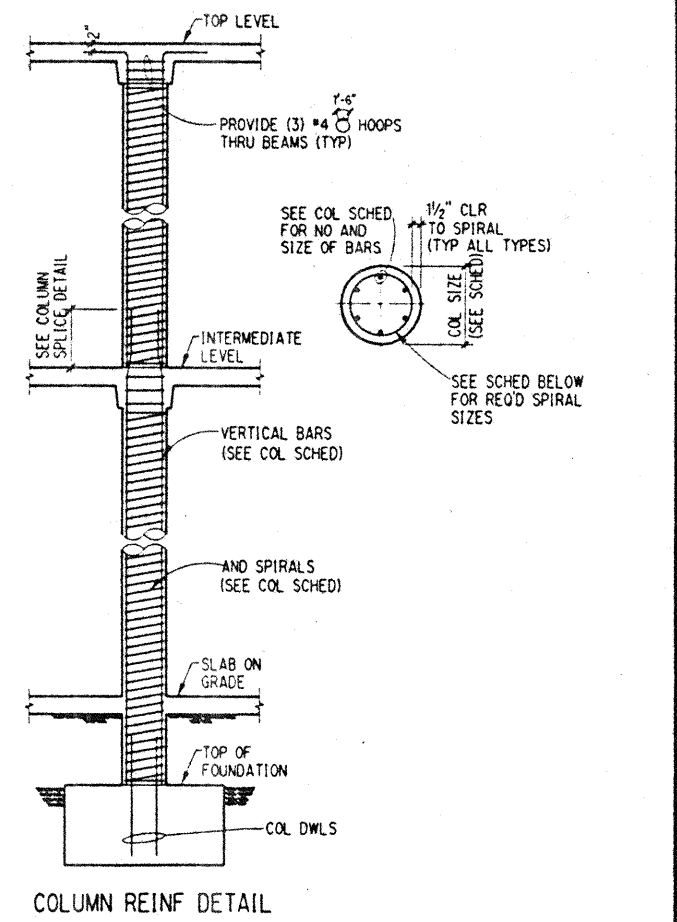
Revisions

12-15-95	FINAL DOCUMENTS
9-15-95	BUILDING PERMIT SET
8-4-95	
6-19-95	
4-17-95	ISSUED FOR FOUNDATION PERMIT

No. Date By Description

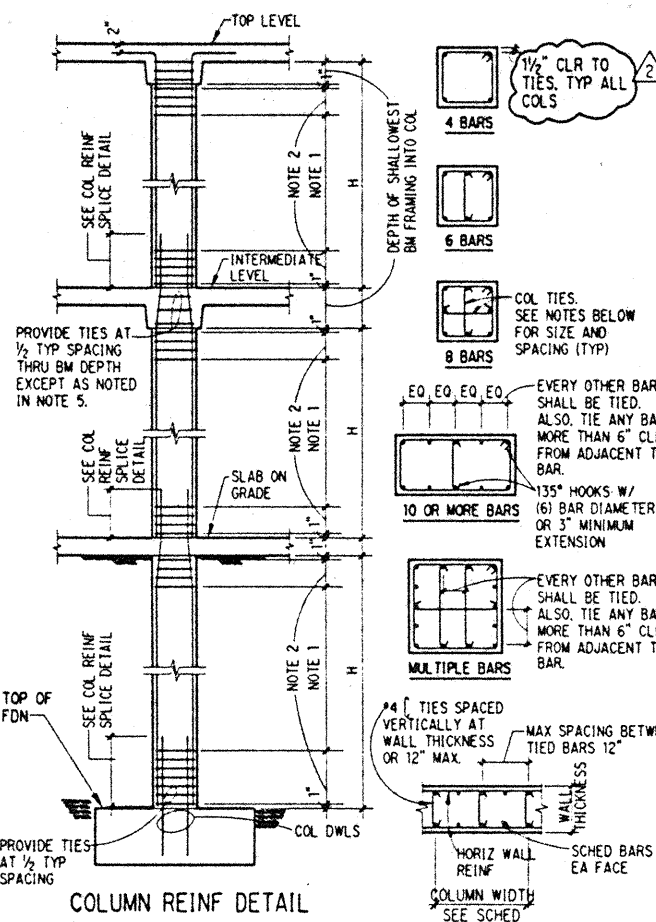
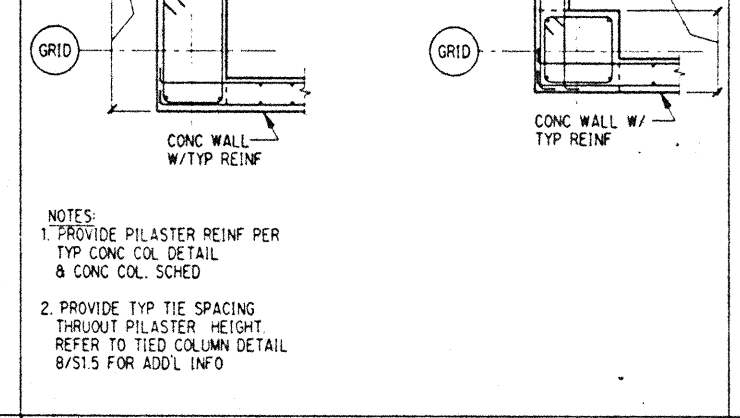
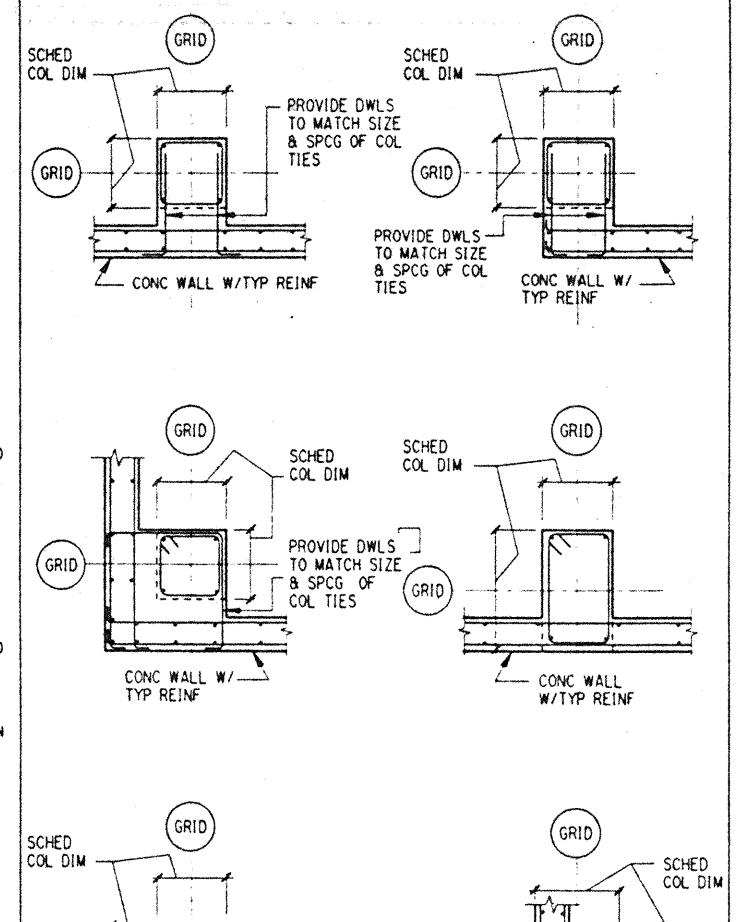
TYPICAL CONCRETE DETAILS

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Checked JDM
Date 4 / 12 / 95
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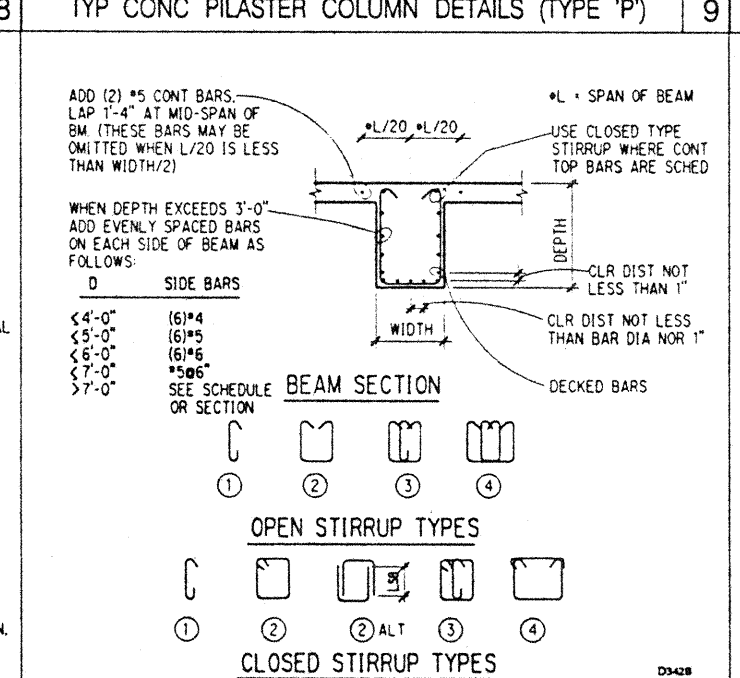
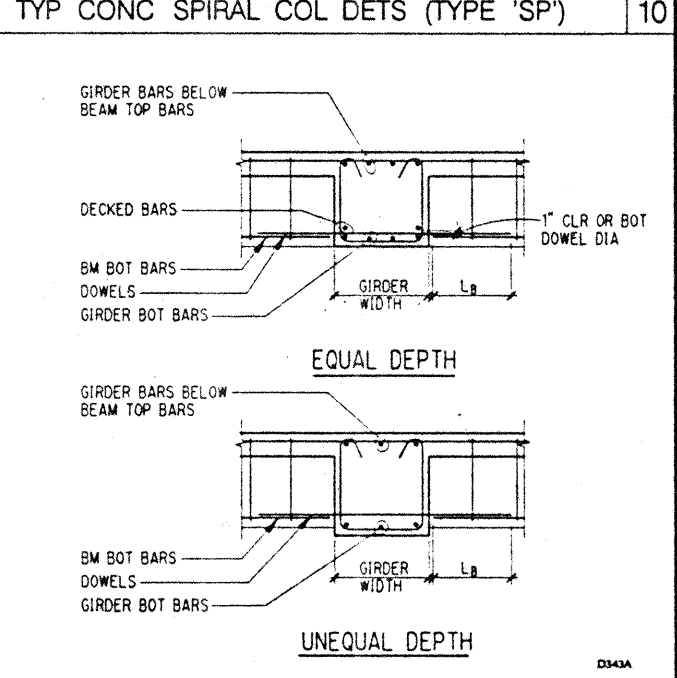
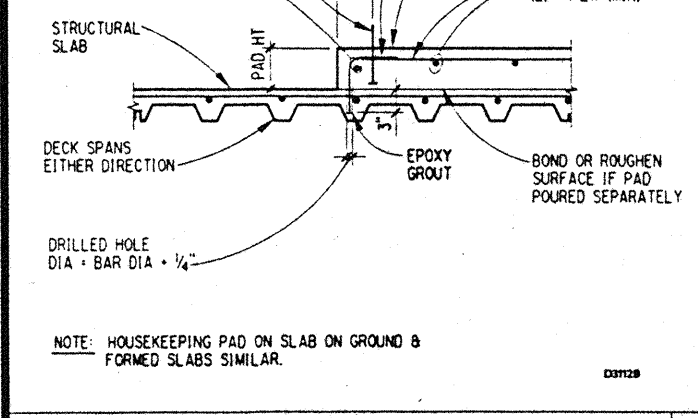
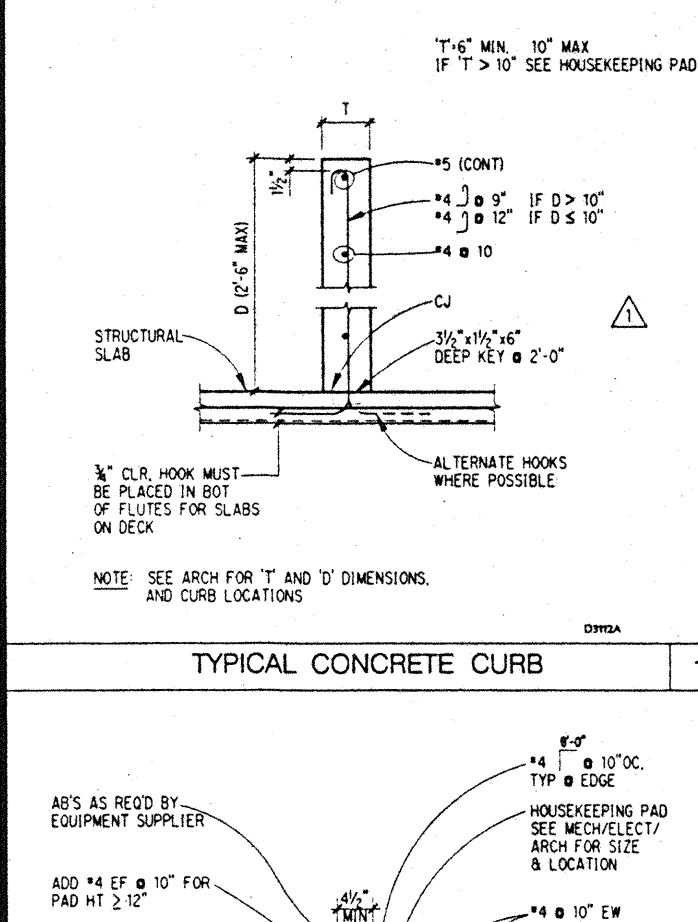
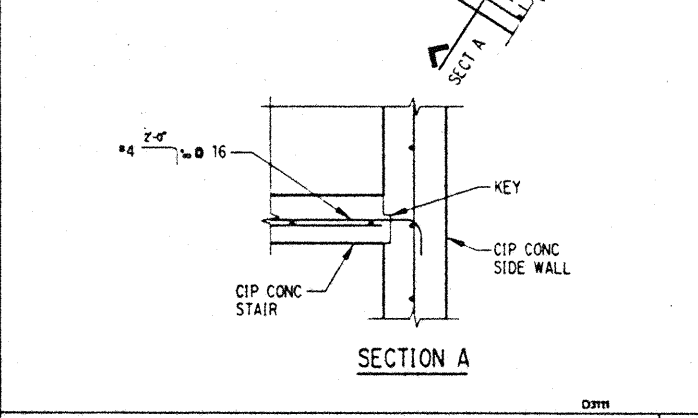
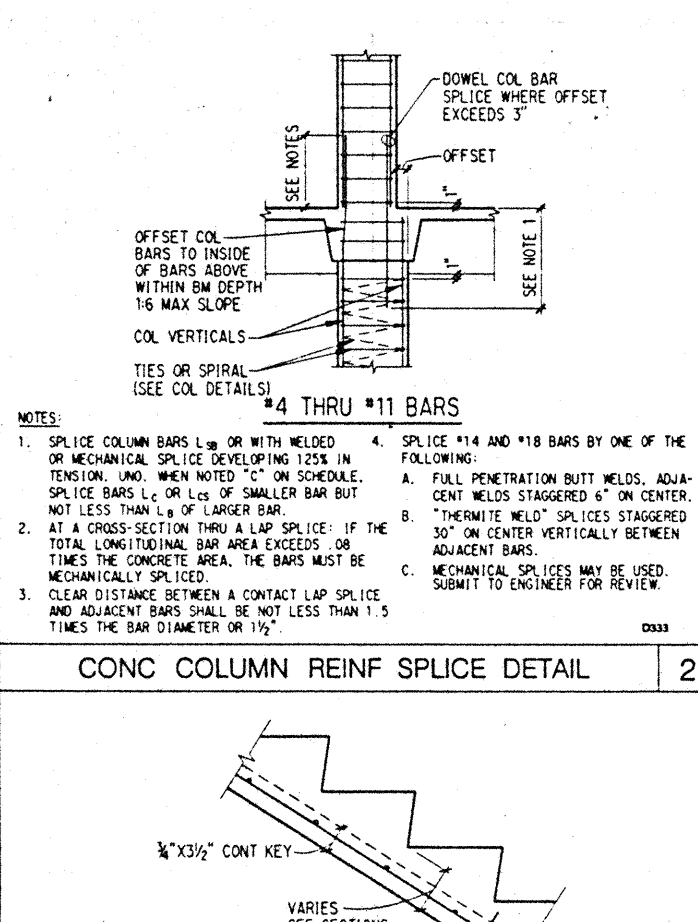


NOTE: SPIRAL SHALL BE CONTINUOUS FROM TOP OF SLAB OR FOOTING TO SOFFIT ABOVE. ADD ONE-HALF EXTRA TURN AT TOP AND BOTTOM.

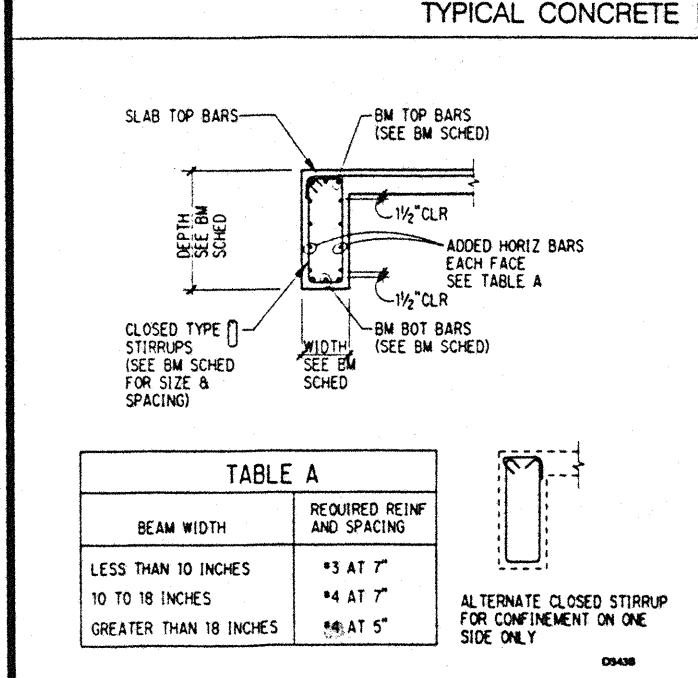
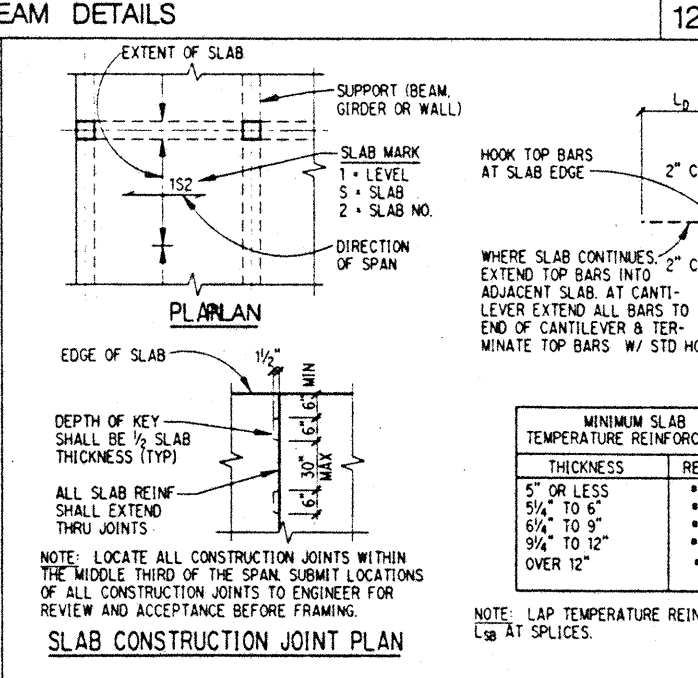
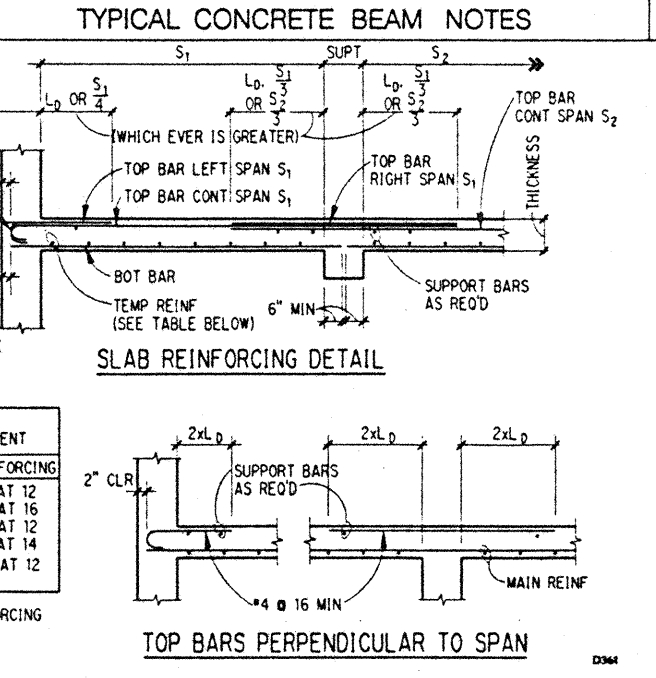
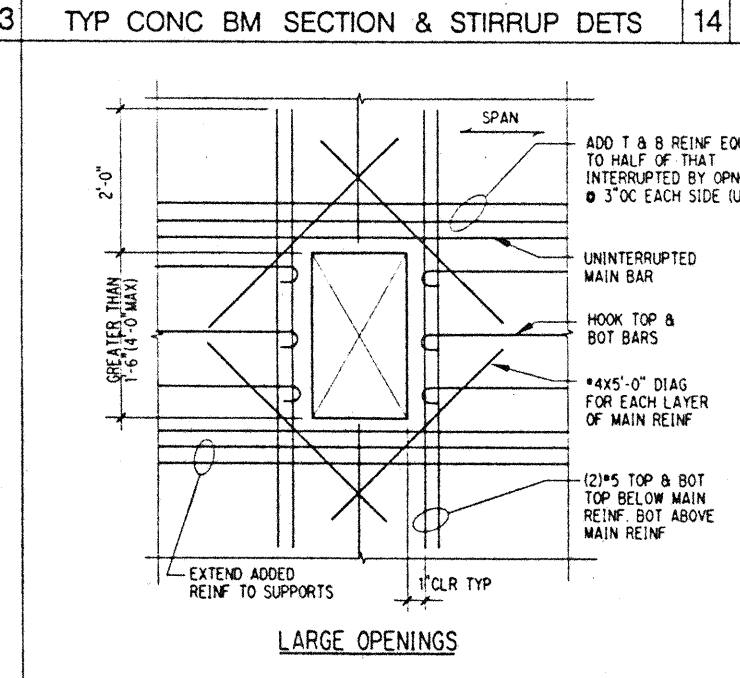
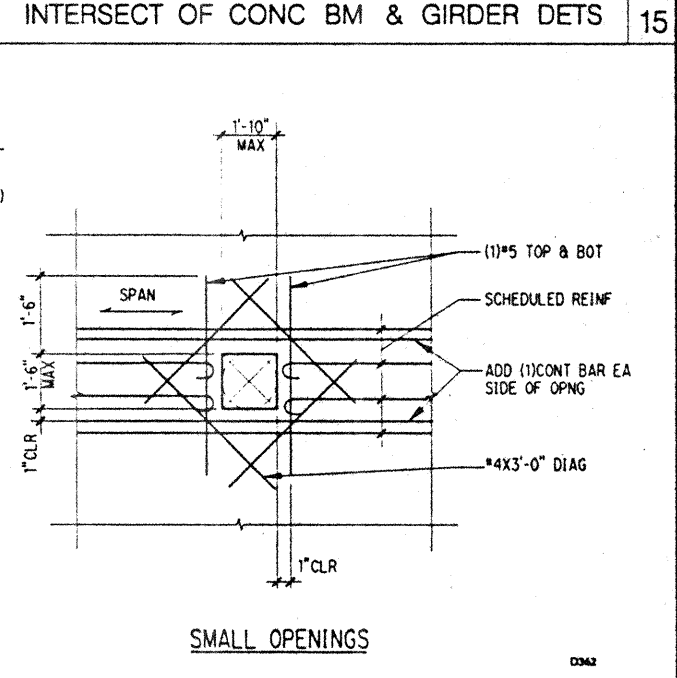
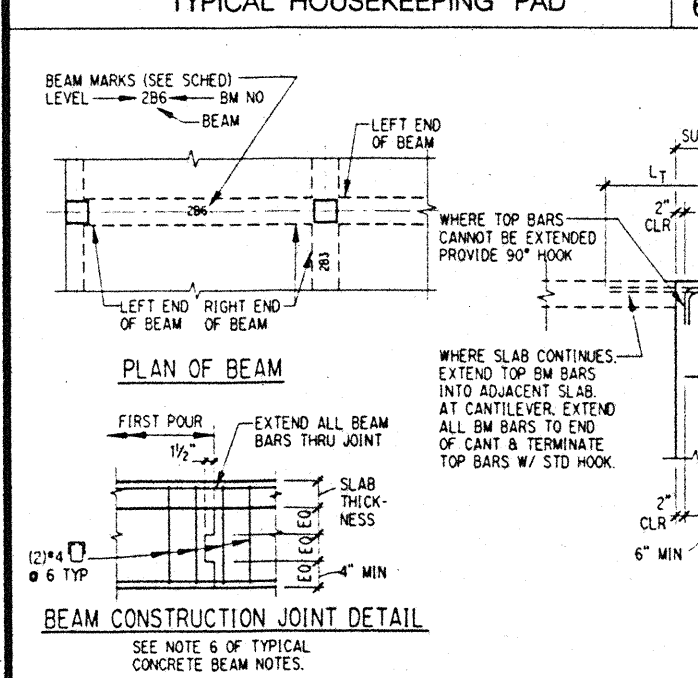
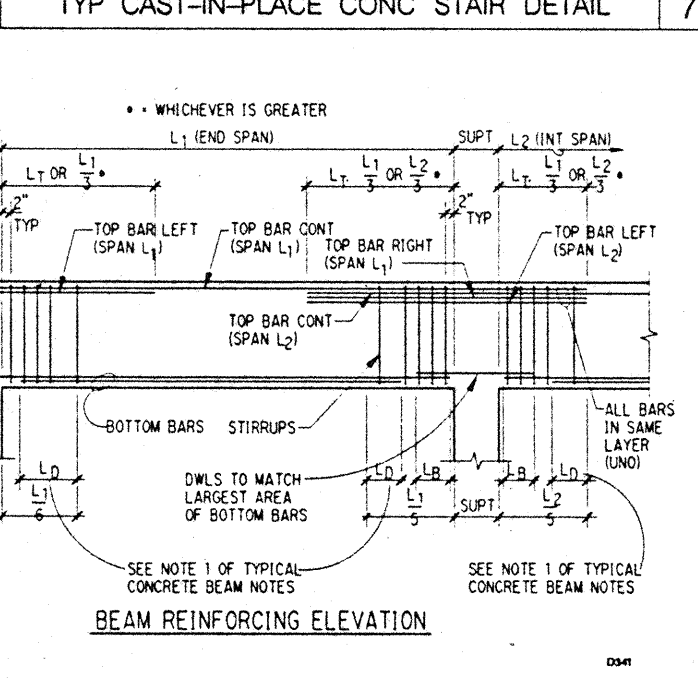
COL DIA.	F'c = 4,000 PSI	F'c = 7,500 PSI
4'-0"	2 1/2" DIA @ 2 1/4"	2 1/2" DIA @ 2 1/4"
6'-0"	3" DIA @ 3"	3" DIA @ 3"
8'-0"	3 1/2" DIA @ 3 1/2"	3 1/2" DIA @ 3 1/2"



- NOTES:
- TYPICAL TIE SPACING SHALL NOT EXCEED SPACING SCHEDULED, 16 LONGITUDINAL BAR DIAMETERS, 48 BAR DIAMETERS, OR LEAST DIMENSION OF COLUMN.
 - TIE SPACING SHALL BE ONE-HALF SPACING REQUIRED BY NOTE #1 FOR 1/4, MAXIMUM COLUMN DIMENSION OR 18 INCHES, WHICHEVER IS GREATER.
 - ALL COLUMN TIES SHALL BE #4, UNO.
 - AT CONTRACTOR'S OPTION LARGER SCHEDULED BARS MAY BE EXTENDED UP ADDITIONAL FLOOR LEVELS WITHOUT SPLICES AT FLOORS.
 - TIES ARE NOT REQUIRED WHERE BEAMS FRAME INTO FOUR SIDES OF COLUMN.

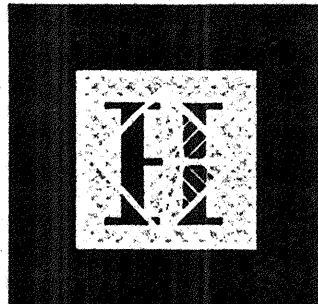


- NOTES:
- AT CONTRACTOR'S OPTION, WHERE REQUIRED TO RELIEVE BAR CONGESTION, NOT MORE THAN 50 PERCENT OF THE AREA OF THE STRAIGHT BOTTOM BARS MAY BE TERMINATED AS SHOWN.
 - BEAM SCHEDULES DO NOT INDICATE REQUIREMENTS FOR ARRANGING BARS. THE CONTRACTOR SHALL DETAIL AND PLACE REINFORCING STEEL IN NO MORE THAN TWO LAYERS ONLY WHERE REQUIRED TO PROVIDE PROPER CLEARANCES BETWEEN BARS IN A LAYER AND WHERE REQUIRED IN ORDER TO PROPERLY CLEAR COLUMN VERTICALS AND SIMILAR REINFORCING.
 - REFER TO TABLE OF SPLICE AND DEVELOPMENT LENGTHS FOR L_d AND L_d.
 - EITHER 90- OR 180-DEGREE STANDARD HOOK BARS MAY BE USED FOR LONGITUDINAL BARS.
 - WHERE TOP AND SIDE BARS ARE INDICATED AS CONTINUOUS AND RUN OVER 60 FEET IN LENGTH, SPLICES MAY BE MADE USING THE L_{sb} COLUMN IN THE TABLE OF DEVELOPMENT LENGTHS. TOP BAR SPLICES SHOULD BE IN THE MIDDLE THIRD OF BEAM SPAN, WHERE BOTTOM BARS ARE SHOWN AS CONTINUOUS AND RUN IN EXCESS OF 60 FEET, A LAP SPLICE MAY BE USED EQUAL TO THE DEVELOPMENT LENGTHS SHOWN IN COLUMN L_{sb} OF THE TABLE OF DEVELOPMENT LENGTHS. THE BOTTOM BAR SPLICES SHOULD BE OUTSIDE THE MIDDLE THIRD OF THE BEAM SPAN. THE SIDE BAR SPLICES MAY BE MADE, WHERE CONVENIENT.
 - LOCATE ALL CONSTRUCTION JOINTS WITHIN THE MIDDLE THIRD OF SPANS. JOINTS SHALL BE OFFSET AT A MINIMUM DISTANCE OF TWO TIMES THE WIDTH OF INTERSECTING BEAMS. SUBMIT LOCATION OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW AND ACCEPTANCE BEFORE FORMING.
 - STANDARD HOOKS FOR STIRRUPS SHALL BE 135 DEGREE BEND PLUS 6d EXTENSION, BUT NOT LESS THAN 3 INCHES.



BEAM WIDTH	REQUIRED REINF AND SPACING
LESS THAN 10 INCHES	#3 AT 7"
10 TO 18 INCHES	#4 AT 7"
GREATER THAN 18 INCHES	#4 AT 5"

11/26/95 vsp5-dgn



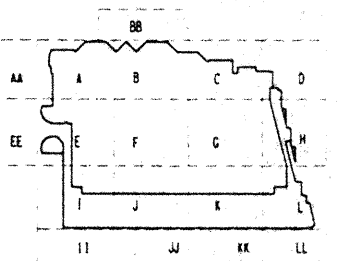
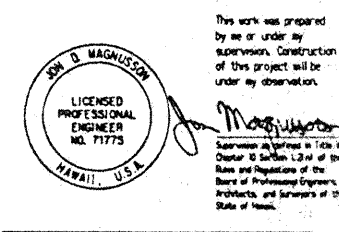
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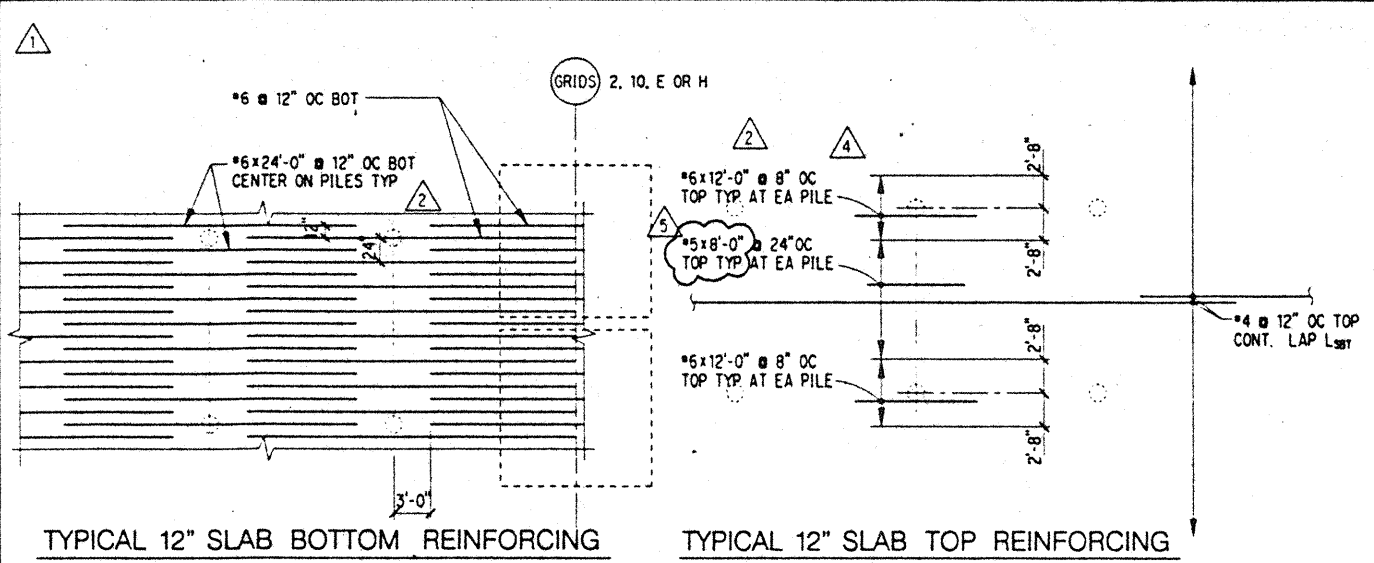
Revisions

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11-7-95	
9-25-95	BUILDING PERMIT SET
8-31-95	
8-4-95	
6-19-95	
4-17-95	ISSUED FOR FOUNDATION PRINT

No. Date By Description

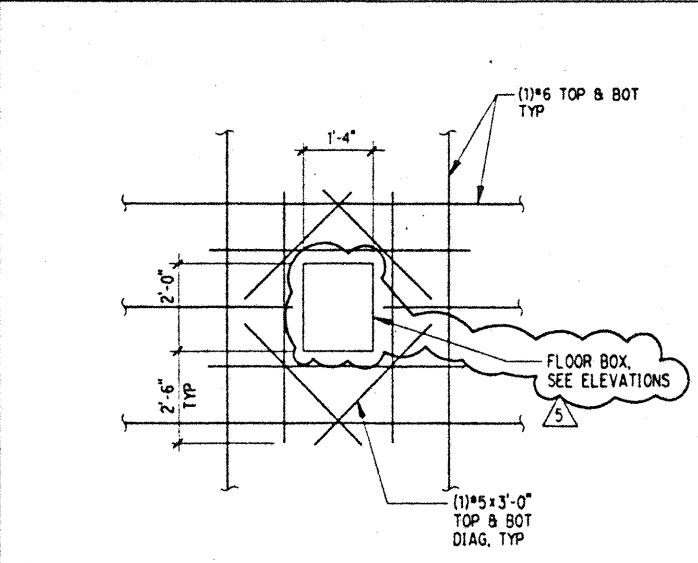
Sheet Title
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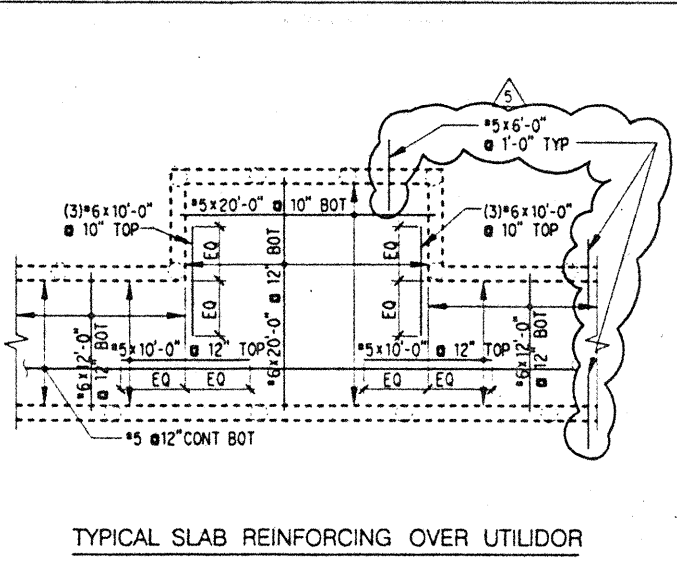


NOTES:
1. BARS SHOWN IN DETAILS ARE SHOWN IN ONLY ONE DIRECTION FOR CLARITY. REINFORCING SHALL BE TYPICAL FOR BOTH DIRECTIONS.
2. SEE PLANS FOR ADDITIONAL REINFORCING NOT SHOWN IN TYPICAL DETAILS.

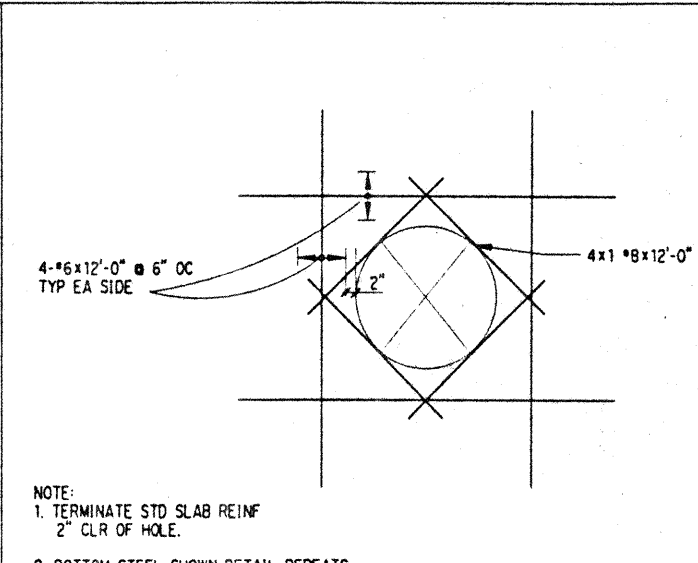
TYPICAL SLAB REINFORCING FOR EXHIBIT HALL SLAB WITHIN GRIDS 2-10 & E-H



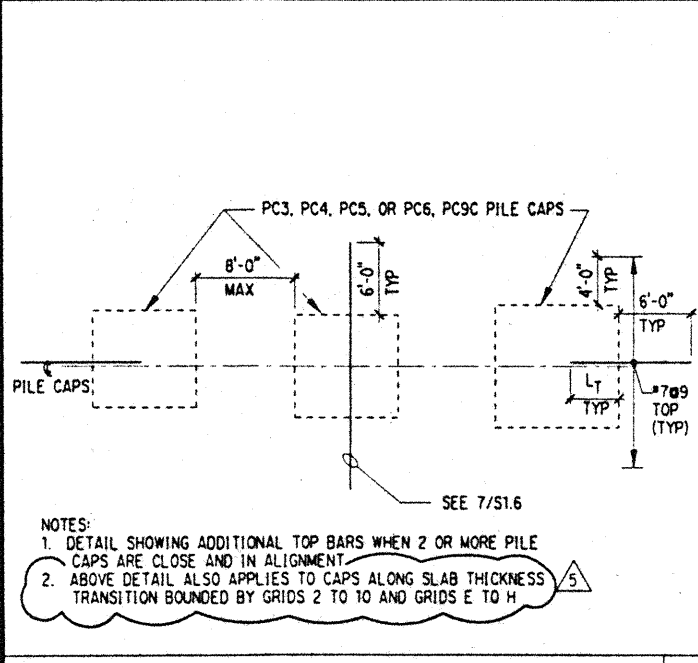
TYPICAL FLOOR BOX DETAIL



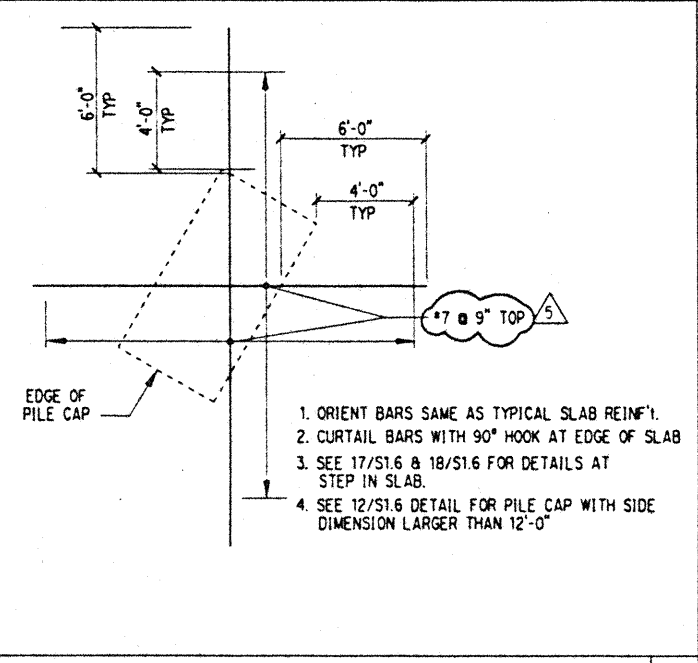
TYPICAL SLAB REINFORCING DETAIL



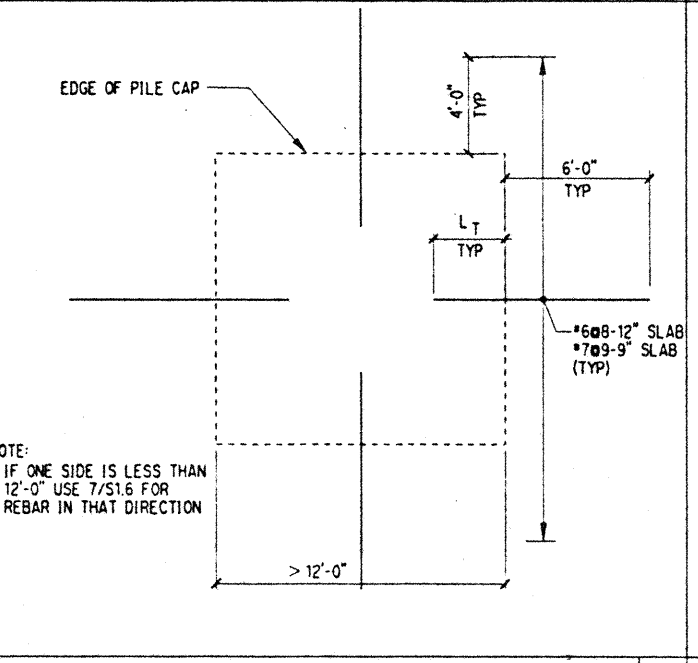
ADD'L REINF AT 6'-0" DIA OPENING



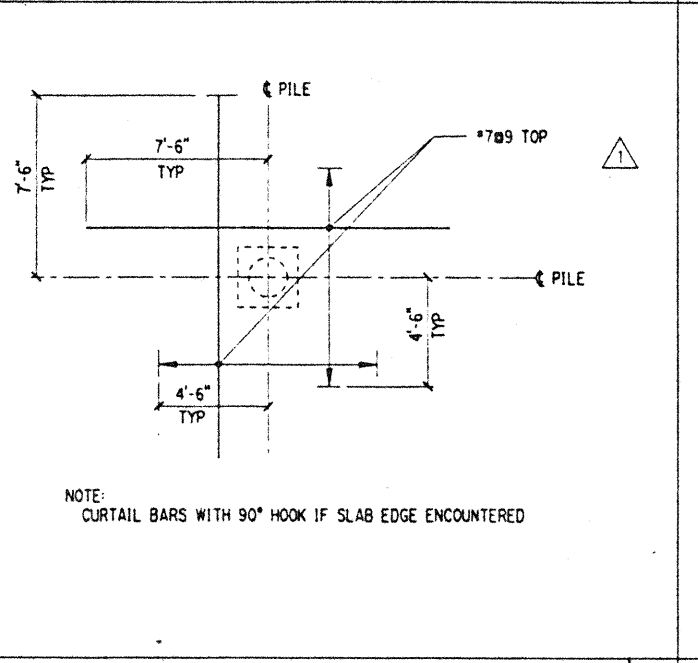
TOP REINF'T AT PILE CAP STRING



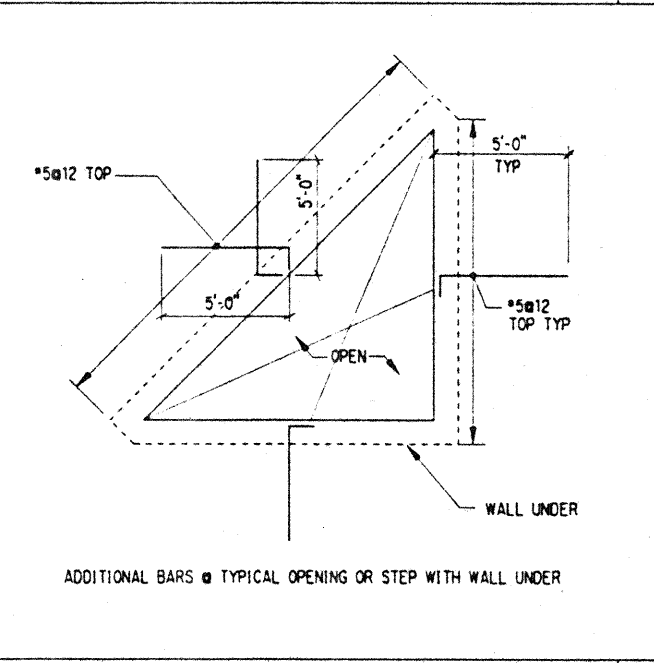
ADD'L SLAB TOP REINF AT MULTI-PILE CAP



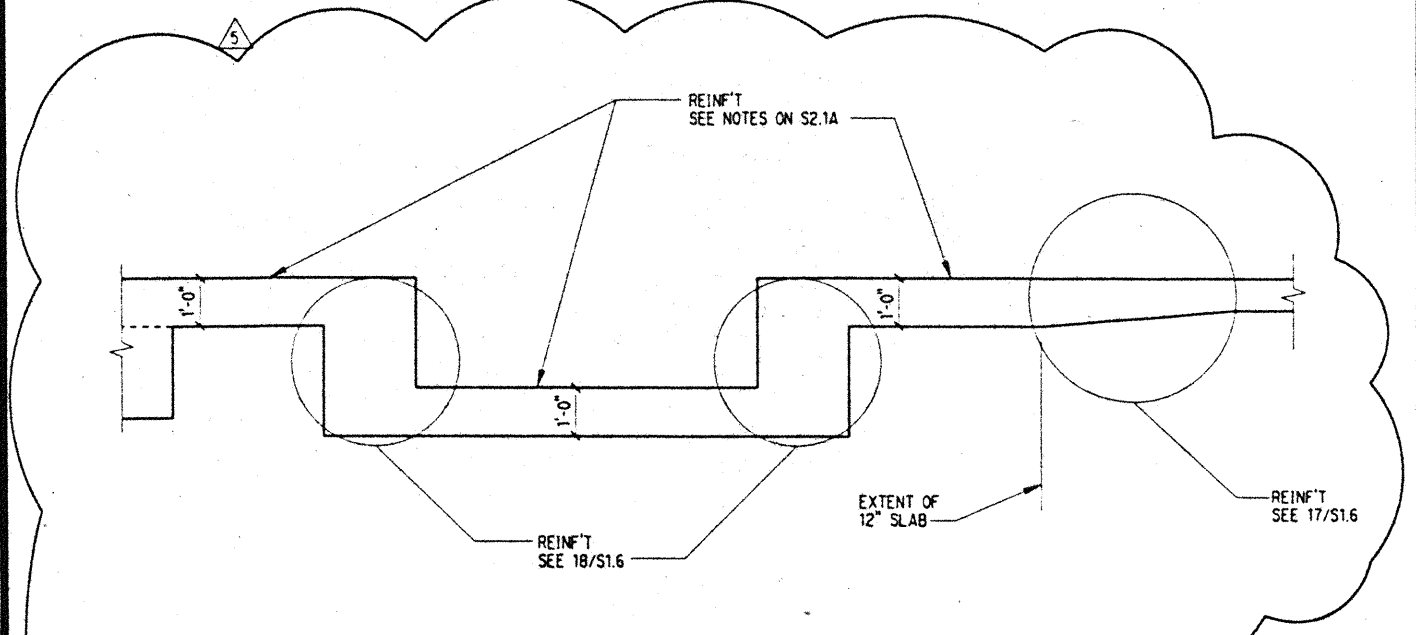
ADD'L TOP REBAR AT LARGE PILE CAPS



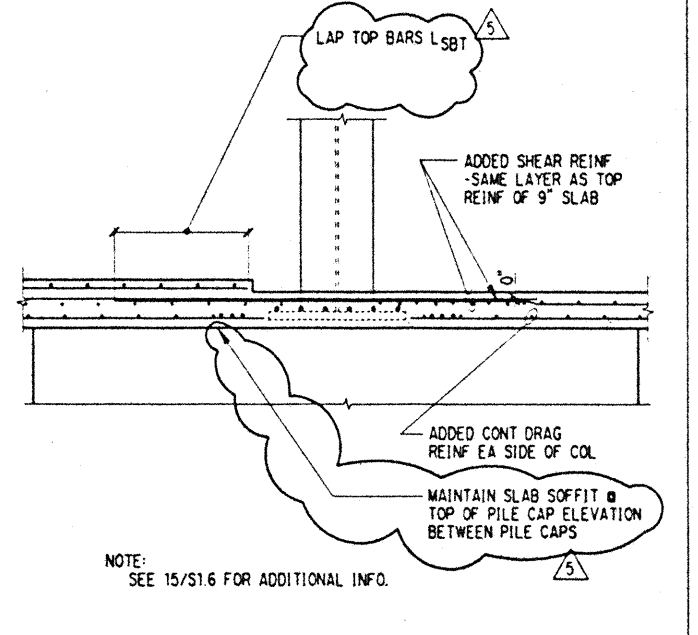
ADD'L SLAB TOP REINF @ SINGLE PILE CAP



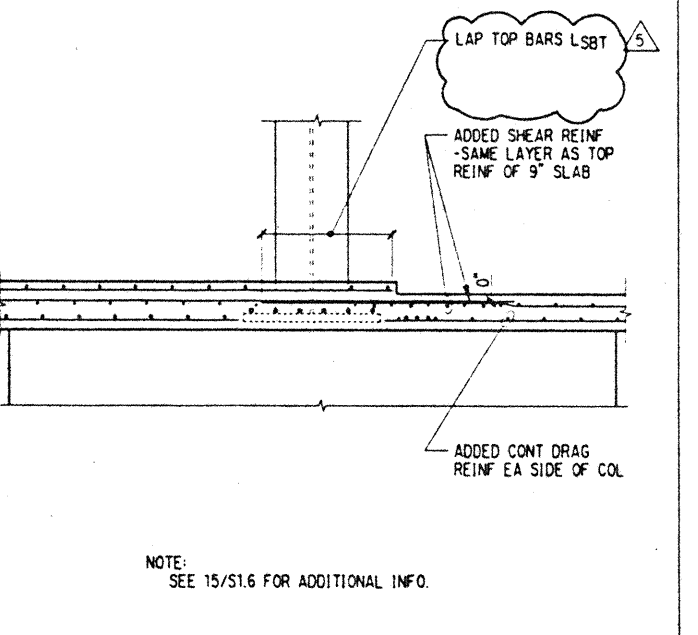
DETAIL



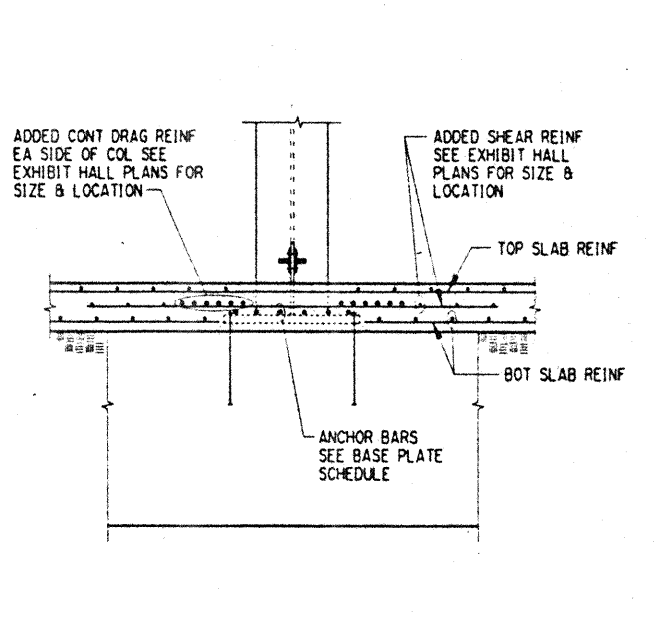
SECTION



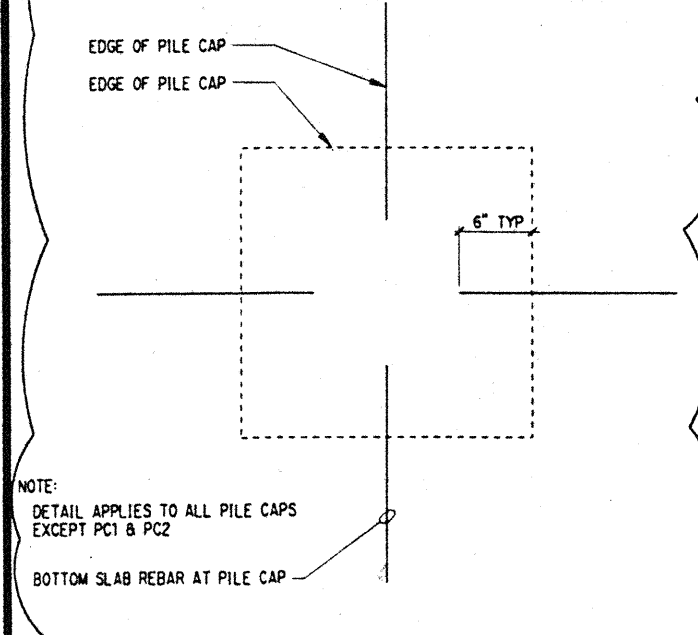
TYPICAL SLAB STEP @ PILE CAP @ GRID E



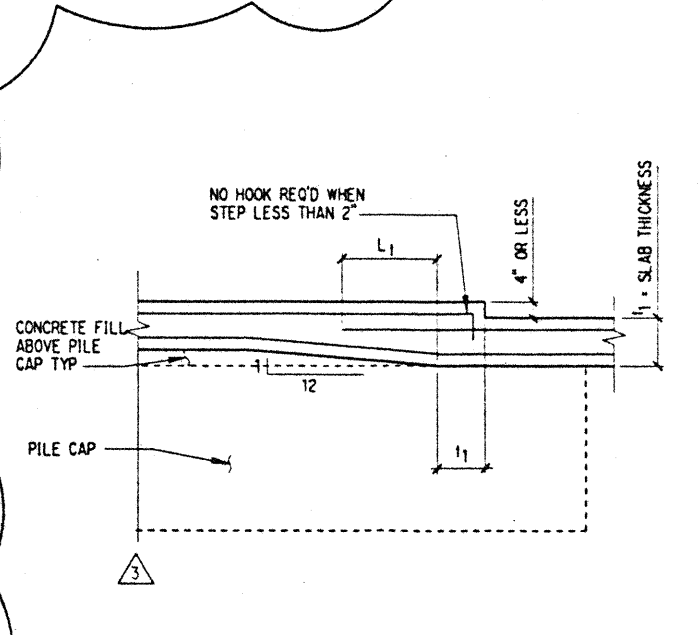
TYP SLAB STEP @ PILE CAP @ GRIDS E & 2



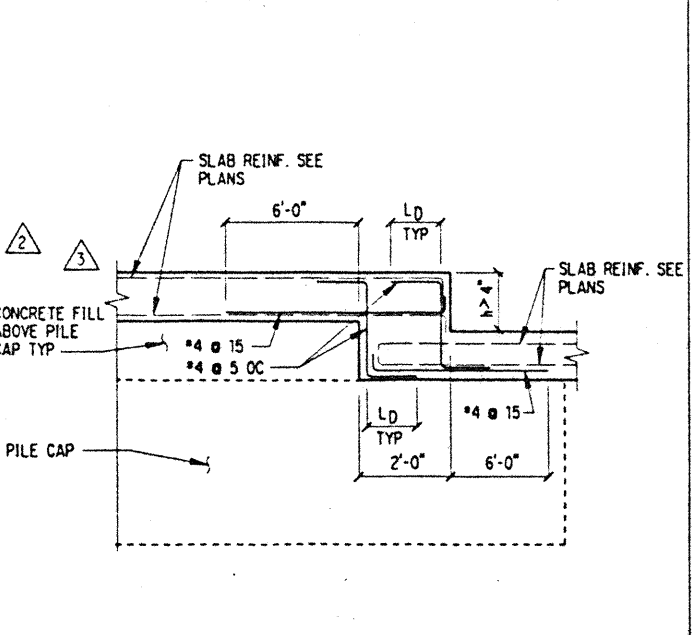
SECTION



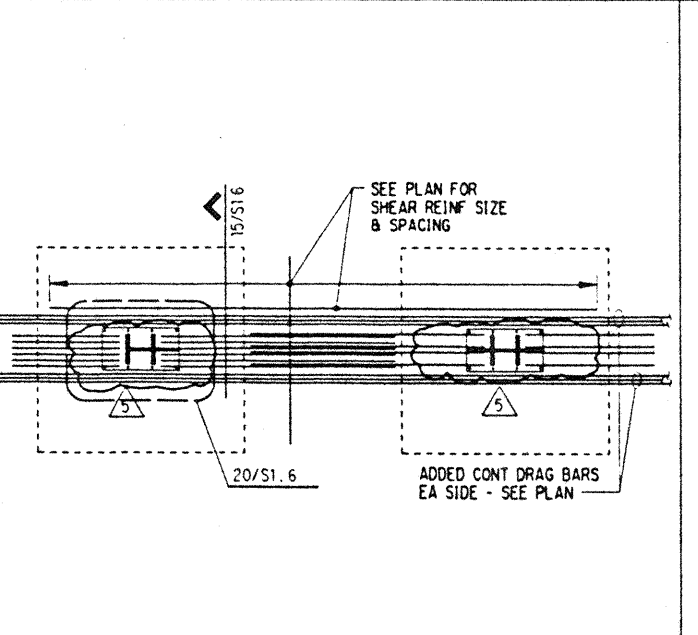
BOTTOM SLAB REBAR AT PILE CAPS



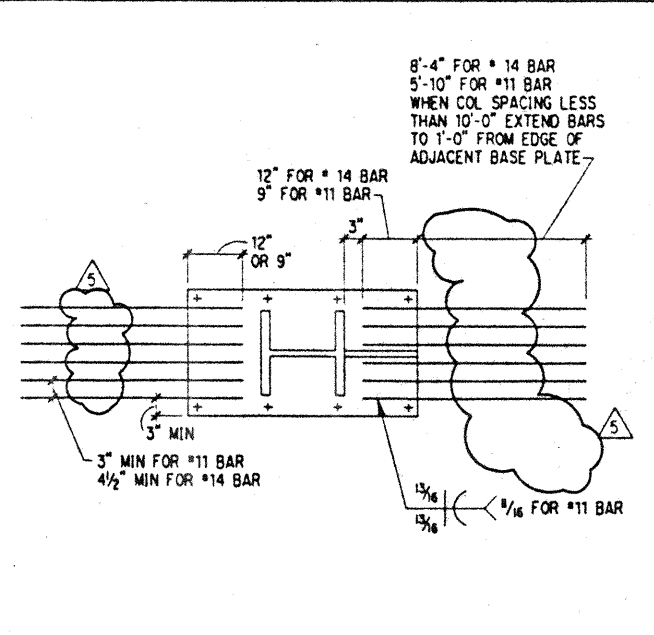
SLAB STEP 4" OR LESS



SLAB STEP GREATER THAN 4" TO 2'-0"

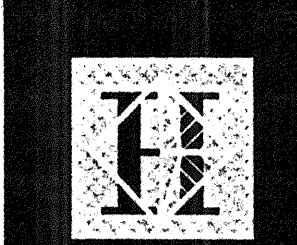


TYP ADDED SLAB REINF @ FRAME COLS



ANCHOR BAR DETAIL

Fri Dec 11 13:38:30 1995 j:\hcc\wyp6.dgn JWS pit

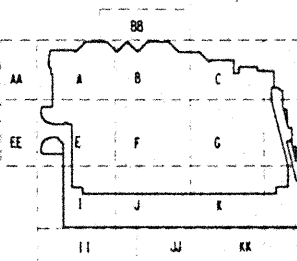
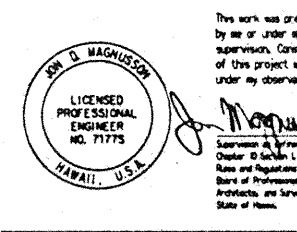


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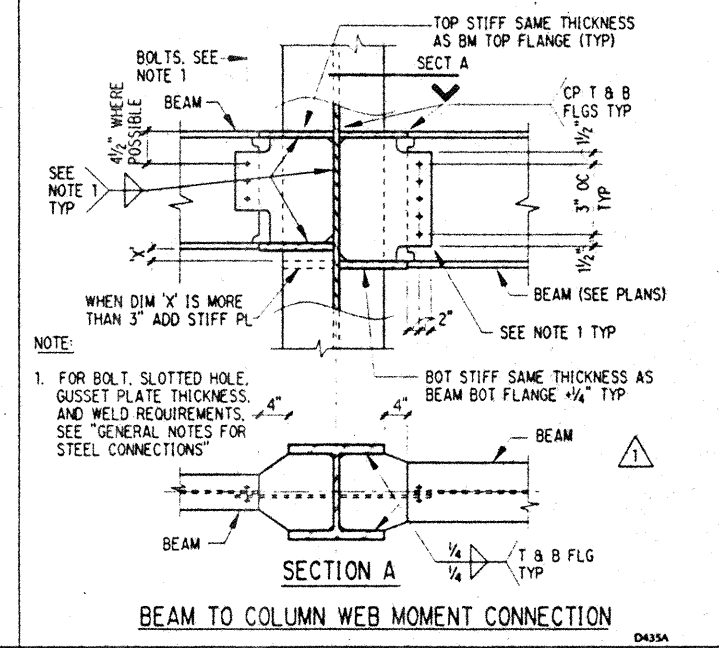


Revisions

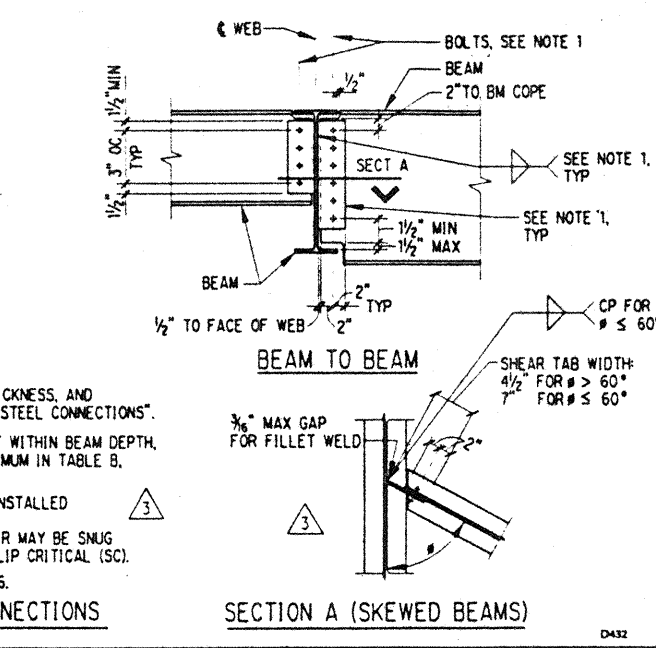
12-15-95	FINAL DOCUMENTS
9-15-95	BUILDING PERMIT SET
8-31-95	
8-4-95	
6-19-95	
5-8-95	ADDENDUM #3
No. Date By Description	

TYPICAL STEEL DETAILS

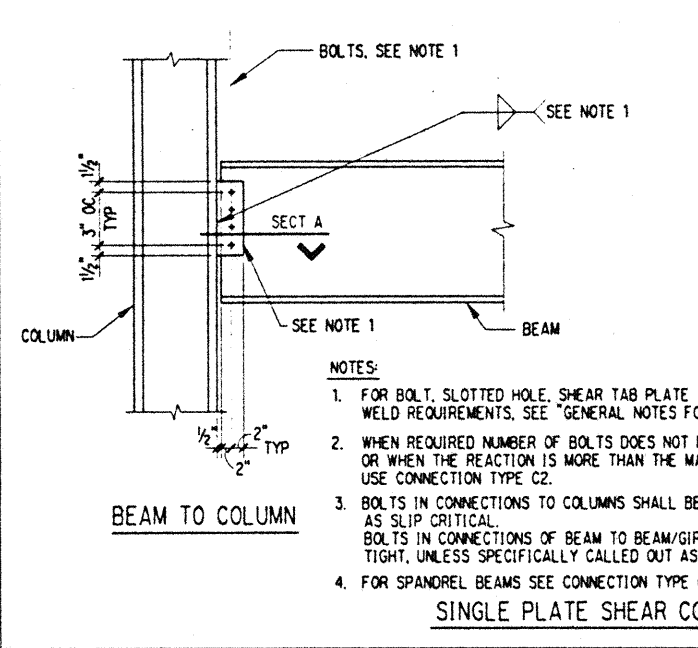
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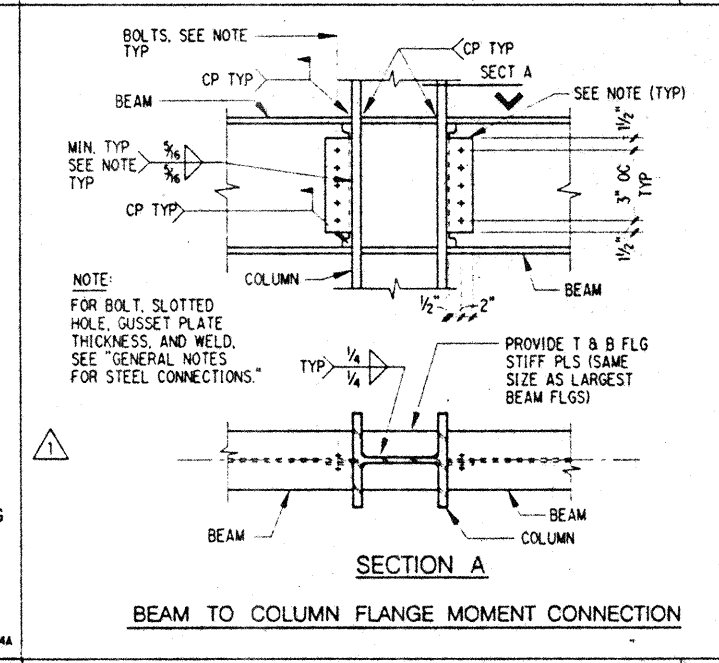
TYPICAL STEEL CONNECTION, TYPE C5



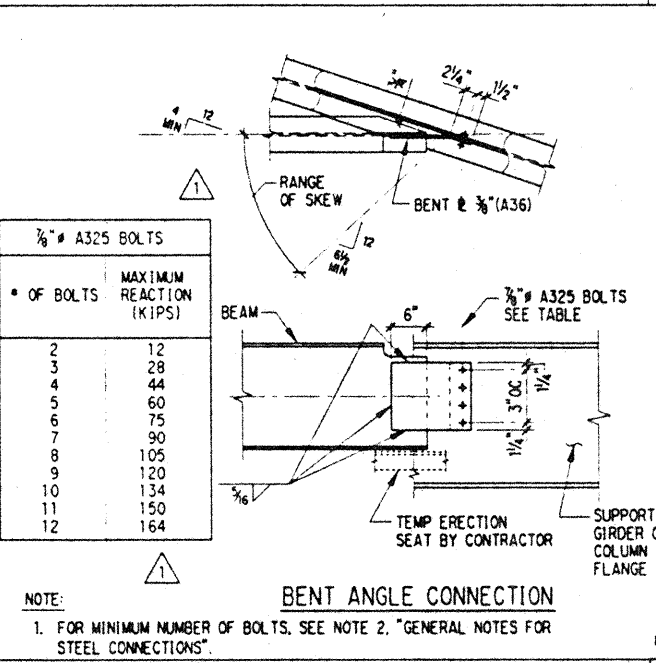
TYPICAL STEEL CONNECTION, TYPE C1



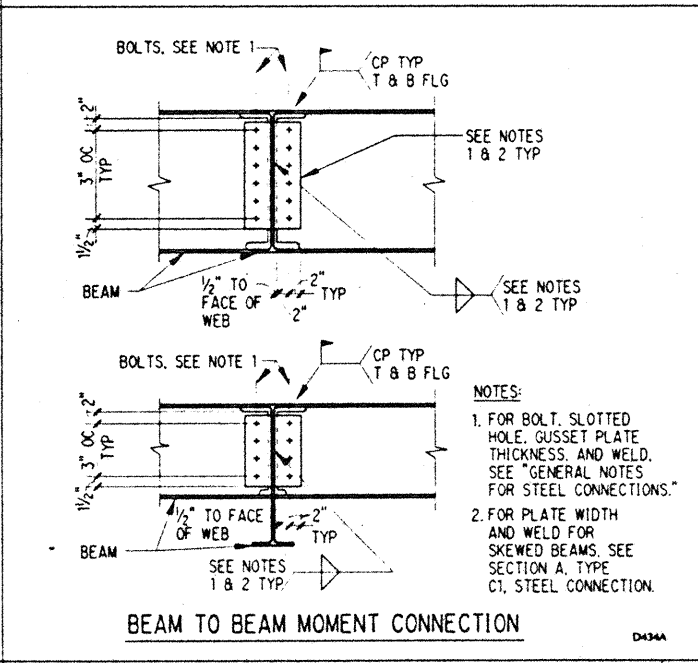
TYPICAL STEEL CONNECTION, TYPE C2



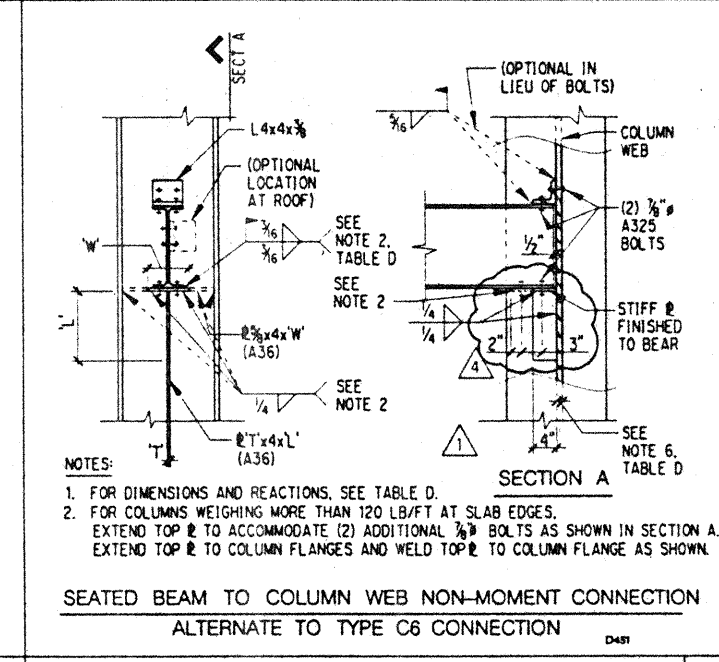
TYPICAL STEEL CONNECTION, TYPE C4



TYPICAL SKEWED BEAM, TYPE C8



TYPICAL STEEL CONNECTION, TYPE C3



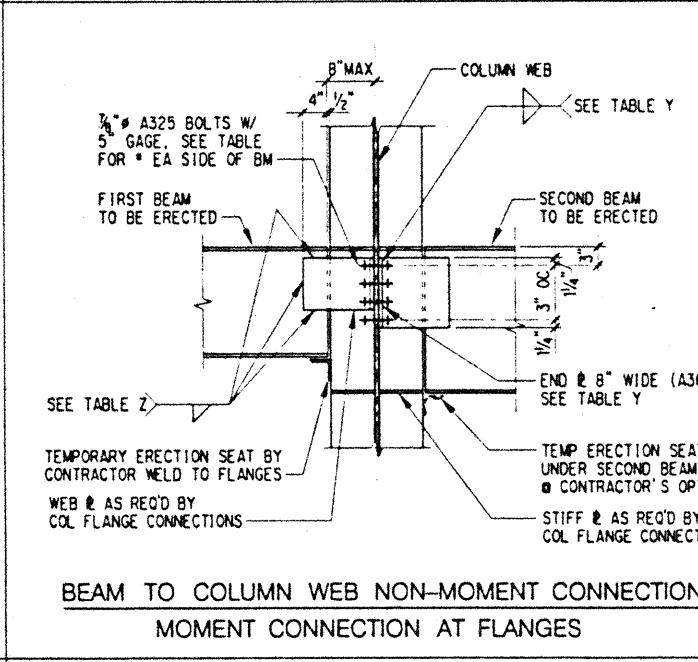
TYPICAL STEEL CONNECTION, TYPE C20

TABLE Z

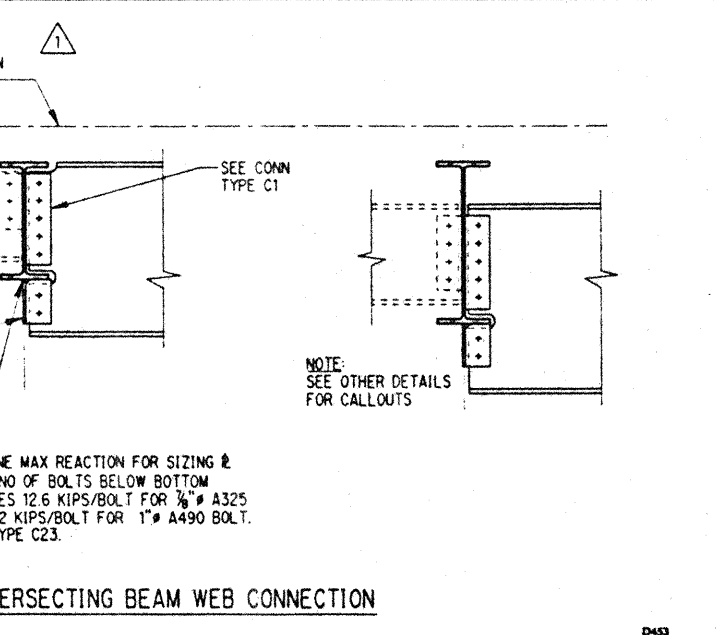
# OF BOLTS PER SIDE	MAXIMUM REACTION (KIPS)	WELD WEB PLATE TO BM WEB	WEB PLATE THICKNESS (INCHES)
2	7	1/4	1/2
3	17	1/4	1/2
4	31	1/4	1/2
5	48	1/4	1/2
6	105	3/8	3/8
7	145	3/8	3/8
8	190	3/8	3/8
9	228	3/8	3/8
10	254	3/8	3/8

NOTE: 1. SEE CONNECTION C6 FOR END PLATE SIZE, WELD AND MINIMUM BEAM WEB THICKNESS.

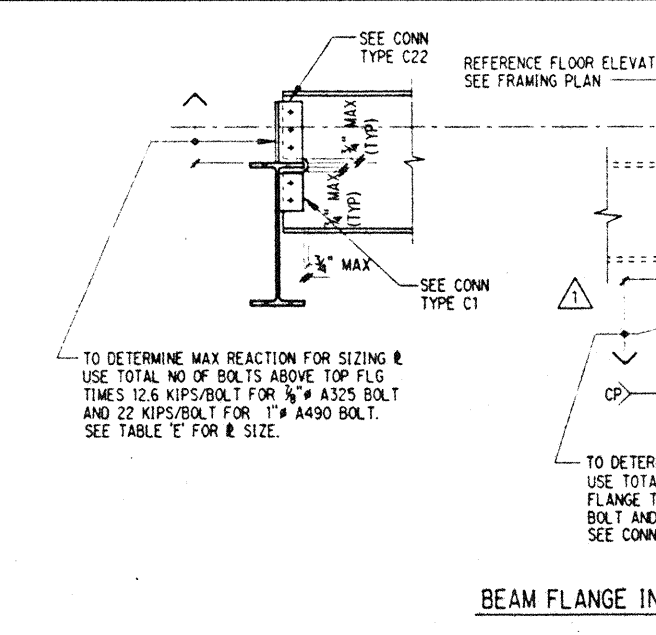
TYPICAL STEEL CONNECTION, TYPE C7



TYPICAL STEEL CONNECTION, TYPE C7



TYPICAL STEEL CONNECTION, TYPE C2



TYPICAL STEEL CONNECTION, TYPE C6

TABLE A - MINIMUM NO. BOLTS

WIDE-FLANGE BEAM DEPTH	BUILT-UP BEAM DEPTHS (IN)	MIN # OF BOLTS REQUIRED
W8, W10, W12	8 - 13	2
W14, W16, W18	To 19	4
W21, W24	To 29	5
W27, W30	To 31	6
W33, W36, W40	To 39	7
W44	To 45	7

TABLE B - MAXIMUM REACTION

# OF BOLTS	MAX REACTION (KIPS)	3/4" DIA A325 BOLTS		1" DIA A490 BOLTS		REDUCTION FACTORS FOR SKEWED CONNS. # ≤ 60°
		PLATE THICKNESS (INCH)	WELD SIZE	MIN WEB THICKNESS A36 (INCH)	MIN WEB THICKNESS FY 50 (INCH)	
2	13	1/4	3/8	.20	.19	63%
3	25	1/4	3/8	.22	.19	65%
4	40	3/8	3/8	.26	.24	71%
5	54	3/8	3/8	.29	.27	76%
6	72	3/8	3/8	.32	.29	80%
7	87	3/8	3/8	.33	.29	83%
8	96	3/8	3/8	.33	.29	85%
9	104	3/8	3/8	.33	.29	85%

GENERAL NOTES FOR STEEL CONNECTIONS

TABLE C

# OF BOLTS	3/4" DIA A325N BOLTS W/ 1/2" THICK ANGLES (A36)		1" DIA A490N BOLTS W/ 1/2" THICK ANGLES (A36)	
	MAX REACTION (KIPS)	MIN WEB THICK (INCH)	MAX REACTION (KIPS)	MIN WEB THICK (INCH)
2	45	.39	21	.26
3	72	.42	30	.33
4	98	.44	32	.36
5	124	.46	33	.39
6	150	.48	35	.42
7	176	.50	36	.45
8	202	.51	37	.48
9	228	.53	38	.51
10	254	.54	39	.54
11	280	.55	39	.57
12	306	.55	40	.60

TYPICAL STEEL CONNECTION, TYPE C2

TABLE Y

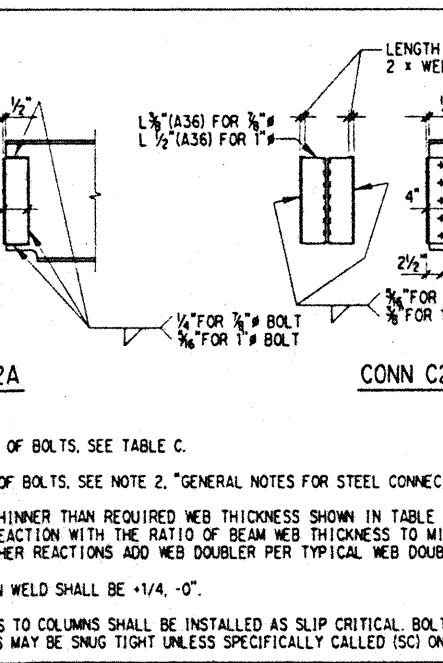
# OF BOLTS PER SIDE	3/4" DIA A325 BOLTS		1" DIA A490 BOLTS	
	ALLOWABLE REACTION (KIPS)	END PLATE THICK (INCH)	MINIMUM BEAM WEB THICKNESS (INCH)	MINIMUM COLUMN WEB THICKNESS (INCH)
2	18	1/4	.28	.2
3	29	1/4	.27	.2
4	45	3/8	.27	.2
5	57	3/8	.27	.2
6	72	3/8	.27	.2
7	88	3/8	.27	.2
8	105	3/8	.27	.2
9	119	3/8	.27	.2
10	124	3/8	.27	.2
11	124	3/8	.27	.2
12	124	3/8	.27	.2

TYPICAL STEEL CONNECTION, TYPE C6

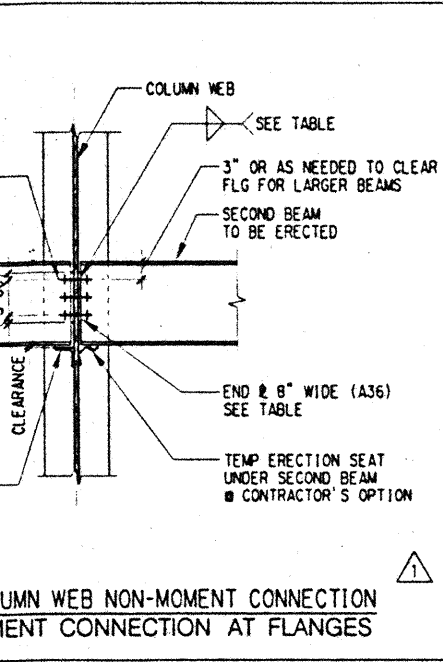
TABLE D MINIMUM BEAM SIZE FOR Fy36 & Fy50 BEAMS

Reaction (kips)	L' (in)	T' min. (in)	W min. (in)	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50						
15	6	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	26	26	22	22	14	14	12	12
23	6	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	26	26	22	22	14	14	12	12
30	7	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	40	31	38	30	35	35	30	26
38	8	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	46	40	45	40	48	38	45	35	39	30
46	9	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	50	45	53	48	58	45	54	54	39	30
55	10	5/8	6	135	135	118	118	90	90	84	84	55	55	44	44	60	55	68	53	65	58	60	60	54	39
64	11	3/4	6	135	135	118	118	90	90	84	84	55	55	44	44	74	68	77	68	74	68	72	65	60	60
73	12	7/8	6	135	135	118	118	99	90	94	84	55	55	44	44	86	77	86	77	86	77	82	73	60	60
82	13	7/8	7	135	135	118	118	108	99	102	94	55	55	44	44	97	86	97	86	97	86	92	79	68	68
92	14	1	7	135	135	130	118	116	99	114	102	117	117	111	83	97	89	99	99	99	99	99	99	88	68
101	15	1-1/8	8	135	135	141	118	124	108	129	114	131	117	122	111	97	97	99	99	99	99	99	99	99	77
111	16	1-1/4	8	150	135	152	130	132	116	124	114	131	117	122	111	97	97	99	99	99	99	99	99	99	77
121	17	1-3/8	9	160	150	152	141	124	114	129	114	131	117	122	111	106	106	106	106	106	106	106	106	106	88
131	18	1-1/2	9	170	150	169	152	132	116	129	114	131	117	122	111	106	106	106	106	106	106	106	106	106	88
140	19	1-1/2	10	182	160	169	152	132	116	129	114	131	117	122	111	106	106	106	106	106	106	106	106	106	100
150	20	1-5/8	10	182	170	169	152	132	116	129	114	131	117	122	111	106	106	106	106	106	106	106	106	106	100
160	21	1-3/4	10	199	170	169	152	132	116	129	114	131	117	122	111	106	106	106	106	106	106	106	106	106	100
169	22	1-7/8	11	210	182	169	152	132	116	129	114	131	117	122	111	106	106	106	106	106	106	106	106	106	100
179	23	2	11-1/4	210	182	169	152	132	116	129	114	131	117	122	111	106	106	106	106	106	106	106	106	106	100

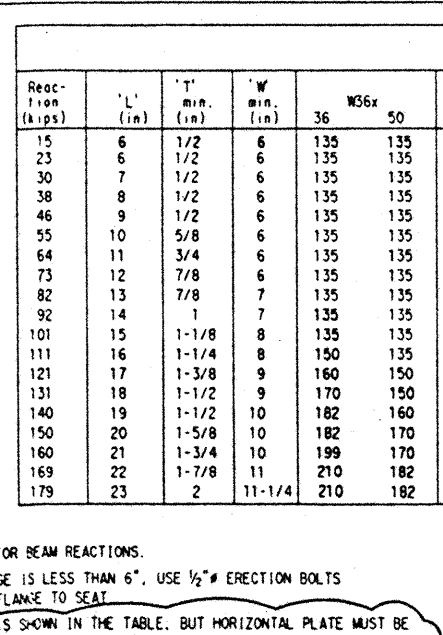
GENERAL NOTES FOR STEEL CONNECTIONS



TYPICAL STEEL CONNECTION, TYPE C2



TYPICAL STEEL CONNECTION, TYPE C6

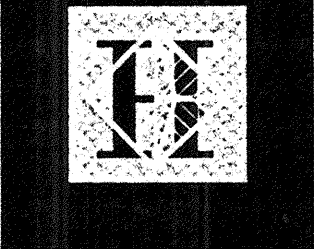


TYPICAL STEEL CONNECTION, TYPE C20

STEEL CONNECTION TYPE C20 TABLE AND NOTES

TABLE D MINIMUM BEAM SIZE FOR Fy36 & Fy50 BEAMS

Reaction (kips)	L' (in)	T' min. (in)	W min. (in)	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50	W36x50						
15	6	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	26	26	22	22	14	14	12	12
23	6	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	26	26	22	22	14	14	12	12
30	7	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	40	31	38	30	35	35	30	26
38	8	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	46	40	45	40	48	38	45	35	39	30
46	9	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	50	45	53	48	58	45	54	54	39	30
55	10	5/8	6	135	135	118	118	90	90	84	84	55	55	44	44	60	55	68	53	65	58	60	60	54	39
64	11	3/4	6	135	135	118	118	90	90	84	84	55	55	44	44	74	68	77	68	74	68	72	65	60	60
73	12	7/8	6	135	135	118	118	99	90	94	84	55	55	44	44	86	77	86	77	86	77	82	73	60	60
82	13	7/8	7	135	135	118	118	108	99	102	94	55	55	44	44	97	86	97	86	97	86	92	79	68	68
92	14	1	7	135	135	130	118	116	99	114	102	117	117	111											



Hawai'i Convention Center

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Honolulu, Hawaii

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Seattle, Washington

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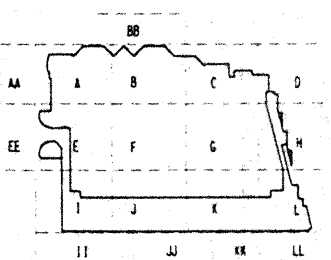
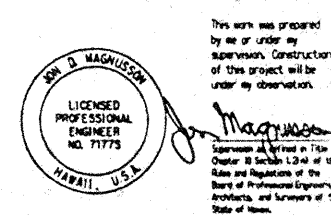
Consultants

SKILLING WARD MAGUISSON BAUMBERG INC.

Consulting Structural and Civil Engineers

1301 Fifth Avenue, Suite 3200 Seattle, WA 98101-3999

(206) 392-1200 FAX: (206) 292-1201



Revisions

02-15-95	FINAL DOCUMENTS
11-7-95	
2-12-95	BUILDING PERMIT SET
8-31-95	
8-4-95	
6-19-95	
5-8-95	ADDENDUM #3
No. Date By Description	

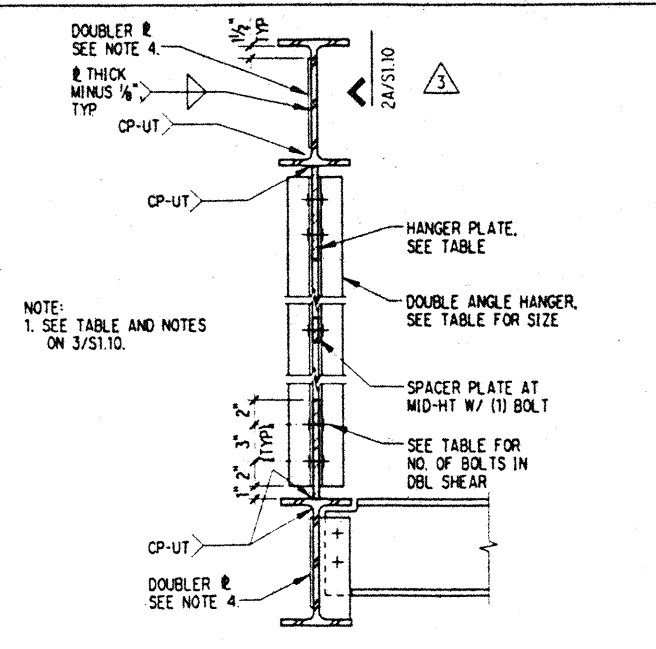
TYPICAL STEEL DETAILS

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

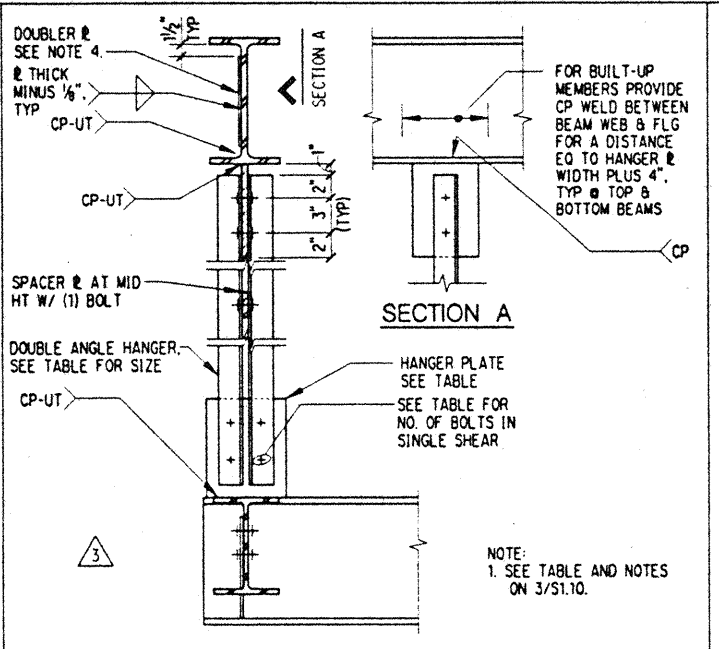
MARK	DIMENSIONS								REMARKS
	E #1 T1 x B1 x L1	W1	E #2 T2 x B2 x L2	W2	E #3 T3 x B3 x L3	W3	DOUBLER E #4 L4 x T4		
A	--	--	--	--	--	--	--	--	
B	--	--	--	--	--	--	--	--	
C	3/8" x 4" x 1'-3"	1/4"	--	--	--	--	--	--	
D	--	--	--	--	3/8" x 7" x 1'-4"	3/8"	--	--	
E	--	--	--	--	3/8" x 1'-0" x 2'-3"	3/8"	--	--	
F	--	--	--	--	3/8" x 10" x 2'-0"	3/8"	--	--	
G	1/2" x 3" x 1'-2"	3/8"	1/2" x 3" x 1'-2"	3/8"	--	--	--	--	
H	3/8" x 6" x 1'-2"	3/8"	3/8" x 6" x 1'-2"	3/8"	--	--	--	--	
J	--	--	1/2" x 3" x 0'-8"	1/4"	--	--	--	--	
K	--	--	--	--	--	--	--	--	
L	3/8" x 3" x 0'-8"	3/8"	(2) 3/8" x 3" x 0'-9"	3/8"	3/8" x 5" x 1'-0"	3/8"	--	--	
M	--	--	--	--	1/2" x 5" x 1'-0"	1/4"	--	--	

MAXIMUM REACTION (kips)	NUMBER OF BOLTS IN SINGLE SHEAR	NUMBER OF BOLTS IN DOUBLE SHEAR	DOUBLE ANGLE SIZE	MIN PLATE THICKNESS (INCHES)	MIN PLATE WIDTH (INCHES)
36	4	2	(2) - L 4 x 4 x 3/4	3/8	10
54	6	3	(2) - L 4 x 4 x 3/4	3/8	14
72	8	4	(2) - L 4 x 4 x 3/4	3/8	14
90	10	5	(2) - L 4 x 4 x 3/4	3/8	14
108	12	6	(2) - L 4 x 4 x 3/4	3/8	14
126	14	7	(2) - L 4 x 4 x 3/4	3/8	14
144	16	8	(2) - L 4 x 4 x 3/4	3/8	14
162	18	9	(2) - L 4 x 4 x 3/4	3/8	14

NOTES:
 1. HANGER LOAD IS EQUAL TO THE SUM OF BEAM END REACTIONS OF ALL THE BEAMS FRAMING INTO THE SUPPORTED JOINT, UNO.
 2. HANGER ANGLES SHALL BE A572 GRADE 50 STEEL.
 3. HANGER BOLTS SHALL BE 3/8" INCH DIA. A325. ANGLES SHALL HAVE STANDARD HOLES. HANGER CONNECTION PLATES SHALL HAVE OVERSIZED HOLES. USE STANDARD AISC ANGLE GAGE.
 4. WEB DOUBLER PLATES ARE REQUIRED WHERE THE BEAM WEB IS THINNER THAN THE HANGER PLATE. THE THICKNESS OF THE BEAM WEB AND DOUBLER PLATE SHALL BE EQUAL TO OR GREATER THAN THE THICKNESS OF THE HANGER PLATE. THE WEB DOUBLER PLATE SHALL BE THE SAME WIDTH AS THE HANGER PLATE.



TYP STEEL CONNECTION, TYPE C31

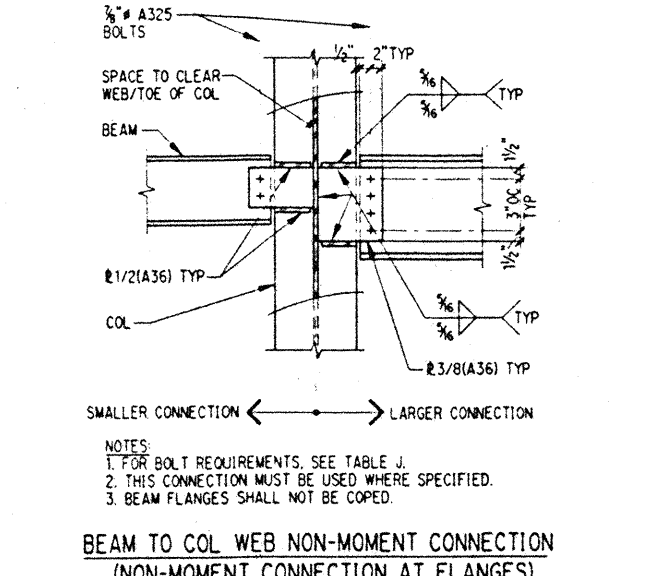


TYP STEEL CONNECTION, TYPE C32

STEEL CONN. TYPE C31 & C32 TABLE

NUMBER OF BOLTS IN SMALLER CONNECTION	SERVICE LOAD OF SMALLER CONNECTION (KIPS)	NUMBER OF BOLTS IN LARGER CONNECTION										
		2	3	4	5	6	7	8	9	10	11	12
0*		4	8	15	23	32	43	54	67	80	93	106
1	25	25	26	27	28	29	30	31	32	33	34	35
2	37	37	37	37	37	37	37	37	37	37	37	37
3	50	50	50	50	50	50	50	50	50	50	50	50
4	63	63	63	63	63	63	63	63	63	63	63	63
5	75	75	75	75	75	75	75	75	75	75	75	75
6	88	88	88	88	88	88	88	88	88	88	88	88
7	100	100	100	100	100	100	100	100	100	100	100	100
8	113	113	113	113	113	113	113	113	113	113	113	113
9	126	126	126	126	126	126	126	126	126	126	126	126
10	138	138	138	138	138	138	138	138	138	138	138	138
11	151	151	151	151	151	151	151	151	151	151	151	151
12	151	151	151	151	151	151	151	151	151	151	151	151

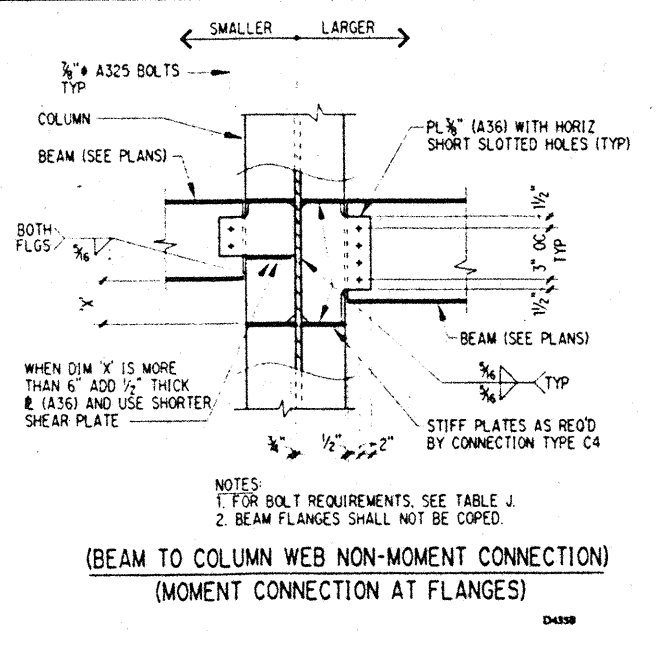
NOTES:
 1. * ONE-SIDED CONNECTION.
 2. THE CAPACITY OF THE LARGER CONNECTION IS DEPENDENT ON THE NUMBER OF BOLTS IN THE SMALLER CONNECTION. TO FIND THE NUMBER OF BOLTS IN THE LARGER CONNECTION, USE THE LOAD VALUES IN THE SAME ROW AS THE PREVIOUSLY-DETERMINED SMALLER CONNECTION.



TYP STEEL CONNECTION, TYPE C34

STEEL CONNECTION TYPE C33 AND C34 TABLE

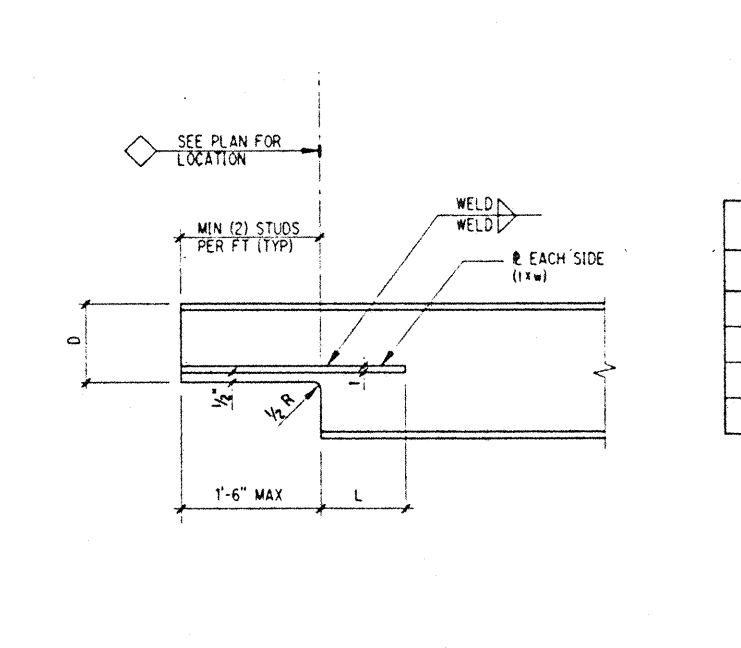
BEAM WEB PENETRATION SCHEDULE



TYP STEEL CONNECTION, TYPE C33



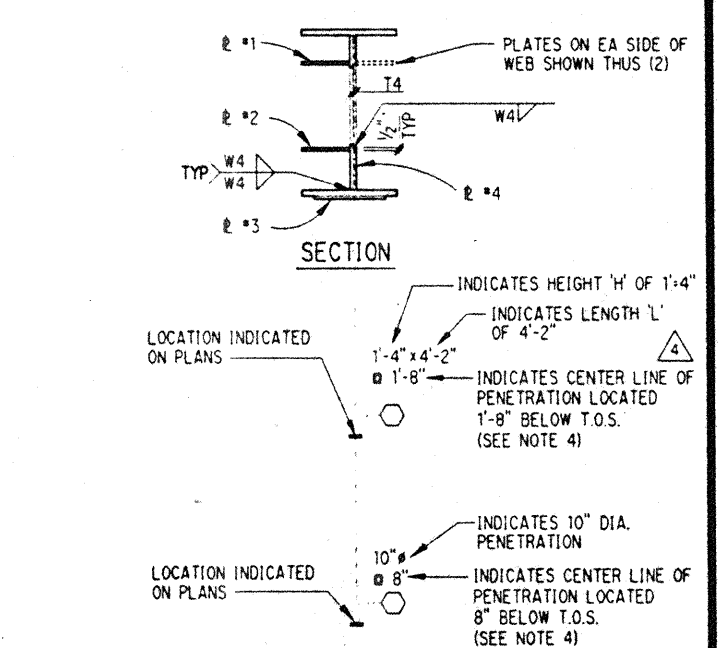
ROOF DECK EDGE @ DECK SPAN PERPENDICULAR TO EDGE BM



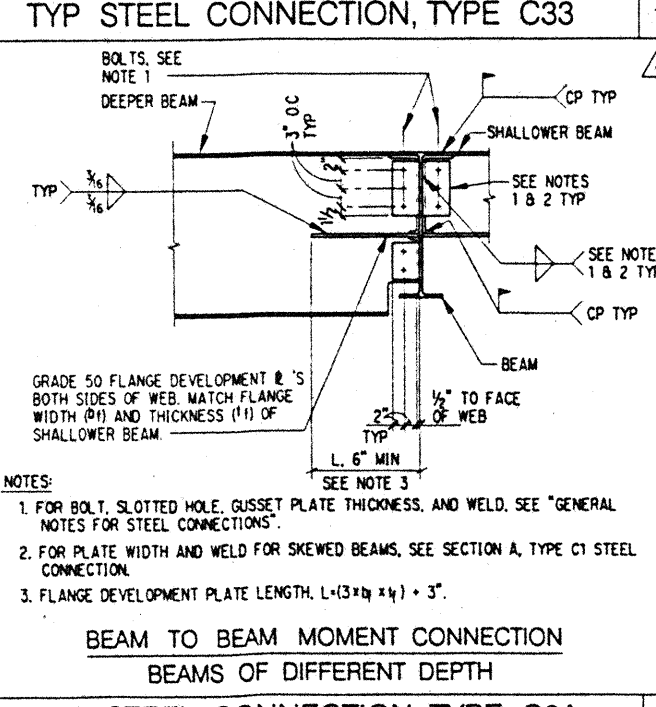
ROOF DECK EDGE @ DECK SPAN PARALLEL TO EDGE BM

MARK	D	(L x W)	L	WELD	REMARKS
◇					
◇					
◇					
◇					

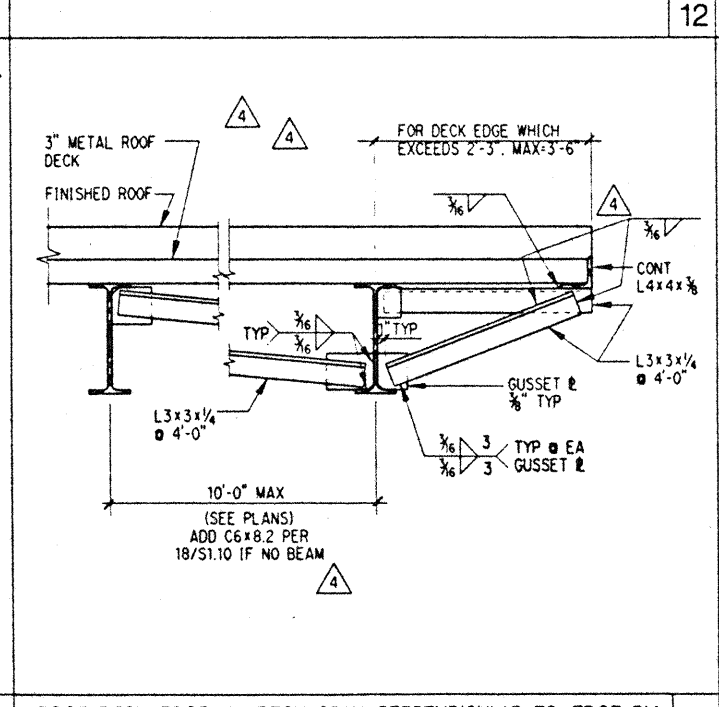
TYPICAL HAUNCHED BEAM TYPE I



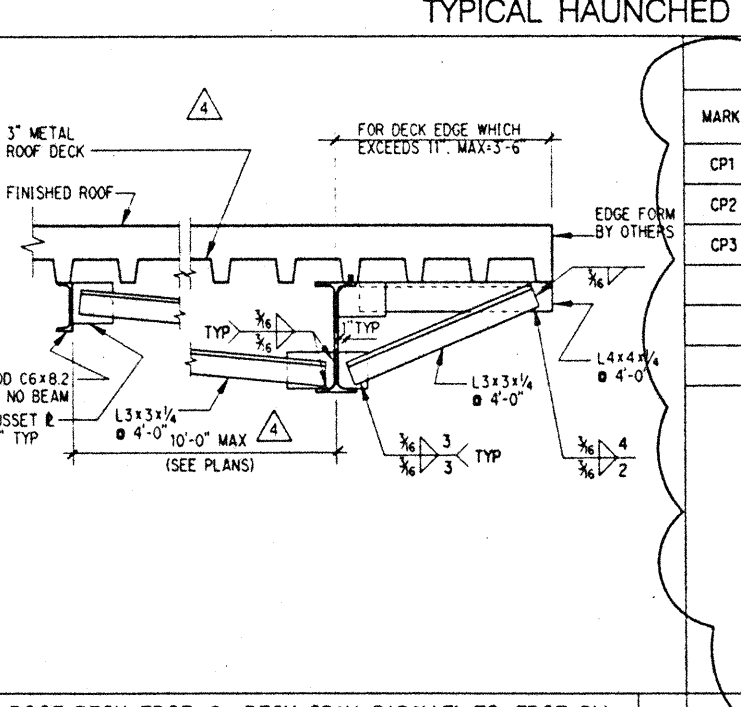
TYPICAL BEAM PENETRATION DETAIL



TYP STEEL CONNECTION, TYPE C3A



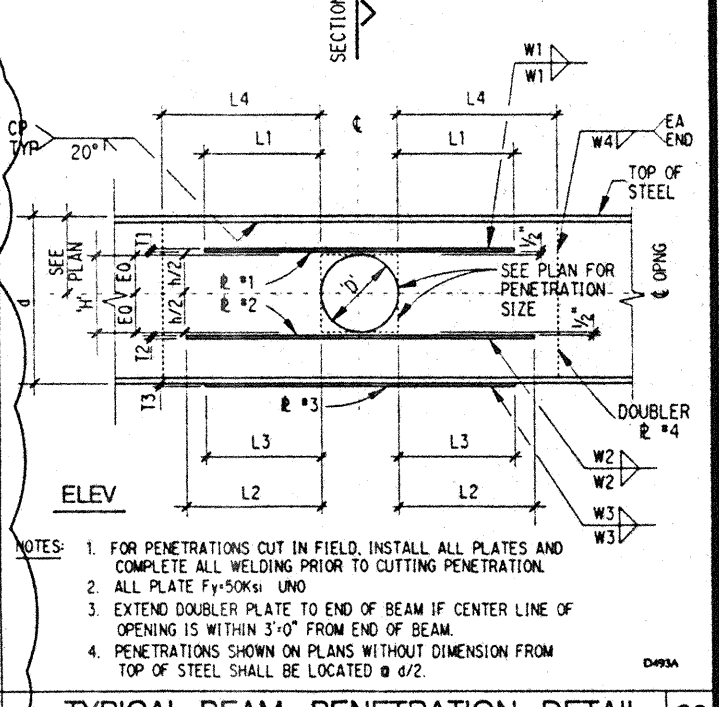
ROOF DECK EDGE @ DECK SPAN PERPENDICULAR TO EDGE BM



ROOF DECK EDGE @ DECK SPAN PARALLEL TO EDGE BM

MARK	BOTTOM PLATE	W _B	R _B	TOP PLATE	W _T	R _T	REMARKS
CP1	1/2" x 4 1/2" x 27'-0"	3/8"	1'-0"	1/2" x 4 1/2" x 27'-0"	3/8"	1'-0"	CENTER AT TRANSFERRED COL.
CP2	1/2" x 10" x 7'-8"	3/8"	1'-8"	1/2" x 10" x 7'-8"	3/8"	1'-8"	
CP3	1/2" x 10" x 17'-4"	1/4"	1'-8"	1/2" x 10" x 17'-4"	1/4"	1'-8"	

TYPICAL BEAM COVER PLATE

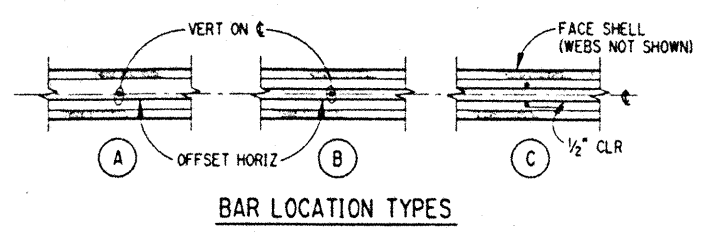


TYPICAL BEAM PENETRATION DETAIL

File Dec 13:48:28 1995
jwccvty10.dgn
1/5 pit

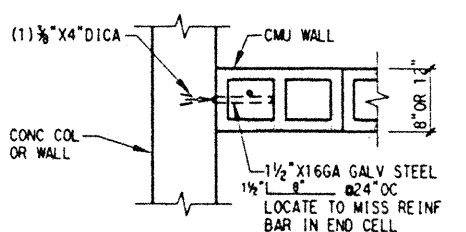
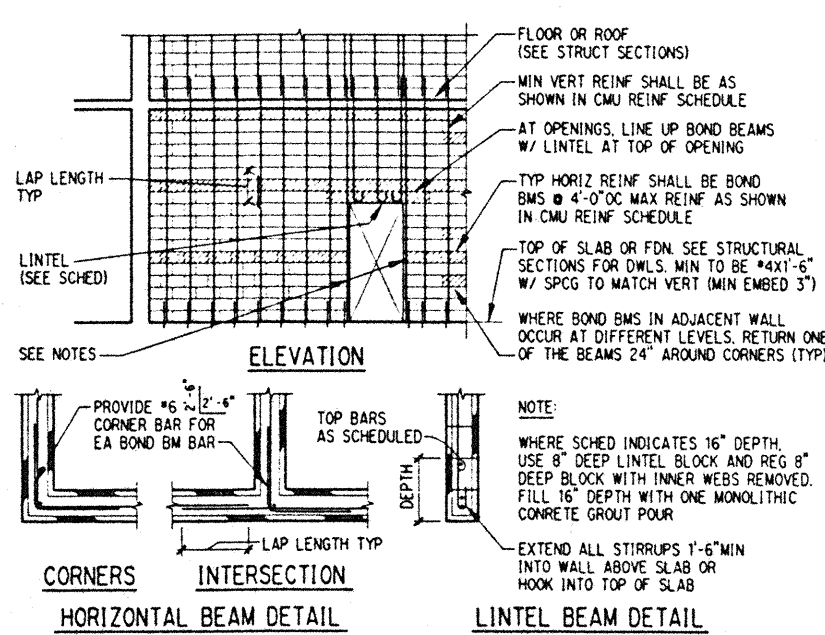
NOMINAL WALL THICKNESS	BOND BEAM REIN		VERTICAL REINFORCING		BAR LOCATION TYPE	MAXIMUM WALL HEIGHT (FT)	REMARKS
	NUMBER & SIZE	LAP LENGTH	NUMBER & SIZE	SPACING			
10"	1 #5	30"	1 #5	16"	A	21'	EXTERIOR

- NOTES:
- BOND BEAM REINFORCEMENT ABOVE AND BELOW OPENINGS SHALL BE CONTINUOUS (I.E. UNSPLICED) AND EXTENDED 2 FEET MINIMUM BEYOND OPENING.
 - WHERE BOND BEAM DOES NOT ALIGN WITH TOP OR BOTTOM OF OPENINGS, ADD A BOND BEAM COURSE EXTENDING 2 FEET BEYOND THE OPENING.
 - LOCATE SCHEDULED BARS FROM TABLE EACH SIDE OF EACH OPENING, EACH CORNER AND WALL ENDS.
 - BOND BEAM BARS MAY BE USED TO ARRIVE AT TOTAL STEEL AREA REQUIRED BY REINFORCING SCHEDULE.
 - GROUT ALL CELLS THAT CONTAIN REINFORCEMENT OR EMBEDDED ITEMS.



BAR LOCATION TYPES

TYPICAL CMU LOAD BEARING WALLS REINFORCING SCHEDULE & NOTES



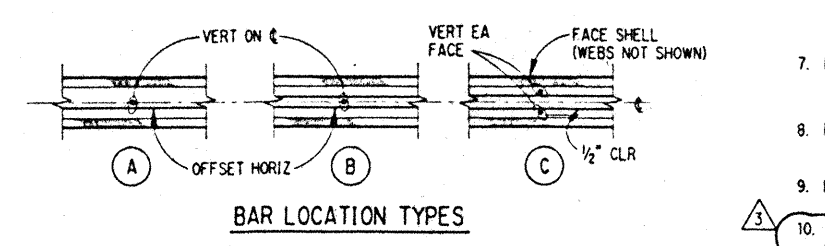
CONN TO ABUTTING CONCRETE

THICK	CLEAR SPAN	DEPTH (IN)	HOR. REINF.	STIRRUPS
8"	4'-0" OR LESS	16"	(2) #4 BOT (1) #4 TOP	#3 J @ 16"
	4'-0" TO 6'-4" TO 8'-0"	16"	(2) #5 BOT (1) #4 TOP	#3 J @ 16"
10"	4'-0" OR LESS	8"	(2) #5 BOT (1) #4 TOP	#3 J @ 16"
	4'-0" TO 6'-4" TO 8'-0"	16"	(2) #6 BOT (1) #4 TOP	#3 J @ 16"
12"	4'-0" OR LESS	16"	(2) #7 BOT (1) #5 TOP	#3 J @ 16"
	4'-0" TO 6'-4" TO 8'-0"	16"	(2) #7 BOT (1) #5 TOP	#3 J @ 16"

TYPICAL CMU LOAD BEARING WALL DETAILS

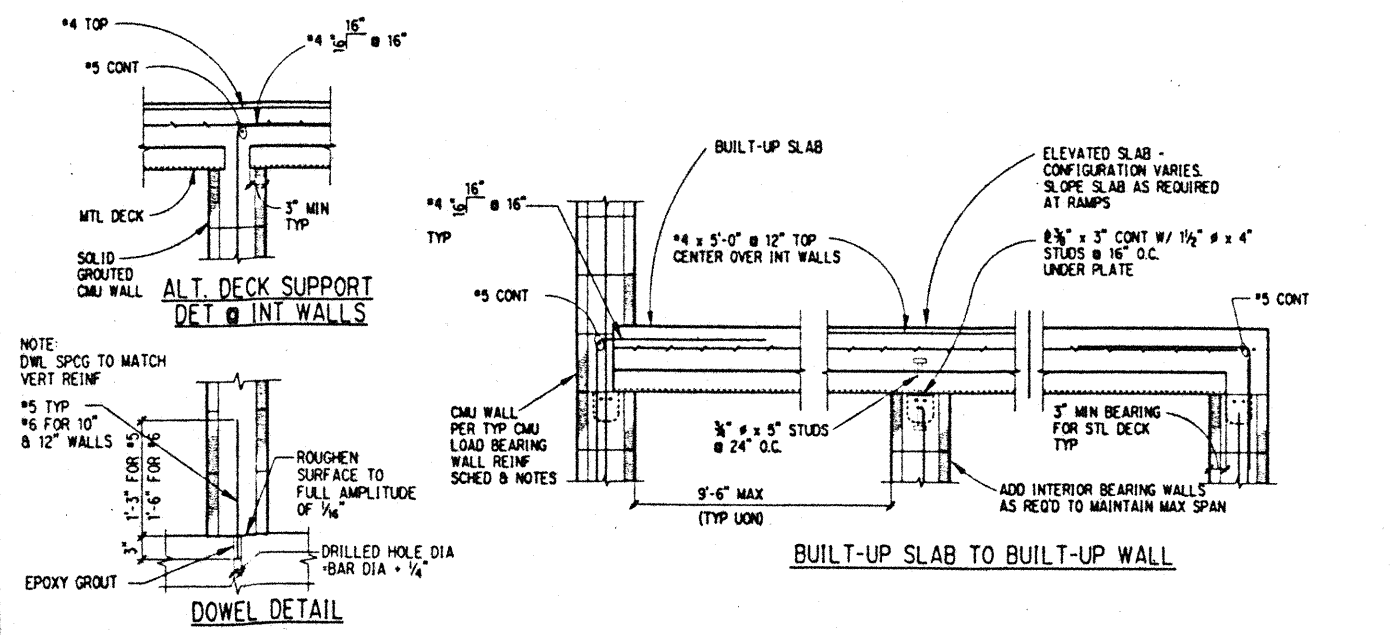
NOMINAL WALL THICKNESS	BOND BEAM REIN		VERTICAL REINFORCING		BAR LOCATION TYPE	MAXIMUM WALL HEIGHT (FT)	REMARKS
	NUMBER & SIZE	LAP LENGTH	NUMBER & SIZE	SPACING			
6"	1 #5	30"	1 #5	48"	A	15'	INT
6"	1 #5	30"	1 #5	48"	A	9'	EXT
6"	1 #5	30"	1 #4	8"	A	12.5'	EXT
8"	2 #5	39"	1 #5	48"	A	20'	INT
8"	1 #5	30"	1 #5	24"	A	15'	EXT
8"	1 #5	30"	1 #4	8"	A	16.5'	EXT/INT

- NOTES:
- BOND BEAM REINFORCEMENT ABOVE AND BELOW OPENINGS SHALL BE CONTINUOUS (I.E. UNSPLICED) AND EXTENDED 2 FEET MINIMUM BEYOND OPENING.
 - WHERE BOND BEAM DOES NOT ALIGN WITH TOP OR BOTTOM OF OPENINGS, ADD A BOND BEAM COURSE EXTENDING 2 FEET BEYOND THE OPENING. WHERE OPENING IS WIDER THAN 8'-0" SEE BLOCK LINTEL SCHEDULE, 20/51.12
 - BOND BEAM BARS MAY BE USED TO ARRIVE AT TOTAL STEEL AREA REQUIRED BY REINFORCING SCHEDULE.
 - GROUT ALL CELLS THAT CONTAIN REINFORCEMENT OR EMBEDDED ITEMS.
 - PACK TOP COURSE CELLS SOLID WITH GROUT, TYPICAL.
 - PROVIDE 1" MINIMUM CLEAR SPACE BETWEEN TOP WALL AND BOTTOM OF PLATE, DECK OR FIREPROOFING ON BEAMS.
 - FOR DICA'S IN BOTTOM OF SLAB, EMBED SO THAT THEY EXTEND A MINIMUM OF 1 INCH ABOVE THE UPPER PART OF THE DECK FLUTES.
 - PROVIDE SPECIAL INSPECTION FOR DICA AT ANCHORS FOR WALLS 16'-0" AND HIGHER.
 - LOCATE BRACE FOR BEAM BOTTOM FLANGE AT EACH CMU TOP SUPPORT AT 4'-0" ON CENTER, MAXIMUM.
 - MAXIMUM WALL HEIGHT IS DEFINED AT DISTANCE FROM TOP OF WALL TO BOT OF WALL OR FROM TOP/BOT OF WALL TO INTERMEDIATE SUPPORT.



BAR LOCATION TYPES

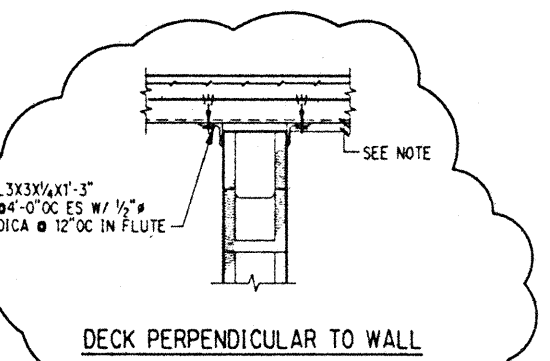
TYPICAL CMU NON-BEARING WALLS REINFORCING SCHEDULE & NOTES



TYPICAL BUILT-UP SLAB DETAIL

THICK	CLEAR SPAN	DEPTH (IN)	HOR. REINF.	STIRRUPS
6"	8'-16"	24"	2 #4 TOP 2 #4 BOT	#3 J @ 16"
8"	8'-16"	24"	2 #4 TOP 2 #4 BOT	#3 J @ 16"
8"	16'-22"	40"	2 #6 TOP 2 #6 BOT	#3 J @ 16"

BLOCK LINTEL SCHEDULE



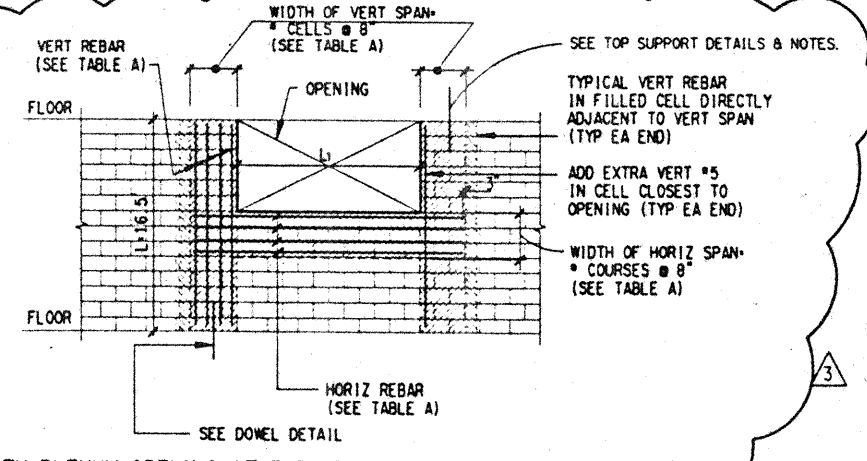
DECK PERPENDICULAR TO WALL

SPAN L ₁	HORIZ SPAN		VERT SPAN	
	* FILLED COURSES	* REBAR	* FILLED CELLS	* REBAR
4'-7"	1	1 #4	1	1 #4
7'-12"	1	1 #4	2	2 #4
12'-13.75"	2	2 #4	2	2 #4
13.75'-17"	2	2 #4	3	3 #4
17'-20.5"	3	3 #4	3	3 #4
20.5'-24"	4	4 #4	4	4 #4

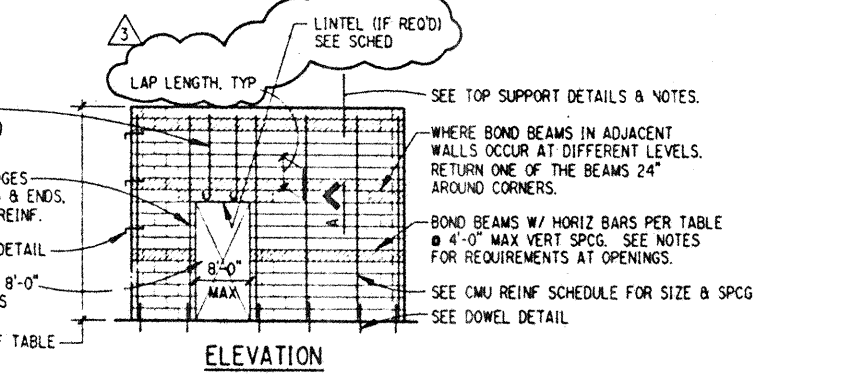
TABLE A



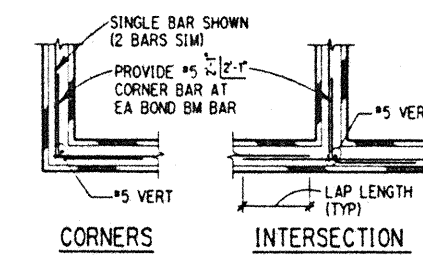
ELEVATION WITH PLENUM OPENING AT TOP OF WALL.



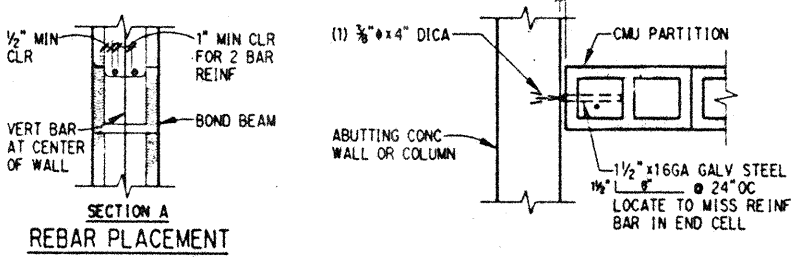
ELEVATION WITH PLENUM OPENING AT TOP OF WALL.



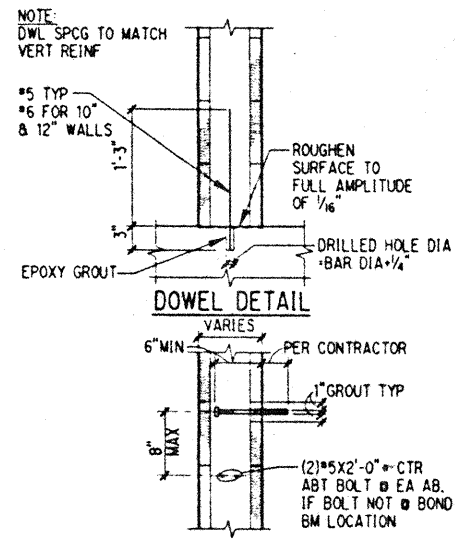
ELEVATION



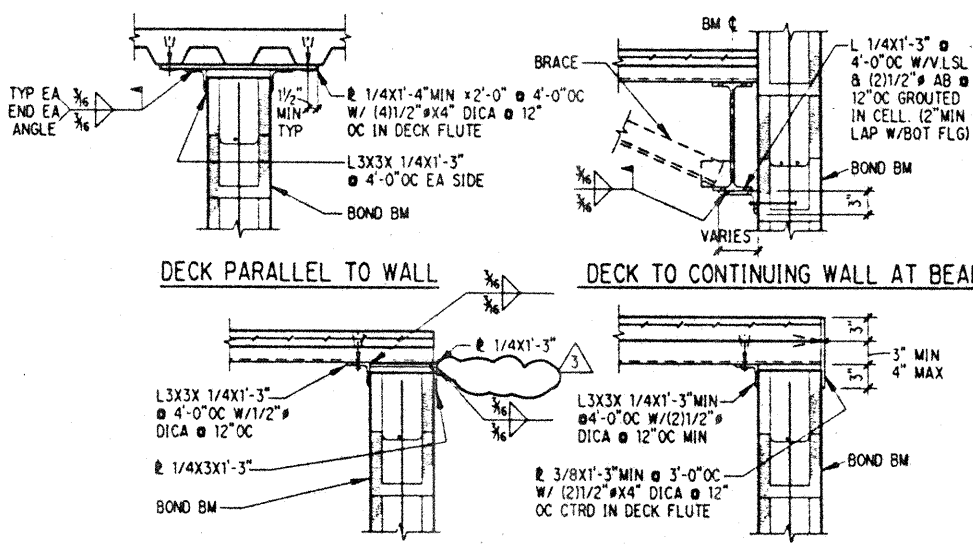
CORNERS INTERSECTION REBAR PLACEMENT



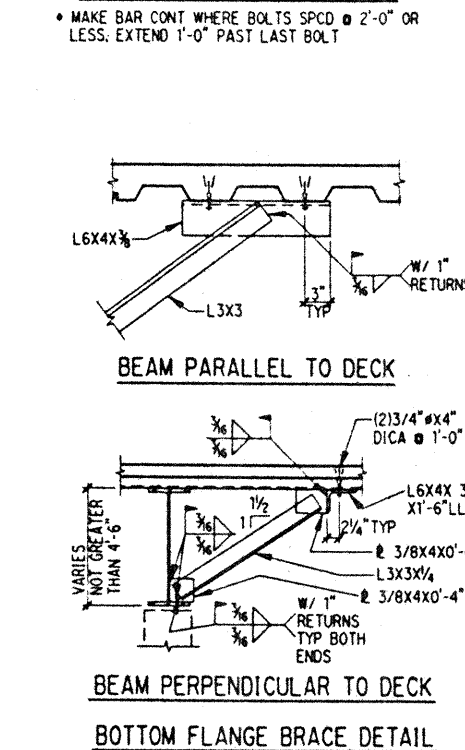
CONNECTION ABUTTING CONCRETE



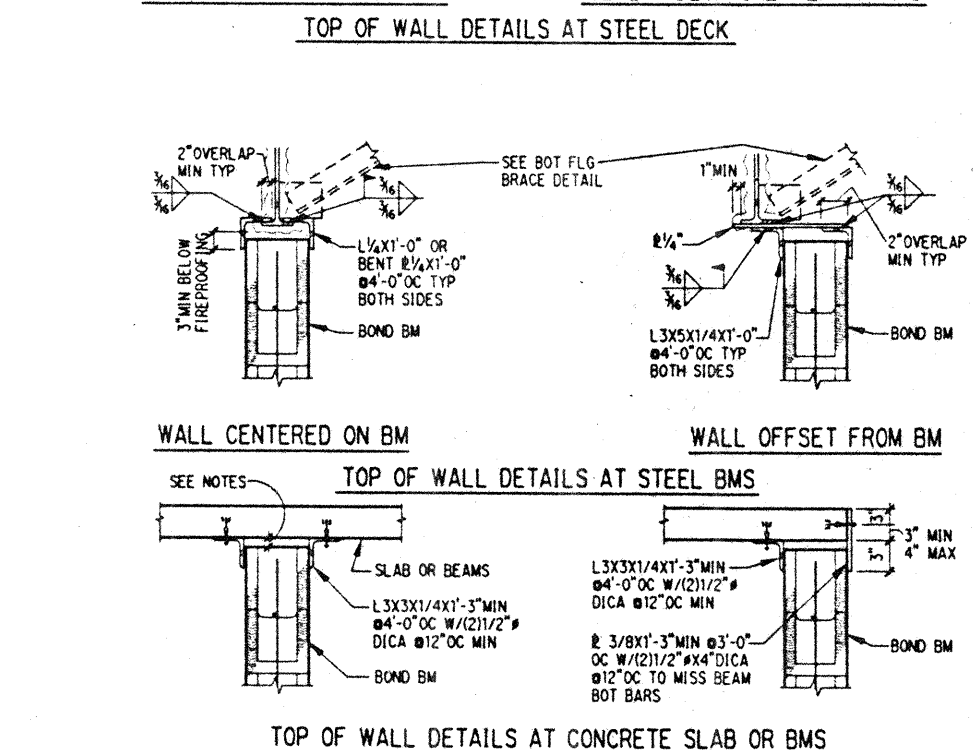
DOWEL DETAIL



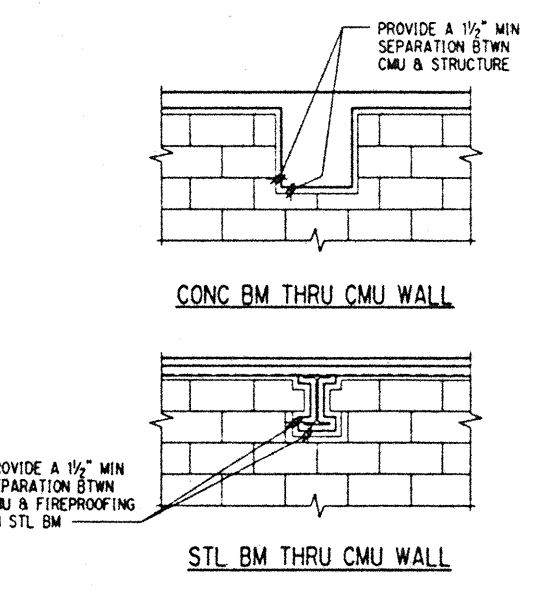
TOP OF WALL DETAILS AT STEEL DECK



BEAM PARALLEL TO DECK



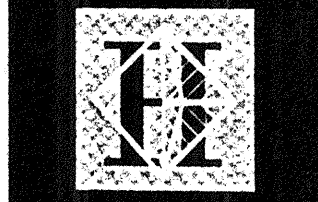
WALL CENTERED ON BM



CONC BM THRU CMU WALL STL BM THRU CMU WALL

BEAM PERPENDICULAR TO DECK BOTTOM FLANGE BRACE DETAIL

TOP OF WALL DETAILS AT CONCRETE SLAB OR BMS



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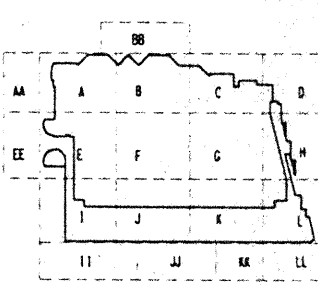
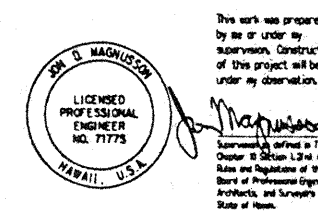
Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

Consultants
SKELLING WARD MAGUISSON BARKSHIRE INC.
Consulting Structural and Civil Engineers

1301 Fifth Avenue, Suite 3200 Seattle, WA 98109-2699
2060 297-7744 FAX 2060 292-0001



Revisions

No.	Date	Description
1	12-15-95	FINAL DOCUMENTS
2	9-21-95	BUILDING PERMIT SET
3	8-31-95	ADDENDUM #3
4	5-8-95	ADDENDUM #3

No. Date By Description

Sheet Title

TYPICAL CMU DETAILS

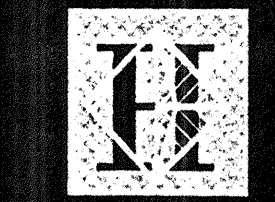
Designed B.M.
Drawn J.M.
Checked J.M.
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

F:\11923\100 1995 J:\VOC\11923\100 1995

CONCRETE COLUMN SCHEDULE

LEVEL	COLUMN MARK	1-F 3 1-F 7	1.5-G 4	1-E 9 1-G 1	3-G 5-F 8-G 3-F 6-G 8-F 4-G 6-F 4-F 7-G 5-G	2.5-B 3.1-B 3.6-B	4.5-B 5.1-B 5.5-C	6.1-C	2.1-C 3.5-C 4.1-C	4.8-C 5.8-D 6.5-D										
FUTURE LOADS			P _u =226k P _u =4649k M _u =12998k-ft V _u =350k	SEE NOTE 6																
MEETING	SIZE				6'-0" #															
PARKING	TYPE				SP															
	REINF				(30) #14															
	SPLICE																			
	REMARKS				f'c=7.5ksi															
MEZZANINE	SIZE	3'-0" x 3'-3"	SEE 4/5.15	8'-0" #	6'-0" #															
	TYPE	P	P	SP	SP															
	REINF	(12) #10	(8) #8	(33) #14	(30) #14															
	SPLICE																			
EXHIBIT	SIZE	3'-0" x 3'-3"	SEE 4/5.15	8'-0" #	6'-0" #	3'-4" #	3'-4" #													
	TYPE	P	P	SP	SP	SP	SP													
	REINF	(12) #10	(8) #8	(33) #14	(30) #14	(21) #9	(21) #11													
	SPLICE					TENSION	TENSION													
FDN	SIZE	3'-0" x 3'-3"	SEE 4/5.15	8'-0" #	6'-0" #	3'-4" #	3'-4" #													
	TYPE	P	P	SP	SP	SP	SP													
	REINF	(12) #10	(8) #8	(33) #14	(30) #14	(21) #9	(21) #11													
	SPLICE					TENSION	TENSION													
DOWELS	(12) #7	(8) #6	(33) #10	(30) #10	NOTE 5	NOTE 5														
REMARKS				f'c=7.5ksi																

- NOTES:
1. ALL COLUMNS SHALL BE CAST WITH f'c=4.0 KSI CONCRETE, UNLESS NOTED OTHERWISE.
 2. SEE TYPICAL DETAILS FOR DETAILS OF TIED (T) COLUMNS, SPIRAL (SP) COLUMNS, OR PILASTER (P) COLUMNS.
 3. ALL BARS SHALL BE SPLICED PER TYPICAL SPLICE DETAIL.
 4. SEE 1/4.15 FOR DETAILS OF CONC COL'S @ REGISTRATION LOBBY TREES.
 5. COVER TO MAIN BARS 3/4" AWAY FROM TOP REVEAL. SEE 4/2.25 FOR REVEAL AND NOTCH DETAIL.
 6. FUTURE WORKING LOADS AS FOLLOWS:
 3-F, P=3204k; 3-G, P=3379k; 4-F, P=3200k; 4-G, P=3322k; 5-F, P=2315k; 5-G, P=1695k.



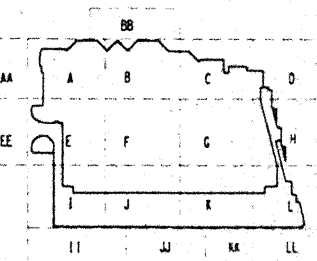
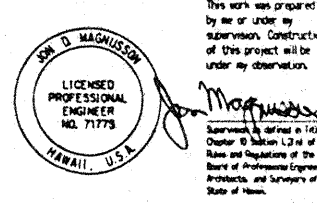
Hawaii Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii sp
in association with
Loschky Marquardt & Neshom
Seattle, Washington

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Honolulu, HI

Consultants
SCILLING WARD MAGNUSON BAKSHINE INC.
Consulting Structural and Civil Engineers
1901 Fifth Avenue, Suite 3000 Seattle WA 98101-2599
206/462-1200 FAX: 206/462-1031



No.	Date	By	Description
12-15-95	12-15-95		FINAL DOCUMENT
9-15-95	9-15-95		BUILDING PERMIT SET
8-31-95	8-31-95		
8-4-95	8-4-95		
4-17-95	4-17-95		ISSUED FOR FOUNDATION PERMIT

CONCRETE COLUMN SCHEDULE

Designed BJM
 Drawn DMS
 Checked JCM
 Date 4 / 12 / 95
 Job No. 93036, 02, 9
 Sheet No.

STEEL COLUMN SCHEDULE

5

Table with columns for MARK, LEVEL, FUTURE LOADS, ROOF, PENTHOUSE, BALLROOM/TERRACE, MEETING, PARKING, INTERMEDIATE, EXHIBIT, FOUNDATION, SIZE, and TYPE. Includes various column specifications and dimensions.

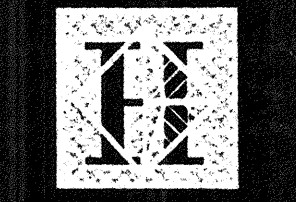
- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFER TO FLOOR PLANS ON S2* FOR COLUMN LOCATIONS AND ORIENTATIONS.
2. UNLESS OTHERWISE NOTED, COLUMNS SHALL BE FY + 50 KSI.
3. UNLESS OTHERWISE NOTED, COLUMN BASEPLATES SHALL BE FY + 36 KSI.
4. FOR COLUMN BASEPLATE AND SPLICE DETAILS SEE S1.7.
5. REFER TO BRACED FRAME AND TRUSS ELEVATIONS ON S4* AND S5* FOR ADDITIONAL COLUMN CALLOUTS.
6. FUTURE LOADS SHOWN ARE ANTICIPATED LOADS FROM THE FUTURE EXPANSION SHOWN ON S9.1 THROUGH S9.3. FUTURE LOADS ARE ASSUMED TO BE APPLIED AT THE BALLROOM / TERRACE LEVEL. UNO. COLUMN TIERS ABOVE THE BALLROOM / TERRACE LEVEL HAVE NOT BEEN DESIGNED FOR THE FUTURE LOADS.
7. ALL COL SPLICES SHALL BE TYPE 1 UNO ON SCHEDULE

STEEL COLUMN SCHEDULE

15

Table with columns for MARK, LEVEL, FUTURE LOADS, ROOF, PENTHOUSE, BALLROOM/TERRACE, MEETING, PARKING, INTERMEDIATE, EXHIBIT, FOUNDATION, SIZE, and TYPE. Includes various column specifications and dimensions.

- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON 5/S1.21.



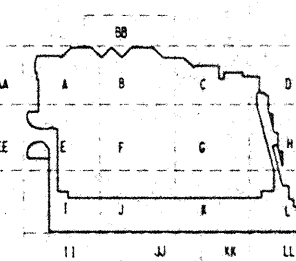
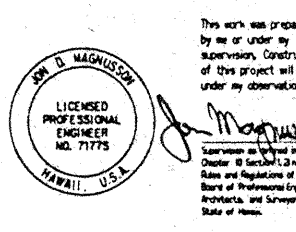
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Convention Center Authority Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc. Honolulu, Hawaii in association with Loschky Marquardt & Nesholm Seattle, Washington

NORDIC /PCL A Joint Venture Honolulu, HI

Consultants SKULLING WARD MAGNUSON BAKESIDE INC. Consulting Structural and Civil Engineers 1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2899 (206) 292-2200 FAX: (206) 292-1201



Revisions table with columns for Date, Description, and initials. Shows dates from 12-15-95 to 4-17-96 and corresponding document actions.

STEEL COLUMN SCHEDULE

Designed BJM Drawn DMS Checked JDM Date 4 / 12 / 96 Job No. 93036.02.9 Sheet No.

S1.21

STEEL COLUMN SCHEDULE

MARK	STEEL COLUMN SCHEDULE																				5													
	2-K4	9.5-J1	8.5-K 9.5-K	8.2-J 5.4-D.75 5.7-E 4.5-J4 4.9-J4 5.9-J3 6.05-J15 6.1-H8	9.3-G9	9-1.1	4.8-J 8.45-G1 8.45-G3 8.55-E 8.9-G5 10-F9 10-F5 10-G1 10-G5 11.8-E5 11.8-E9 11.8-F5 11.8-G1 11.8-G5	5.25-F 5-H95	5.5-F.5	5.5-E.5 5.5-E.9	4.3-E	8.3-E.5 8.3-H	8.3-G1 8.3-G5	8.3-E.9 8.3-F.5	6.3-E	3.7-H 3.7-I	6.4-I 7-1 7.5-1 8-1 8.3-1	7-6.3	7-6.5	3-G.85 3-I 3.5-G.85 4-H 4-I 4-G.85		6.5-H	1-F.3	1-F.75	6.25-J8	3-K4 3.5-K4 4-K4	3.65-E	2.4-E	2-E.65 2.4-E.2 2.7-E.2 3-E.2 3.3-E.2 3.65-E.2 4-E.2 4.3-E.2 4.6-E.5 4.6-E.7 4.7-E.2 4.85-E.4 4.85-E.5	4.85-E.7 4.85-E.9 4.6-F.2 4.6-F.5 4.6-F.7 4.85-F.2 4.85-F.5 4.85-F.7 4.85-F.9 4.85-G.3 4.85-G.6 4.4-H 4.05-H 4.4-I 6.8-1	2-K3 2.05-K	3.5-H	2.05-K.3	9.7-J1
LEVEL	[Grid lines and levels as per schedule]																																	
FUTURE LOADS	[Future load specifications]																																	
ROOF	[Roof specifications]																																	
PENTHOUSE	[Penthouse specifications]																																	
BALL ROOM/ TERRACE	[Ball room/terrace specifications]																																	
MEETING	[Meeting room specifications]																																	
PARKING	[Parking specifications]																																	
INTERMEDIATE	[Intermediate floor specifications]																																	
EXHIBIT	[Exhibit specifications]																																	
FOUNDATION	[Foundation specifications]																																	
BASE & SIZE	[Base and size details]																																	
TYPE	[Column types]																																	

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON 5/S121.



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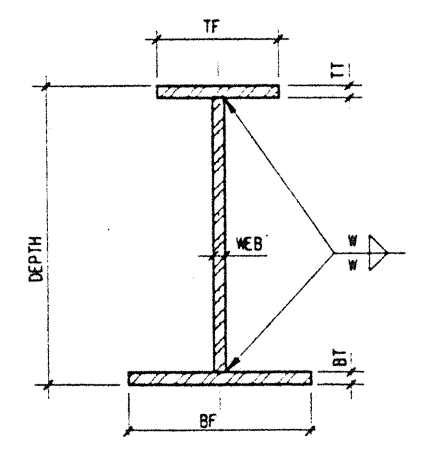
Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
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Seattle, Washington

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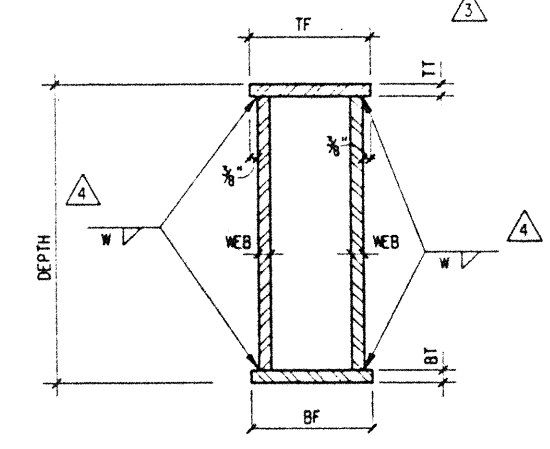
Consultants
SKILLING WARD MAGNUSON BANKSIDE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 292-1001

BUILT-UP BEAM SCHEDULE

BEAM MARK	DIMENSIONS							BEAM MARK	DIMENSIONS						
	DEPTH	WEB	TOP FLANGE		BOTTOM FLANGE		WELD		DEPTH	WEB	TOP FLANGE		BOTTOM FLANGE		WELD
			TF	TT	BF	BT					TF	TT	BF	BT	
BU60x119	60"	1/2"	7"	3/8"	7"	3/8"	3/16"	BU60x414	60"	1"	20"	1-1/2"	20"	1-3/4"	5/8"
BU60x150	60"	11/16"	5"	5/16"	5"	5/16"	1/4"	BU60x438	60"	3/4"	10"	1-1/4"	20"	3-3/4"	5/8"
BU60x162	60"	11/16"	7"	1/2"	7"	1/2"	1/4"	BU60x463	60"	1"	20"	2"	20"	2"	5/8"
BU60x172	60"	11/16"	10"	1/2"	10"	1/2"	1/4"	BU60x471	60"	1-1/4"	20"	1-1/2"	20"	1-7/8"	3/4"
BU60x188	60"	11/16"	10"	5/8"	10"	7/8"	5/16"	BU60x478	60"	3/4"	10"	2"	20"	4"	5/8"
BU60x200	60"	3/4"	10"	3/4"	10"	3/4"	5/16"	BU60x510	60"	1-1/4"	20"	2"	20"	2"	3/4"
BU60x212	60"	3/4"	10"	3/4"	11"	1"	3/8"	BU28x354	28"	1"	20"	2"	20"	2"	3/4"
BU60x220	60"	3/4"	12"	7/8"	12"	7/8"	5/16"	BU30x108	30"	5/8"	8"	7/8"	8"	7/8"	5/16"
BU60x226	60"	3/4"	10"	3/4"	20"	3/4"	7/16"	BU30x162	30"	1"	8"	1-1/4"	8"	1-1/4"	1/2"
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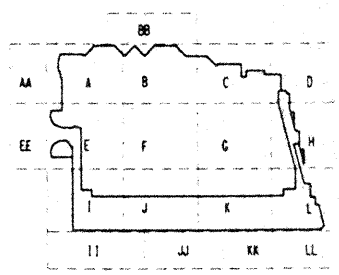
TYP. BU SECTION



TYP. BUTS SECTION

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

[Signature]
LICENSED PROFESSIONAL ENGINEER
 NO. 71775
 HAWAII, U.S.A.



Revisions

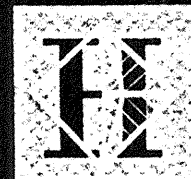
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11-7-95			
9-25-95	BUILDING PERMIT SET		
8-31-95			
8-4-95			
6-19-95			
5-8-95	ADDENDUM #3		
4-17-95	ISSUED FOR FOUNDATION PERMIT		
No.	Date	By	Description

Sheet Title

STEEL COLUMN & BUILT-UP BEAM SCHEDULES

Designed: BJM
 Drawn: DMS
 Checked: JCM
 Date: 4 / 12 / 95
 Job No.: 93036, 02, 9
 Sheet No.:

Fri Dec 1 13:55:16 1995
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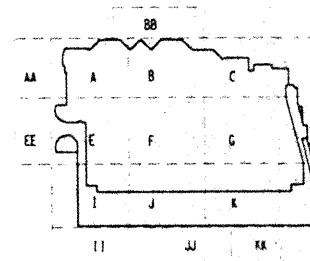
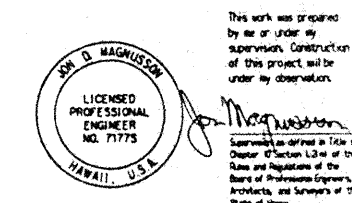
Loschky Marquardt & Nesholm
Seattle, Washington

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SELLING WARD MAGNUSON BARRISIDE INC.

Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle, WA 98101-2699
206 465-1200 FAX: 206 465-1201



Revisions

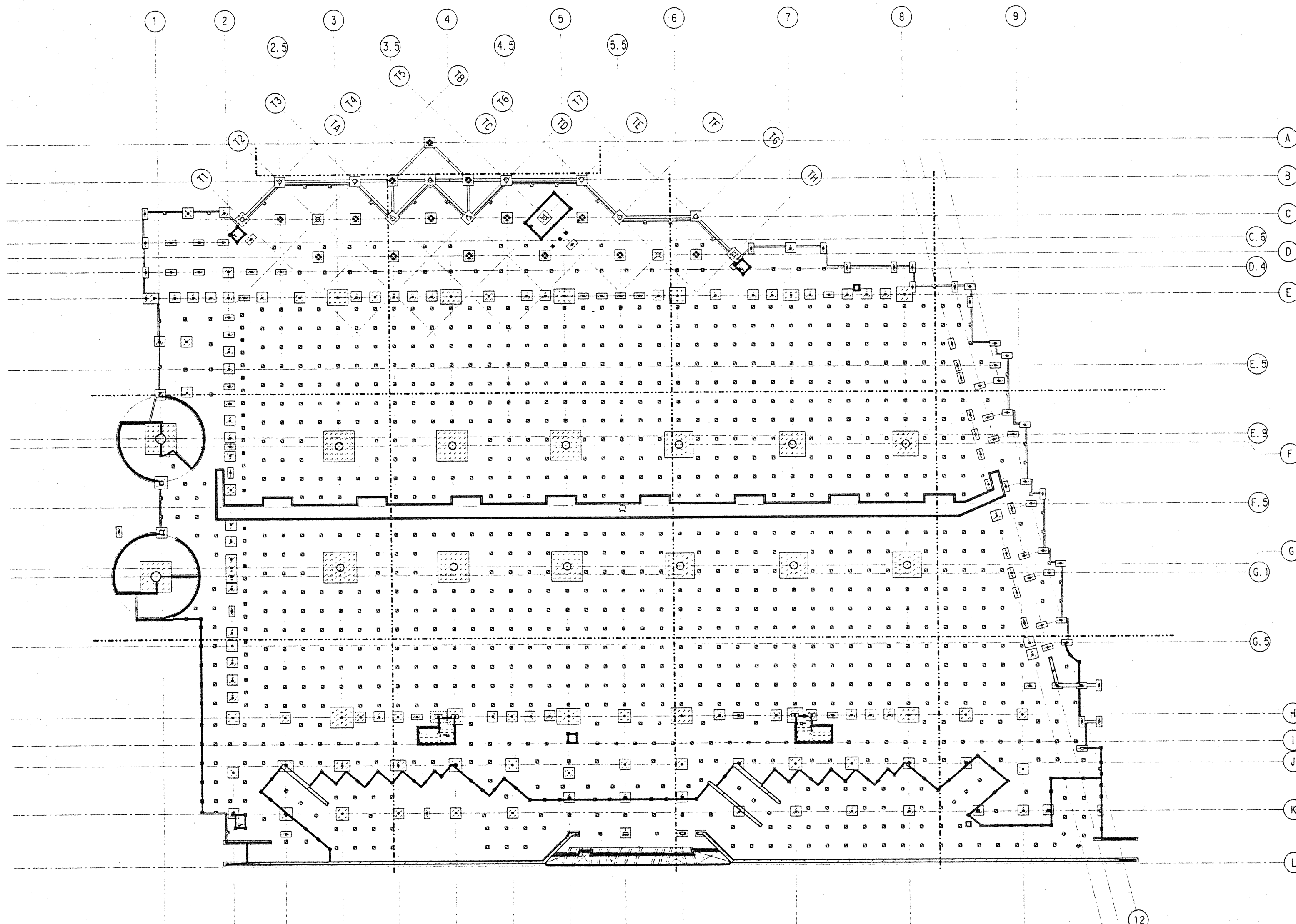
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12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
4-28-95			ADDENDUM #1
4-17-95			ISSUED FOR FOUNDATION PERMIT

Sheet Title

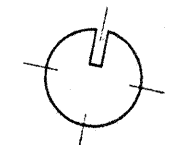
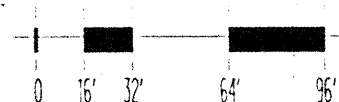
FOUNDATION PLAN

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036, 02, 9
 Sheet No.

S2.0.0



FOUNDATION PLAN



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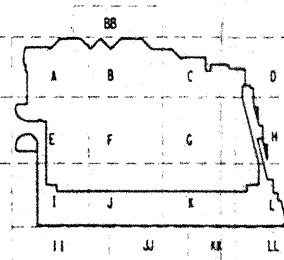
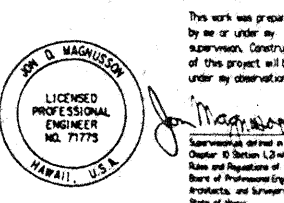
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(206) 232-1200 FAX: (206) 232-1201



No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
4-17-95			ISSUED FOR FOUNDATION PERMIT

Sheet Title	
Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
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Sheet No.	

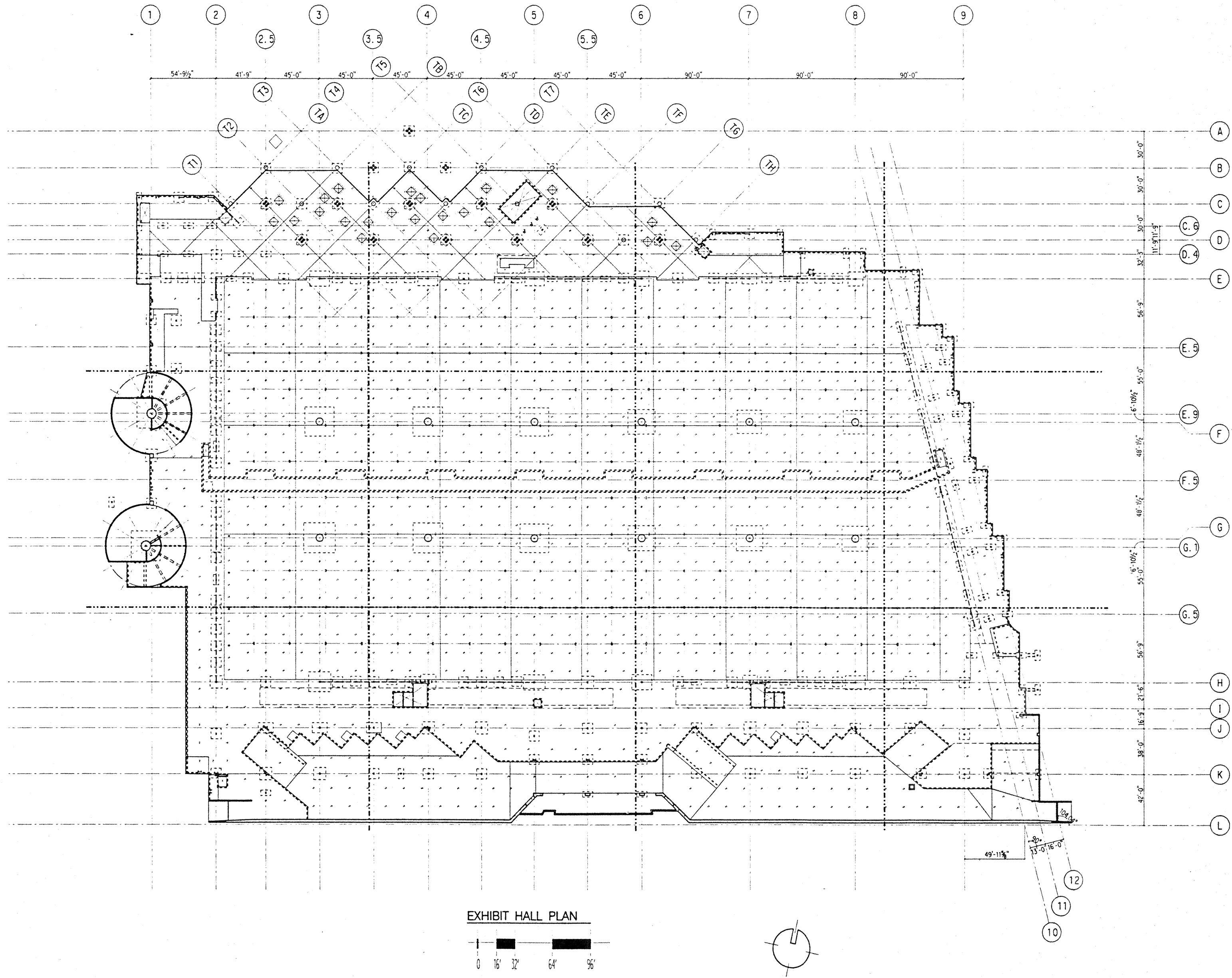
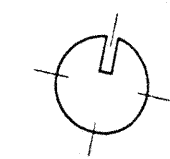
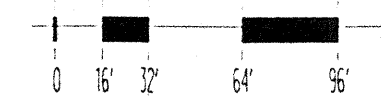
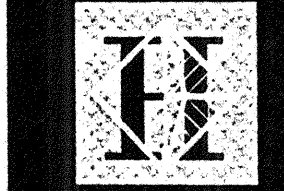


EXHIBIT HALL PLAN



j:\ncce\exhibit.dgn Sat Dec 2 11:09:06 1995



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Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

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Seattle, Washington

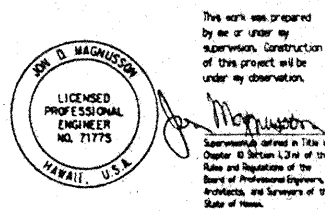
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SKILLING WARD MAGNUSSEN BARKSHIRE INC.

Consulting Structural and Civil Engineers

1301 Fifth Avenue, Suite 3200 Seattle, WA 98101-2699
206 222-1200 FAX: 206 222-1201



Revisions

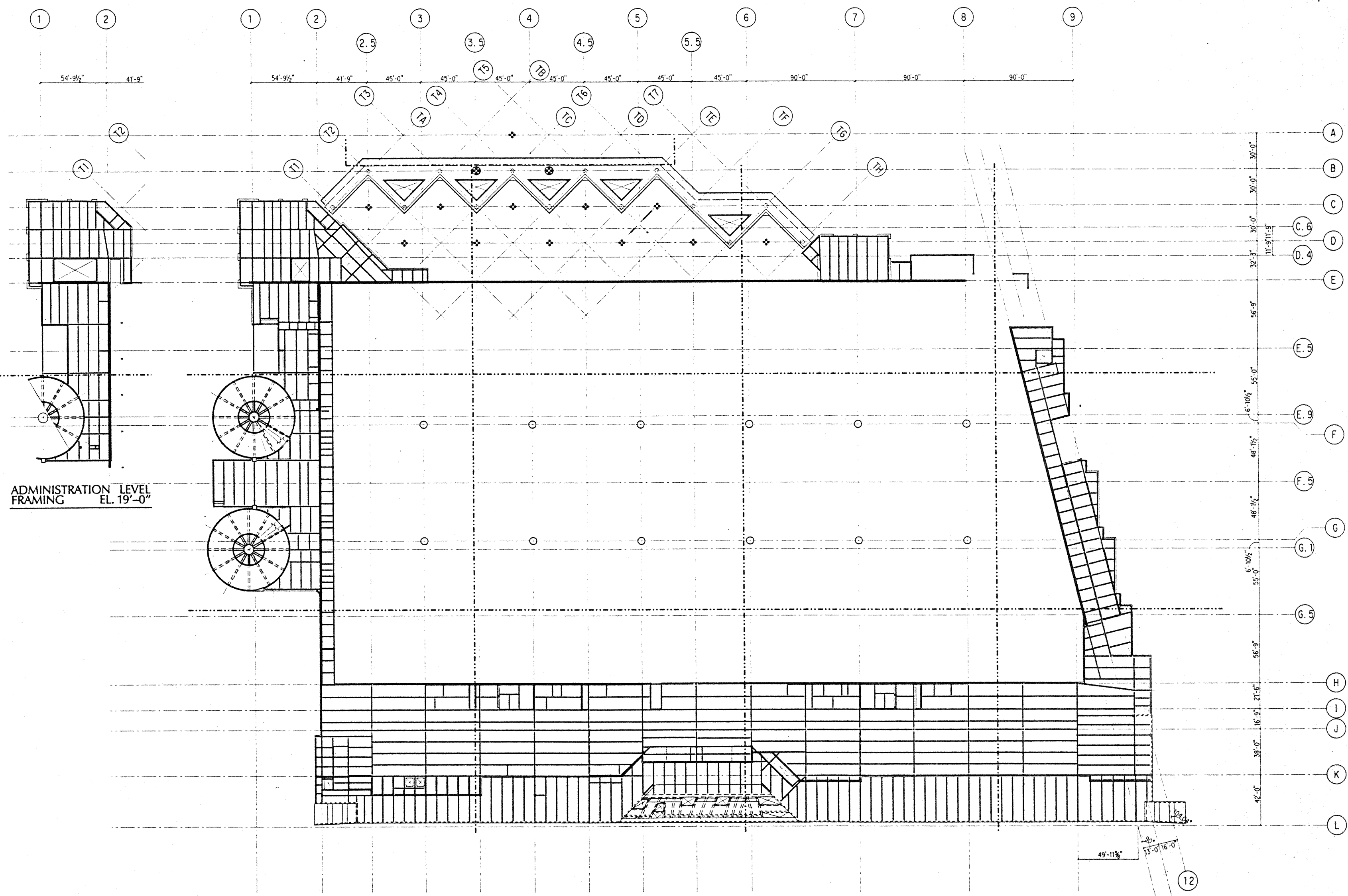
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No. Date By Description

Sheet Title

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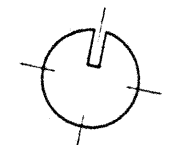
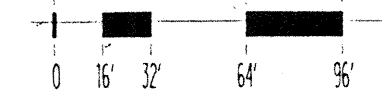
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Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.0.2



ADMINISTRATION LEVEL
FRAMING EL. 19'-0"

INTERMEDIATE LEVEL FRAMING PLAN



Fri Dec 1 16:01:05 1995

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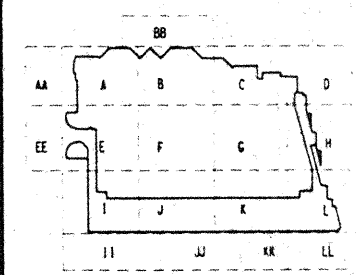
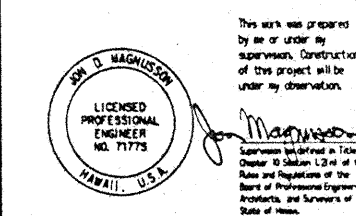
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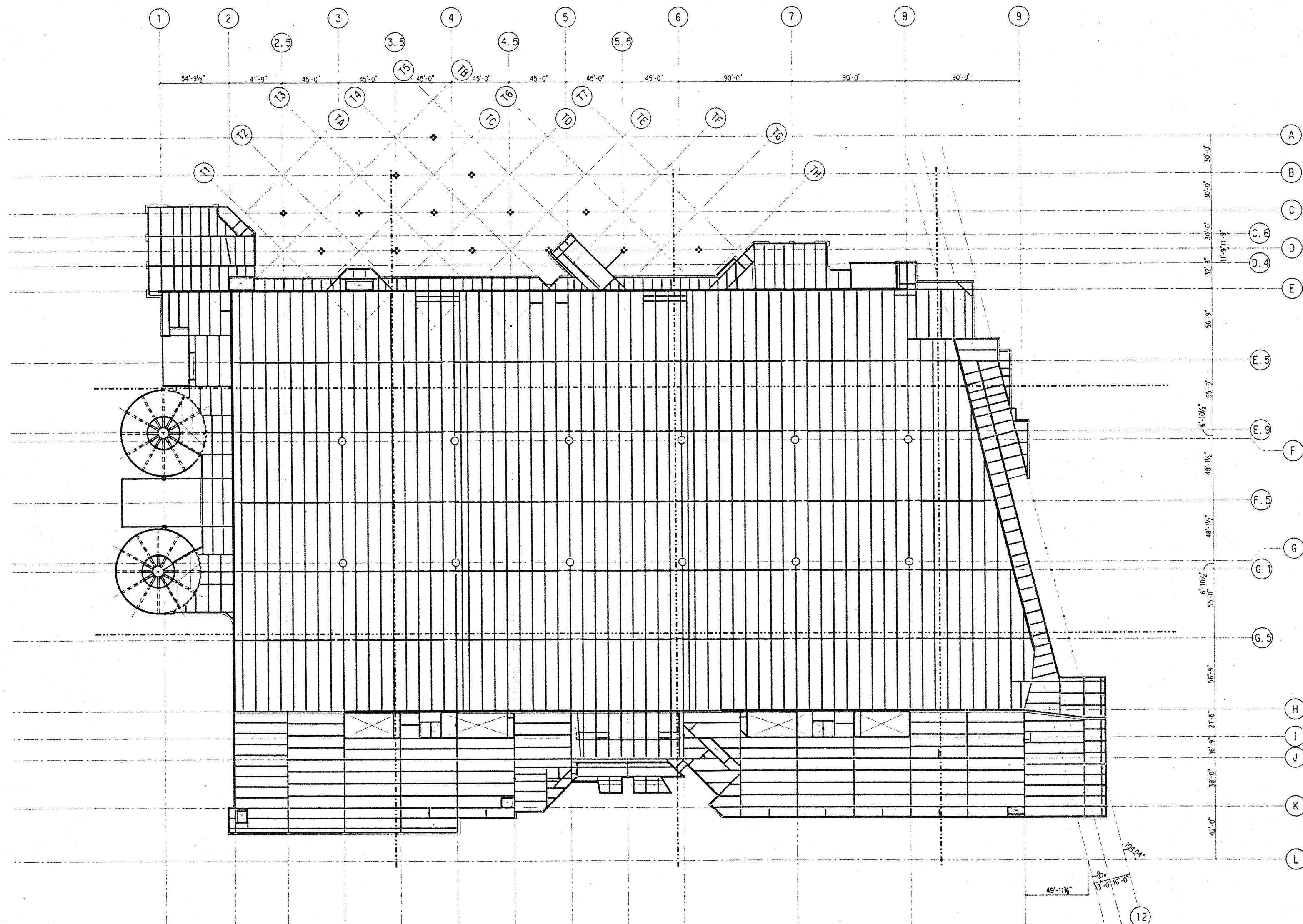
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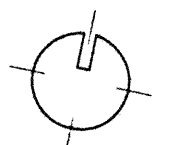
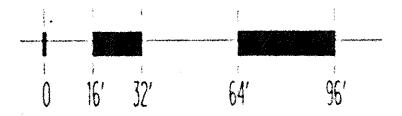
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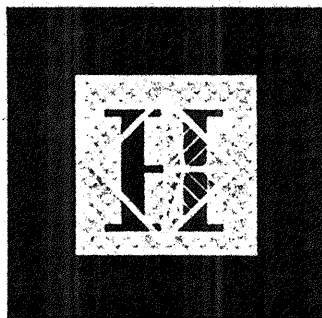
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 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036, Q2, 9
 Sheet No.



PARKING FRAMING PLAN



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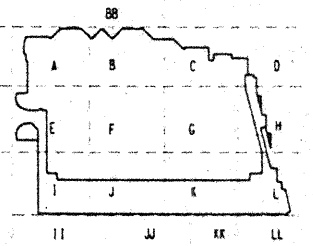
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SELLING WARD MAGNUSON BAKER INC.

Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 2000 Seattle WA 98101-2698
206/292-1200 FAX: 206/292-1201

This work was prepared by me or under my supervision. Construction of this project will be under my observation.



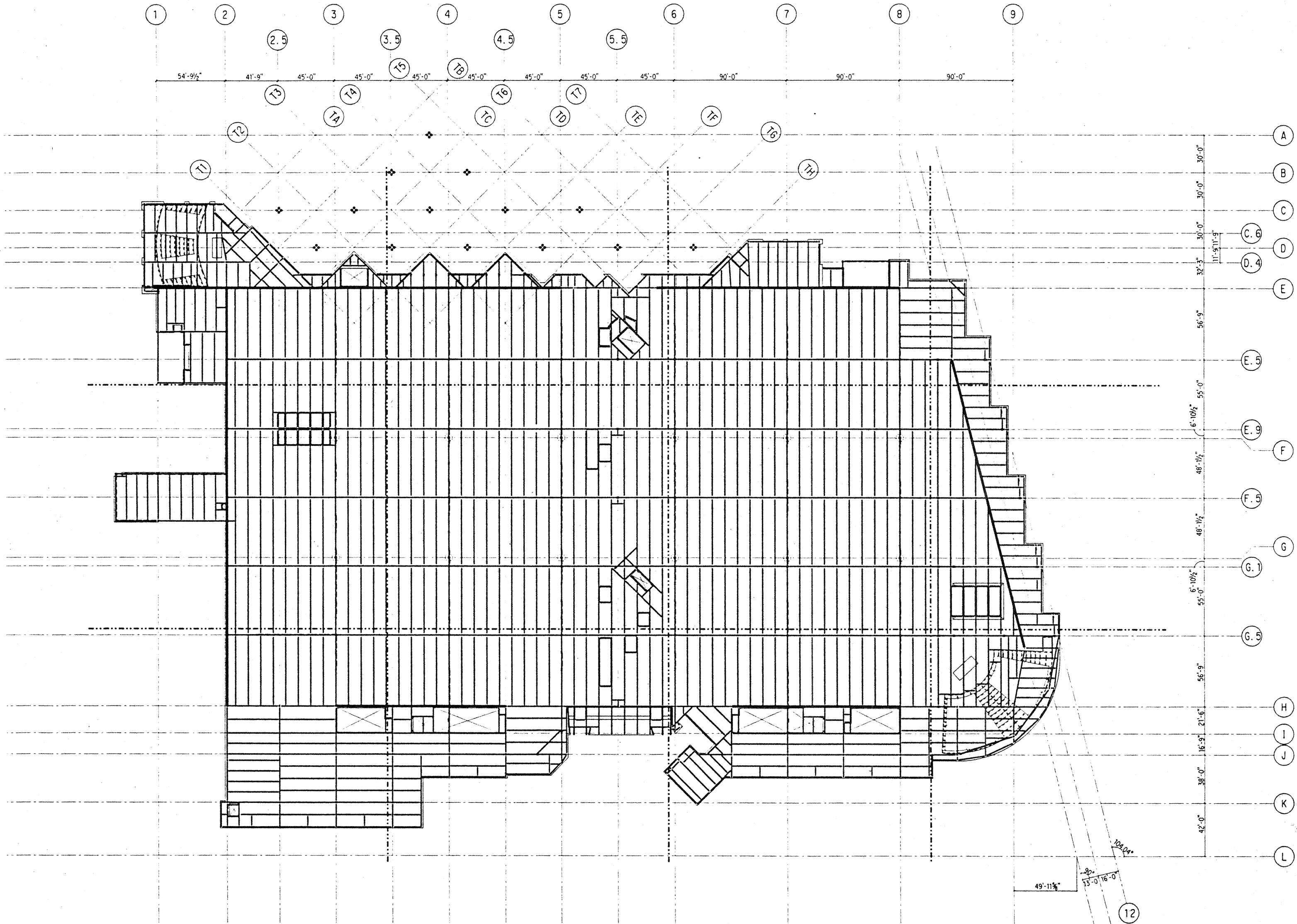
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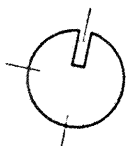
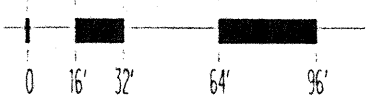
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MEETING FRAMING PLAN

Designed **BJM**
 Drawn **DMS**
 Checked **JCM**
 Date 4 / 12 / 95
 Job No. 93036, 02, 9
 Sheet No.



MEETING FRAMING PLAN



Thu, Nov 30 18:00:48 1995

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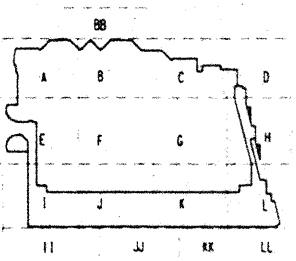
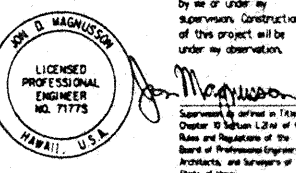
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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2899
206.292.1000 FAX: 206.292.1001



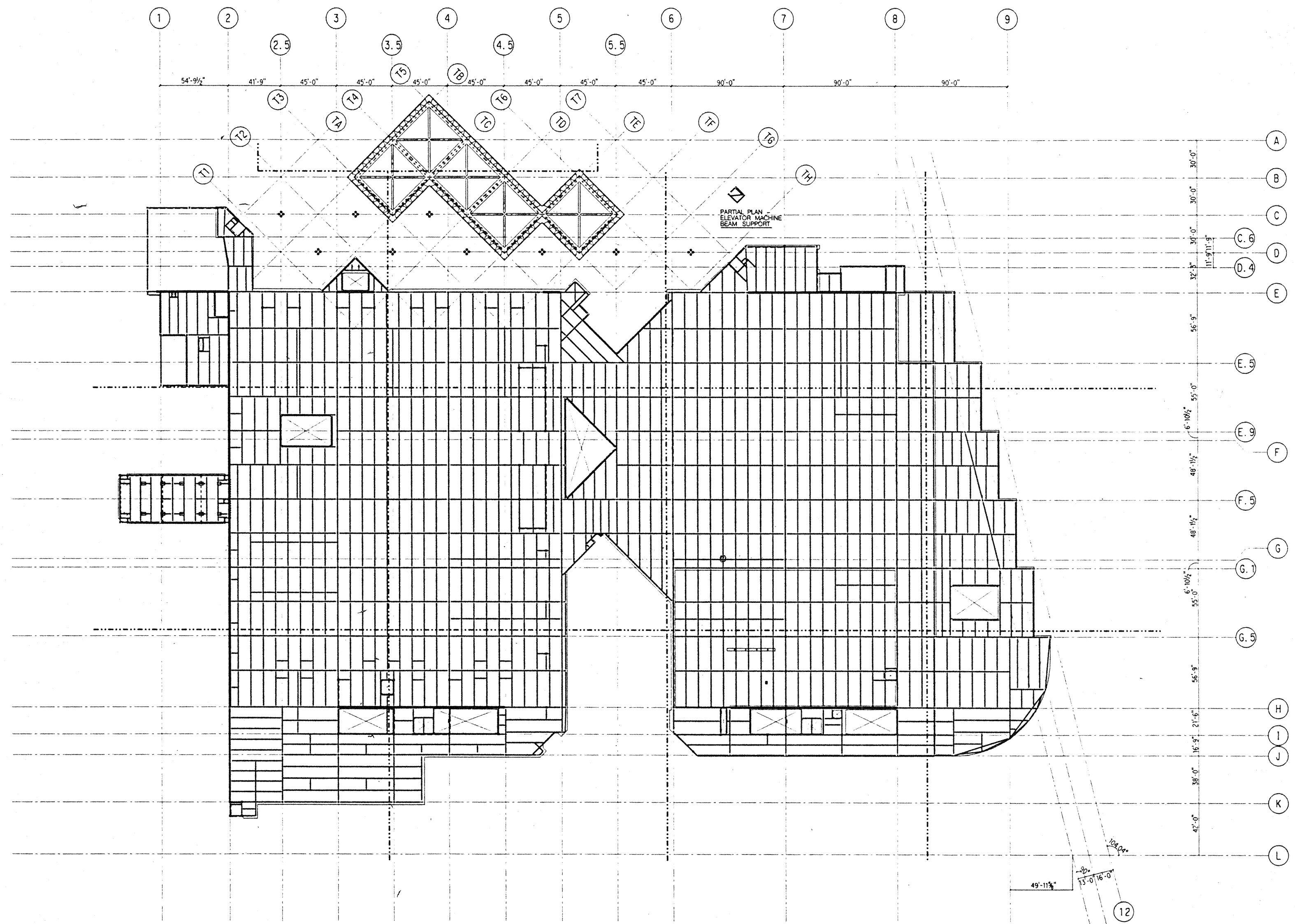
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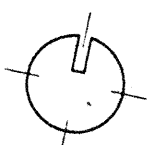
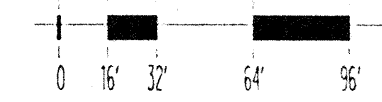
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BALLROOM FRAMING PLAN

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	



BALLROOM FRAMING PLAN



Tue Nov 30 17:22:46 1995

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1/12 pl1



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Seattle, Washington

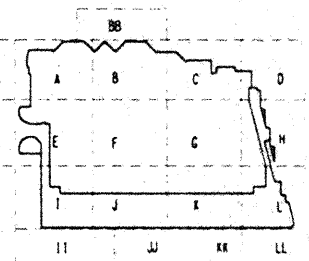
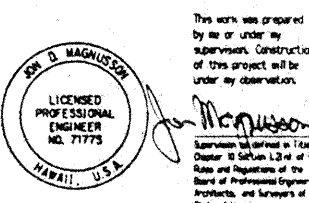
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Consultants

SKILLING WARD MAGUIRE BANKS INC.

Consulting Structural and Civil Engineers

1901 Fifth Avenue, Suite 3200 Seattle, WA 98107-2699
206 220-1200 FAX 206 453-2211



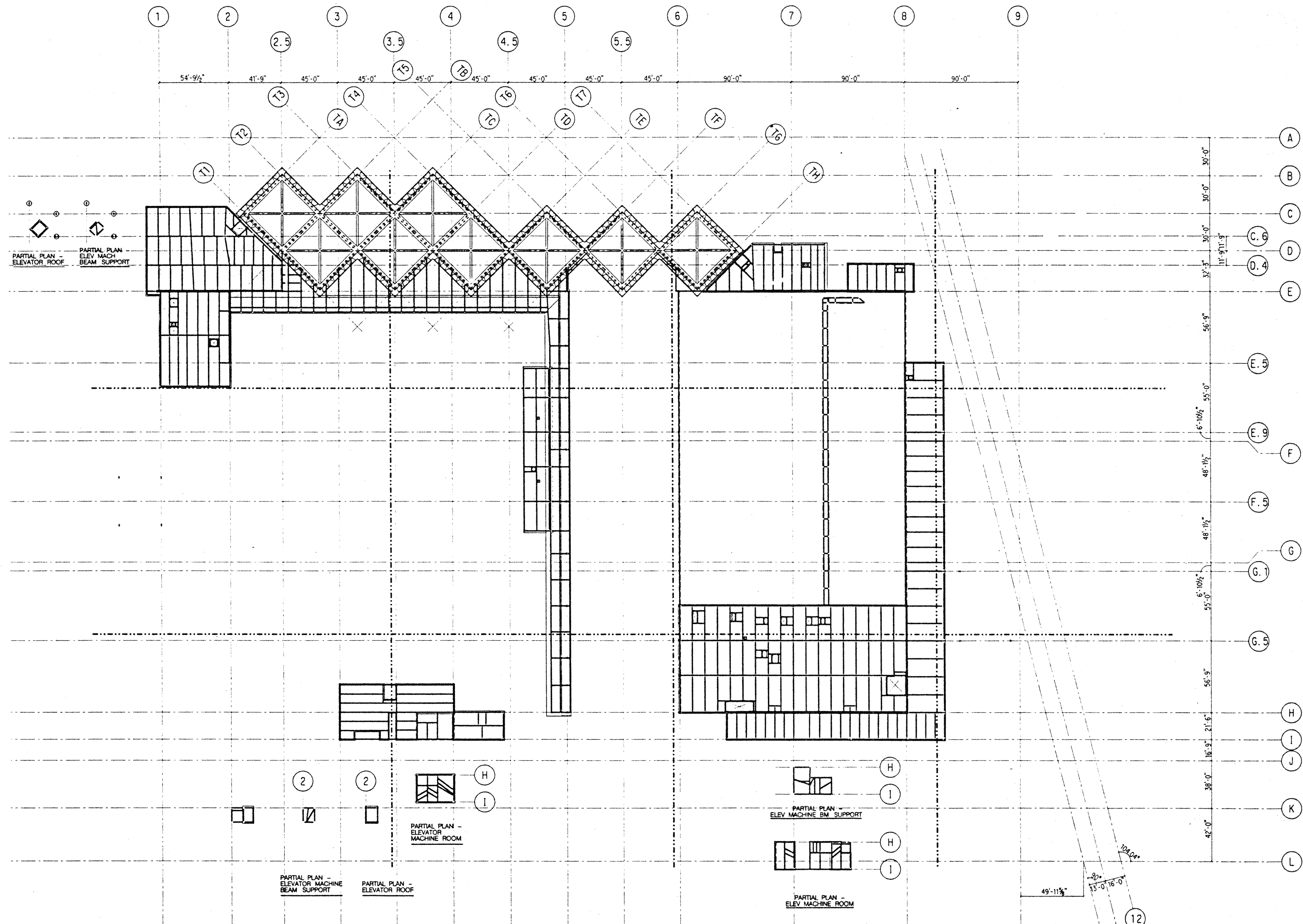
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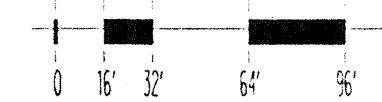
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Designed BJM
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 Date 4 / 12 / 95
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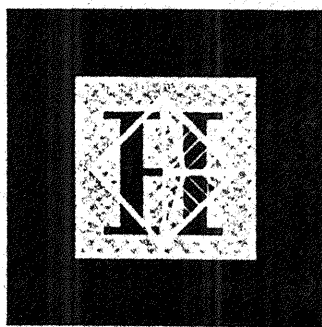
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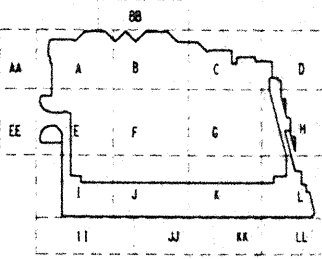
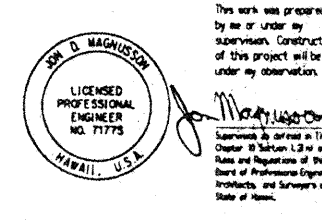
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1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2599
(206) 292-1200 FAX: (206) 623-1201

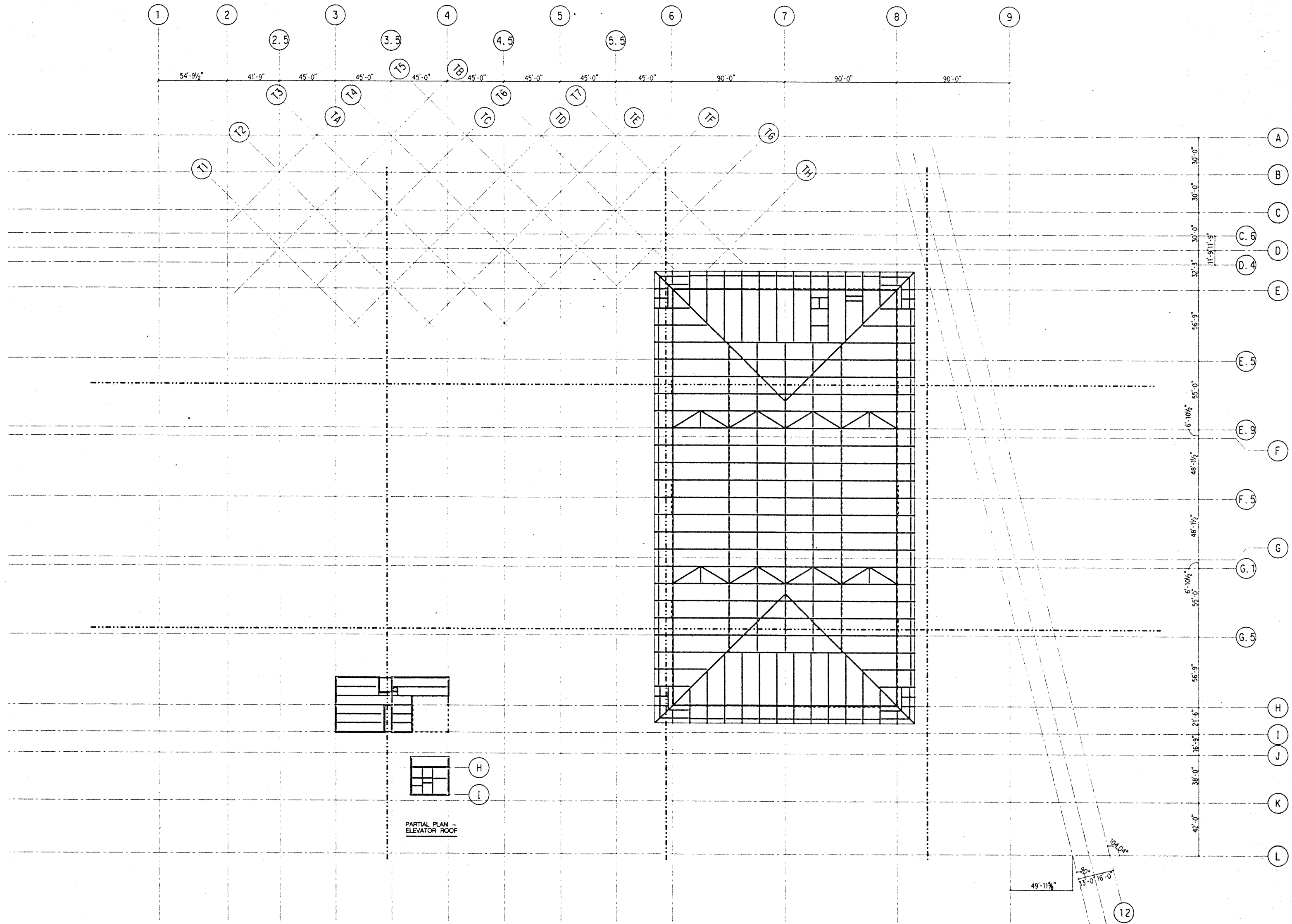


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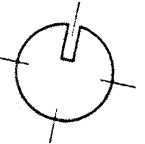
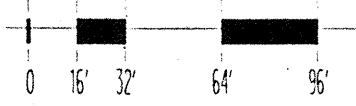
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9-15-95			BUILDING PERMIT SET

Sheet Title
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Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.



ROOF FRAMING PLAN



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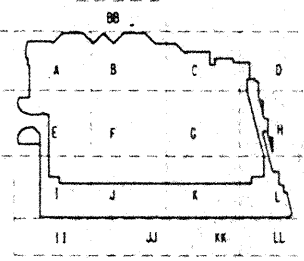
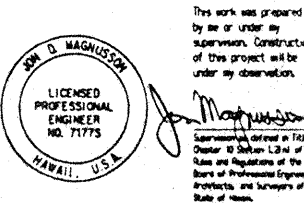
Loschky Marquardt & Nesholm
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SEILING WARD MAGNUSON BARKSHIRE INC.

Consulting Structural and Civil Engineers
1331 Fifth Avenue, Suite 3200 Seattle WA 98101-2899
(206) 292-1200 FAX: (206) 623-1201

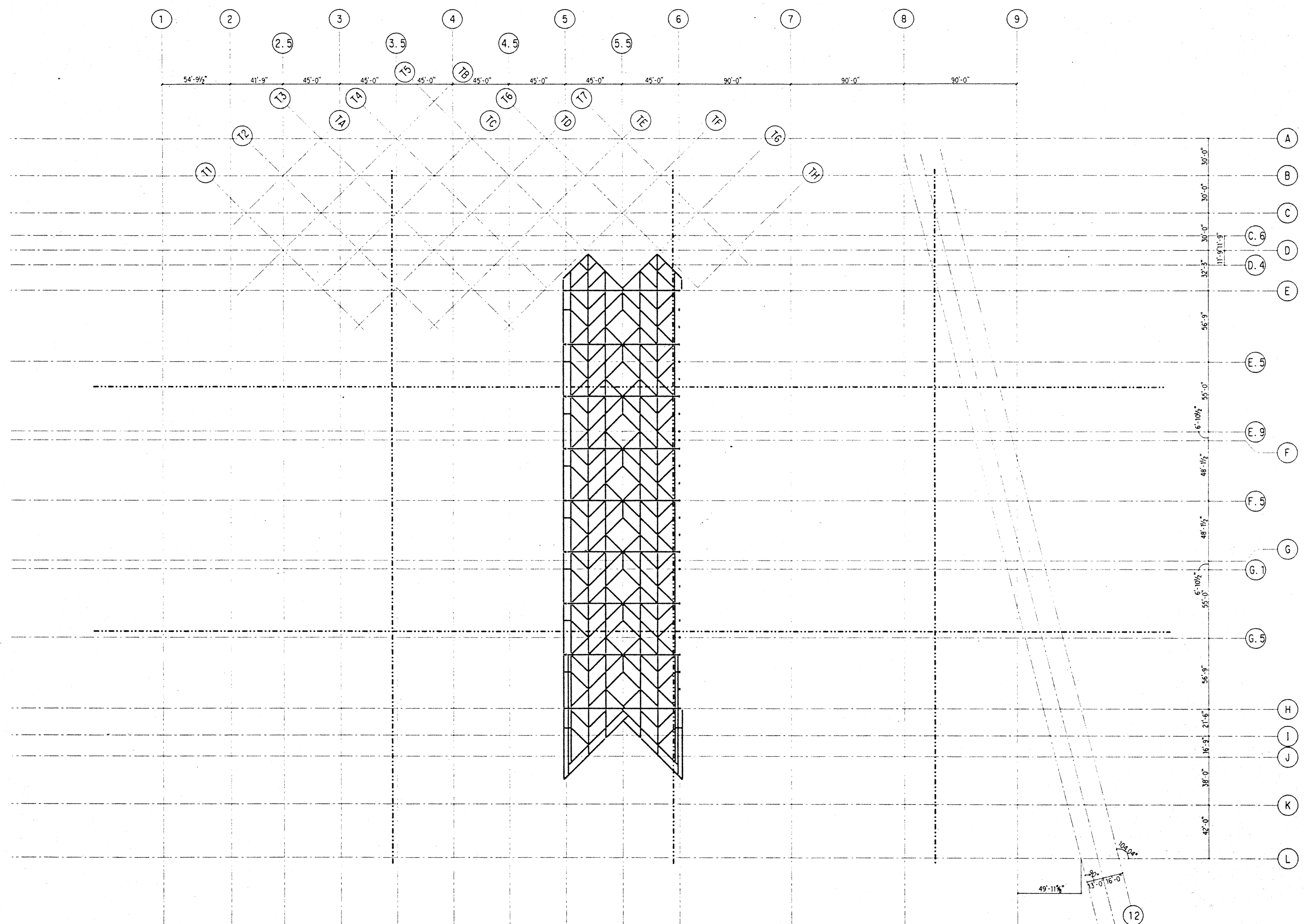


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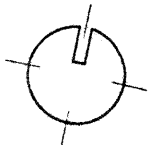
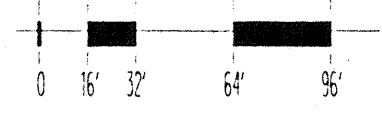
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Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93035.02.9
Sheet No.	

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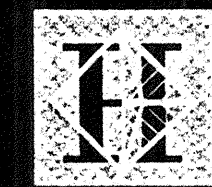
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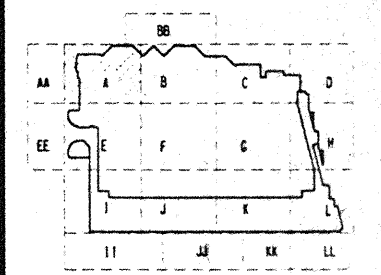
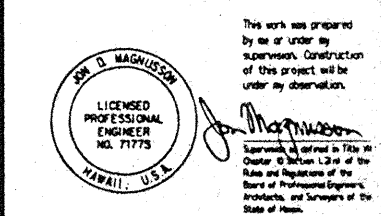
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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98109-2699
(206) 292-1200 FAX: (206) 292-1001



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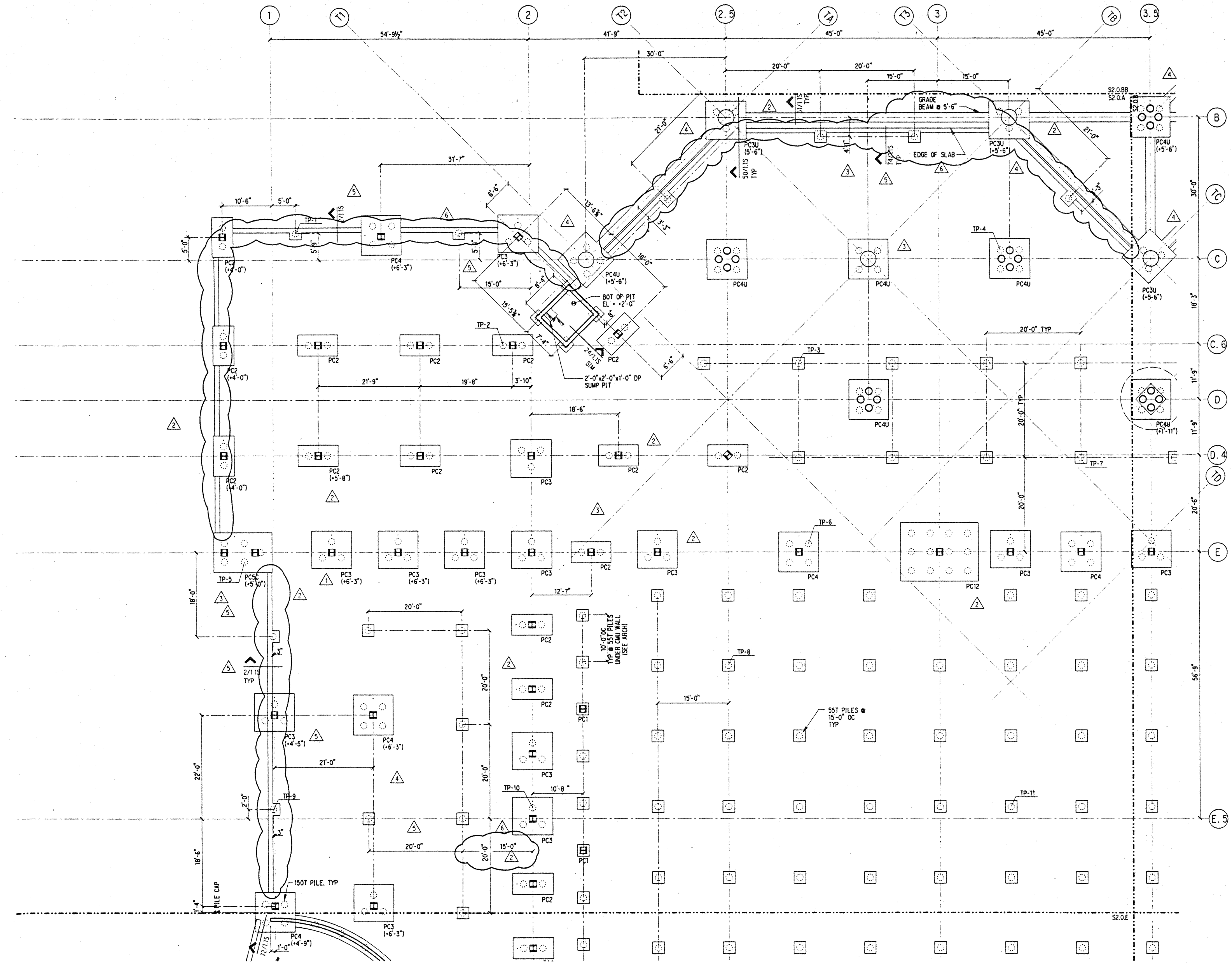
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△ 8-14-95			
△ 6-19-95			
△ 4-28-95			ADDENDUM #1
4-17-95			ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN SECTOR A

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.0.A



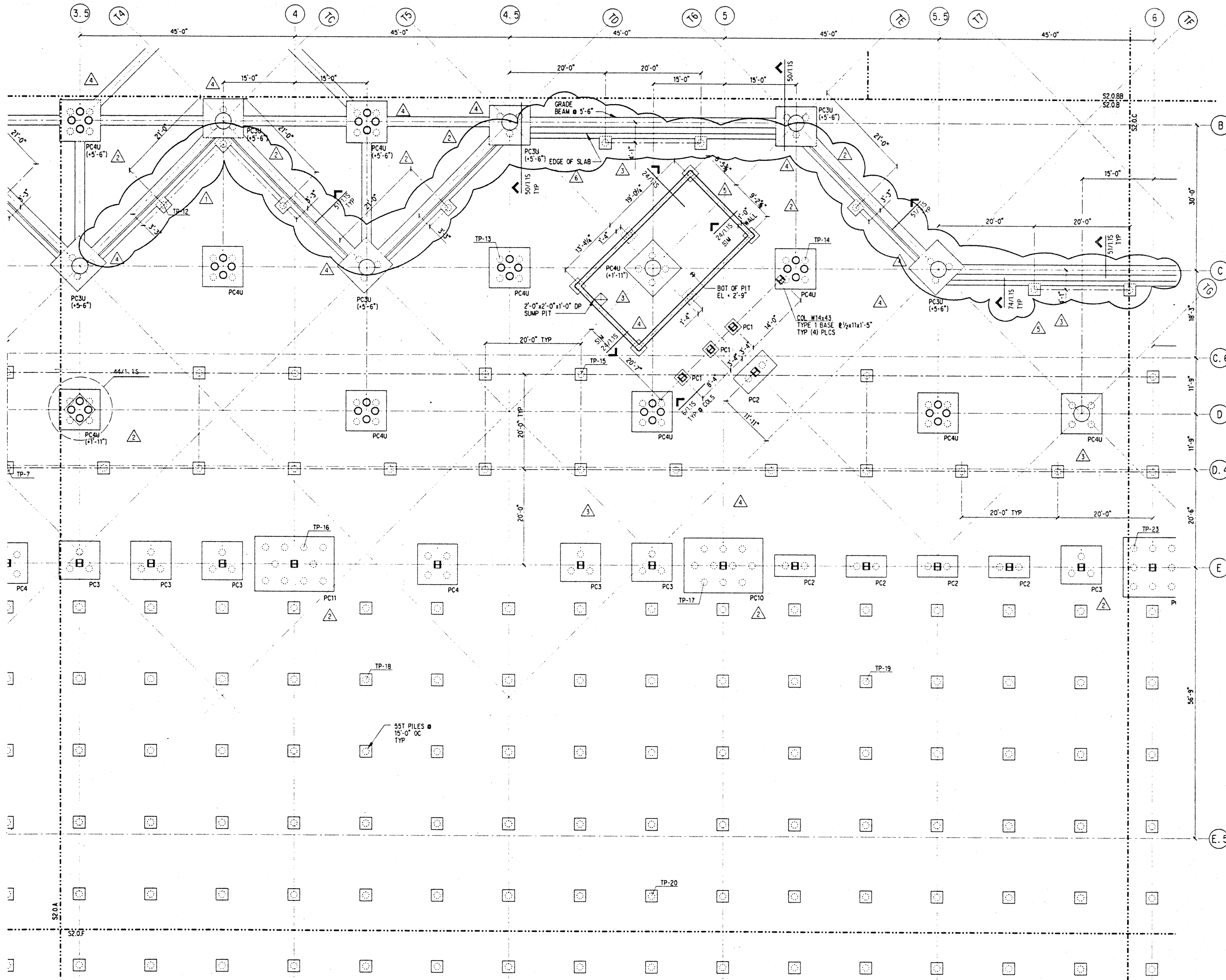
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- REFERENCE DRAWINGS:
S0. SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
S1. TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU PLANS
S1.20 CONCRETE SCHEDULES
S1.21 S1.22 STEEL SCHEDULES
S3. TYPICAL STEEL COLUMN TREE ELEVATIONS AND DETAILS
S2. PLANS
S4. BRACED FRAME ELEVATIONS
S5. TRUSS ELEVATIONS
S6. BRACED FRAME & TRUSS DETAILS
DETAIL BOOK
- PILES FOR PC2 THRU PC40 ARE 150 TON CAPACITY PILES. ALL SINGLE PILES ARE 55 TON CAPACITY PILES.
- PILECAPS AND PILES REQUIRING UPLIFT CAPACITY ARE DENOTED WITH A "U" ON THE PLANS. EX: PC14U. PROVIDE TENSION SPLICES FOR PILES AT THESE PILECAPS.
- FOR TOP OF PC1 ELEV SEE 18/S1.2.
- TOP OF PILECAP AT ELEVATION +6.0 FEET, TYP UNLESS NOTED OTHERWISE.
- REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, BOLLARDS, EDGE ANGLES, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
- TP-# DENOTES TEST PILE NUMBER. SEE SPECIFICATIONS.

F:\1\Dec 116\53\36 1995

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p:\pjt

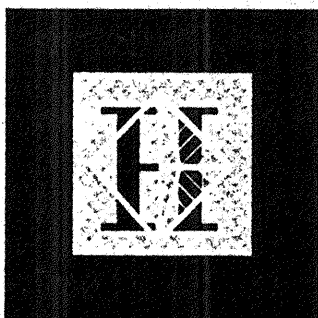


Fri Dec 11 16:56:03 1995

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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
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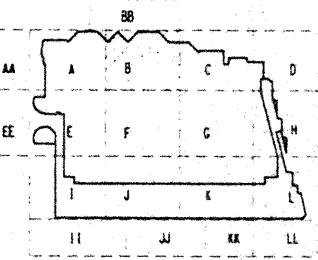
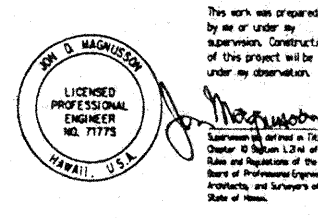
Hawai'i Convention Center

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 Honolulu, Hawaii

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SKELLING WARD MAGNUSON BAUCKE INC.
 Consulting Structural and Civil Engineers
 1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
 (206) 292-1200 FAX: (206) 292-1201

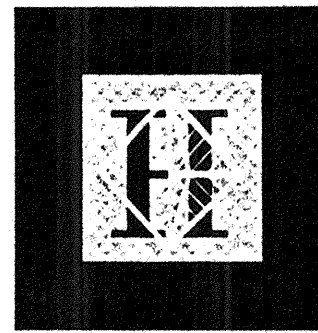


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△ 8-4-95		
△ 6-19-95		
△ 4-28-95		ADDENDUM #1
△ 4-17-95		ISSUED FOR FOUNDATION PERMIT

FOUNDATION PLAN SECTOR B

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4/12/95
 Job No. 93036.02.9
 Sheet No.

S2.0.B



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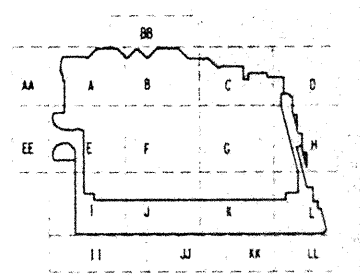
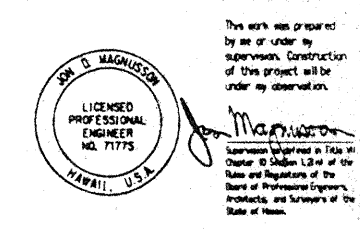
Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

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SKILLING WARD MCGUIRE BARKER INC.

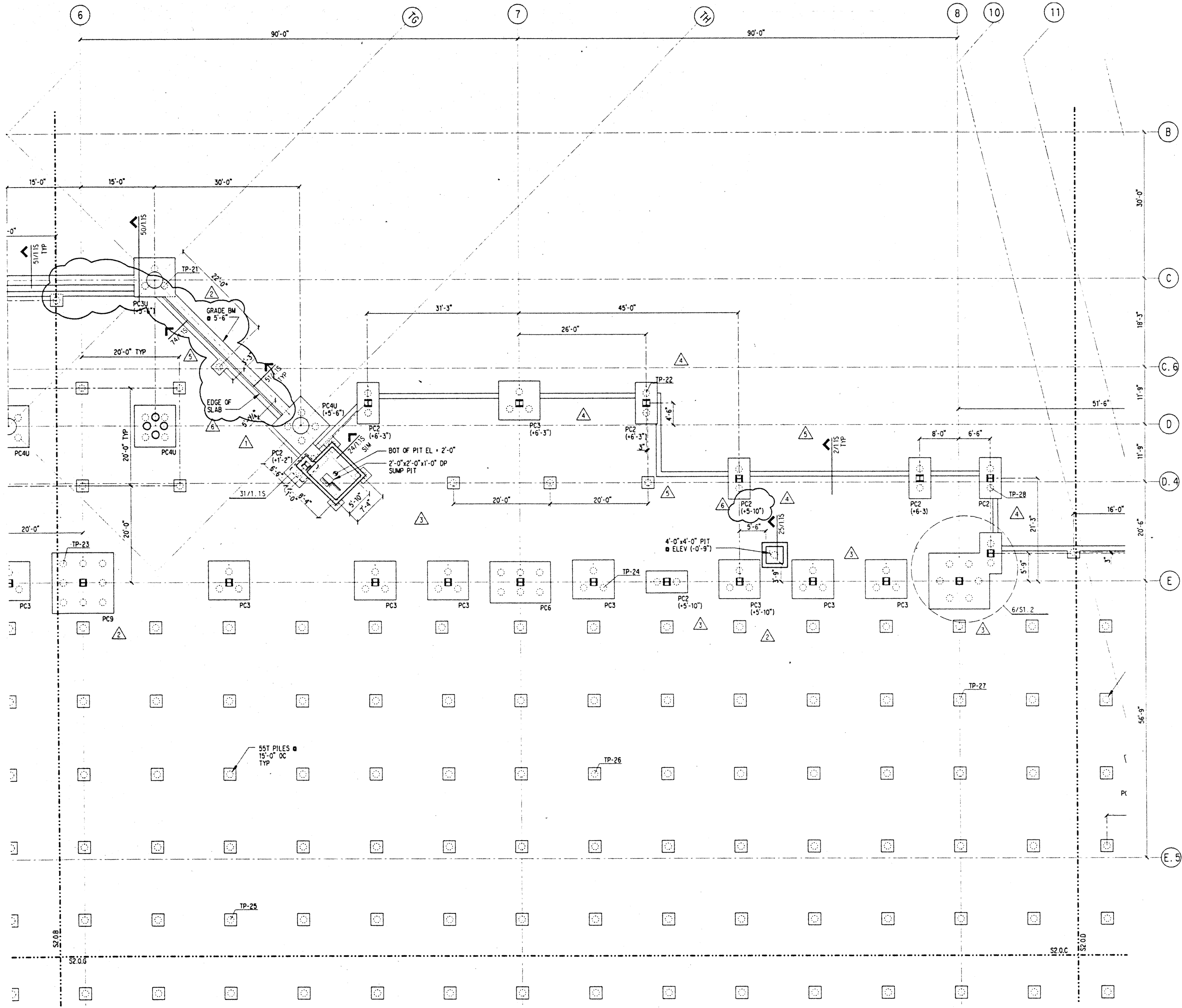
Consulting Structural and Civil Engineers
1001 Fifth Avenue, Suite 2200 Seattle WA 98101-2696
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△ 8-31-95			
△ 8-4-95			
△ 6-19-95			
△ 4-17-95			ISSUED FOR FOUNDATION PERMIT

FOUNDATION PLAN SECTOR C

Designed BJM
 Drawn DMS
 Checked JCM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.0.A

S2.0.C

Fri Dec 11 16:58:30 1995 j:\hcc\fdn.c.dgn 2/9 pit



Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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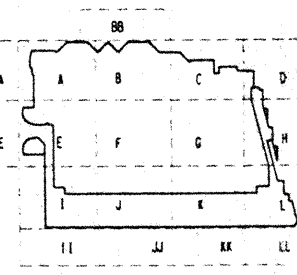
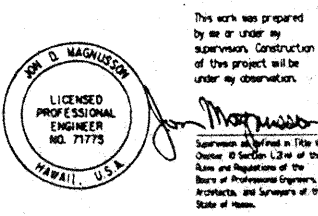
Loschky Marquardt & Nesholm
Seattle, Washington

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Consultants

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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2599
(206) 292-1200 FAX: (206) 292-1231



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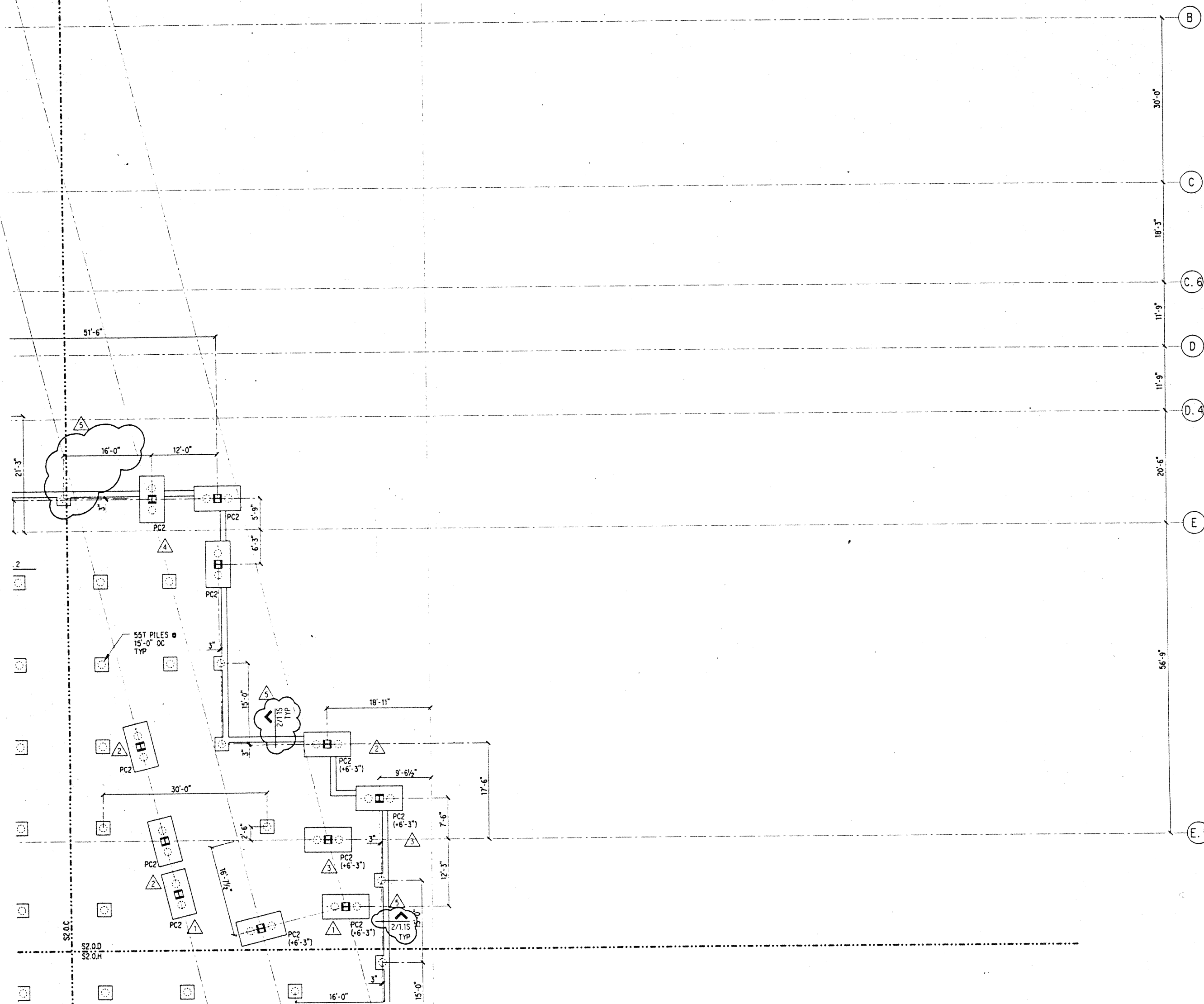
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FOUNDATION PLAN SECTOR D

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Date	4 / 12 / 95
Job No.	93036.02.9
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S2.0.D

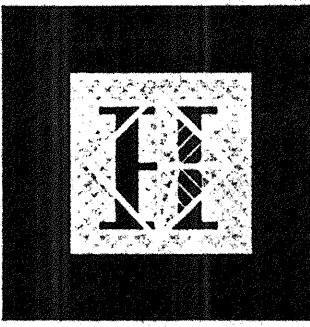
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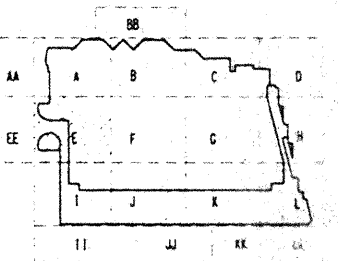
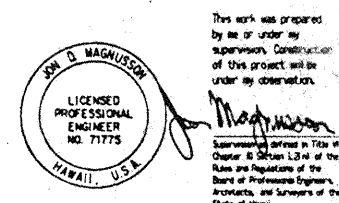
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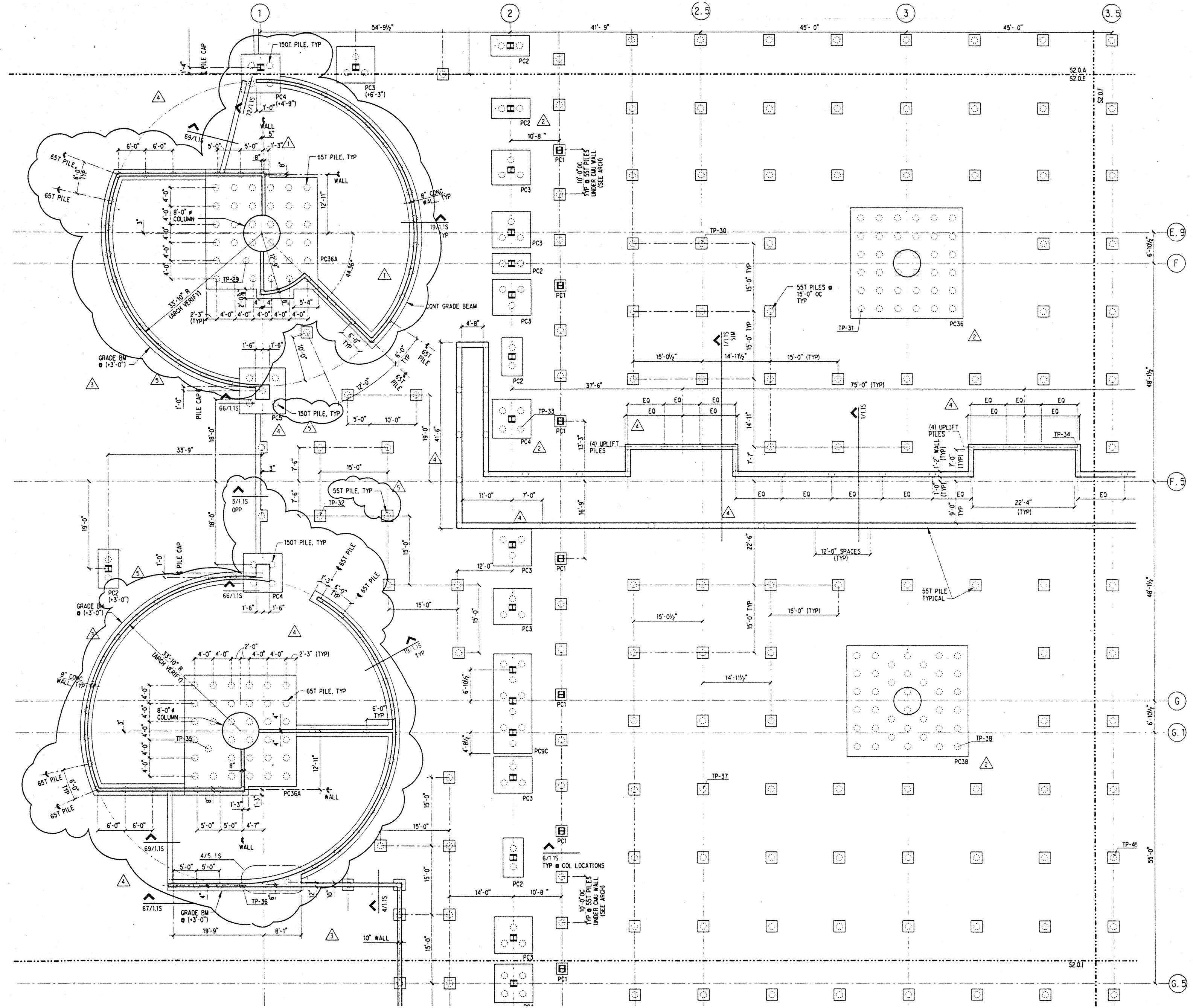


Revisions

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8-4-95			
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4-17-95	ISSUED FOR FOUNDATION PERMIT		
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Sheet Title
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Date **4 / 12 / 95**
Job No. **93036.02.9**
Sheet No.



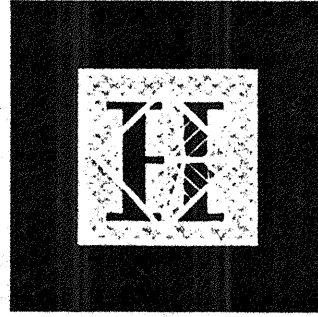
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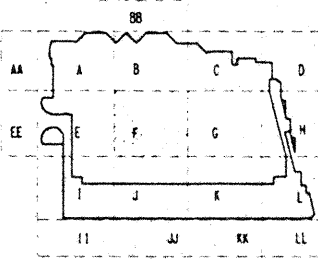
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LICENSED PROFESSIONAL ENGINEER
NO. 71775
HAWAII, U.S.A.

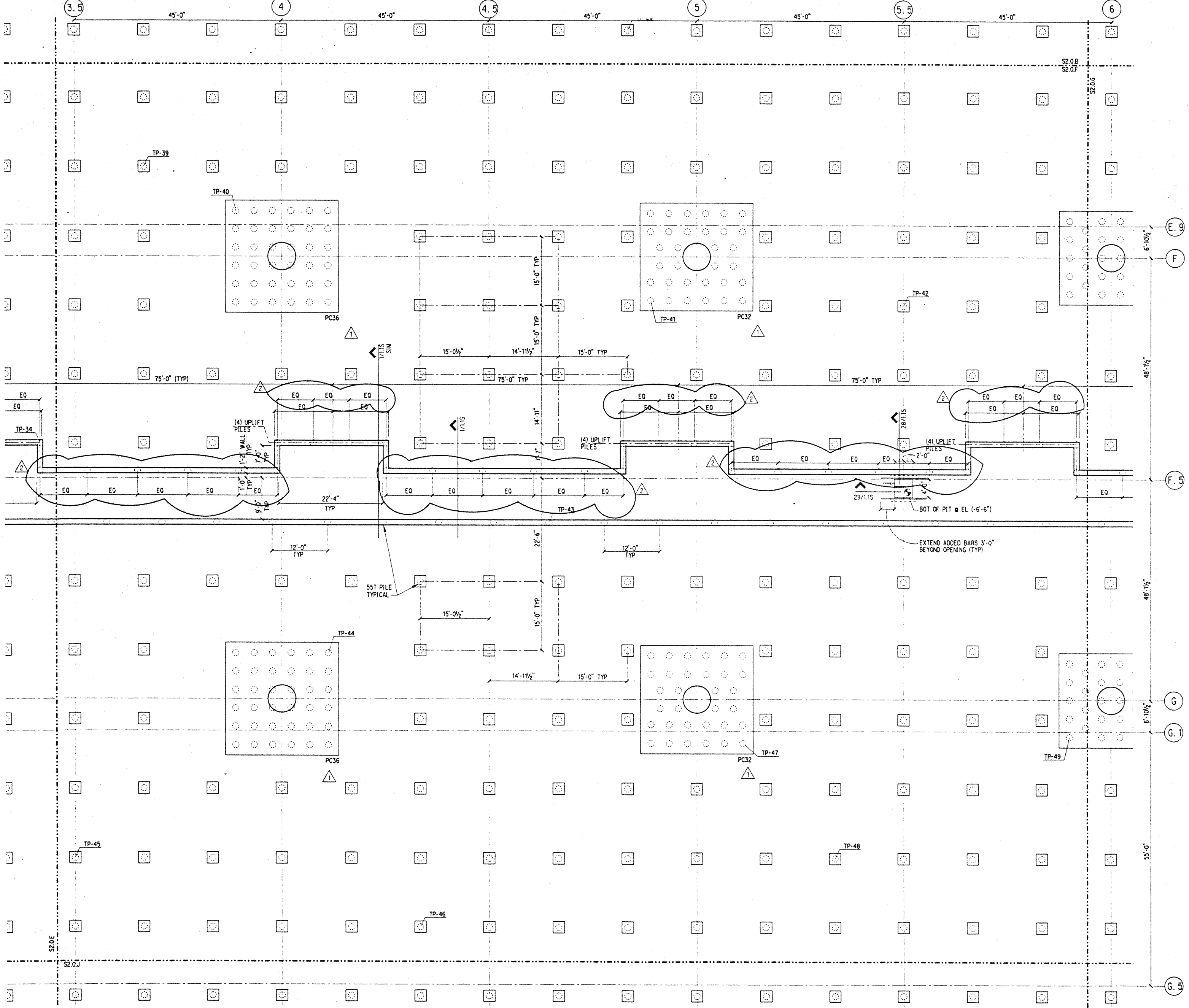


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No.	Date	By	Description

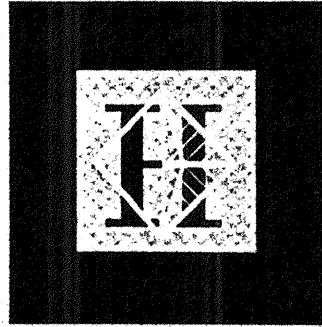
FOUNDATION PLAN SECTOR F

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Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.



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1. REFERENCE NOTES ON S2.0.A

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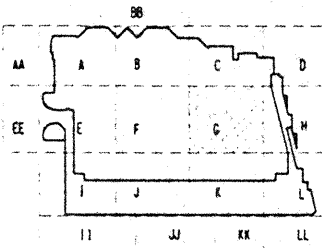
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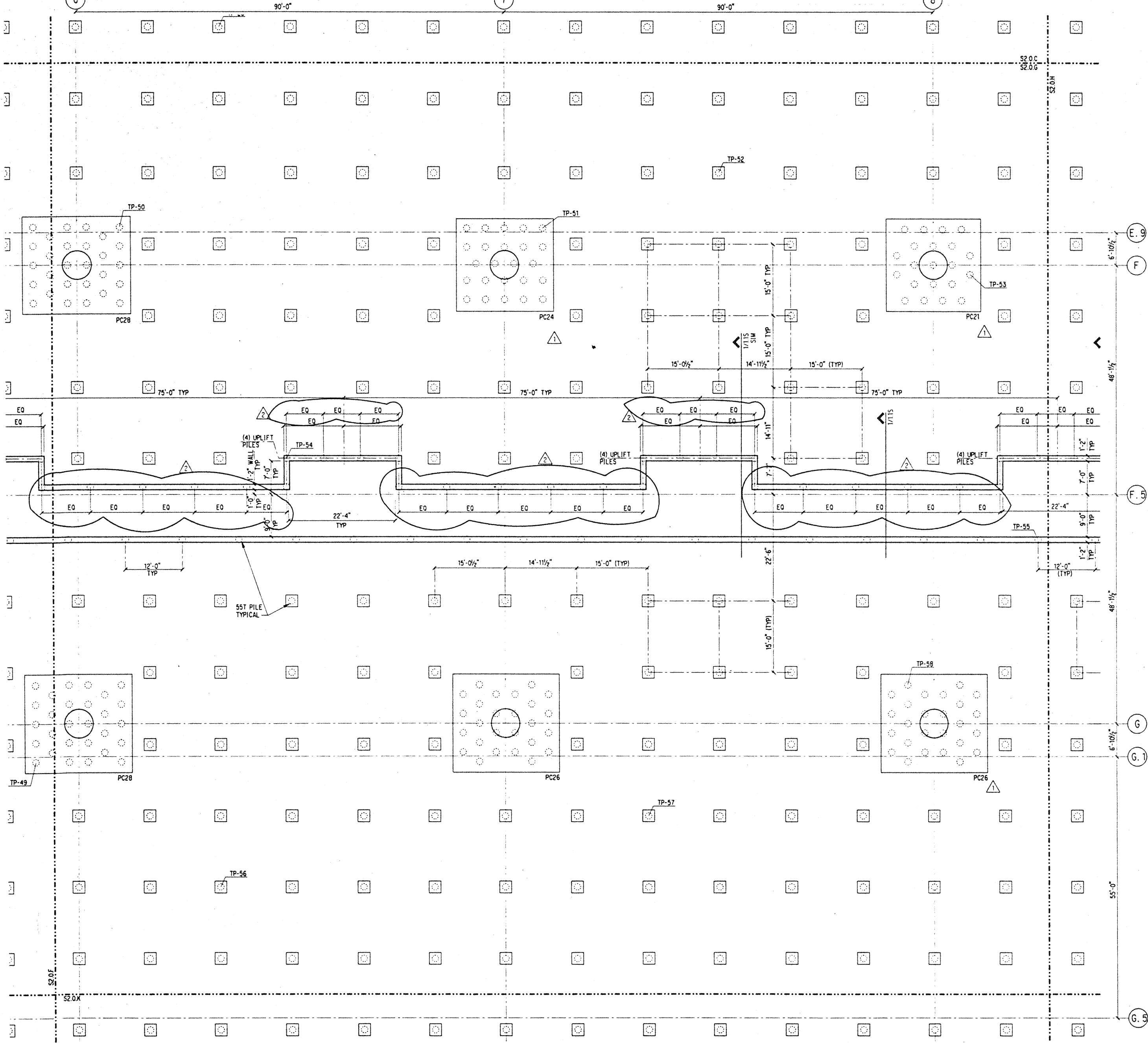
S. D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 71775
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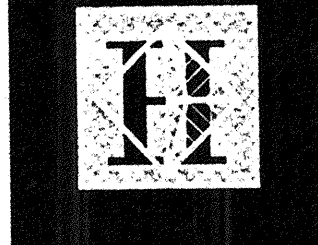
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Date 4 / 12 / 95
Job No. 93036.02.9
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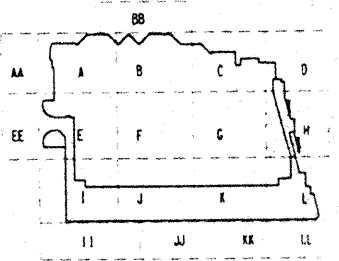
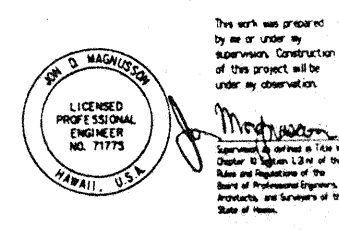
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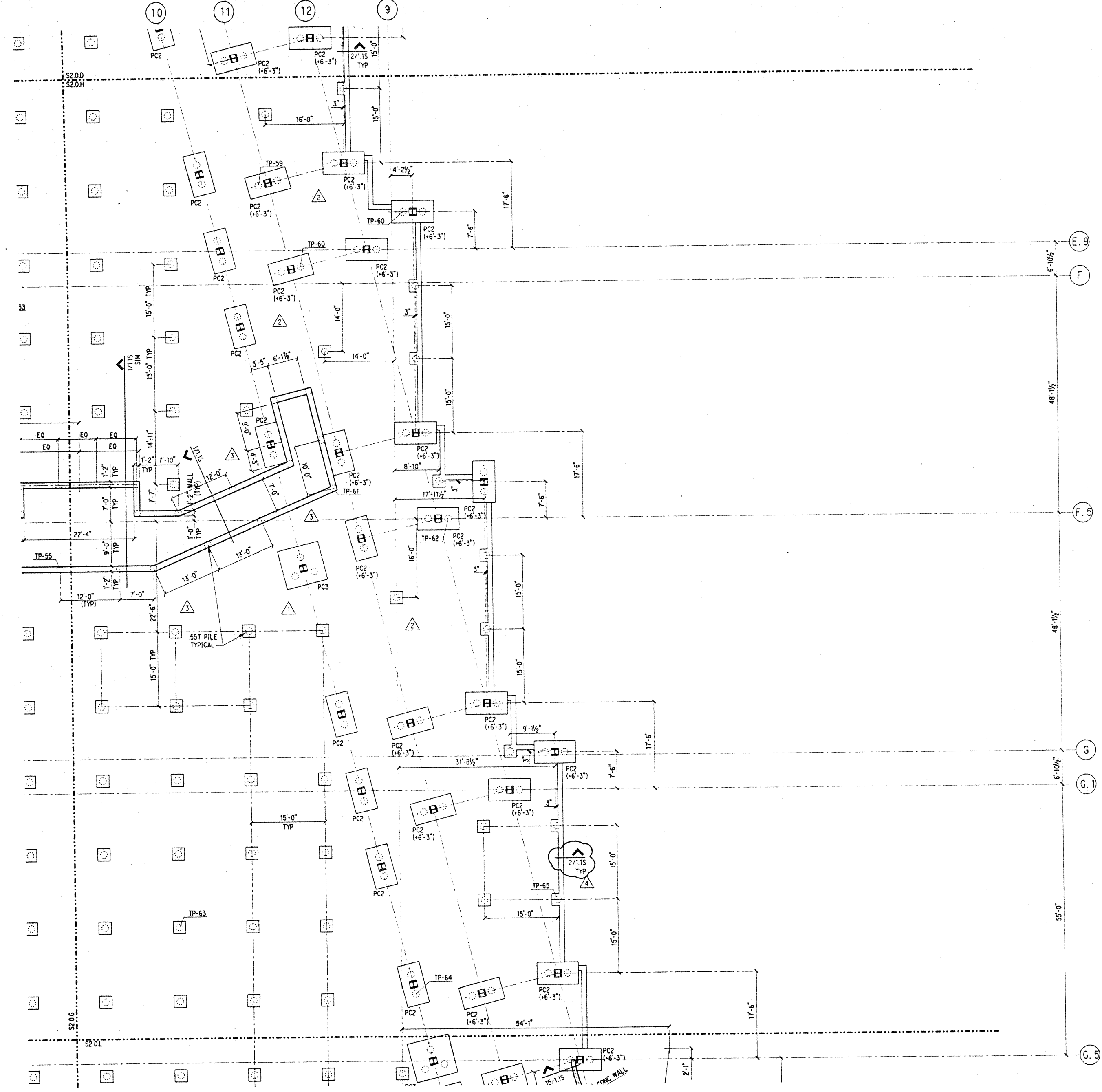


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6-19-95			
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FOUNDATION PLAN SECTOR H

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Job No.	93036.02.9
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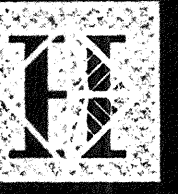


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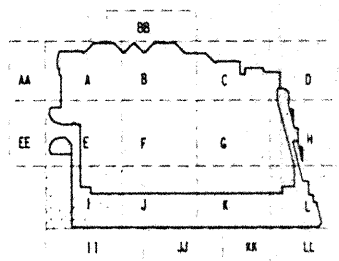
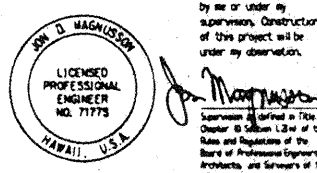
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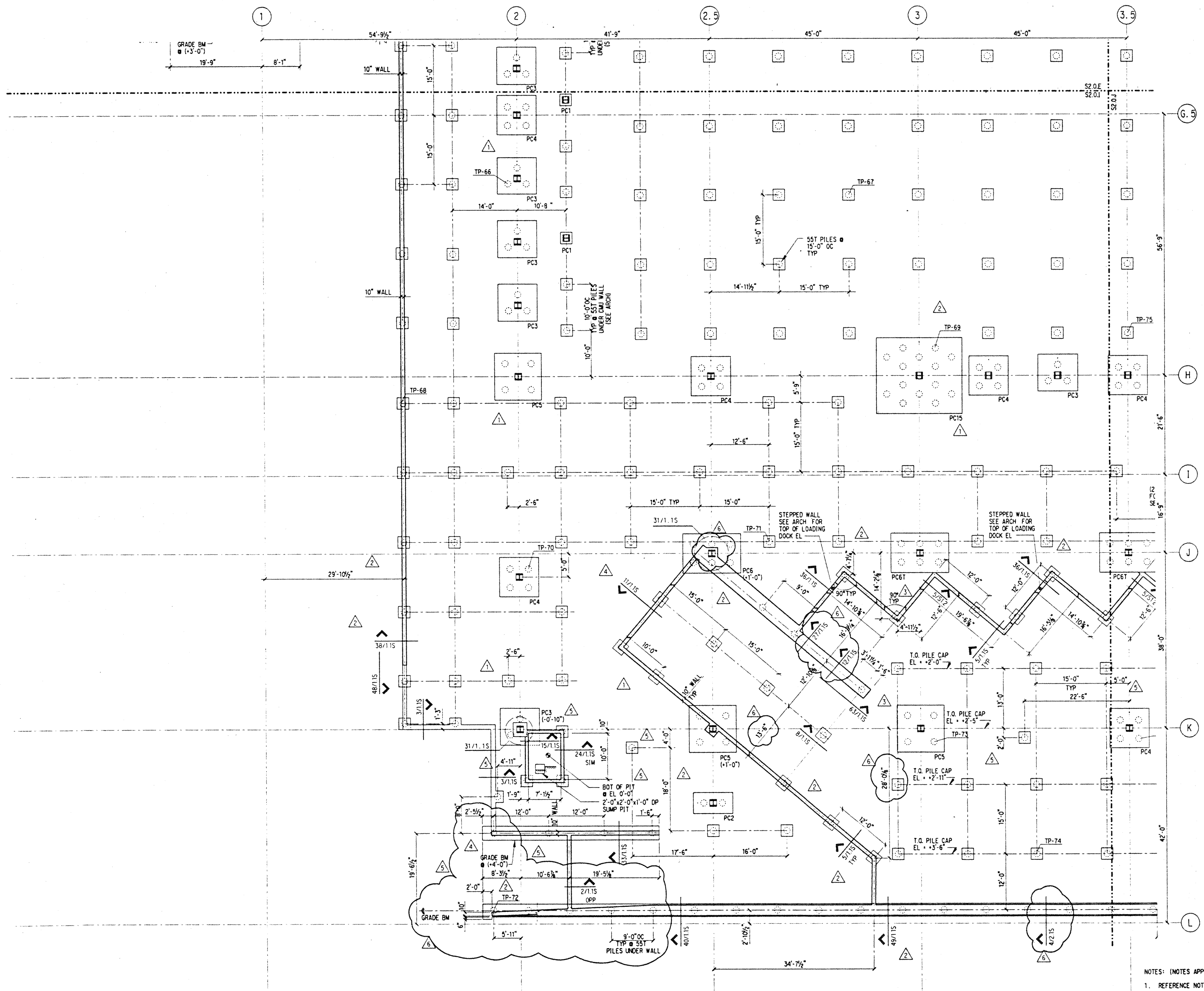
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8	4-17-95	ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN SECTOR I

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 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

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 1. REFERENCE NOTES ON S2.0.A

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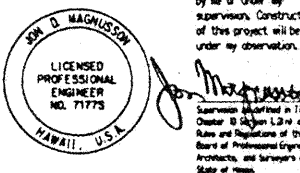
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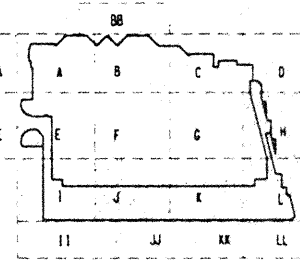
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No.	Date	Description
1	12-15-95	FINAL DOCUMENT
2	11-7-95	
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9	4-17-95	ISSUED FOR FOUNDATION PERMIT

Sheet Title

**FOUNDATION
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SECTOR J**

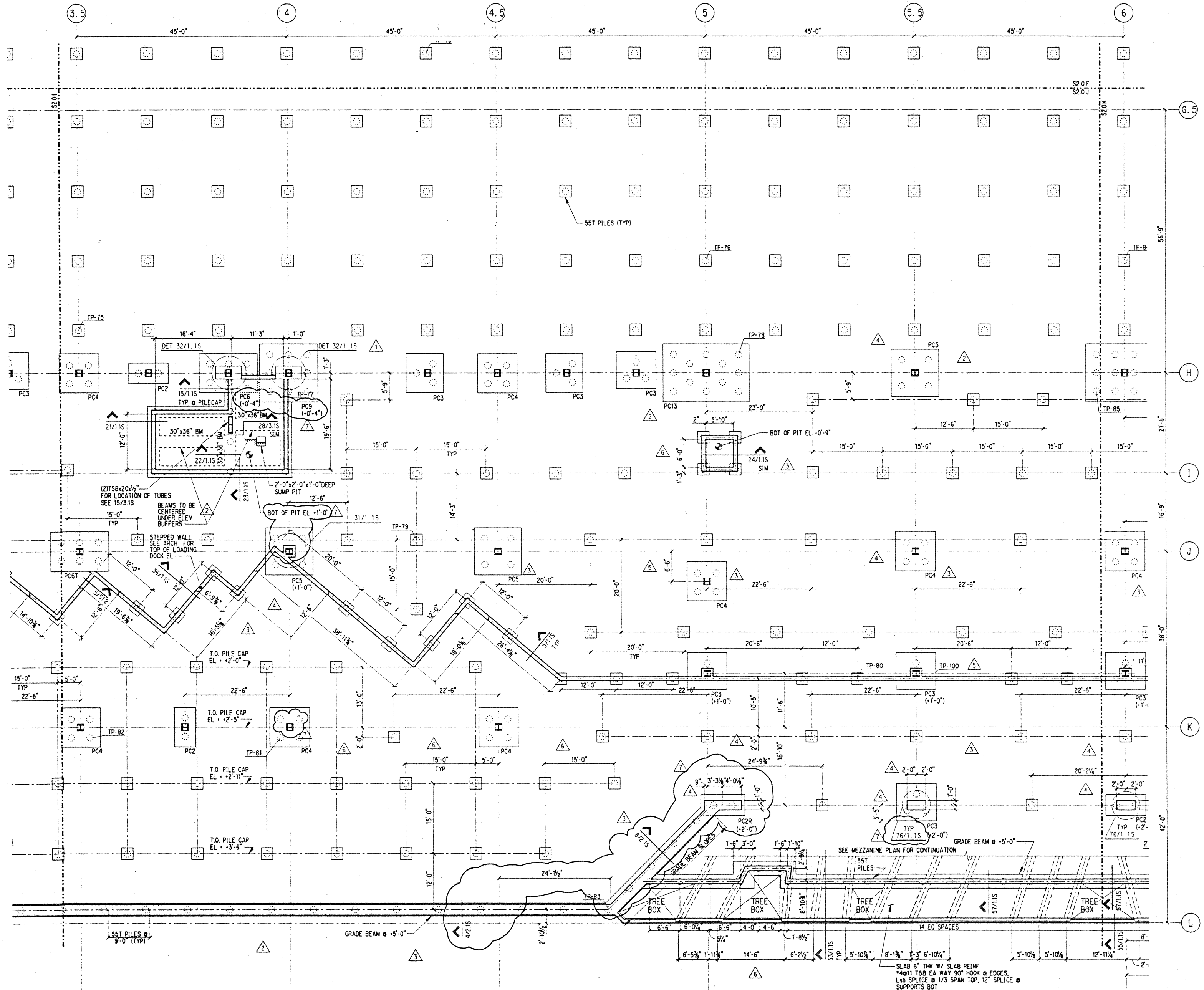
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Sheet No.	

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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.0.A



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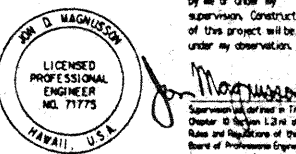
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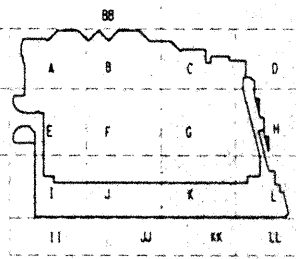
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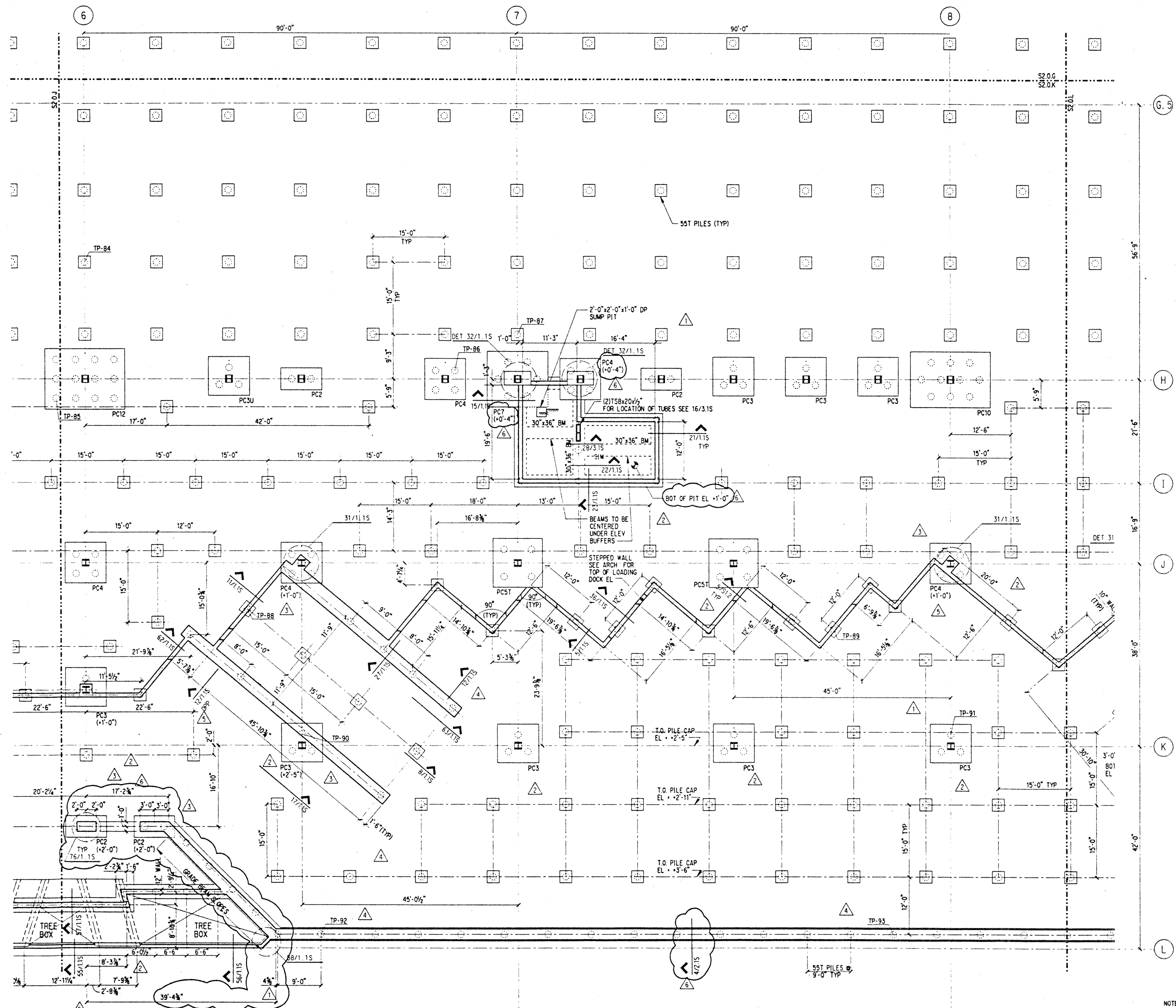
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Sheet Title

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 Job No. 93036, Q2, 9
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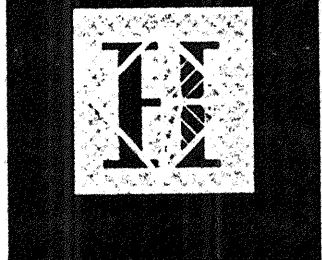


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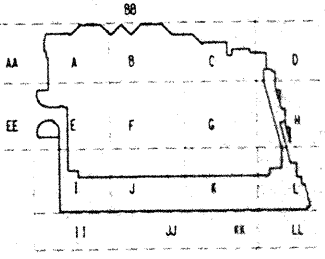
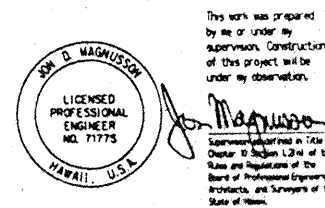
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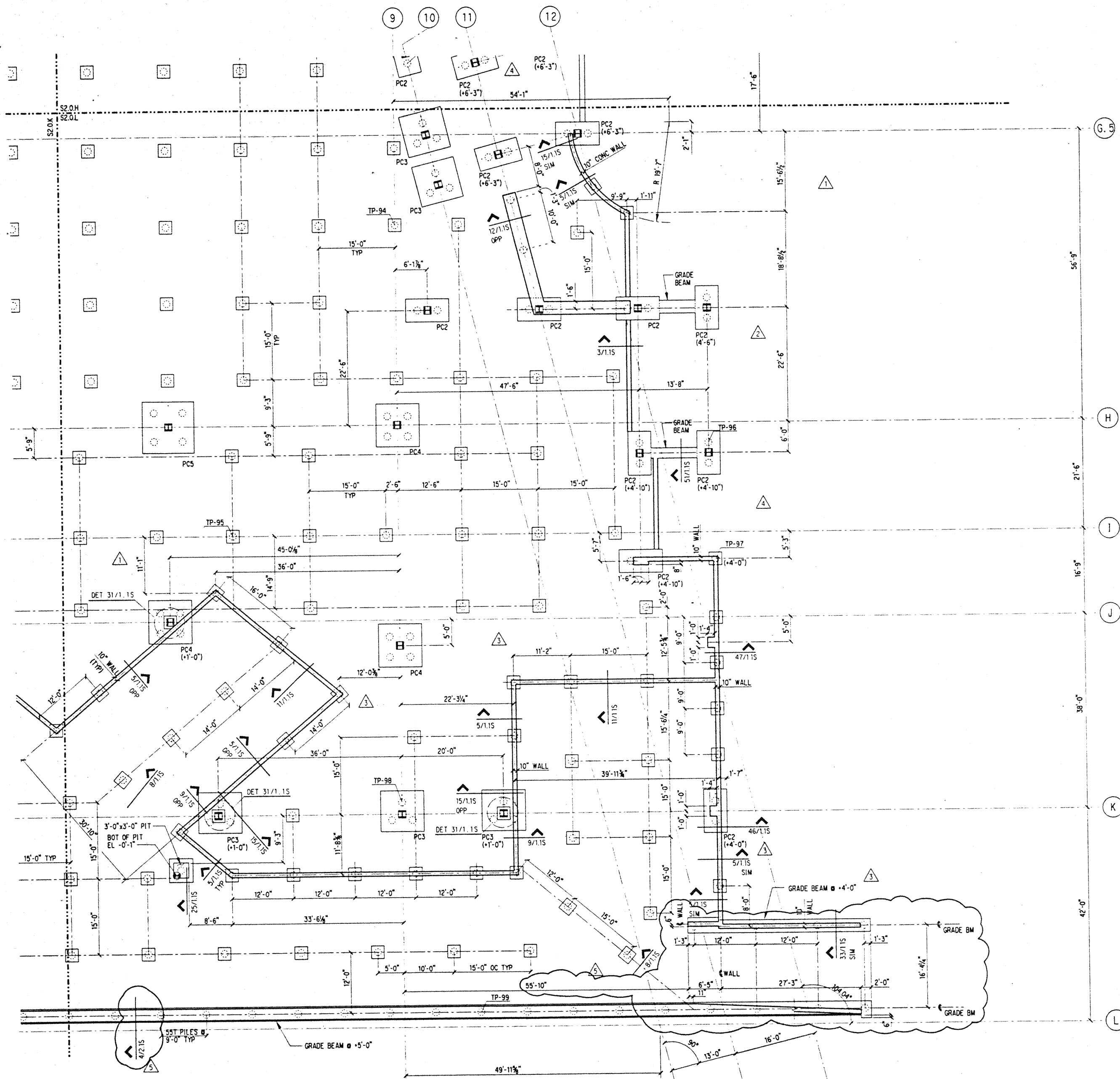
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7	4-17-95		ISSUED FOR FOUNDATION PERMIT

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FOUNDATION PLAN SECTOR L

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Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

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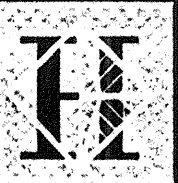


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1. REFERENCE NOTES ON S2.0.A

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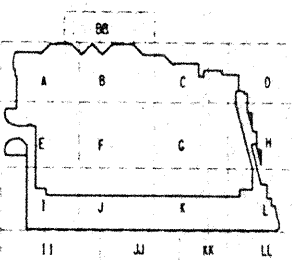
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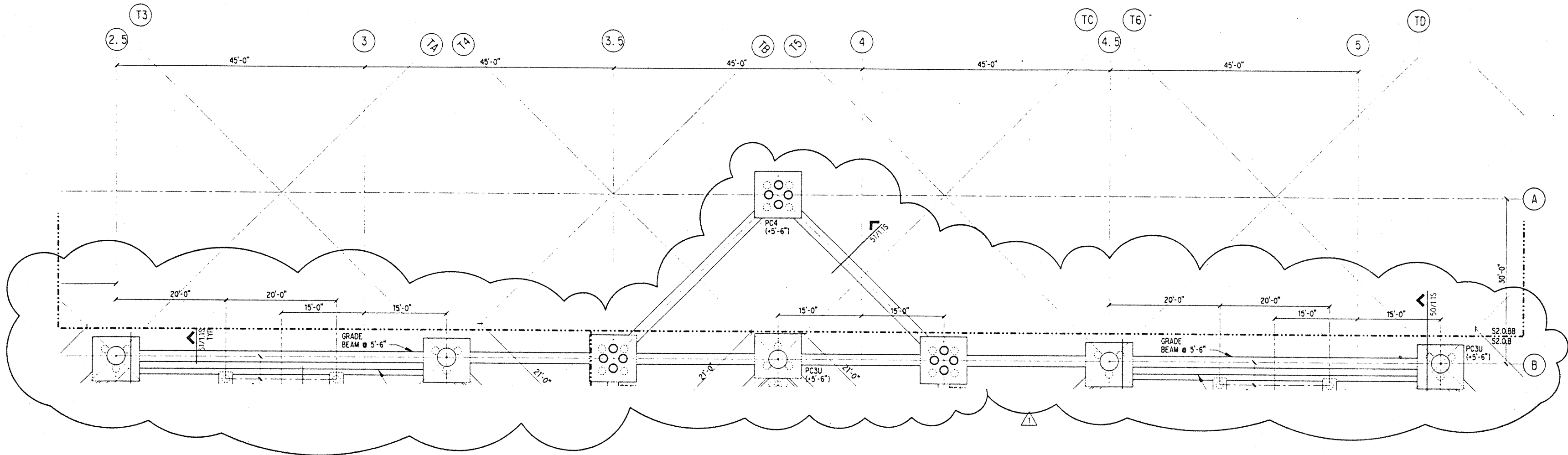
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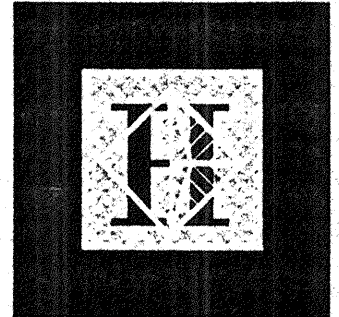
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Date	4 / 12 / 95
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Sheet No.	

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1. REFERENCE NOTES ON S2.0.A



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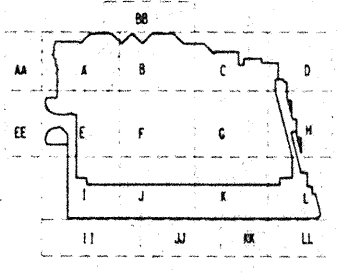
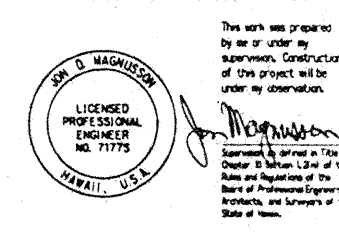
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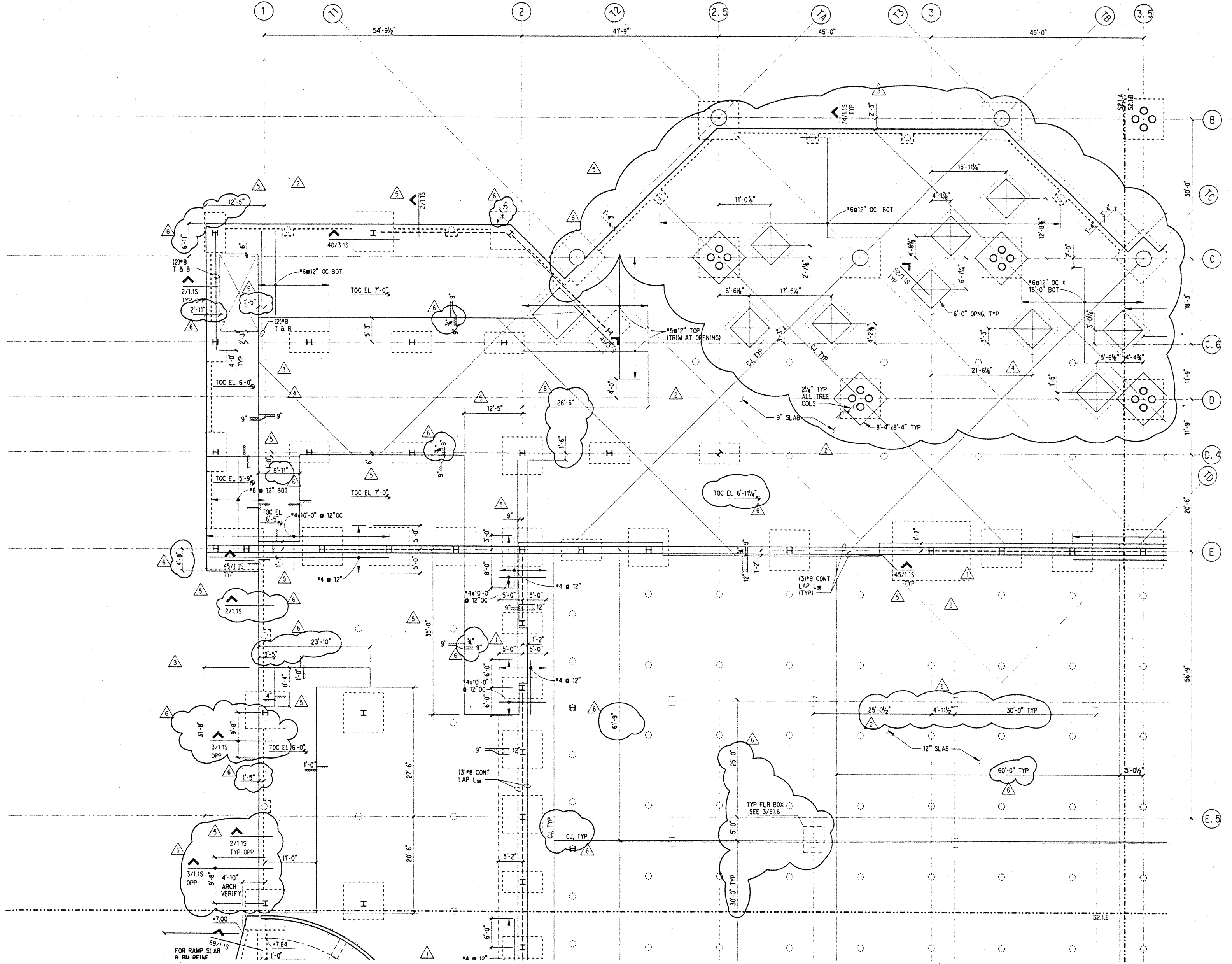
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7	5-8-95		ADDENDUM #3
8	4-17-95		ISSUED FOR FOUNDATION PERMIT

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EXHIBIT HALL SLAB PLAN SECTOR A

Designed **BJM**
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 Date **4 / 12 / 95**
 Job No. **93036.02.9**
 Sheet No.

S2.1.A

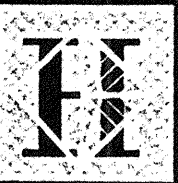


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- REFERENCE DRAWINGS:
 - S0. SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
 - S1. TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
 - S1.20 CONCRETE SCHEDULES
 - S1.21 S1.22 STEEL SCHEDULES
 - S2. PLANS
 - S3. TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
 - S4. BRACED FRAME ELEVATIONS
 - S5. TRUSS ELEVATIONS
 - S6. BRACED FRAME & TRUSS DETAILS
 - DETAIL BOOK STRUCTURAL DETAILS
- REFERENCE FLOOR ELEVATION = 7'-0"
- TOP OF CONCRETE SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
- REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, BOLLARDS, EDGE ANGLES, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
- REBAR SEQUENCE
 - EAST/WEST BOTTOM BARS
 - NORTH/SOUTH BOTTOM BARS
 - EAST/WEST TOP BARS
 - NORTH/SOUTH TOP BARS
- ALL REINFORCING SHOWN ON PLAN IS IN ADDITION TO THAT SPECIFIED IN NOTES 7 AND 8.
- FOR TYPICAL SLAB REINFORCING WITHIN GRIDS
 - 2-10@E-H SEE 2/S1.6
- SLAB REINFORCING OUTSIDE GRIDS 2-10@E-H SHALL BE AS FOLLOWS.
 - 6#12 EA WAY BOT
 - 4#12 EA WAY TOP
 - 4#BARS CONTINUOUS - LAP LSB (TYP)
- SEE DETAIL 5/S1.6 FOR ADDITIONAL REINFORCEMENT AT LARGE SLAB OPENINGS IN LOBBY.
- SEE DETAILS 6/S1.6-9/S1.6 FOR ADDITIONAL TOP BARS AT PILE CAPS
- SEE DETAIL 11/S1.6 FOR BOTTOM STEEL LAYOUT AT PILE CAP
- SEE DETAILS 10/S1.6 FOR ADDITIONAL REINFORCEMENT AT WALL SUPPORTED SLAB EDGES.

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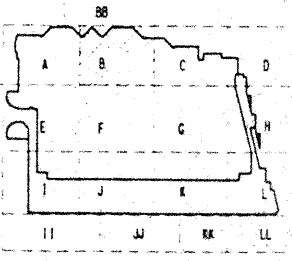
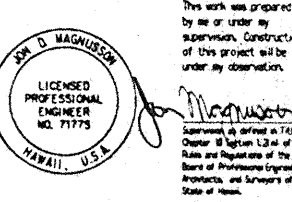
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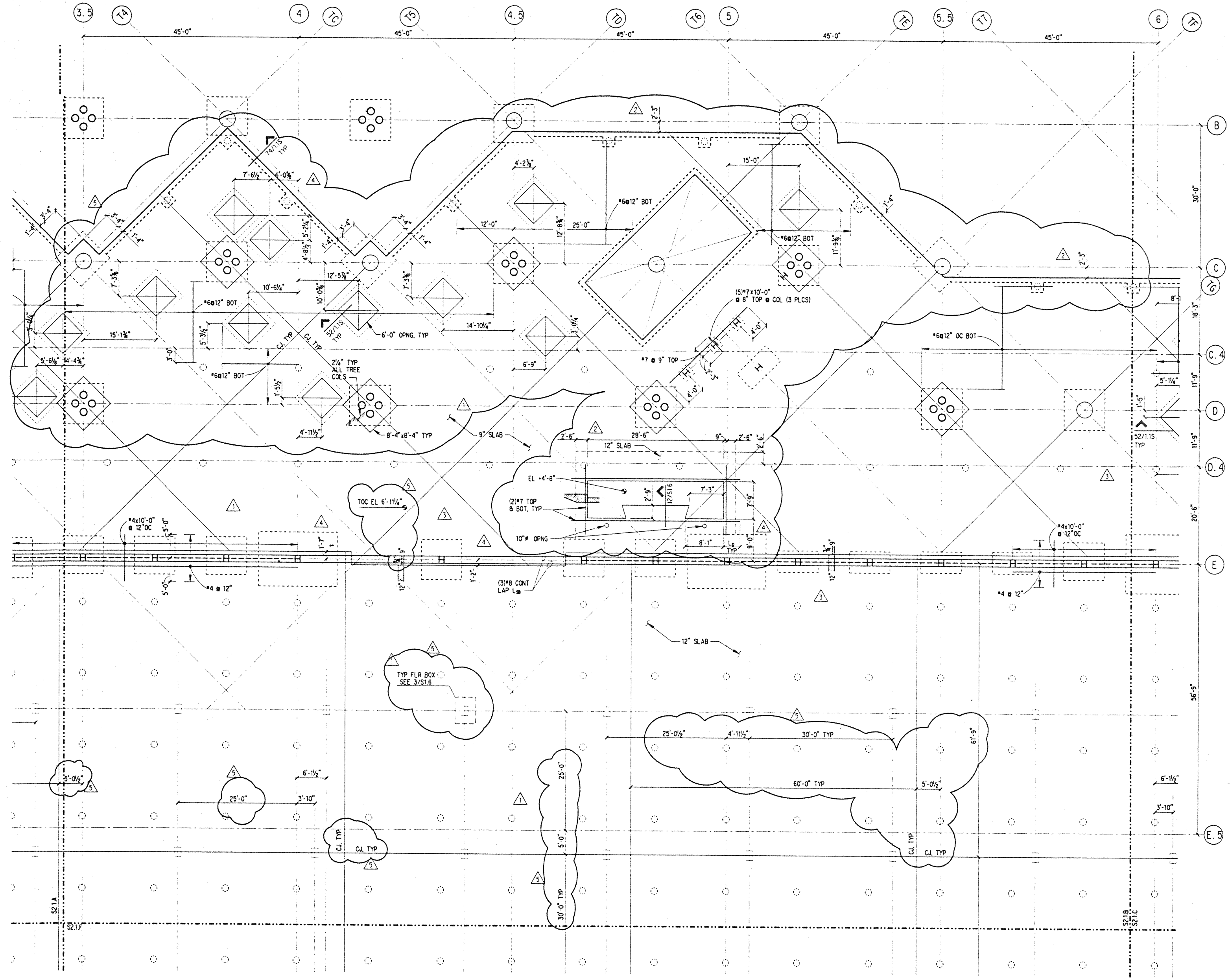
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5	8-4-95		
6	6-19-95		
7	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL SLAB PLAN SECTOR B

Designed	BJM
Drawn	DMS
Checked	JJM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.1.B

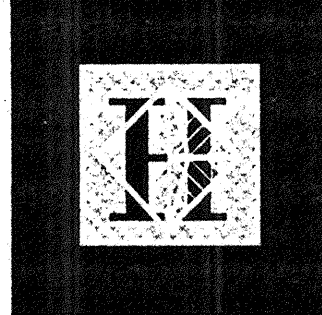


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.1.A

Mon Dec 4 12:13:57 1995

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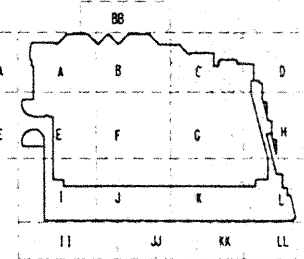
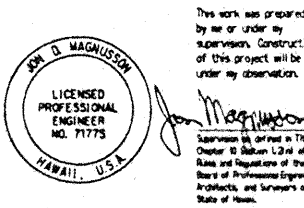
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SKILLING WARD MAGNUSSEN BAKERSHIRE INC.

Consulting Structural and Civil Engineers
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206 462-1200 FAX: 206 292-1201

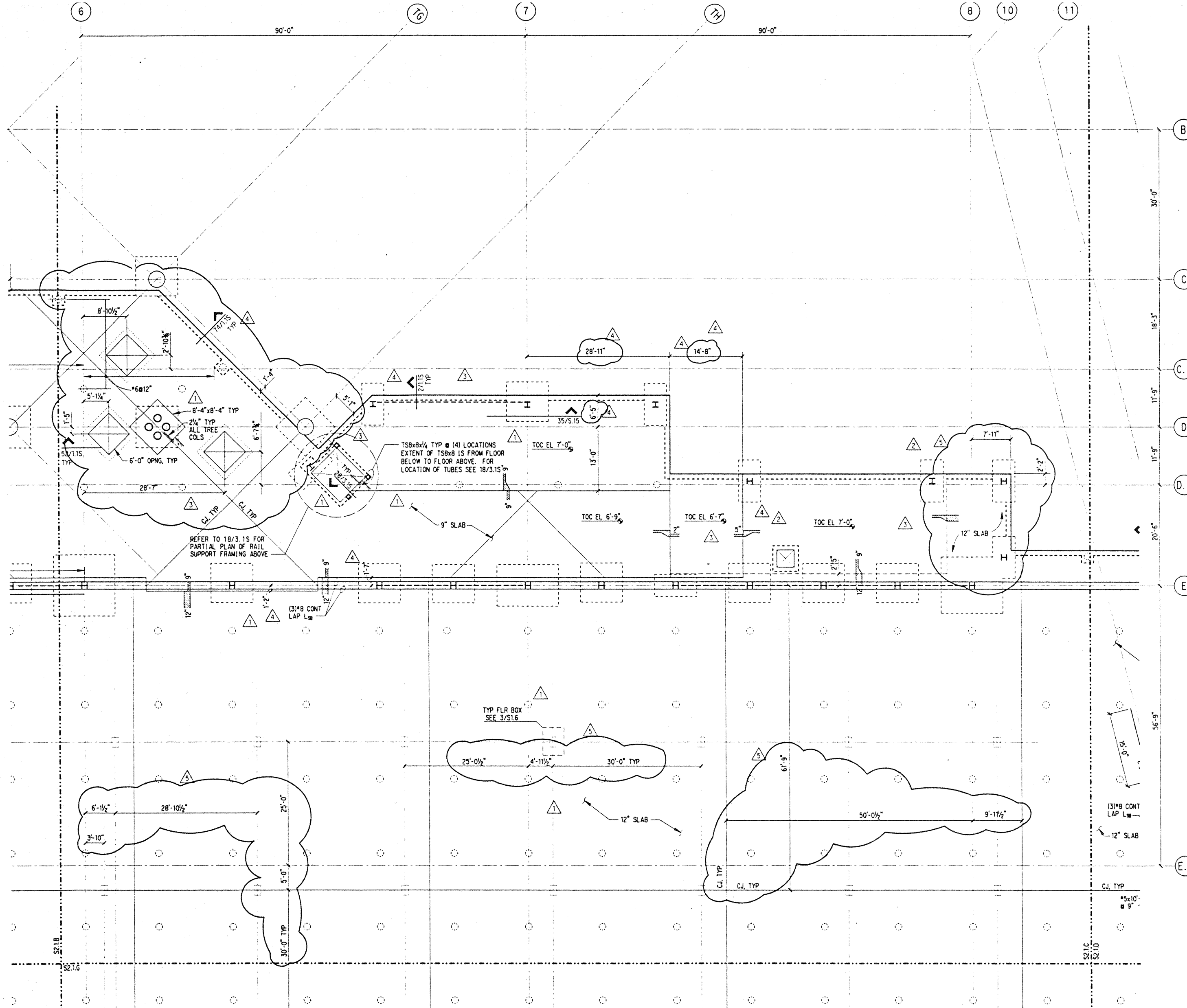


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6	6-19-95		
7	4-17-95		ISSUED FOR FOUNDATION PERMIT

EXHIBIT HALL SLAB PLAN SECTOR C

Designed **BJM**
 Drawn **DMS**
 Checked **JDM**
 Date **4 / 12 / 95**
 Job No. **93036.02.9**
 Sheet No.

S2.1.C

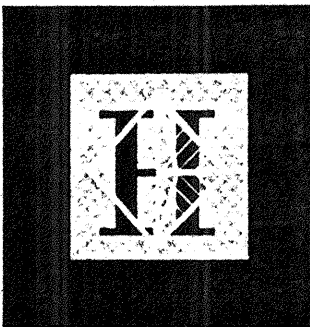


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.1.A

Mon, Dec 4 12:16:04 1995

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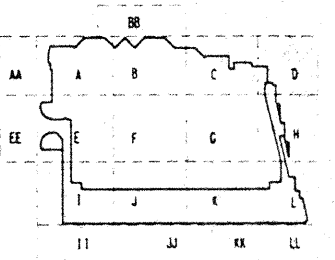
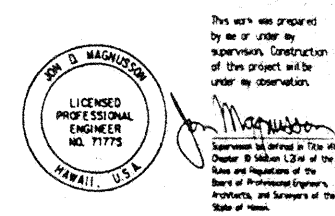
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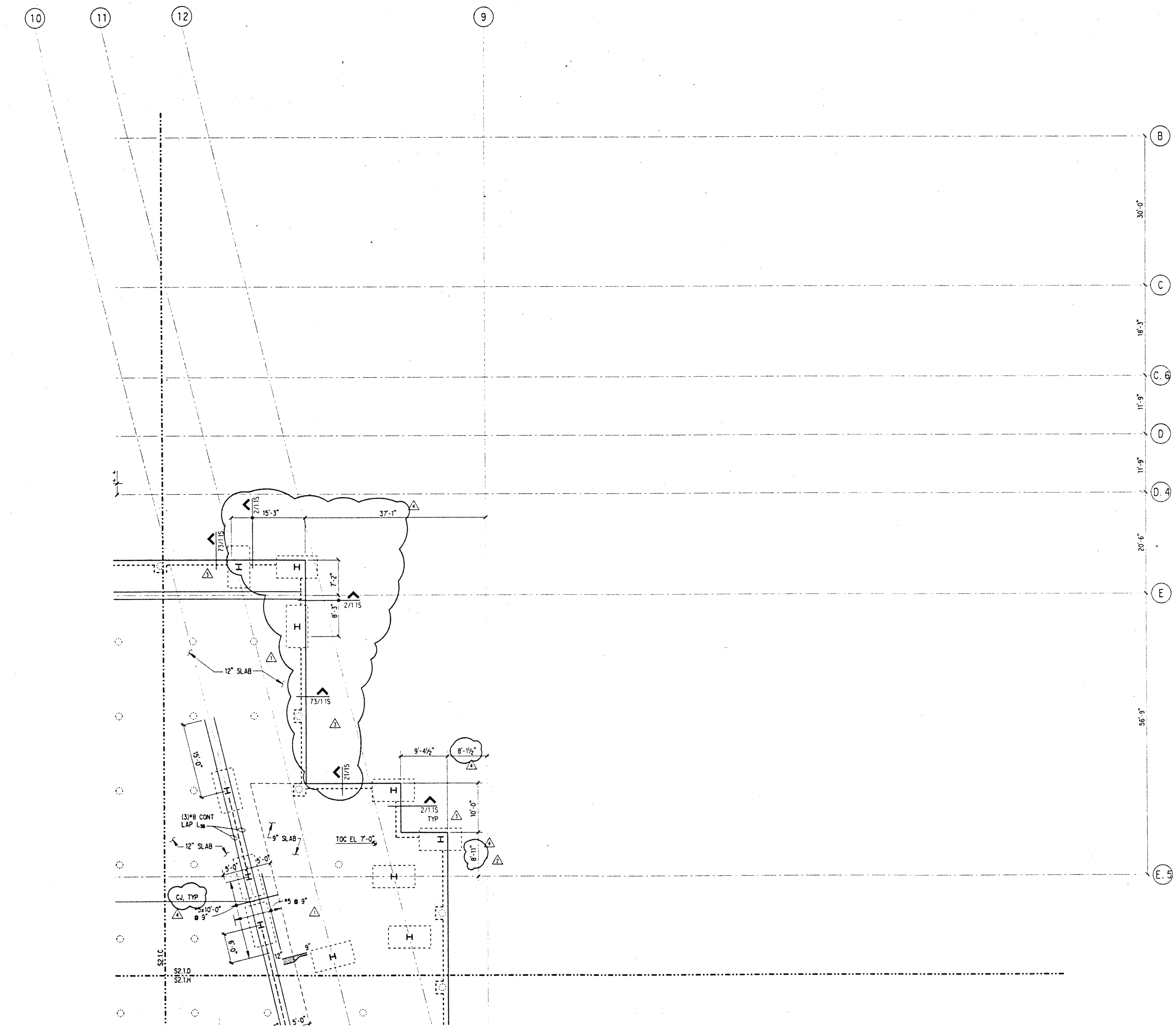
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Sheet Title

**EXHIBIT HALL
SLAB PLAN
SECTOR D**

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.1.A



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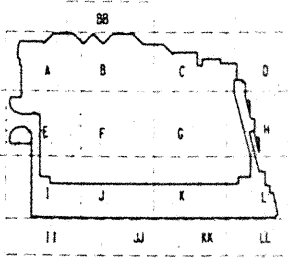
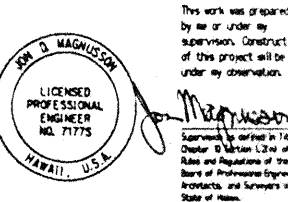
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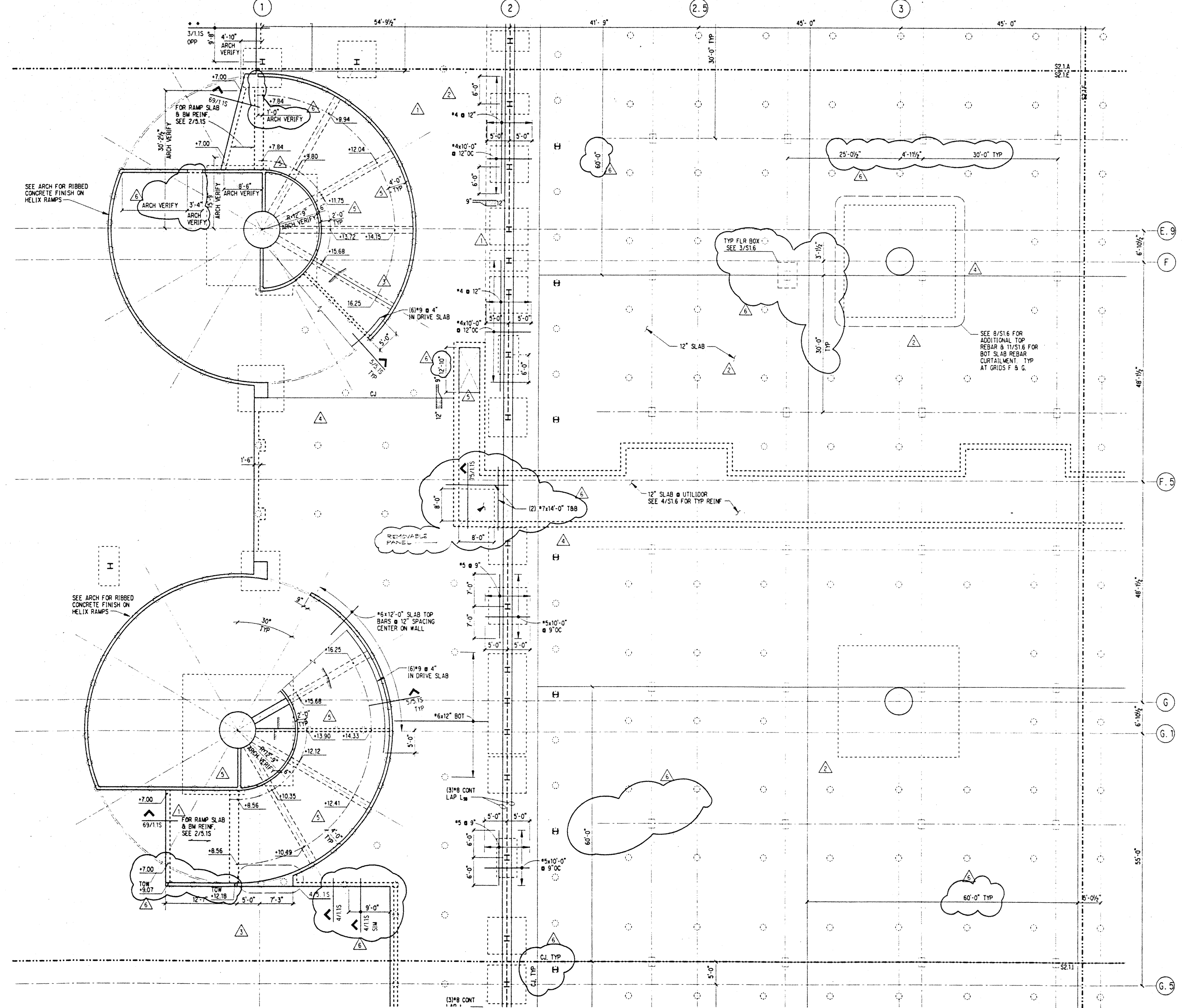


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7	5-8-95		APPENDIX #3
8	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title
**EXHIBIT HALL
SLAB PLAN
SECTOR E**

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036, 02.9
Sheet No.

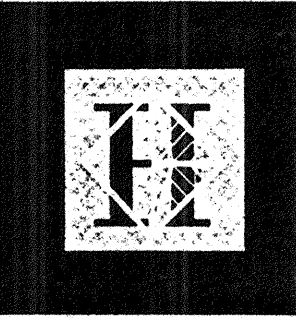
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1. REFERENCE NOTES ON S2.1.A



Sat Dec 2 11:20:39 1995

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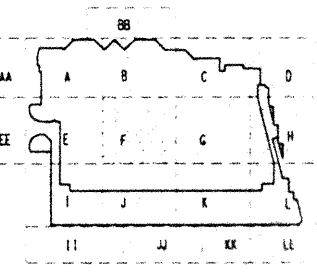
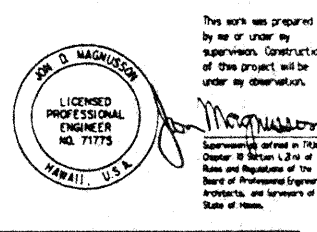
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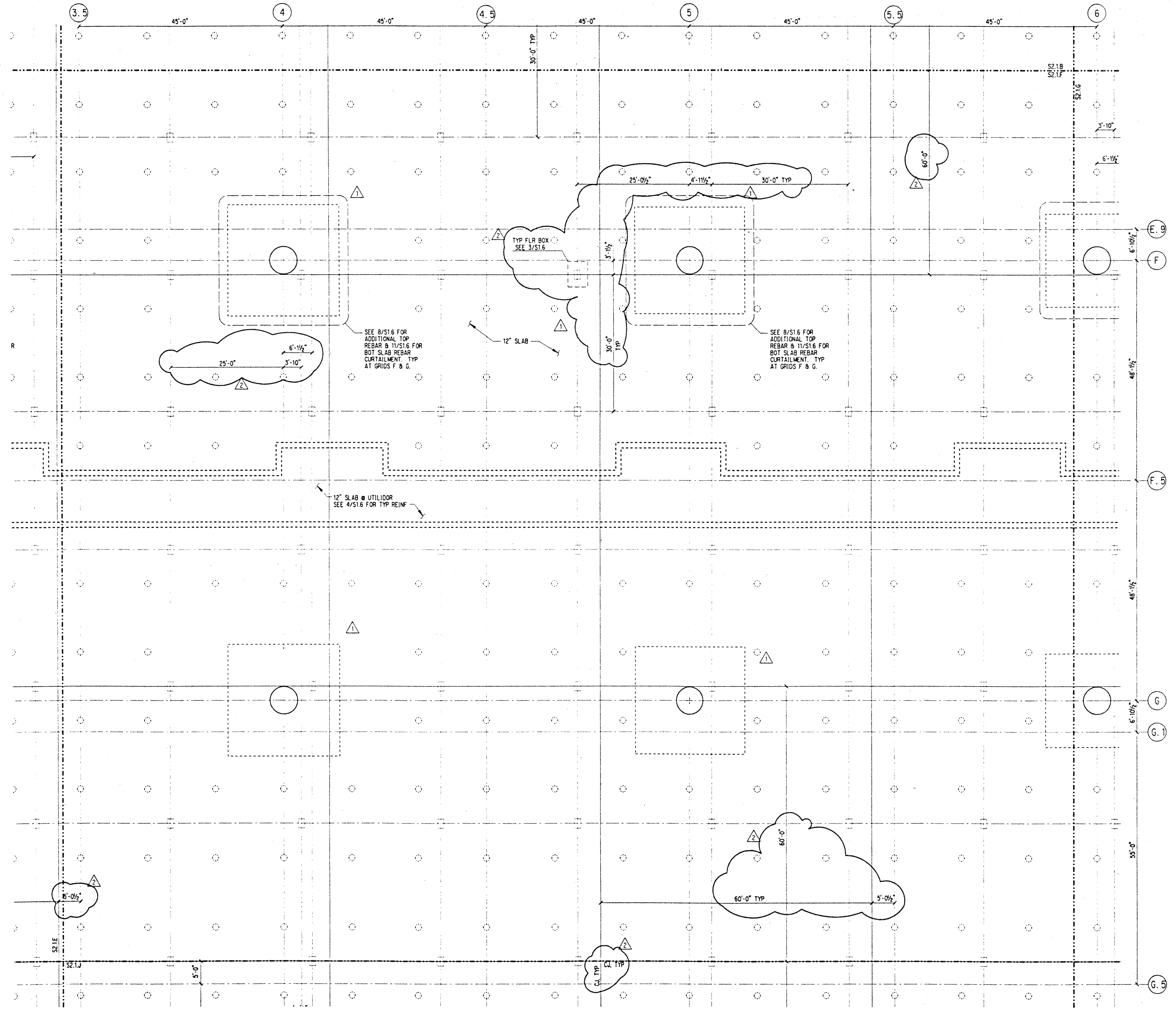
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Sheet Title

EXHIBIT HALL SLAB PLAN SECTOR F

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

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1. REFERENCE NOTES ON S2.1.A



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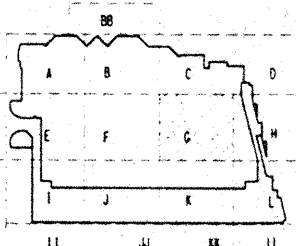
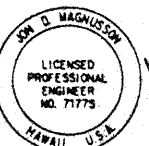
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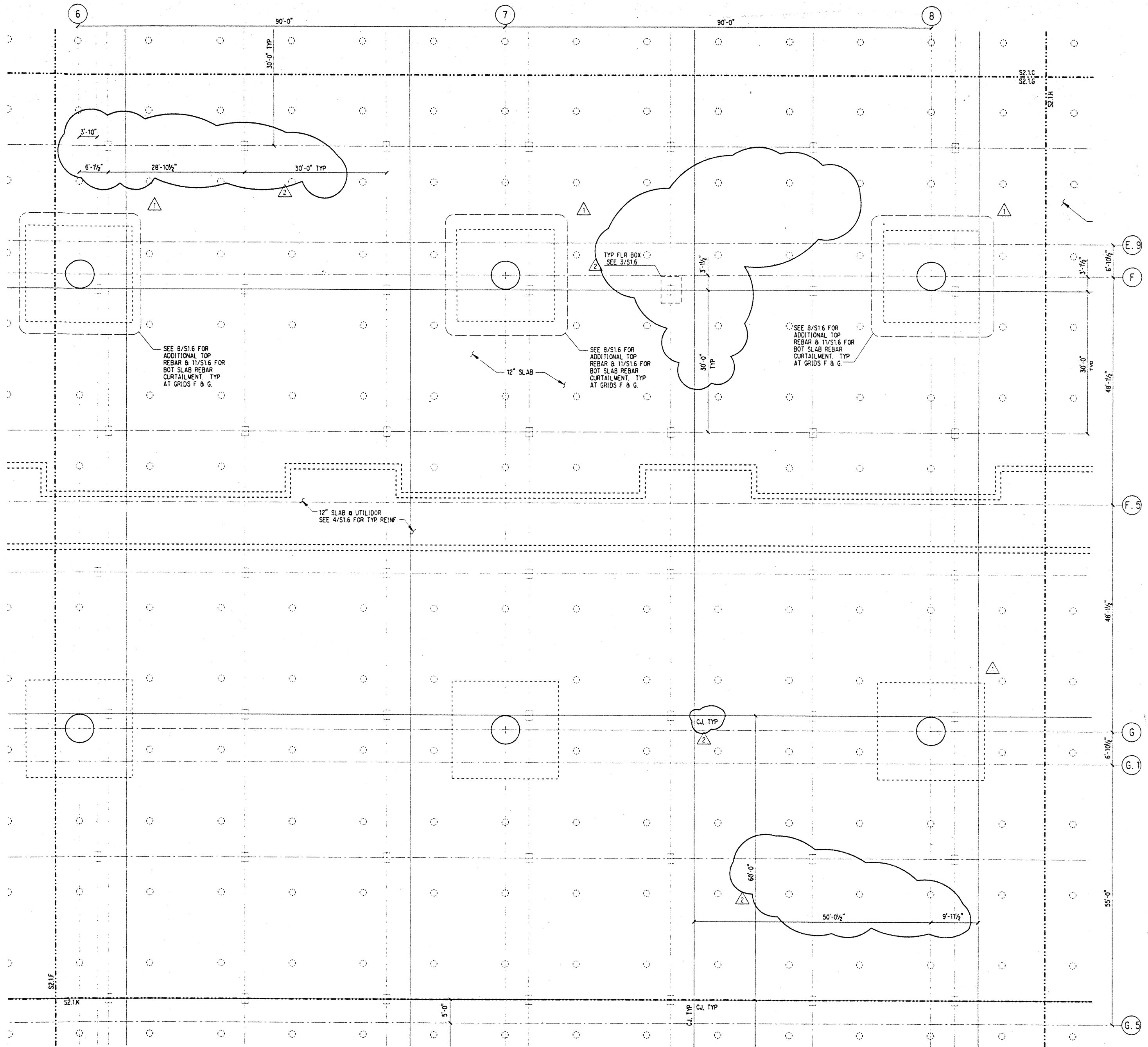
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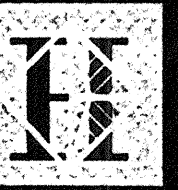
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 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.1.G

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 1. REFERENCE NOTES ON S2.1.A



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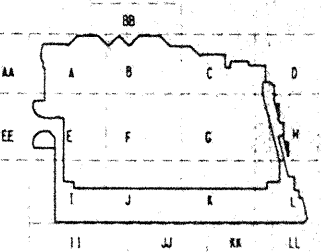
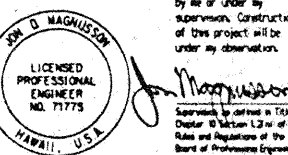
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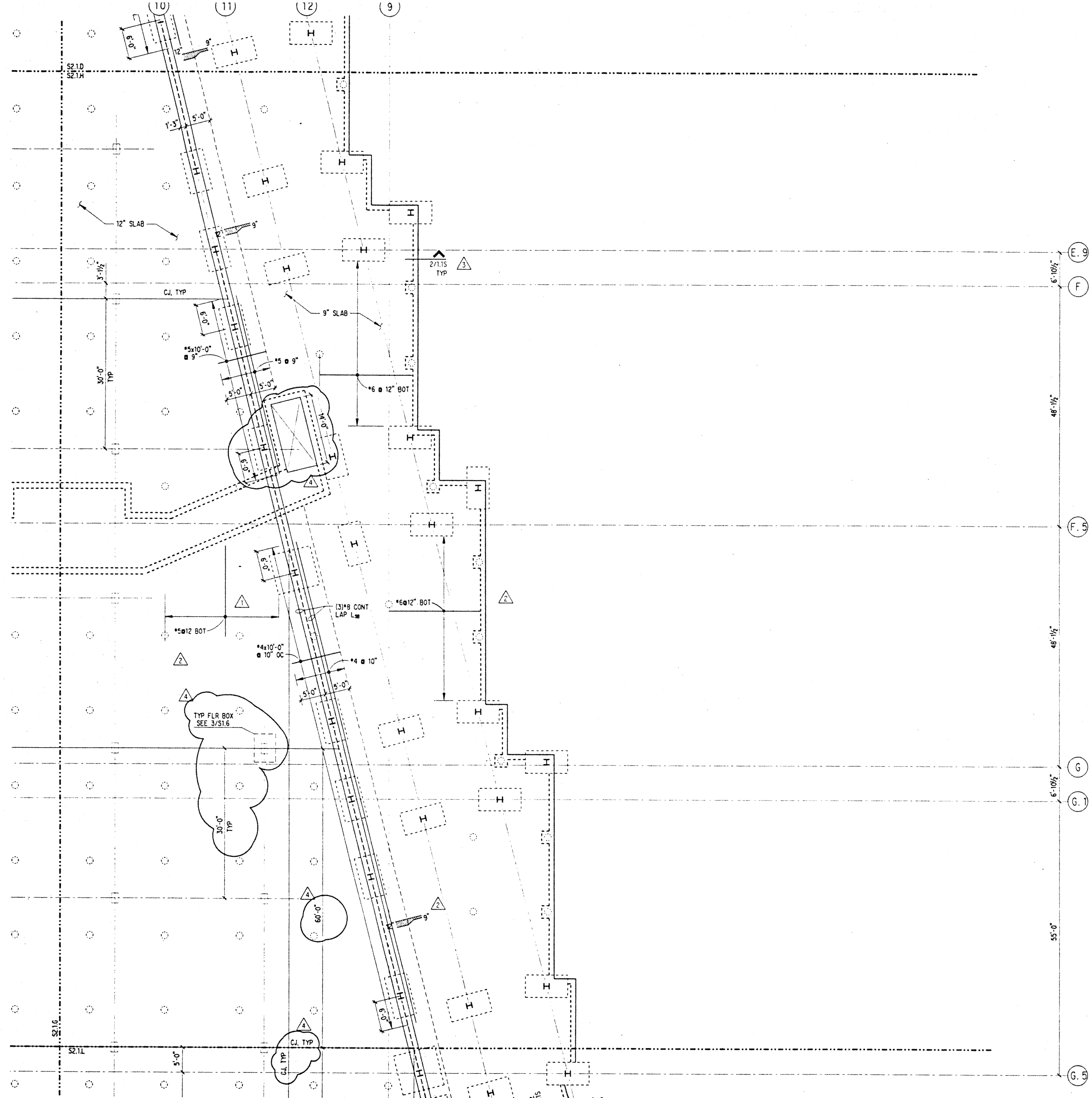
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4-17-95			

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR H

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036, 02, 9
 Sheet No.

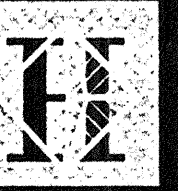
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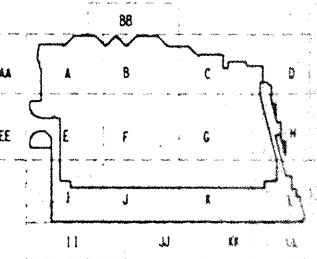
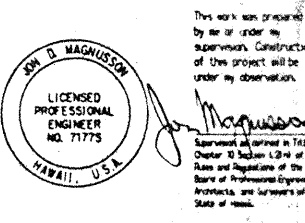
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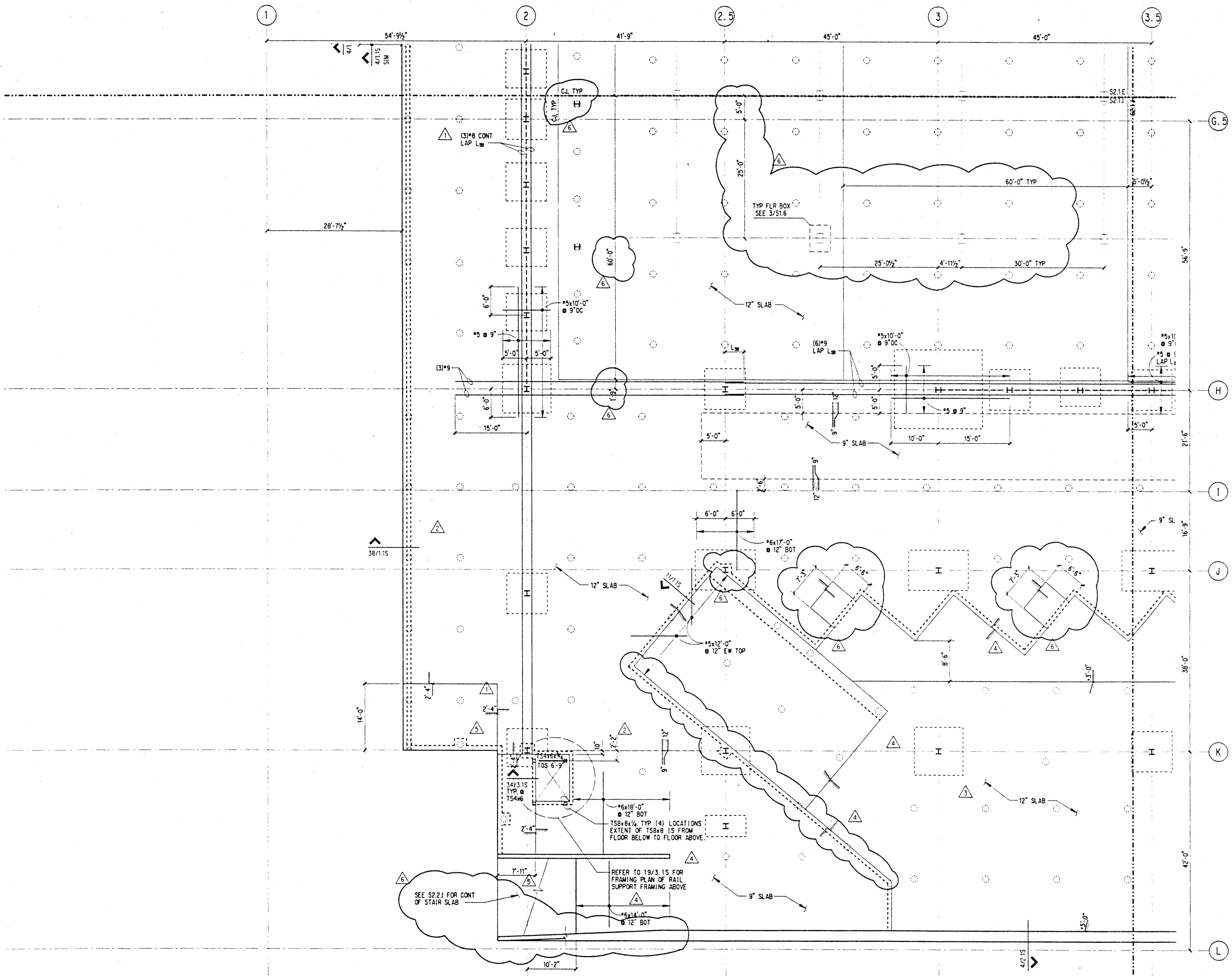
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Sheet Title

EXHIBIT HALL SLAB PLAN SECTOR I

Designed	BJM
Drawn	DMS
Checked	IDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.1.1



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.1.A

Sat Dec 2 11:29:32 1995

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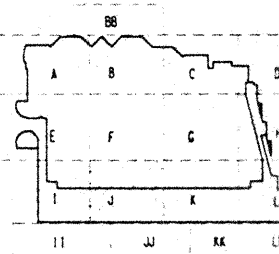
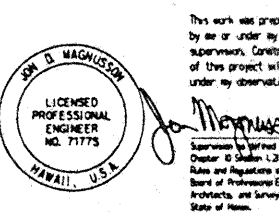
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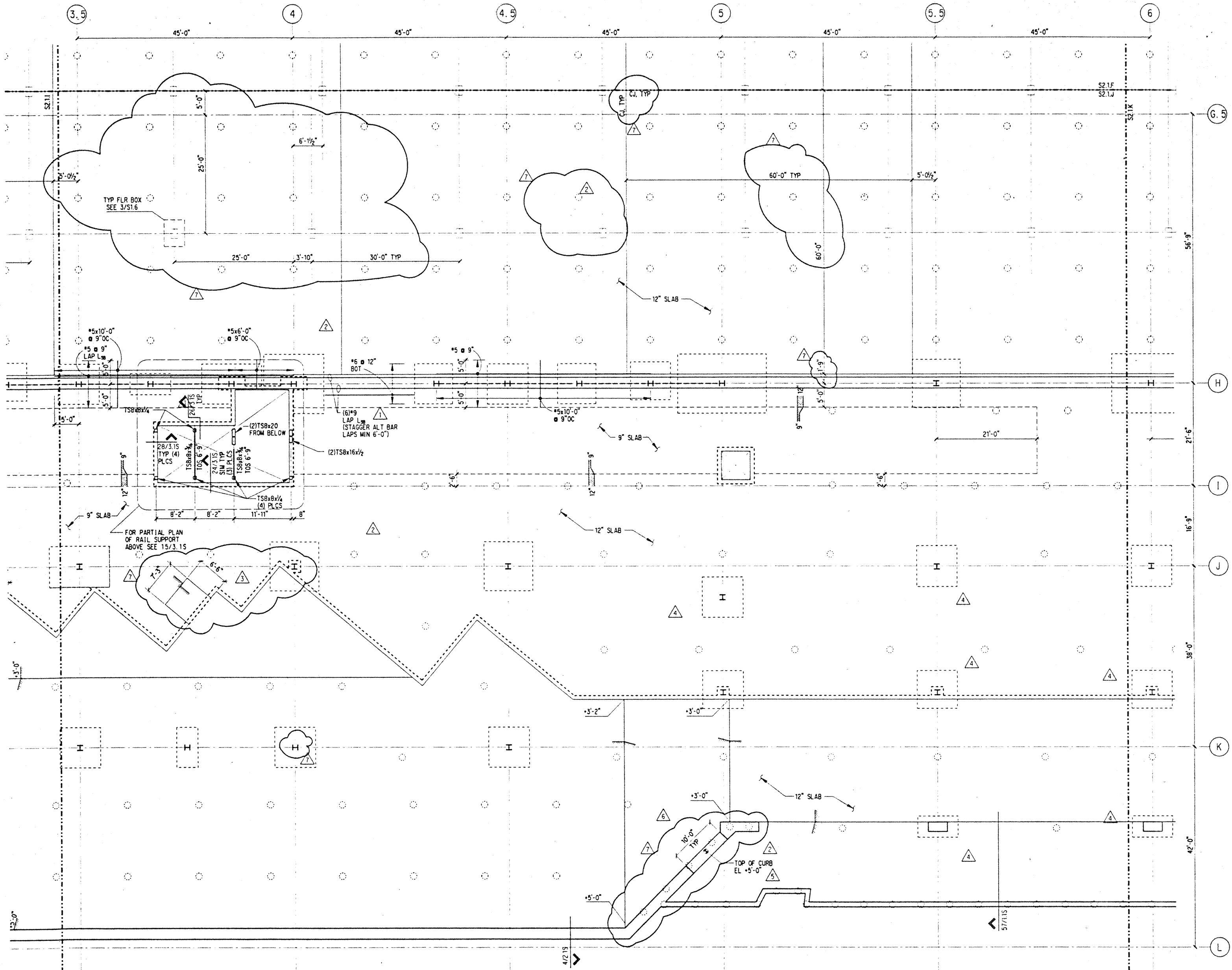
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3	4-17-95		ISSUED FOR FOUNDATION PERMITS

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR J

Designed **BJM**
 Drawn **DMS**
 Checked **JDM**
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.1.J

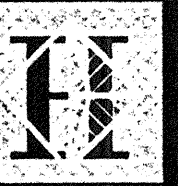


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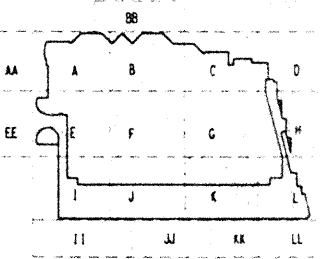
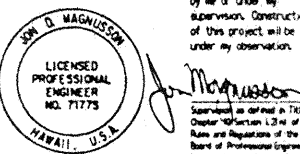
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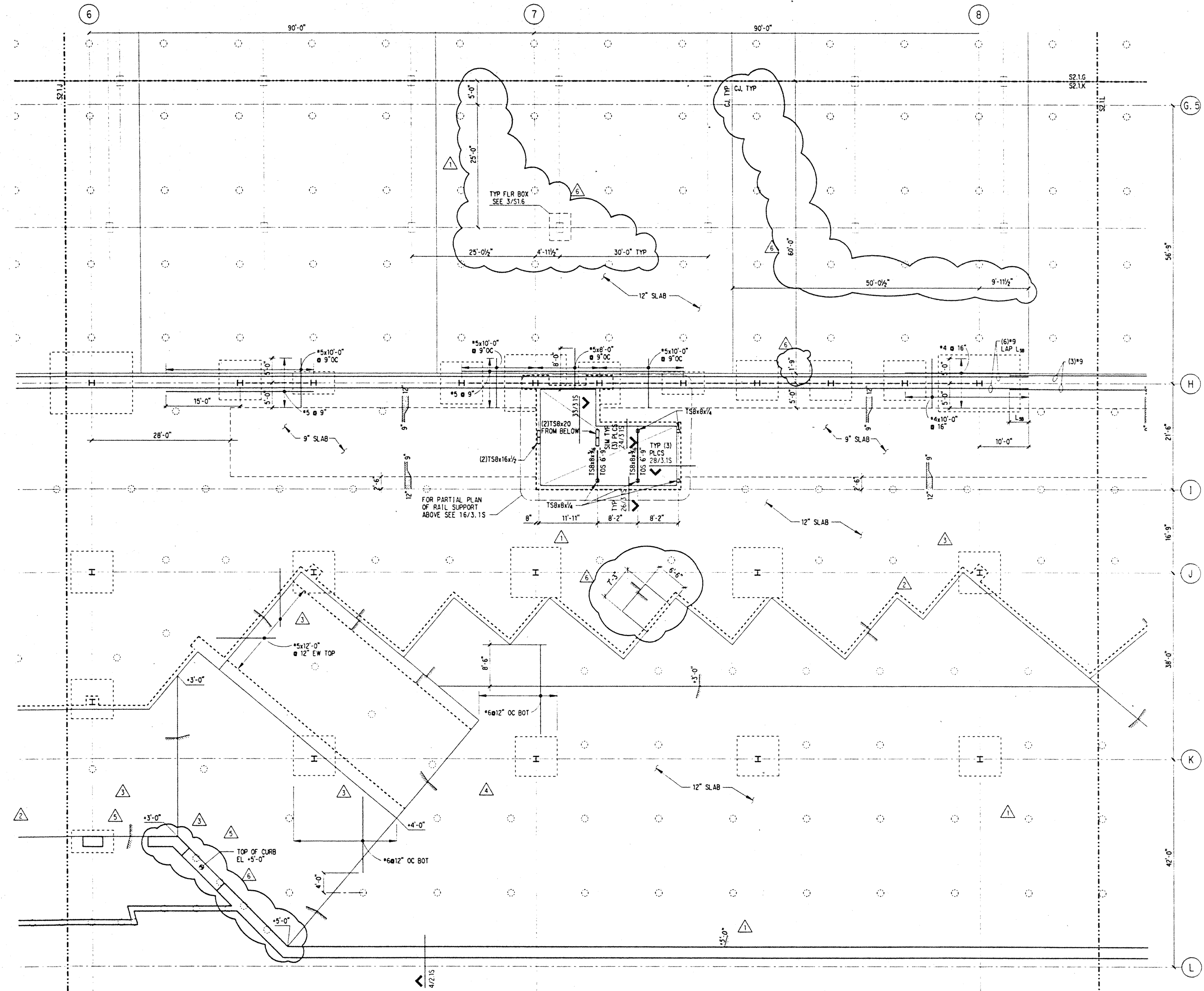
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Designed BJM
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 Date 4 / 12 / 95
 Job No. 93036, 02, 9
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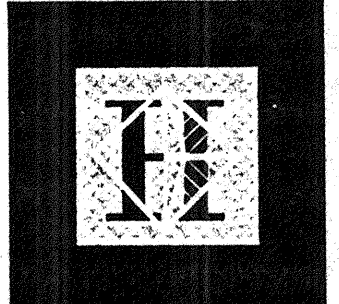
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
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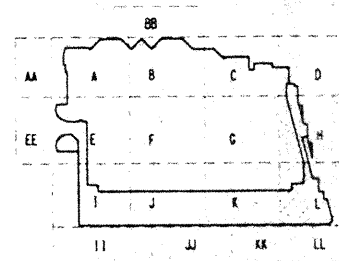
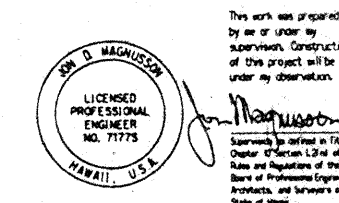
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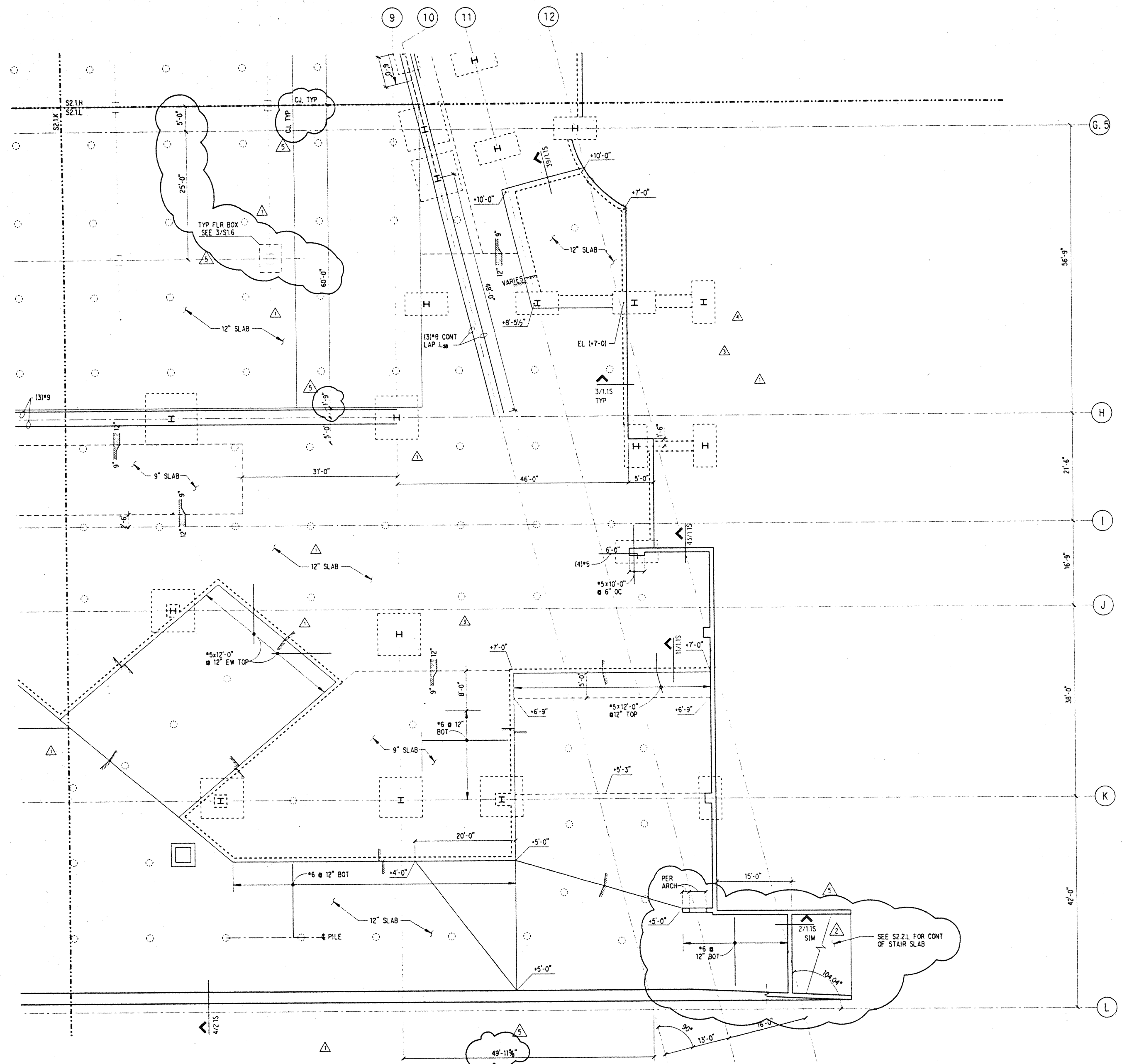
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4-17-95		

EXHIBIT HALL
SLAB PLAN
SECTOR L

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.1.L



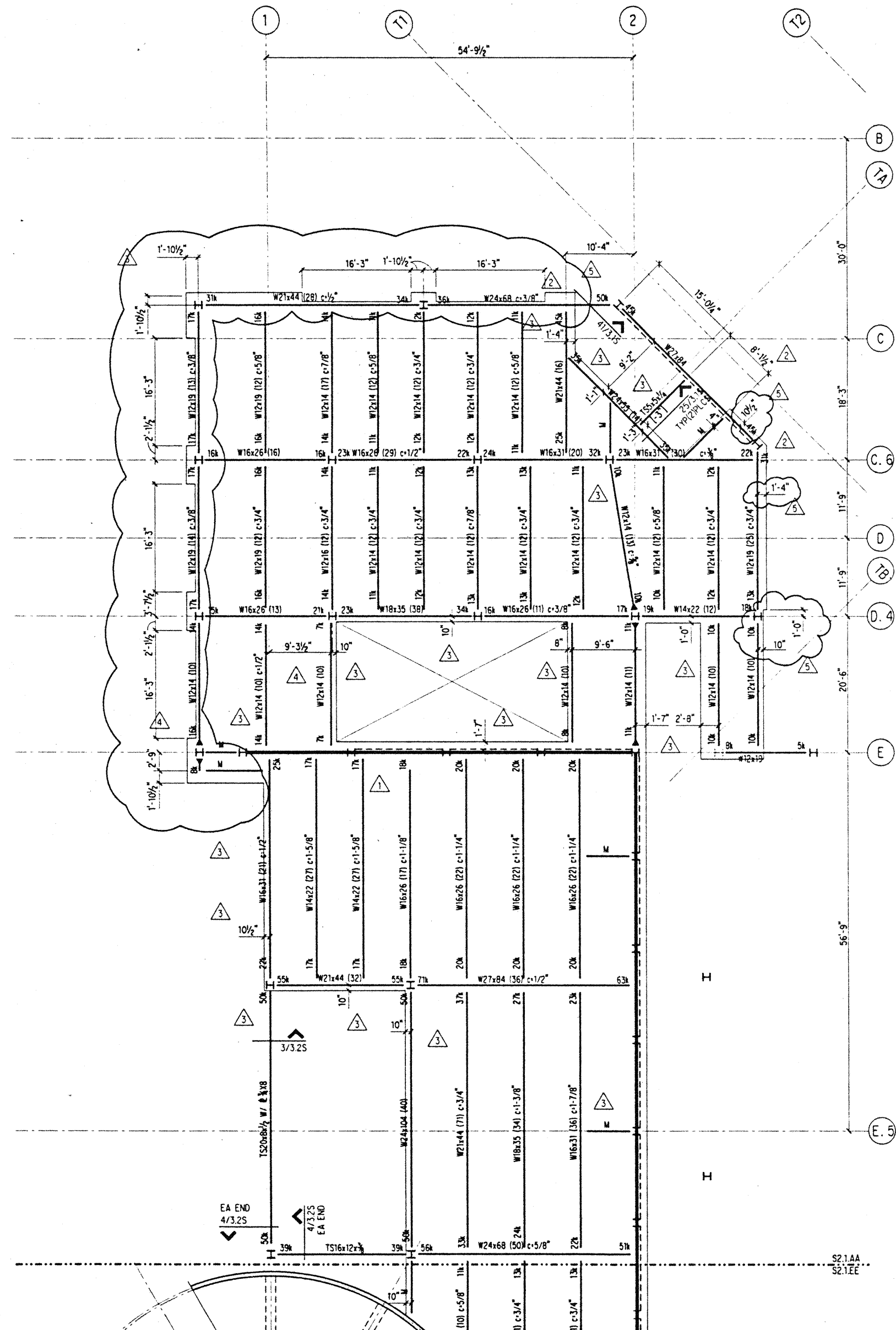
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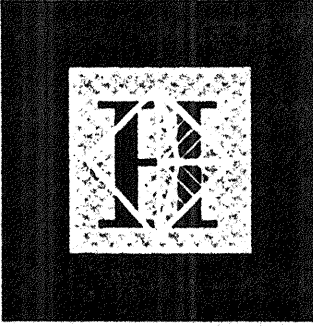
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
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 2. REFERENCE FLOOR ELEVATION = 19'-0"



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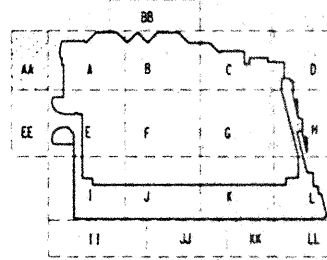
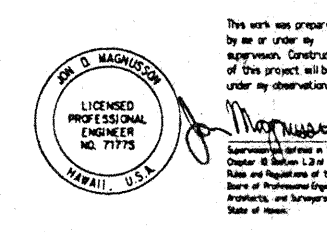
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ADMINISTRATION FRAMING PLAN SECTOR AA

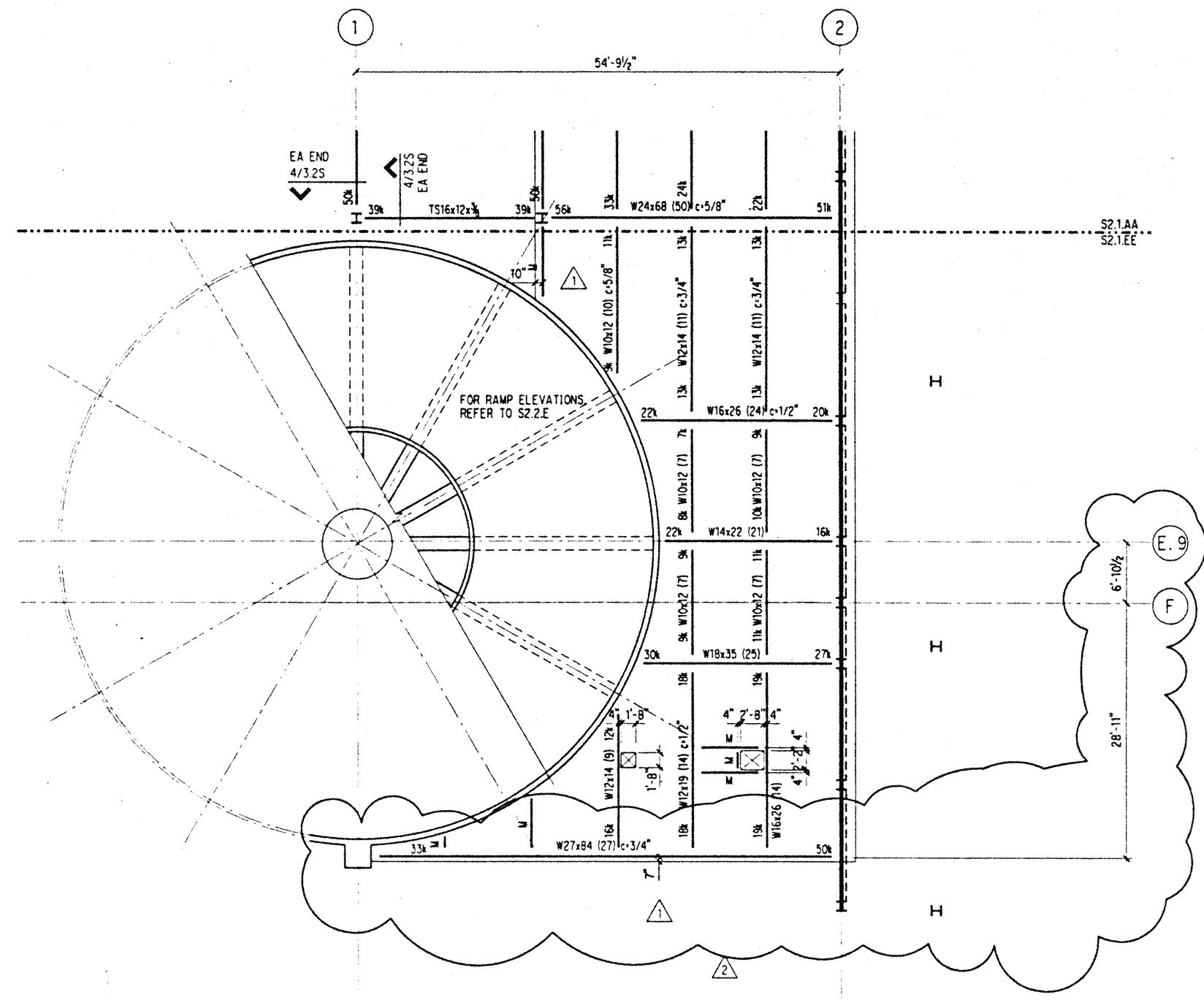
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Sheet No.	

S2.1.AA

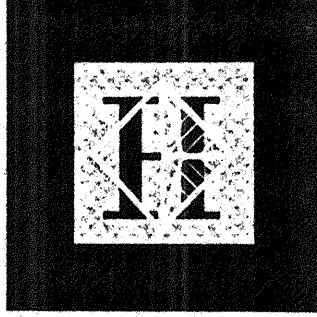
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.2.A
 2. REFERENCE FLOOR ELEVATION = 19'-0"



Hawai'i Convention Center

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 Honolulu, Hawaii

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 Honolulu, Hawaii
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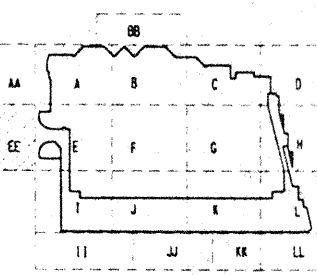
Loschky Marquardt & Nesholm
 Seattle, Washington

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 A Joint Venture
 Honolulu, HI

Consultants
SELLING WOOD MAGNUSSEN BAKERSIDE INC.
 Consulting Structural and Civil Engineers
 1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2999
 (206) 292-1200 FAX: (206) 292-1201

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DR. D. MAGNUSSEN
 LICENSED PROFESSIONAL ENGINEER
 NO. 71775
 HAWAII, U.S.A.



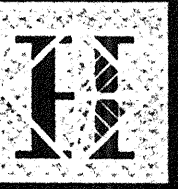
Revisions

No.	Date	By	Description
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11-7-95			BUILDING PERMIT SET
9-15-95			BUILDING PERMIT SET
8-4-95			

ADMINISTRATION FRAMING PLAN SECTOR EE

Designed **BJM**
 Drawn **DMS**
 Checked **JDM**
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.1.EE



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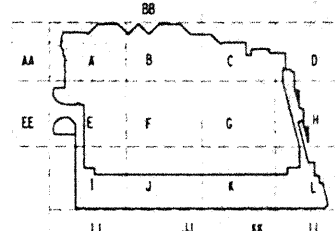
SKELLING WARD MAGNUSSON BAKERSHIRE INC.

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206 292-1200 FAX: 206 292-1201



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Professional Engineer
No. 7772
HAWAII, U.S.A.



Revisions

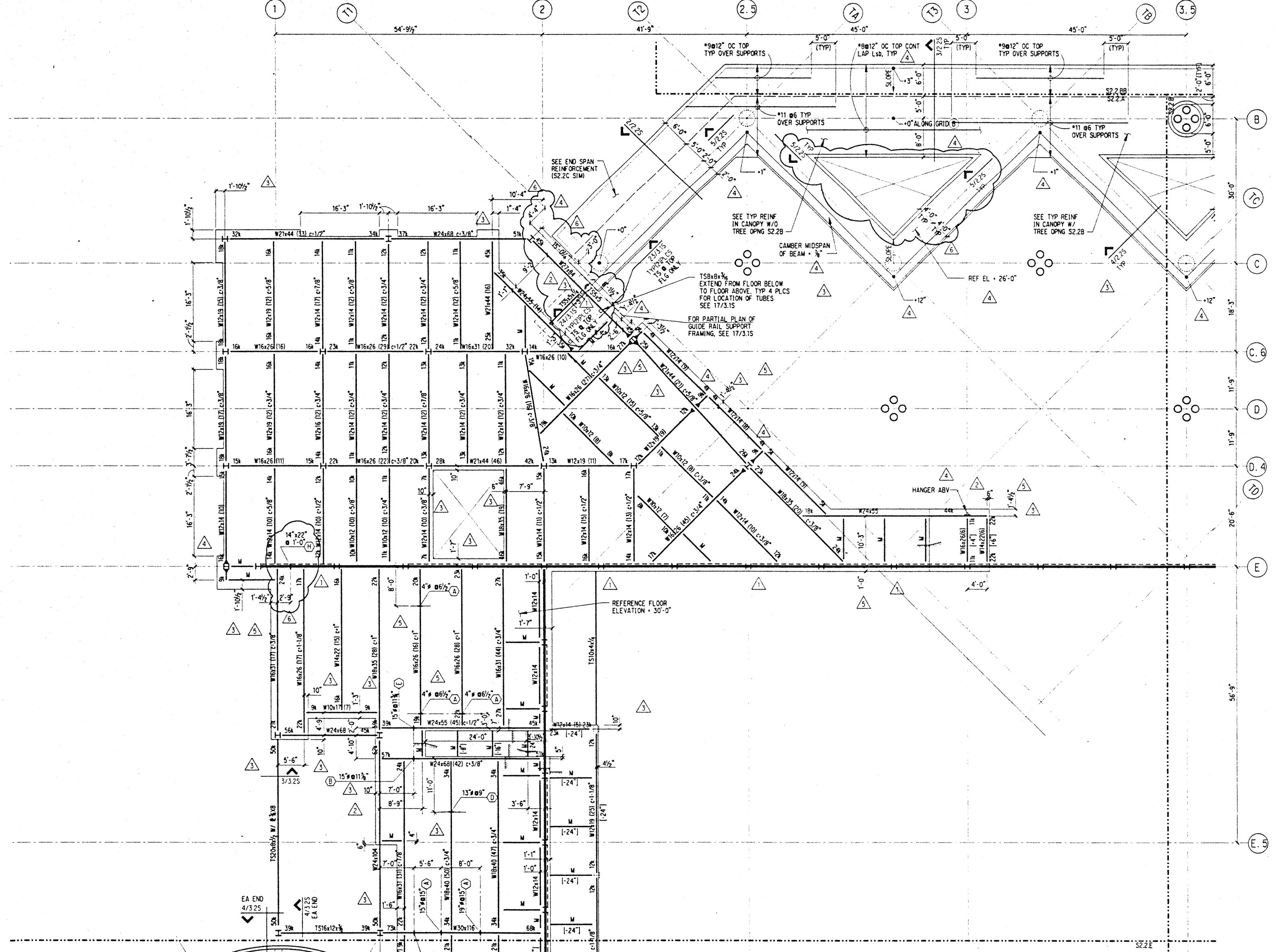
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5-8-95	ADDENDUM #3
No. Date By Description	

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR A

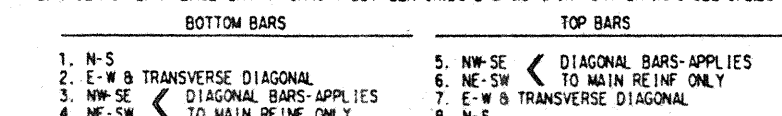
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Drawn **DMS**
Checked **JCM**
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.2.A

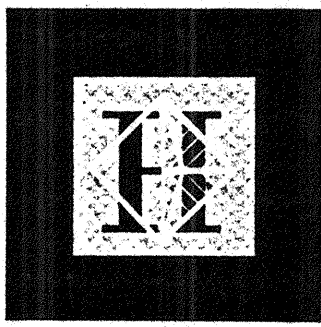


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- REFERENCE DRAWINGS:
S0. SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
S1. TYPICAL FOUNDATION, CONCRETE, STEEL AND CMJ DETAILS
S1.20 CONCRETE SCHEDULES
S1.21 S1.22 STEEL SCHEDULES
S2. PLANS
S3. TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
S4. BRACED FRAME ELEVATIONS
S5. TRUSS ELEVATIONS
S6. BRACED FRAME & TRUSS DETAILS
DETAIL BOOK STRUCTURAL DETAILS
- REFERENCE FLOOR ELEVATION = 25'-6"
- TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
- REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION, UNO.
- WHERE NOTED THUS (+/- 8"), TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
- STRUCTURAL SLAB IS 2-1/2" OF HARDCORE CONCRETE ON 3" STEEL DECK, UNO.
- REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
- "M" DENOTES W10X12 BEAM, FY-50 KSI, MIN. STUDS.
- INDICATES BEAM PENETRATION, SEE S1.10
- INDICATES HAUNCHED BEAM, SEE S1.10
- REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMJ WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
- ORDER OF BAR PLACEMENT AT CANOPY BETWEEN GRIDS 2 & 6.5: (FOR FURTHER INFO SEE 5/2.25)



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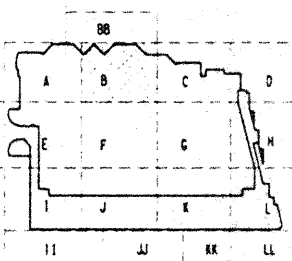
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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 2000 Seattle WA 98101-2699
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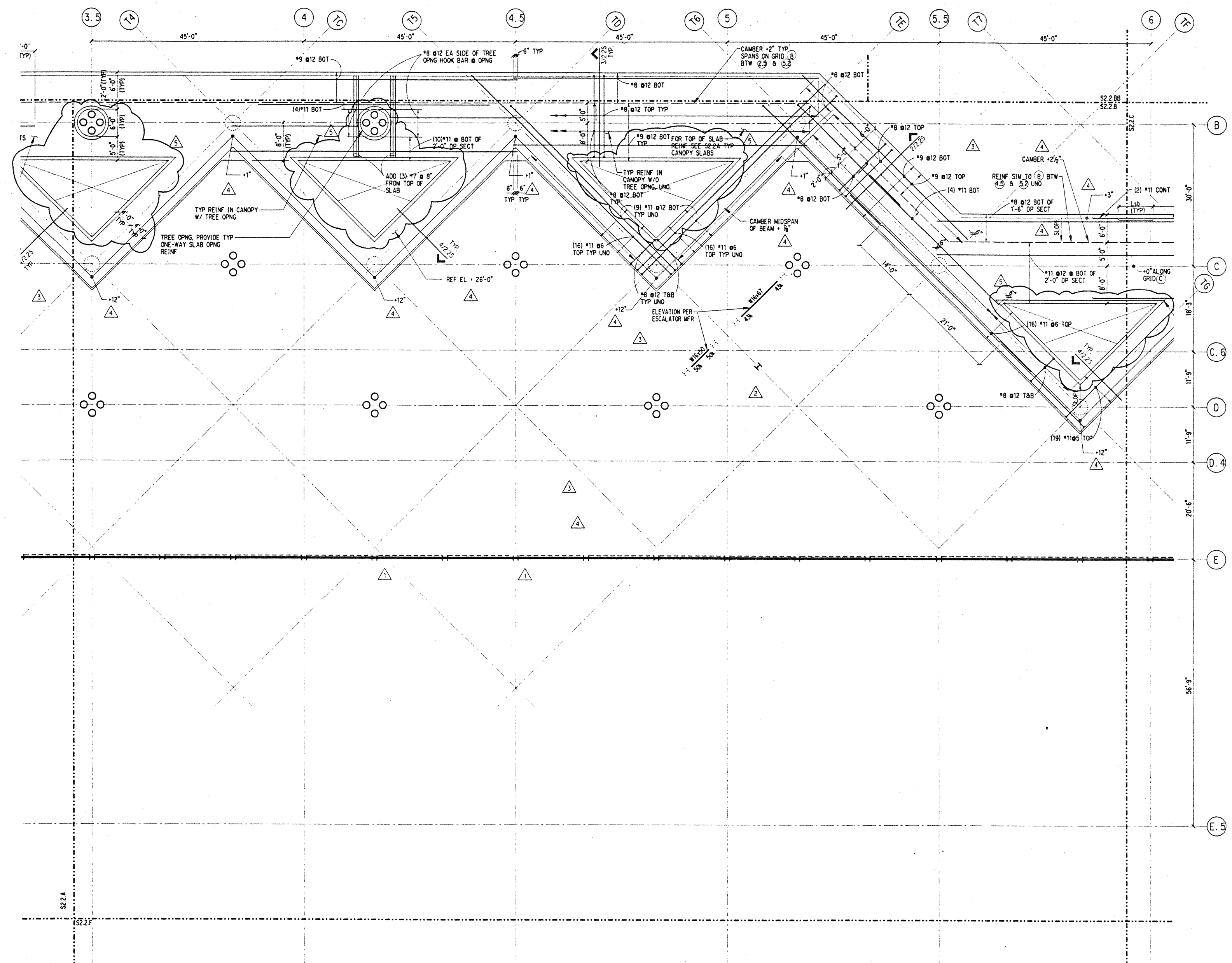
S. D. Magnusson
LICENSED PROFESSIONAL ENGINEER
NO. 71773
HAWAII, U.S.A.



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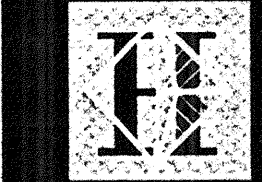
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Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036, Q2.9
Sheet No.	



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.2.A

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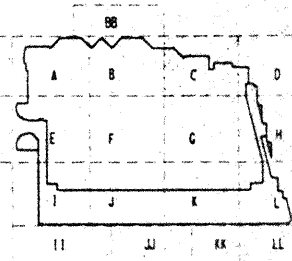
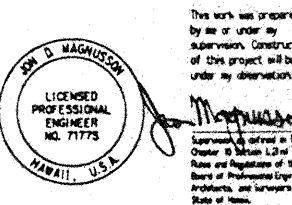
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1301 Fifth Avenue, Suite 1200 Seattle WA 98101-7898

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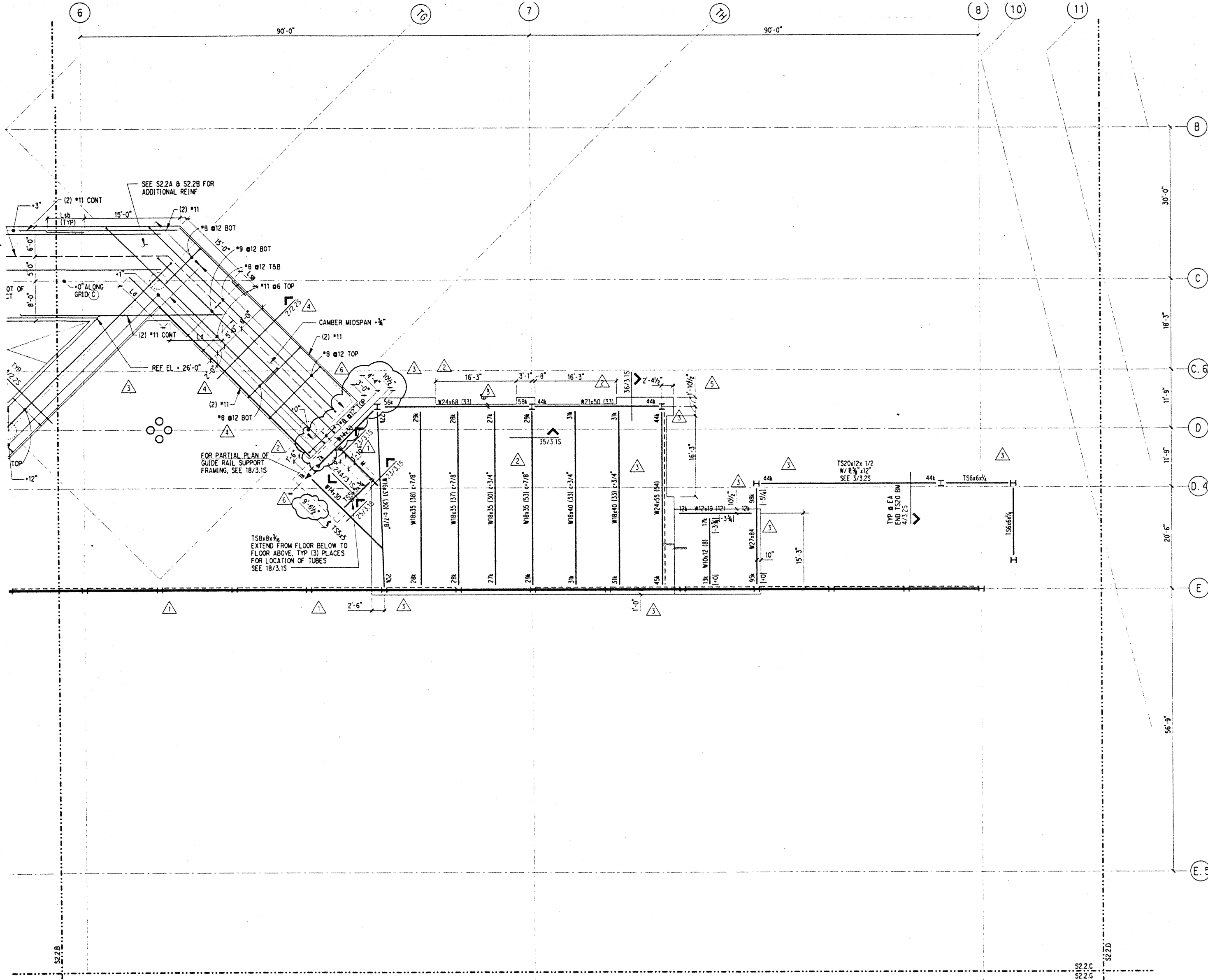
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△	6-19-95		
△	5-8-95		ADDENDUM #3

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR C

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.2.C

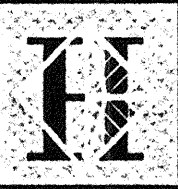


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.2.A
 2. REFERENCE FLOOR ELEVATION + 26'-6"

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Seattle, Washington

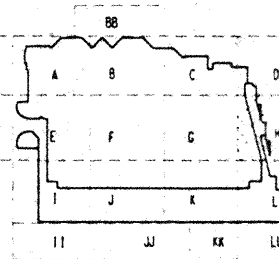
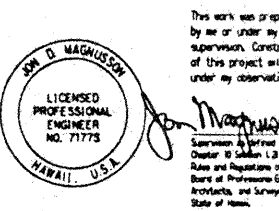
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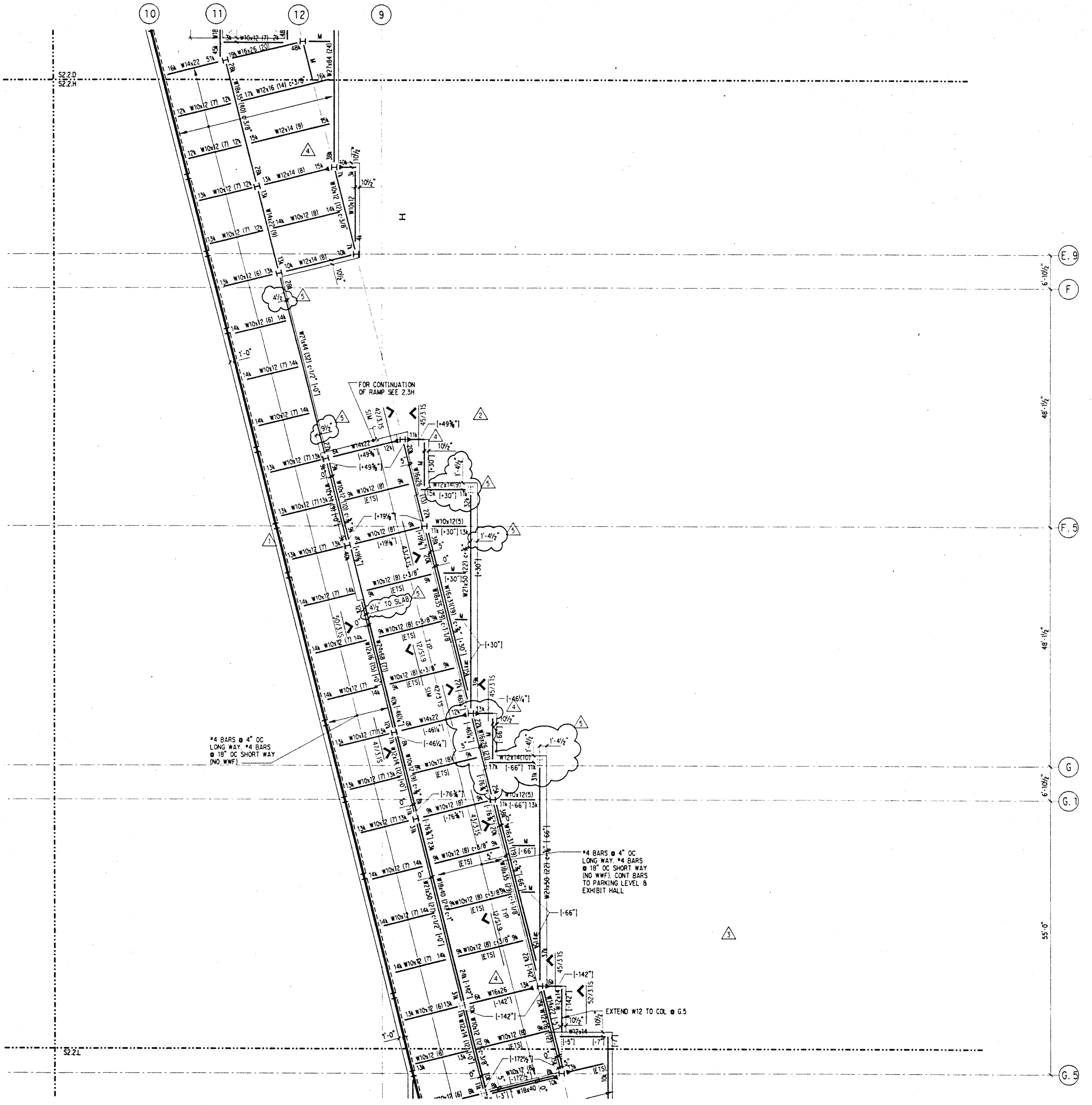
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8-4-95			
6-19-95			
5-8-95			ADDENDUM #3

INTERMEDIATE LEVEL FRAMING PLAN SECTOR H

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036, 02, 9
 Sheet No.

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

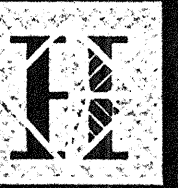
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Seattle, Washington

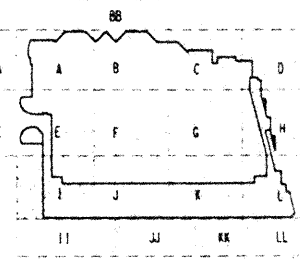
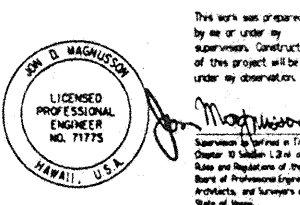
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SKILLING WARD MANNING BARRETT INC.

Consulting Structural and Civil Engineers

1201 Fifth Avenue, Suite 3200 Seattle WA 98101-2999
(206) 292-1200 FAX: (206) 292-1201



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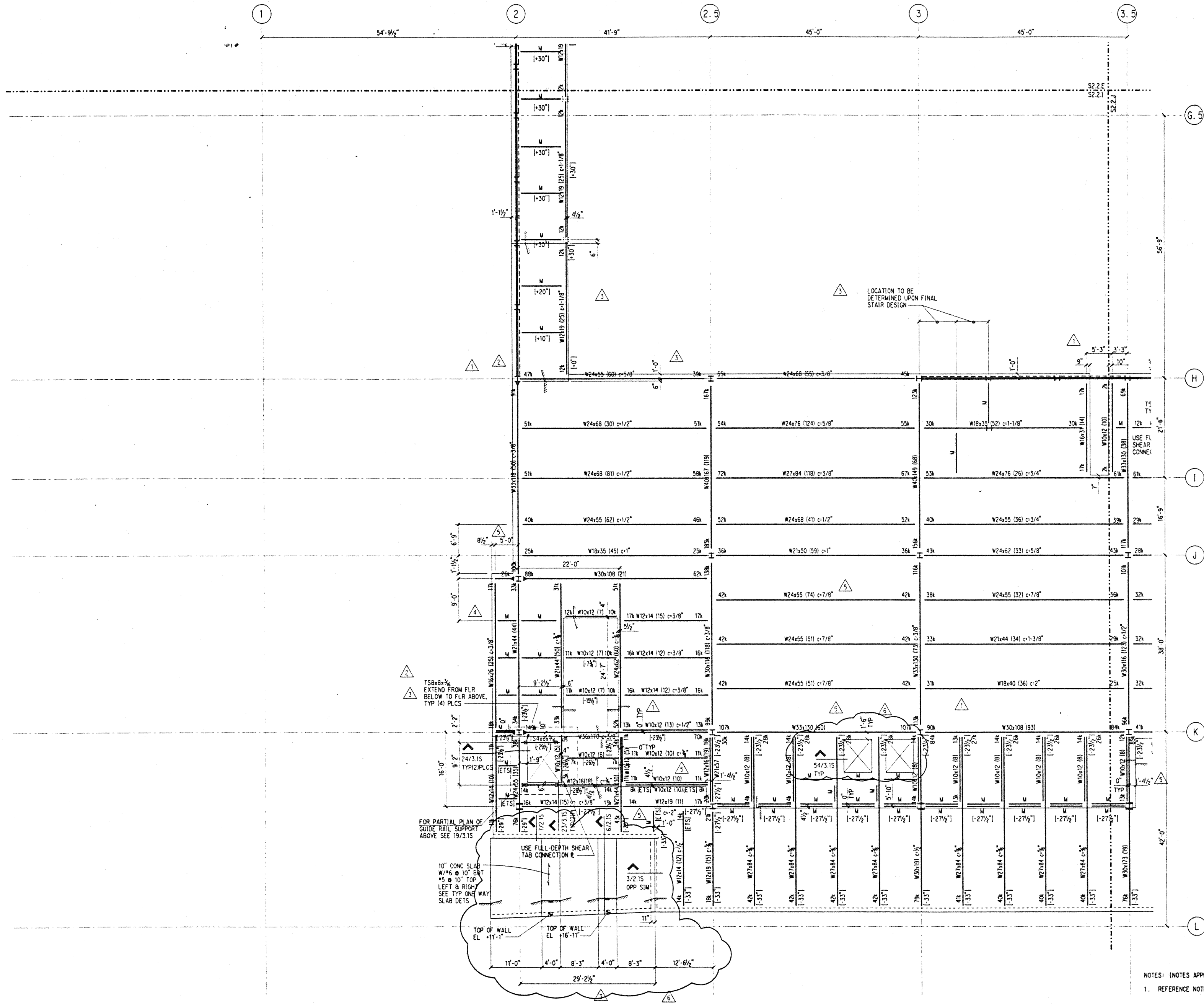
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No.	Date	By	Description

Sheet Title

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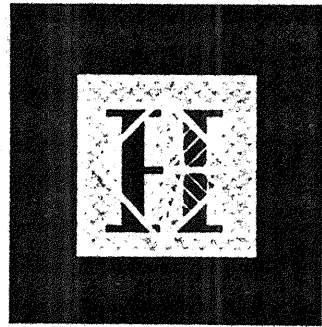
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 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.2.1



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.2.A

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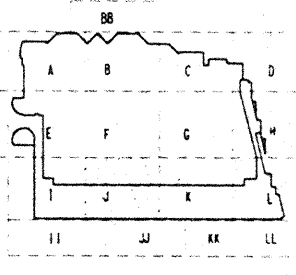
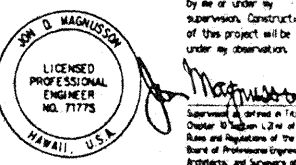
Loschky Marquardt & Nesholm
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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle, WA 98101-2699
(206) 292-1200 FAX: (206) 292-1201

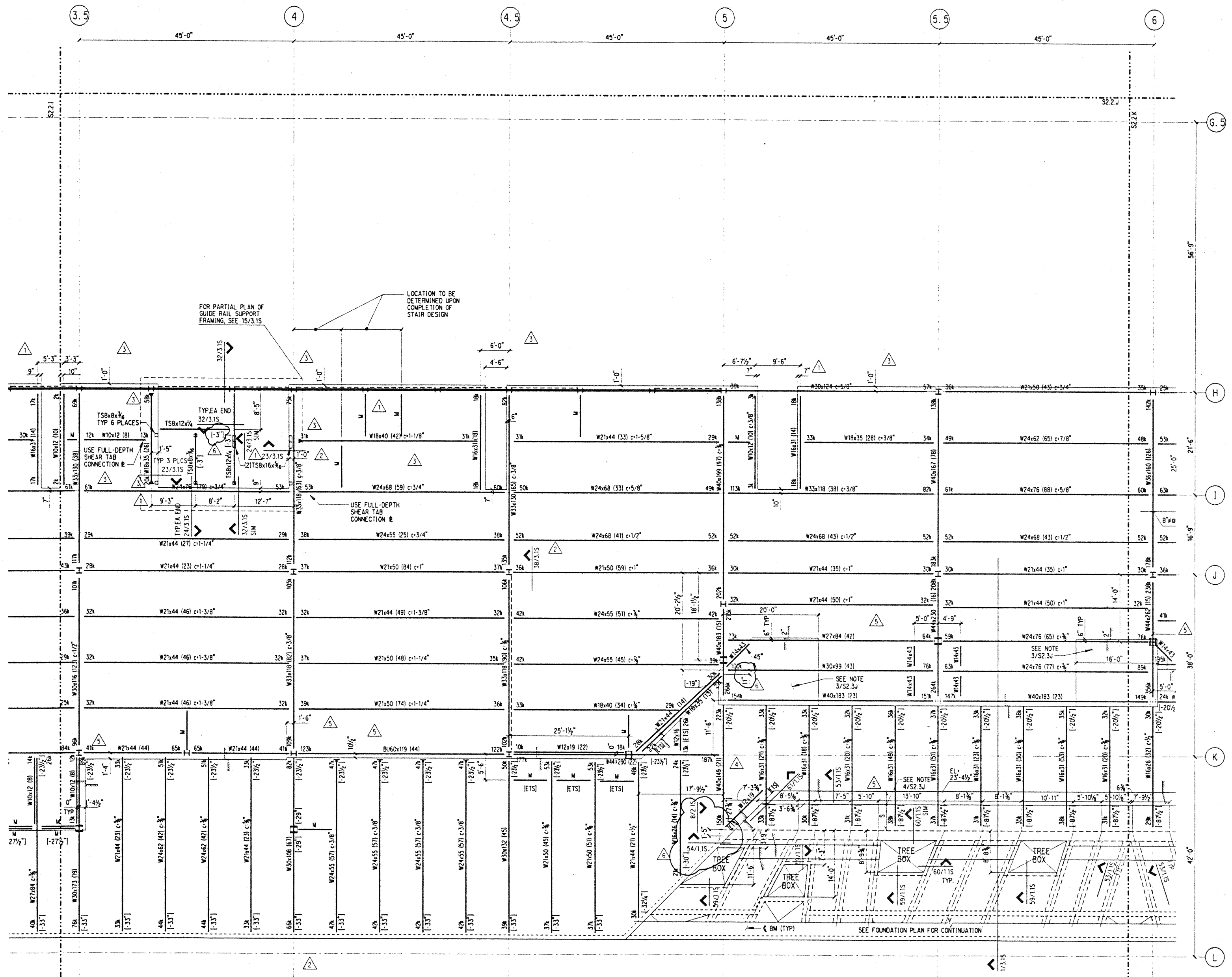


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6	6-19-95		
7	5-8-95		ADDENDUM #3

Sheet Title
INTERMEDIATE LEVEL FRAMING PLAN SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.2.J

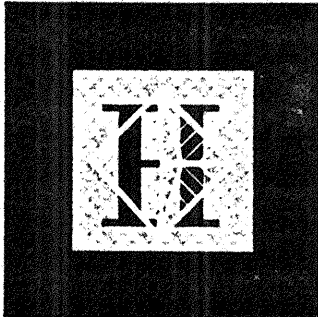


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
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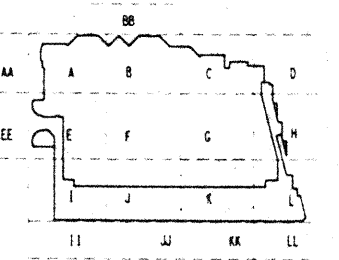
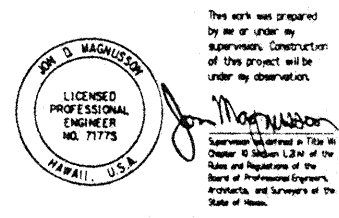
Convention Center Authority
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Seattle, Washington

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Consulting Structural and Civil Engineers
1321 Fifth Avenue, Suite 3200 Seattle, WA 98101-2699
(206) 292-1500 FAX: (206) 242-1251

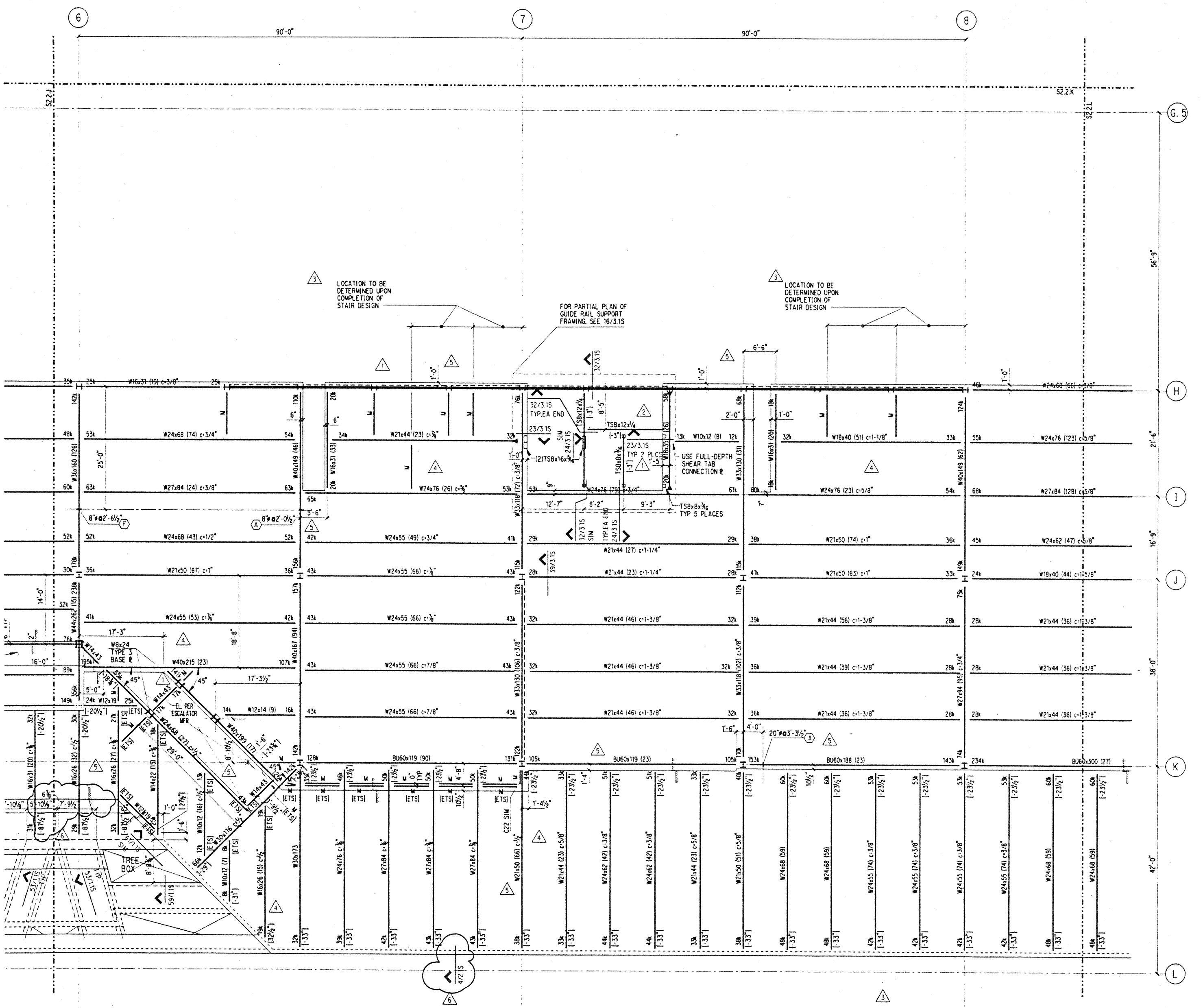


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Sheet Title
INTERMEDIATE LEVEL FRAMING PLAN SECTOR K

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Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.2.K

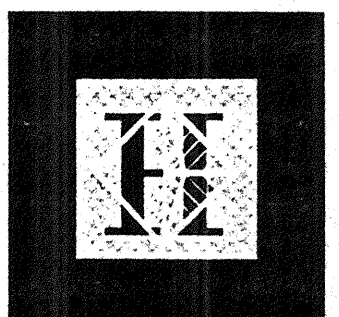


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.2.A

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Honolulu, Hawaii

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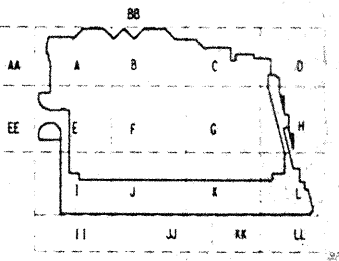
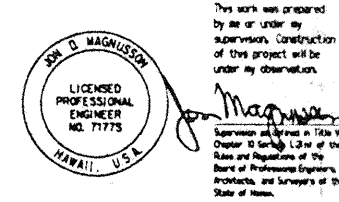
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Seattle, Washington

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SELLING WARD MAGNUSON BANKSIDE INC.

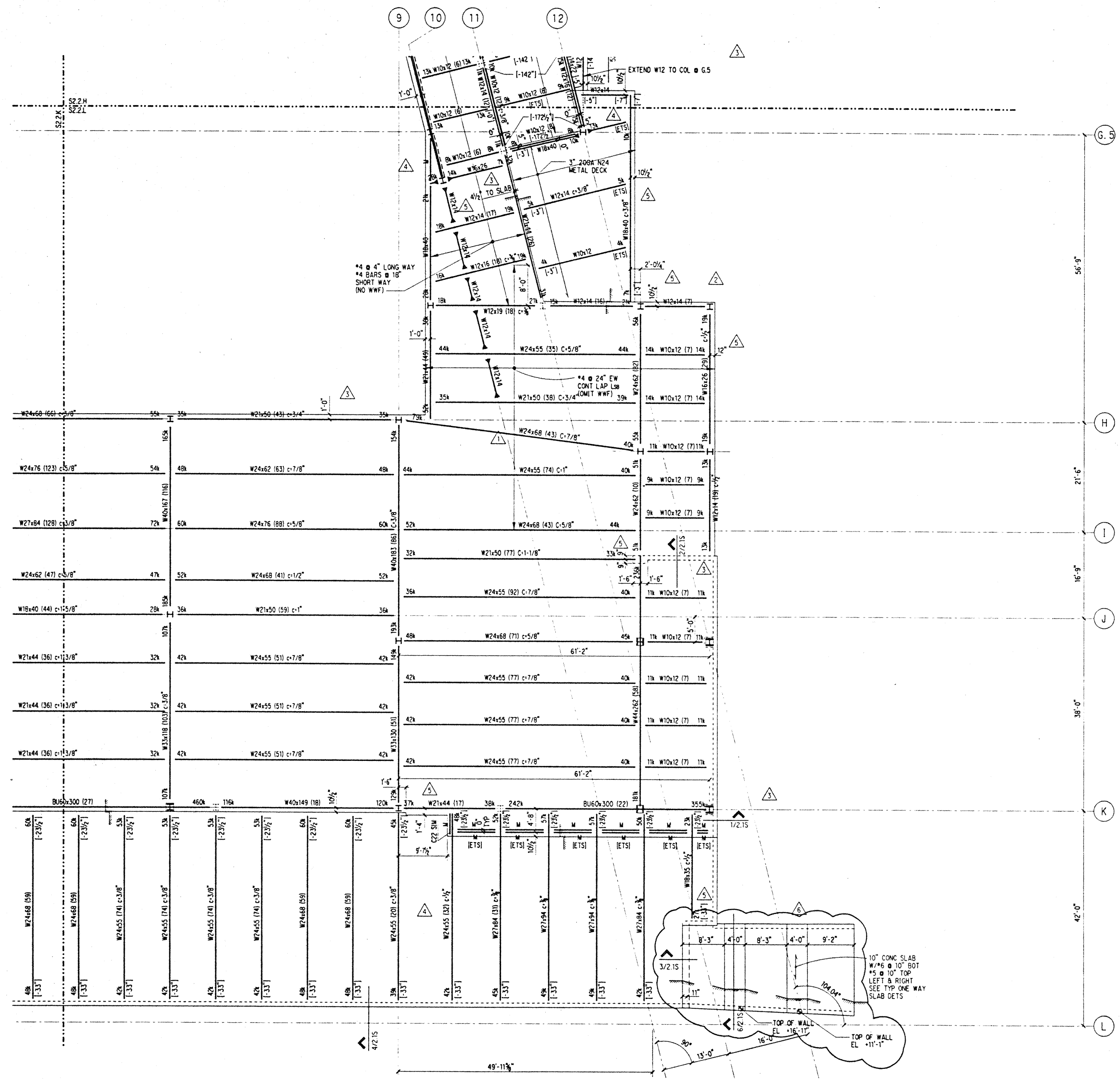
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No.	Date	By	Description
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8-31-95			
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INTERMEDIATE LEVEL FRAMING PLAN SECTOR L

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



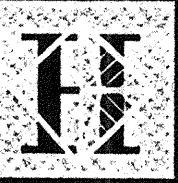
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Sat Dec 2 09:25:17 1995

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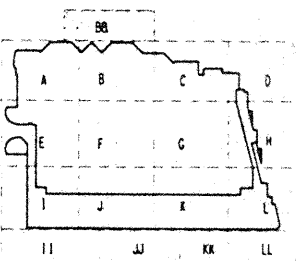
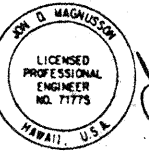
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1301 Fifth Avenue, Suite 2200 Seattle WA 98107-2699
206 462-1200 FAX: 206 462-1201

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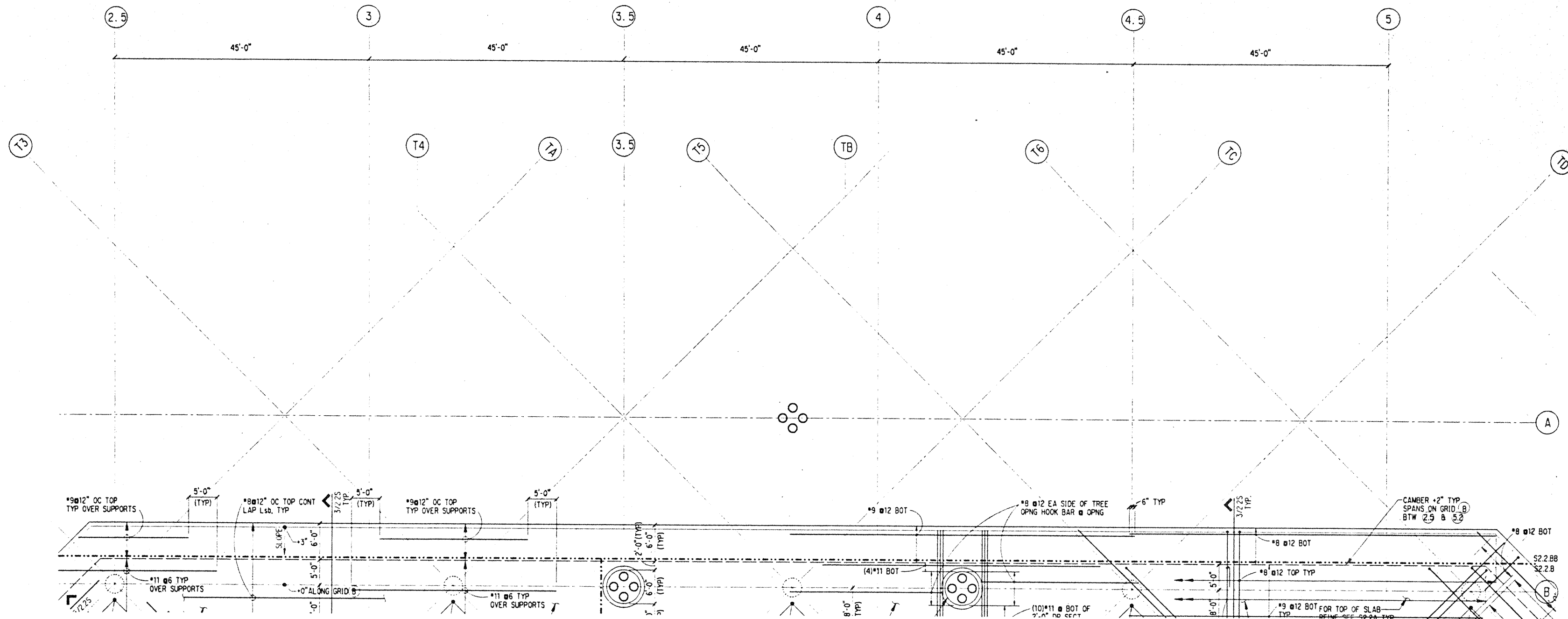
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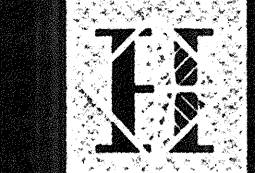
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Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.2BB



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.2.A



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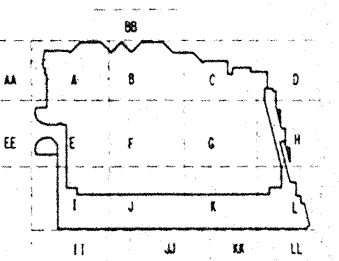
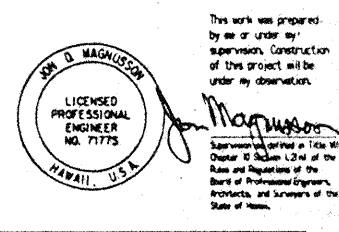
Loschky Marquardt & Nesholm
Seattle, Washington

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(206) 292-1000 FAX: (206) 292-1001



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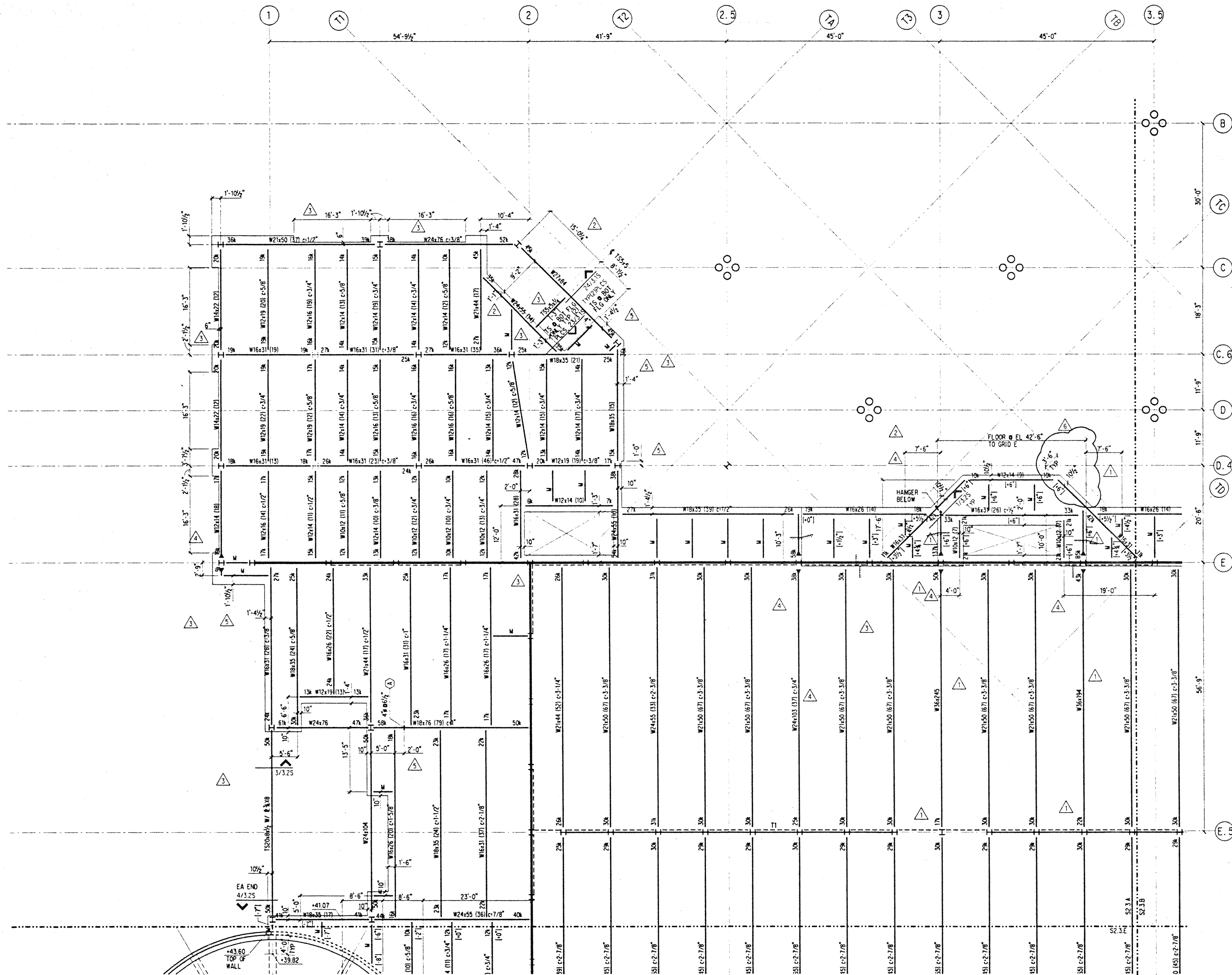
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6-19-95			
5-8-95			ADDENDUM #3

Sheet Title

PARKING FRAMING PLAN SECTOR A

Designed: BJM
 Drawn: DMS
 Checked: JDM
 Date: 4 / 12 / 95
 Job No.: 93036.02.9
 Sheet No.:

S2.3.A



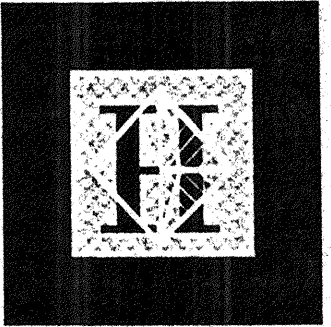
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- REFERENCE DRAWINGS:
 - S0. SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
 - S1. TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
 - S1.20 CONCRETE SCHEDULES
 - S1.21 S1.22 STEEL SCHEDULES
 - S2. PLANS
 - S3. TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
 - S4. BRACED FRAME ELEVATIONS
 - S5. TRUSS ELEVATIONS
 - S6. BRACED FRAME & TRUSS DETAILS
 - DETAIL BOOK STRUCTURAL DETAILS
- REFERENCE FLOOR ELEVATION = 42'-0"
- TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
- REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION, UNO.
- WHERE NOTED THUS (+/- 8"), TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
- STRUCTURAL SLAB IS 2-1/4" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO. STEEL DECK SHALL HAVE A ZINC COATING CONFORMING TO ASTM A525-G90.
- REINFORCE SLAB WITH #4 @ 18" OC EACH WAY, UNO.
- "M" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
- INDICATES BEAM PENETRATION, SEE S1.10
- INDICATES HAUNCHED BEAM, SEE S1.10
- SEE DETAIL 1/2.25 FOR THICKENED CONCRETE ALONG TRUSS/FRAME LOCATIONS. OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
- REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION

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Honolulu, Hawaii

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Honolulu, Hawaii

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Seattle, Washington

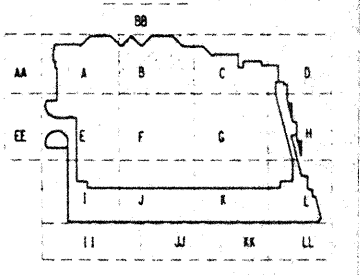
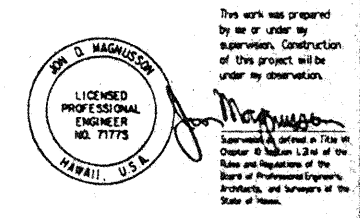
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Consultants

SKELLING WARD MAGNUSSON BANKS INC.

Consulting Structural and Civil Engineers

1301 Fifth Avenue, Suite 3200 Seattle, WA 98101-2599
(206) 292-1200 FAX: (206) 292-1201



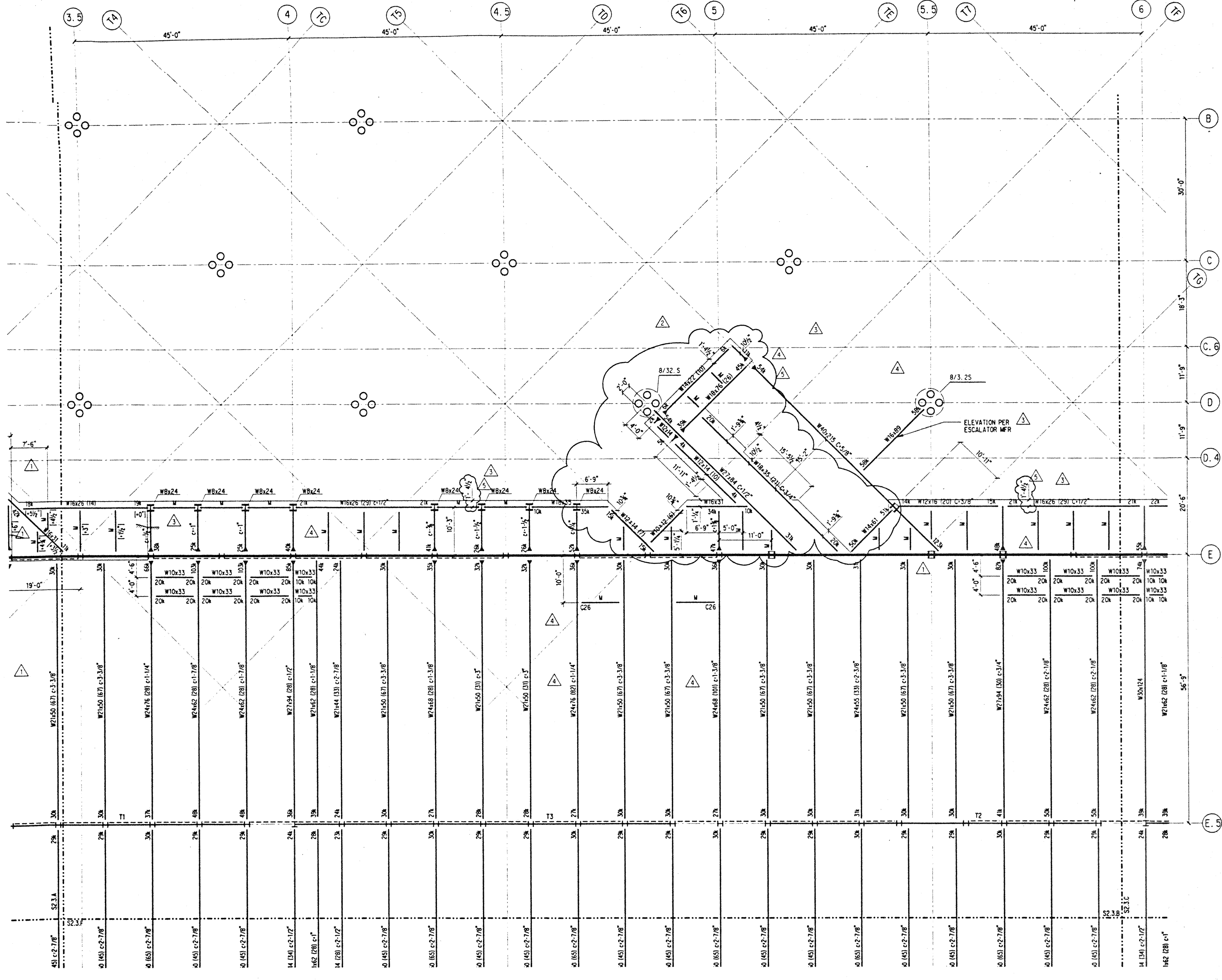
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11-7-95			
9-15-95			BUILDING PERMIT SET
8-31-95			
8-4-95			
6-19-95			
5-8-95			ADDENDUM #3

Sheet Title

PARKING FRAMING PLAN SECTOR B

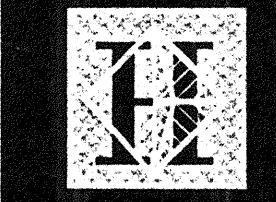
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Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.3.B



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.3.A

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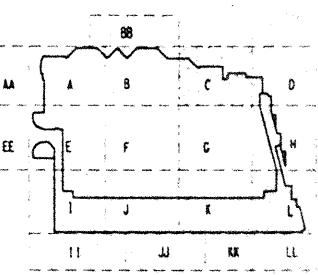
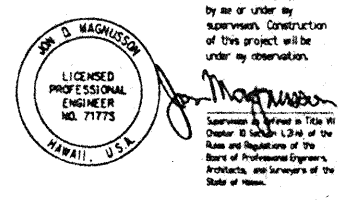
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(206) 292-1200 FAX: (206) 292-1201



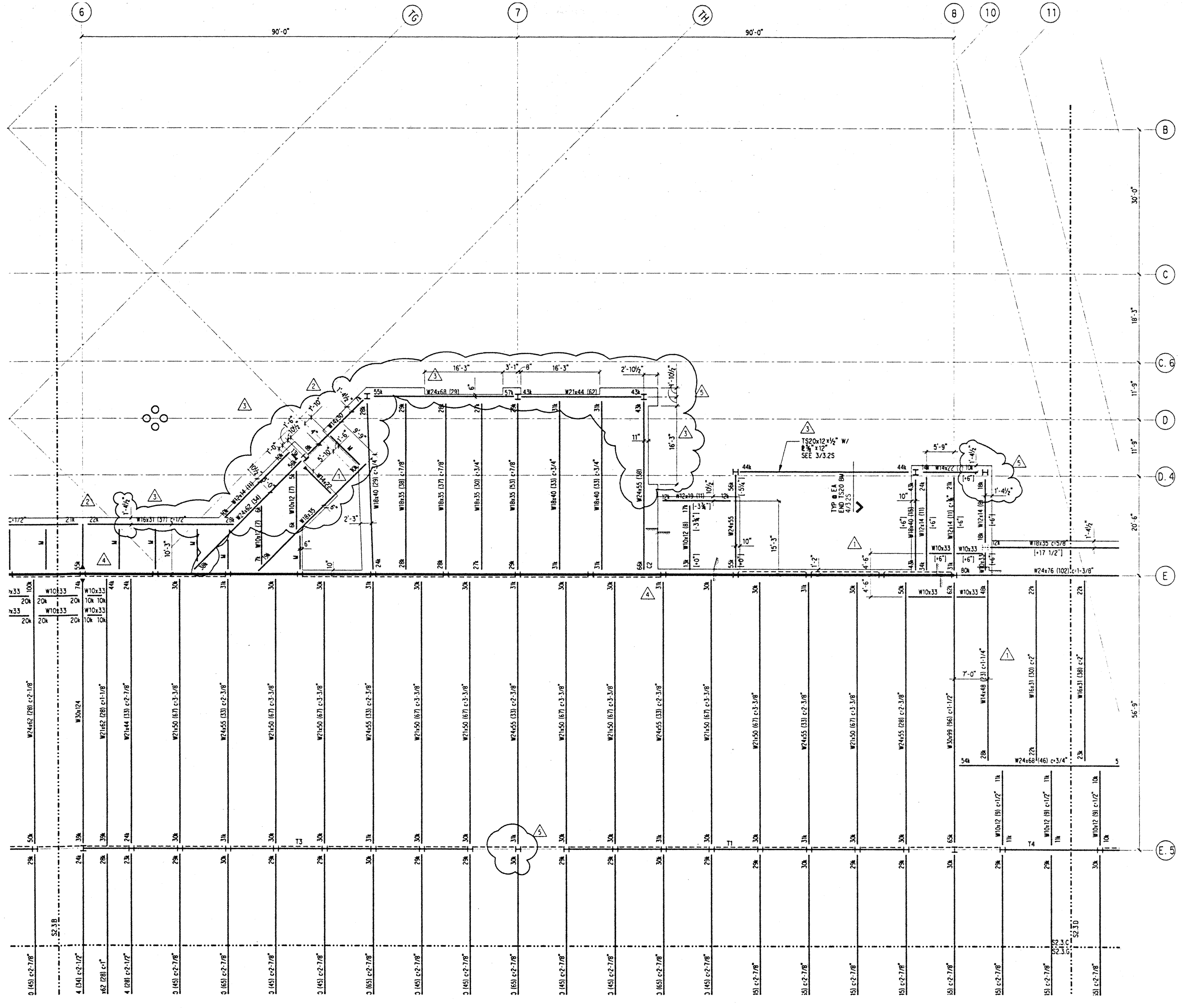
Revisions

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11-7-95			
9-15-95			BUILDING PERMIT SET
8-31-95			
8-4-95			
6-19-95			ADDENDUM #3
5-8-95			

PARKING FRAMING PLAN SECTOR C

Designed **BJM**
 Drawn **DMS**
 Checked **JCM**
 Date **4 / 12 / 95**
 Job No. **93036, 02, 9**
 Sheet No.

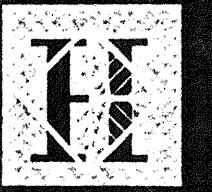
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.3.A



Fri Dec 1 08:40:55 1995

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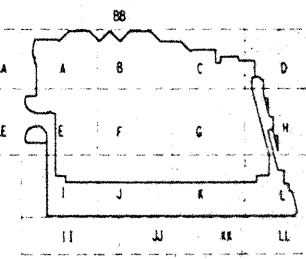
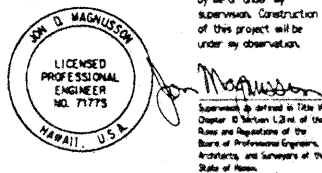
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206-292-1000 FAX: 206-292-1021



Revisions

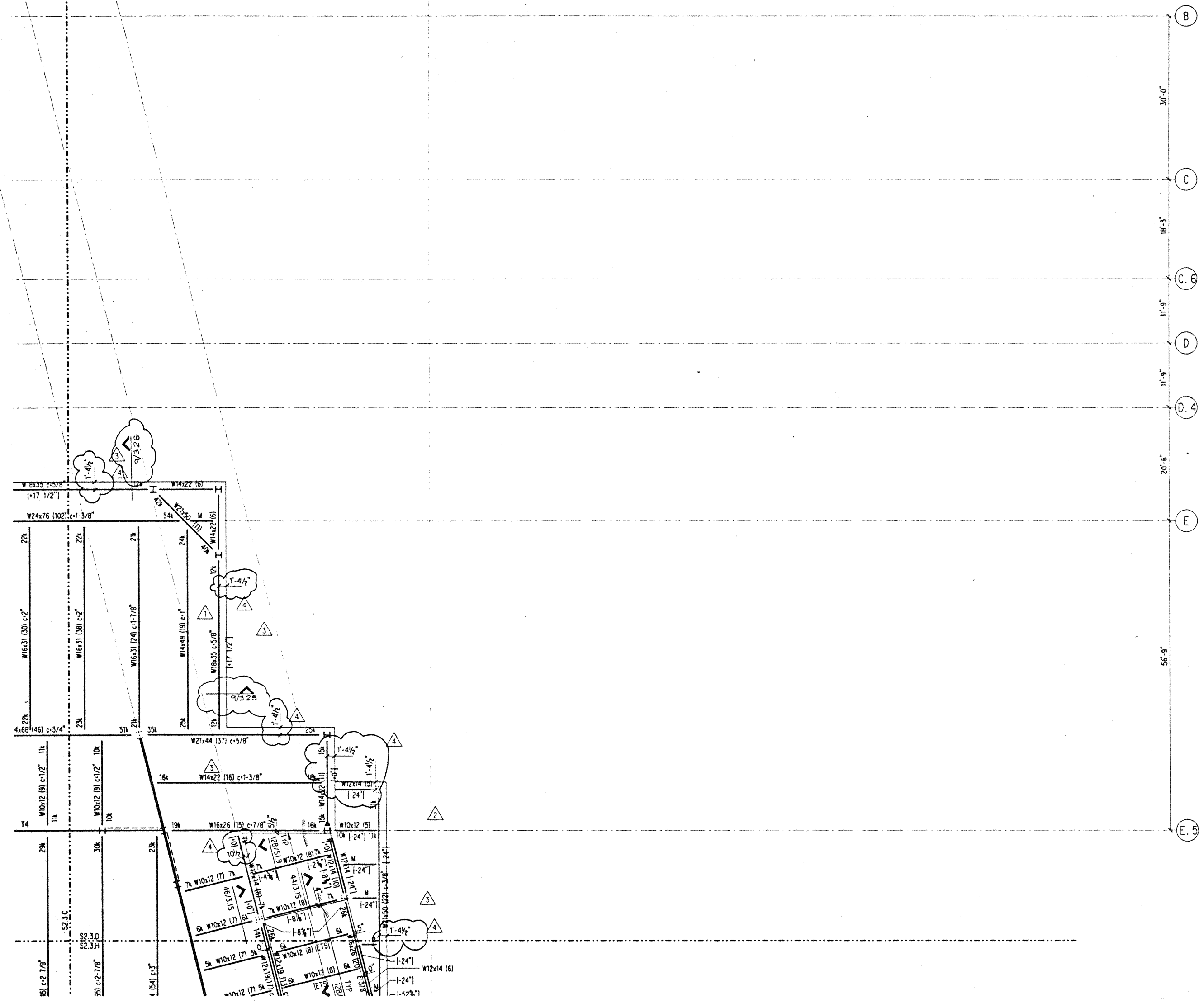
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5-8-95			ADDENDUM #3

Sheet Title

PARKING FRAMING PLAN SECTOR D

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.3.D

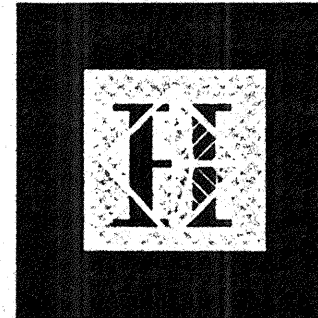


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
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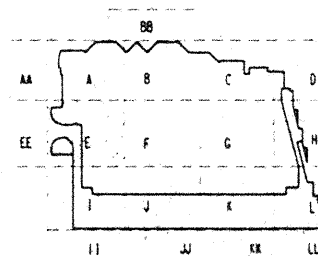
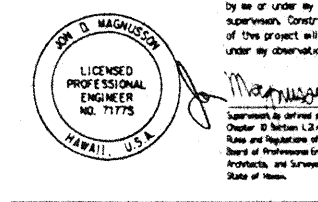
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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2599
(206) 342-1200 FAX: (206) 292-1201

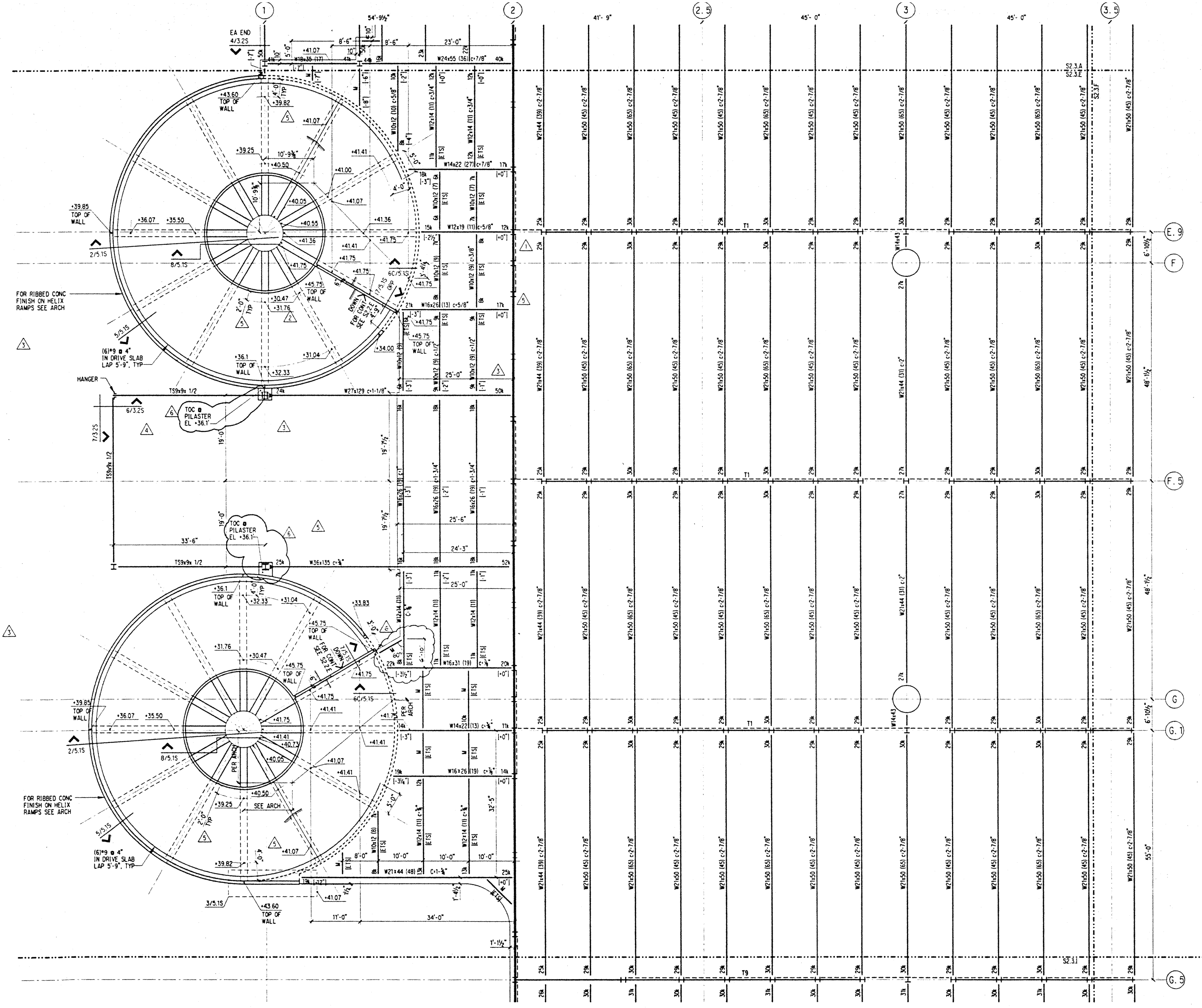


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△	8-31-95	
△	8-4-95	
△	6-19-95	
△	5-8-95	ADD COLUMN #3
No.	Date	By Description

PARKING FRAMING PLAN SECTOR E

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036, Q2, 9
 Sheet No.

S2.3.E



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.3.A

Fri Dec 1 08:45:15 1995

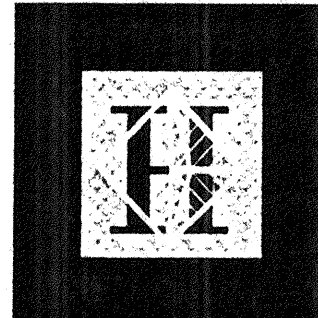
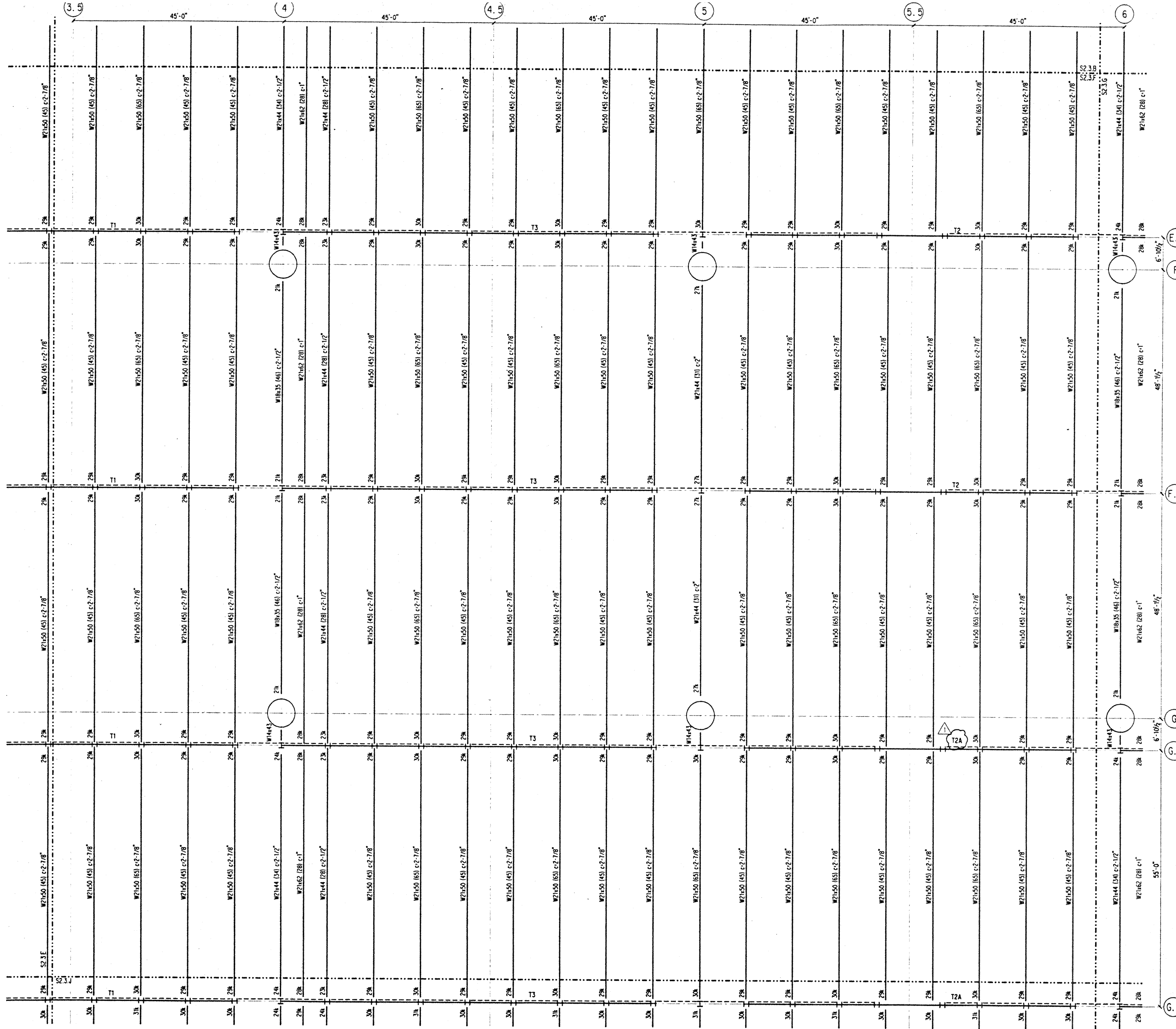
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Honolulu, Hawaii

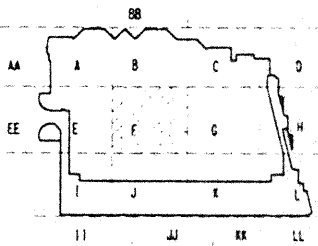
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1301 Fifth Avenue, Suite 2000, Castle, WA 98105-2899
(206) 292-1200 FAX: (206) 292-1021

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

DR. D. MAGUISSON
LICENSED PROFESSIONAL ENGINEER
NO. 71773
HAWAII, U.S.A.



Revisions

No.	Date	By	Description
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2	9-15-95		BUILDING PERMIT SET
3	9-14-95		

No. Date By Description

Sheet Title

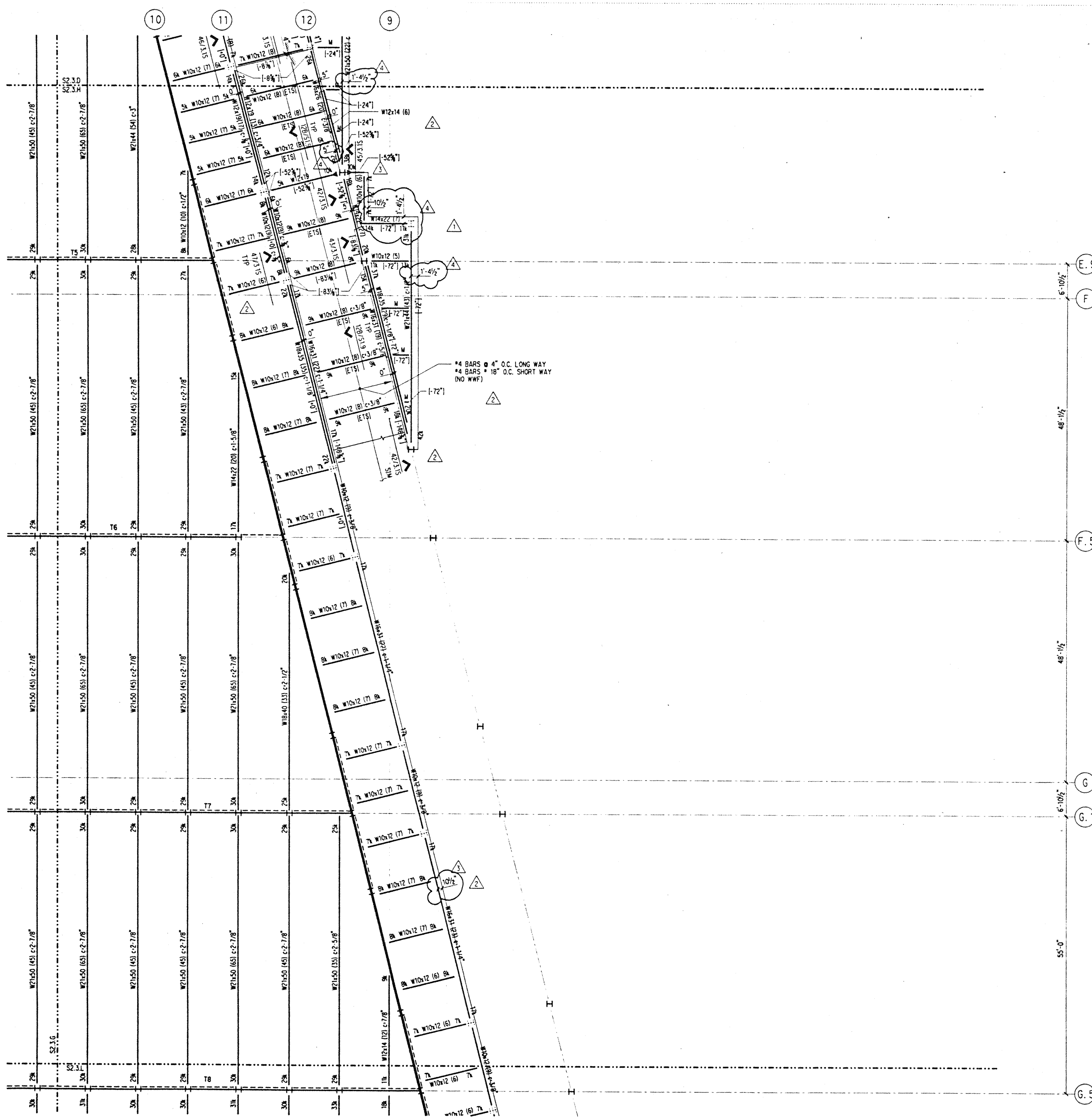
PARKING FRAMING PLAN SECTOR F

Designed **BJM**
Drawn **DMS**
Checked **JDM**
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

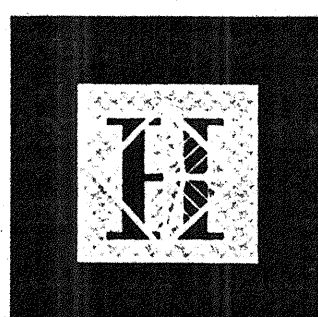
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.3.A

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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.3.A



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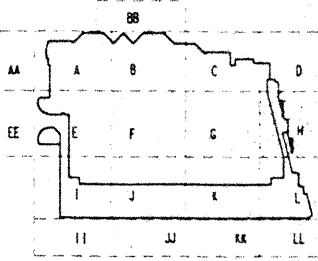
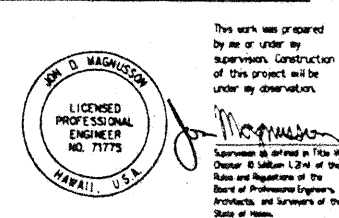
Convention Center Authority
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SKILLING WARD MAGNUSSON BAKERSHIRE INC.
 Consulting Structural and Civil Engineers
 1301 Fifth Avenue, Suite 3200 Seattle, WA 98101-2699
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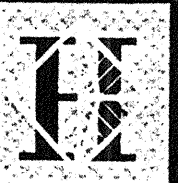
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11-7-95			BUILDING PERMIT SET
9-15-95			
8-31-95			
7-28-95			
6-19-95			

PARKING FRAMING PLAN SECTOR H

Designed: BJM
 Drawn: DMS
 Checked: JDM
 Date: 4 / 12 / 95
 Job No.: 93036.02.9
 Sheet No.:

S2.3.H



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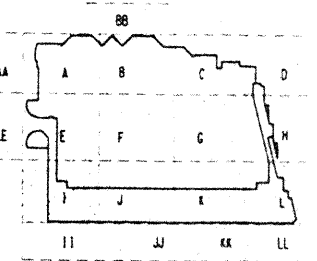
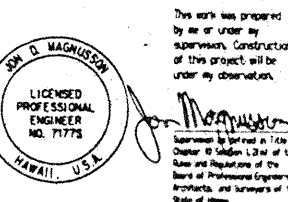
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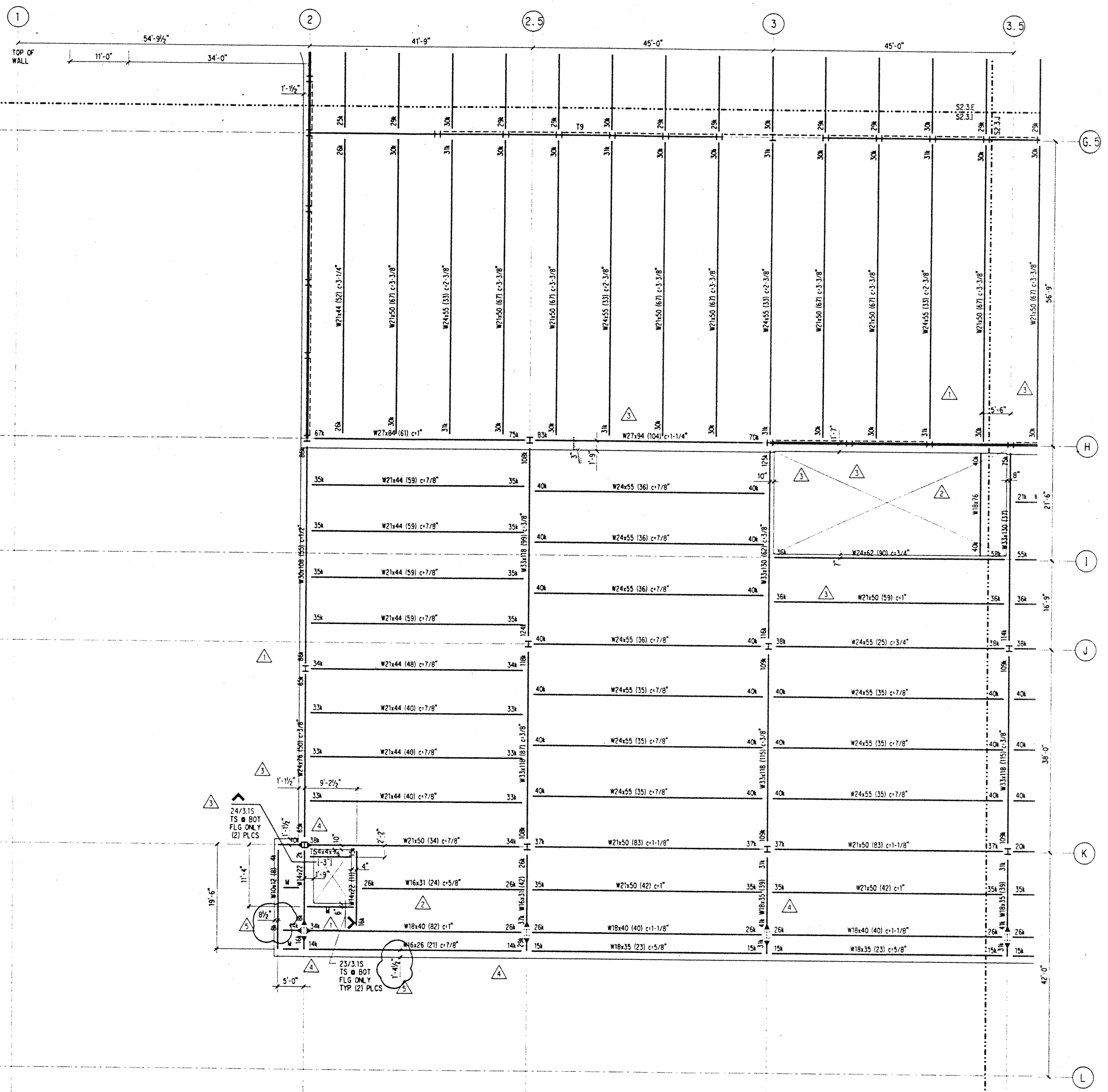


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8-31-95			
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5-8-95			ADDENDUM #3

Sheet Title

PARKING FRAMING PLAN SECTOR I

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

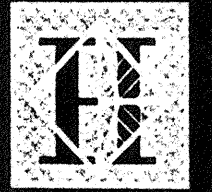


- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- REFERENCE NOTES ON S2.3.A
 - SLOPE STRUCTURAL STEEL DOWN AT 2.45 PERCENT SOUTH OF GRID H, UNO.

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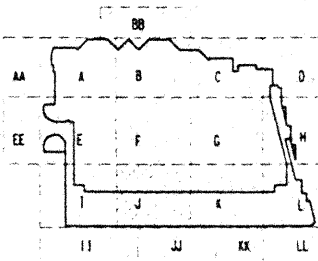
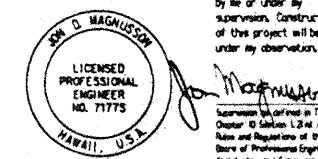
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Seattle, Washington

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SKILLING WARD MCGUIRE BAUER INC.

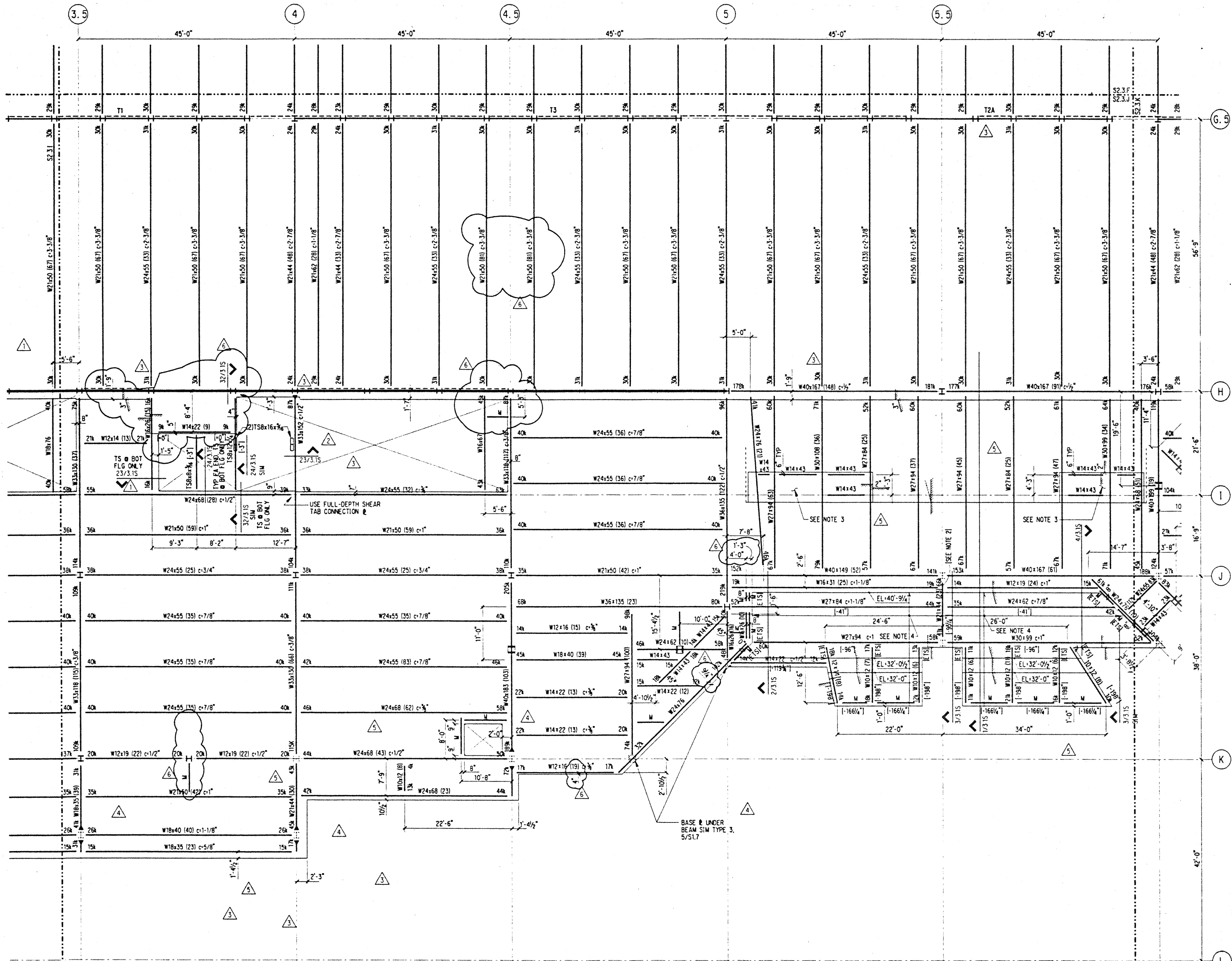
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1301 First Avenue, Suite 3200, Seattle, WA 98101-2099
206 222-1200 FAX 206 222-2201



No.	Date	By	Description
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3	8-31-95		
2	8-4-95		
1	6-19-95		
	5-8-95		ADDENDUM #1

PARKING FRAMING PLAN SECTOR J

Designed: BJM
 Drawn: DMS
 Checked: JDM
 Date: 4 / 12 / 95
 Job No.: 93036, 02, 9
 Sheet No.:

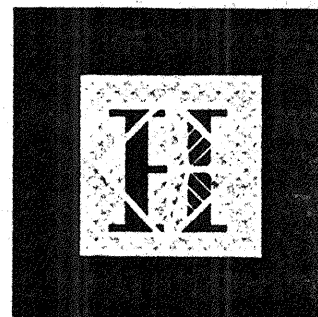


- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- REFERENCE NOTES ON S2.3.A
 - SLOPE STRUCTURAL STEEL DOWN AT 2.45 PERCENT SOUTH OF GRID H, UNO.
 - STRUCTURAL SLAB IS 4 1/2" OF HARDROCK CONCRETE ON 3" 16 GA STEEL DECK. REINFORCE PER SECTION ON 7/3.35.
 - STRUCTURAL SLAB IS 2 1/2" OF HARDROCK CONCRETE ON 3" 16 GA STEEL DECK.

Print Date: 1/15/08, 3:38 1995

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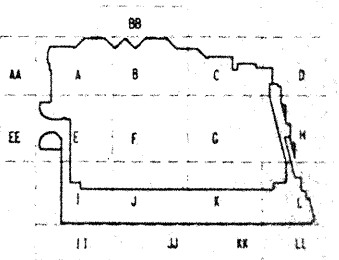
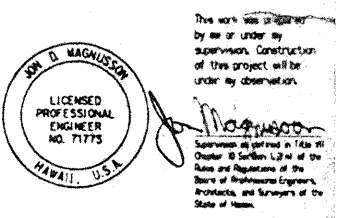
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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3000 Seattle WA 98107-2699
(206) 292-1200 FAX: (206) 292-1031



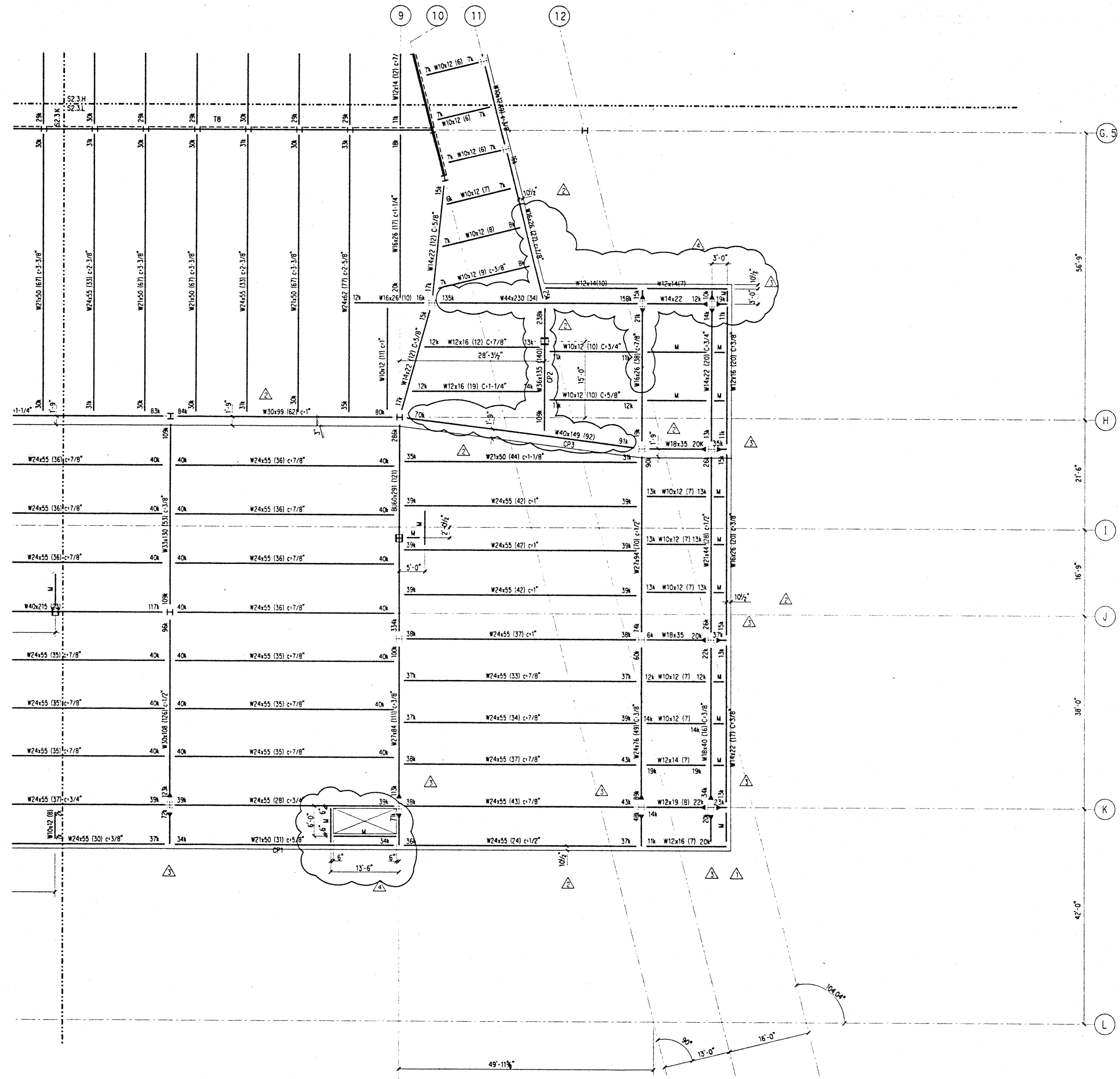
Revisions

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9-15-95			BUILDING PERMIT SET
8-31-95			
8-4-95			
6-19-95			

Sheet Title

PARKING FRAMING PLAN SECTOR L

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

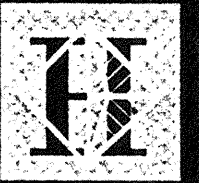


- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.3.A
 2. SLOPE STRUCTURAL STEEL DOWN AT 2.45 PERCENT SOUTH OF GRID H, UNO.

Fri Dec 1 09:00:23 1995

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1/4" = 1'



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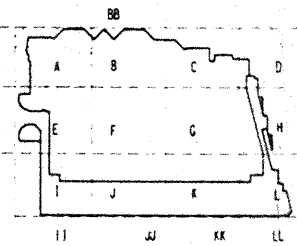
SKELLING WARD MCGUIRE BARKSHIRE INC.

Consulting Structural and Civil Engineers

1301 Fifth Avenue, Suite 3200, Seattle, WA 98101-2699
(206) 292-1200 FAX: (206) 292-1001

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

D. D. Magnusson
LICENSED PROFESSIONAL ENGINEER
NO. 71775
HAWAII, U.S.A.

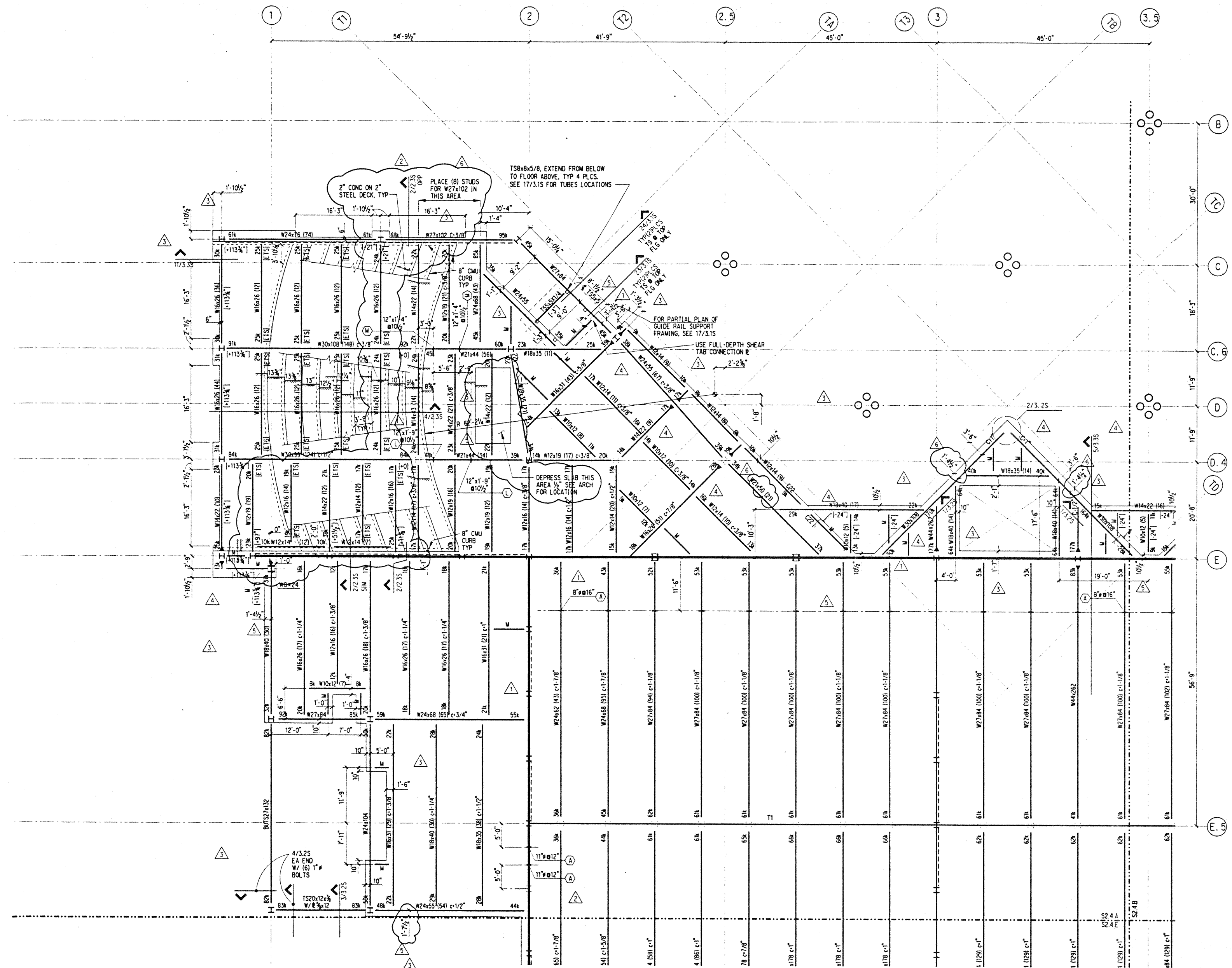


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4	8-14-95		REVISION #1
5	6-19-95		REVISION #2
6	5-8-95		REVISION #3

MEETING FRAMING PLAN SECTOR A

Designed **BJM**
 Drawn **DMS**
 Checked **JCM**
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.4.A



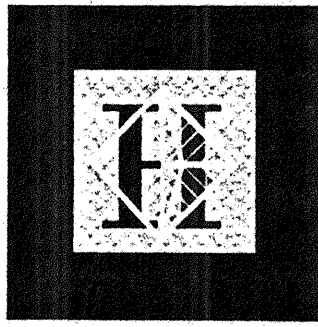
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- REFERENCE DRAWINGS:
 S0. • SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
 S1. • TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
 S1.20 CONCRETE SCHEDULES
 S1.21 S1.22 STEEL SCHEDULES
 S3. • TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
 S4. • BRACED FRAME ELEVATIONS
 S5. • TRUSS ELEVATIONS
 S6. • BRACED FRAME & TRUSS DETAILS
 DETAIL BOOK
- REFERENCE FLOOR ELEVATION = 54'-6"
- TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
- REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION, UNO.
- WHERE NOTED THIS (+/- 8"), TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
- STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
- REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
- "M" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
- INDICATES BEAM PENETRATION, SEE S1.10
- INDICATES HAUNCHED BEAM, SEE S1.10
- REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS, REINFORCE PER TYPICAL DETAILS.

Tue Nov 30 18:03:24 1995

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1/5 pl1



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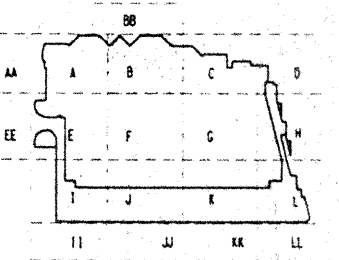
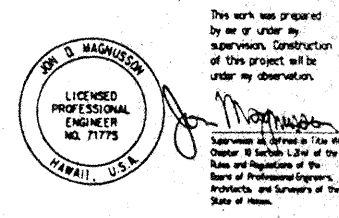
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No.	Date	By	Description
△ 12-15-95			FINAL DOCUMENT
△ 11-7-95			BUILDING PERMIT SET
△ 9-15-95			
△ 8-31-95			
△ 8-4-95			
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△ 5-8-95			ADDENDUM #3

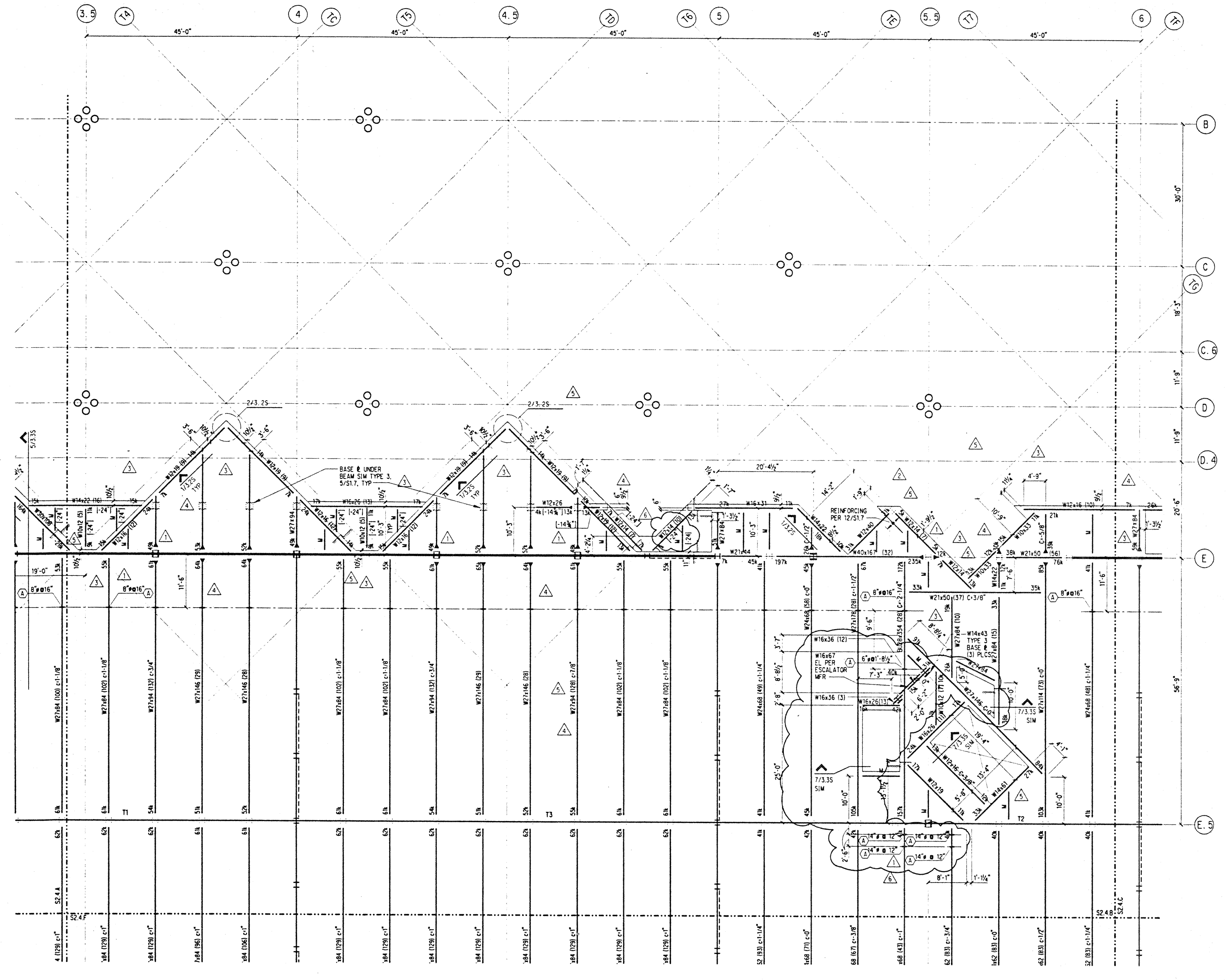
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Sheet Title

MEETING FRAMING PLAN SECTOR B

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Drawn	DMS
Checked	JCM
Date	4 / 12 / 95
Job No.	93036, 02, 9
Sheet No.	

S2.4.B

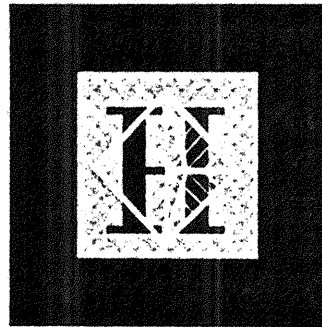


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.4.A

Thu Nov 30 18:05:59 1995

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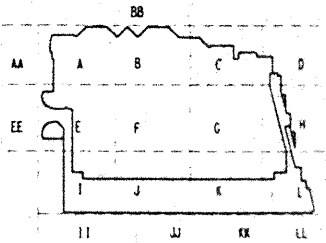
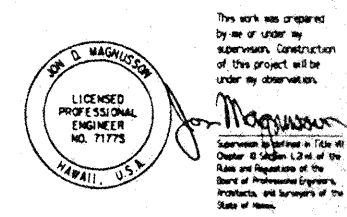
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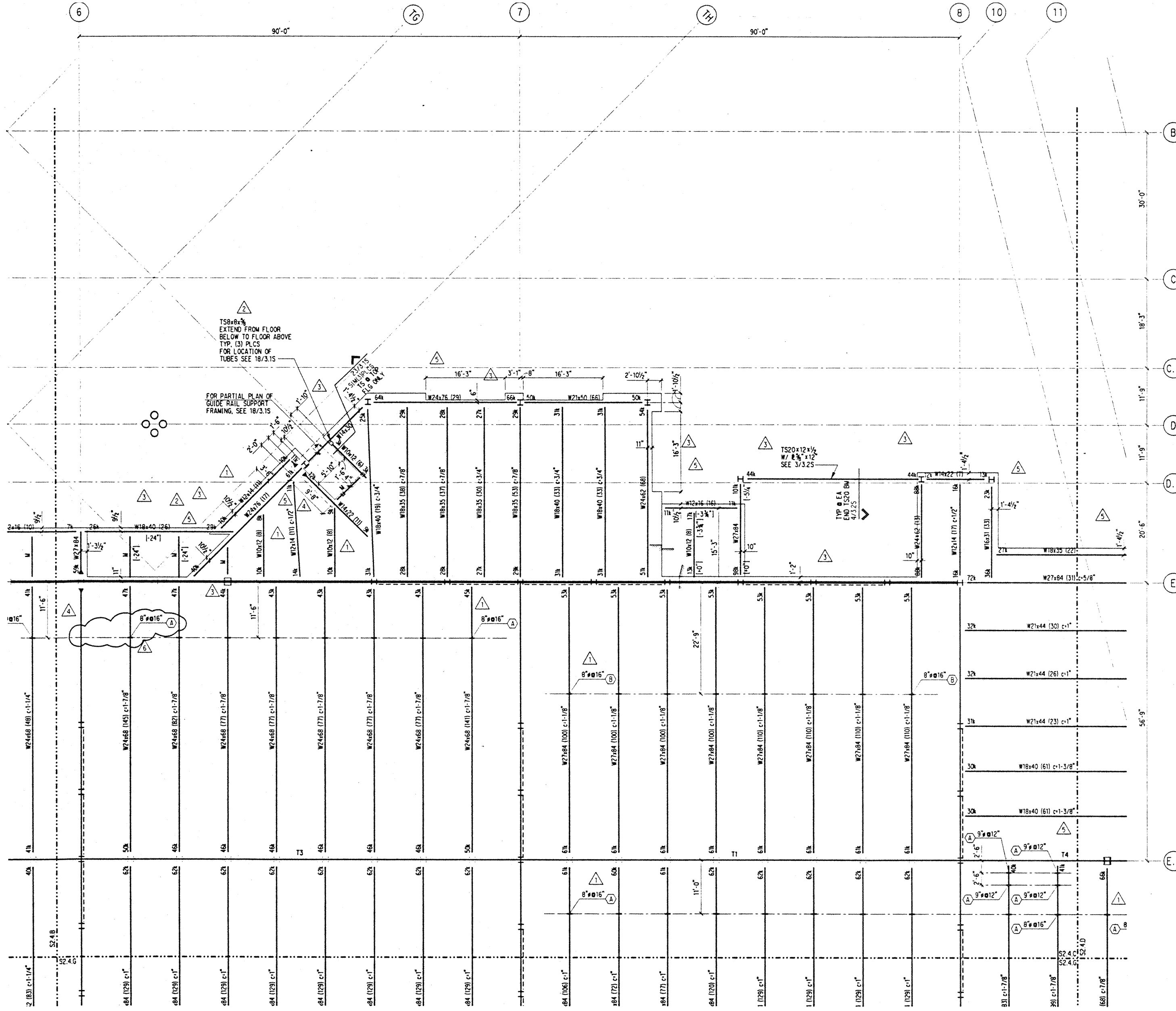
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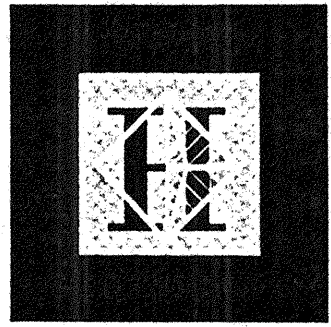
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Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.4.C



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.4.A

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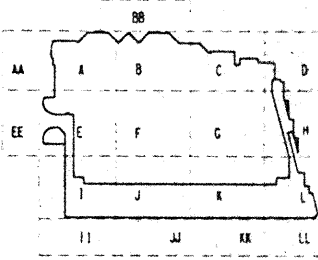
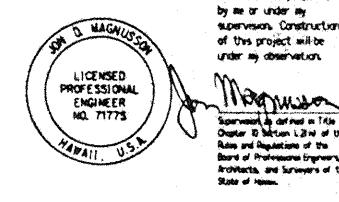
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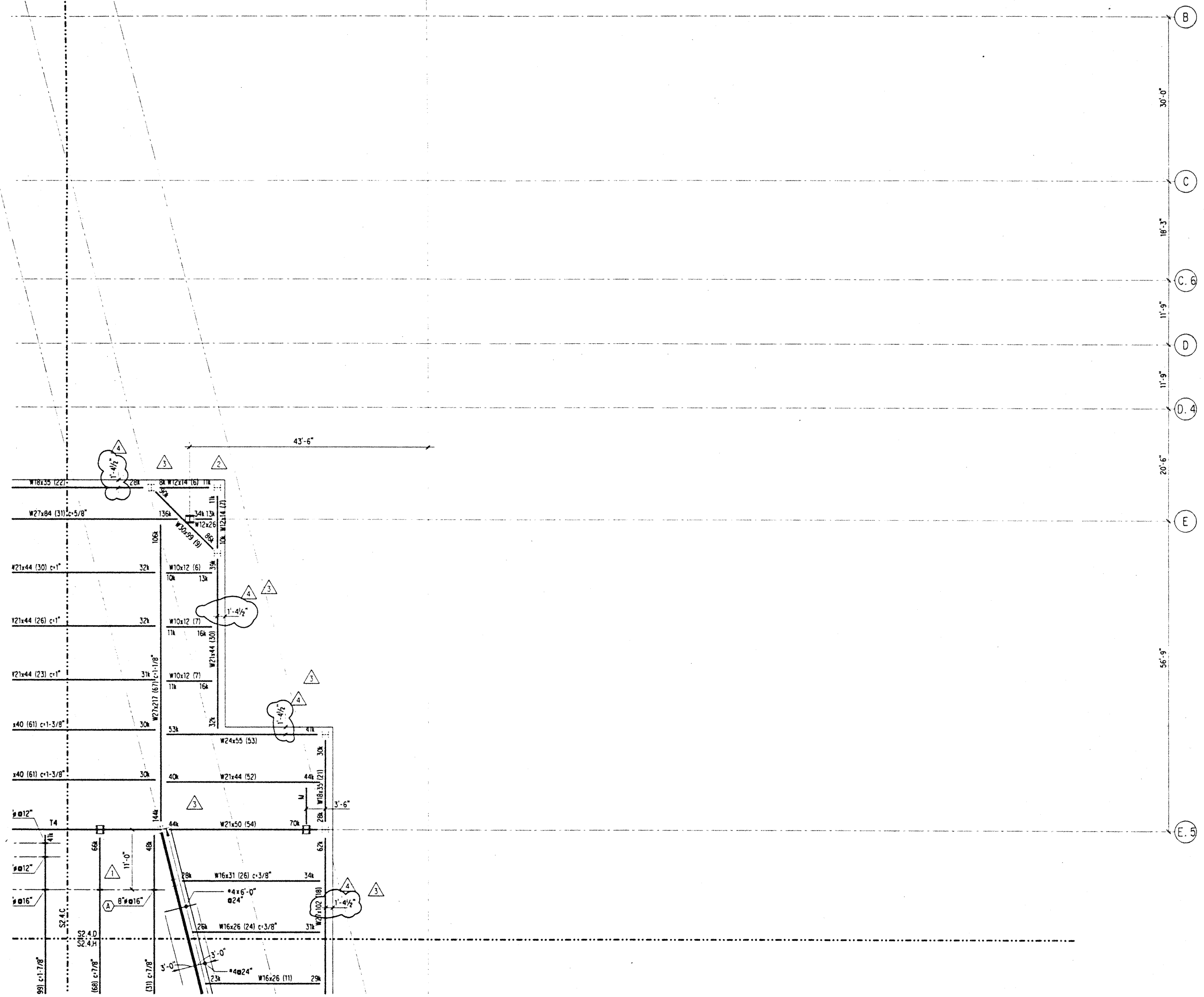
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S2.4.D

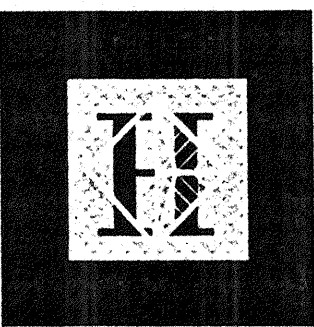
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1. REFERENCE NOTES ON S2.4.A



Thu Nov 30 18:11:02 1995

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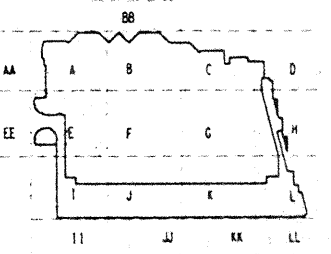
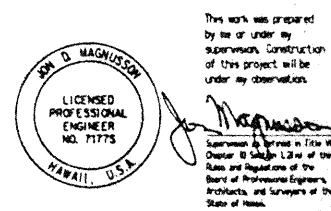
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No.	Date	By	Description
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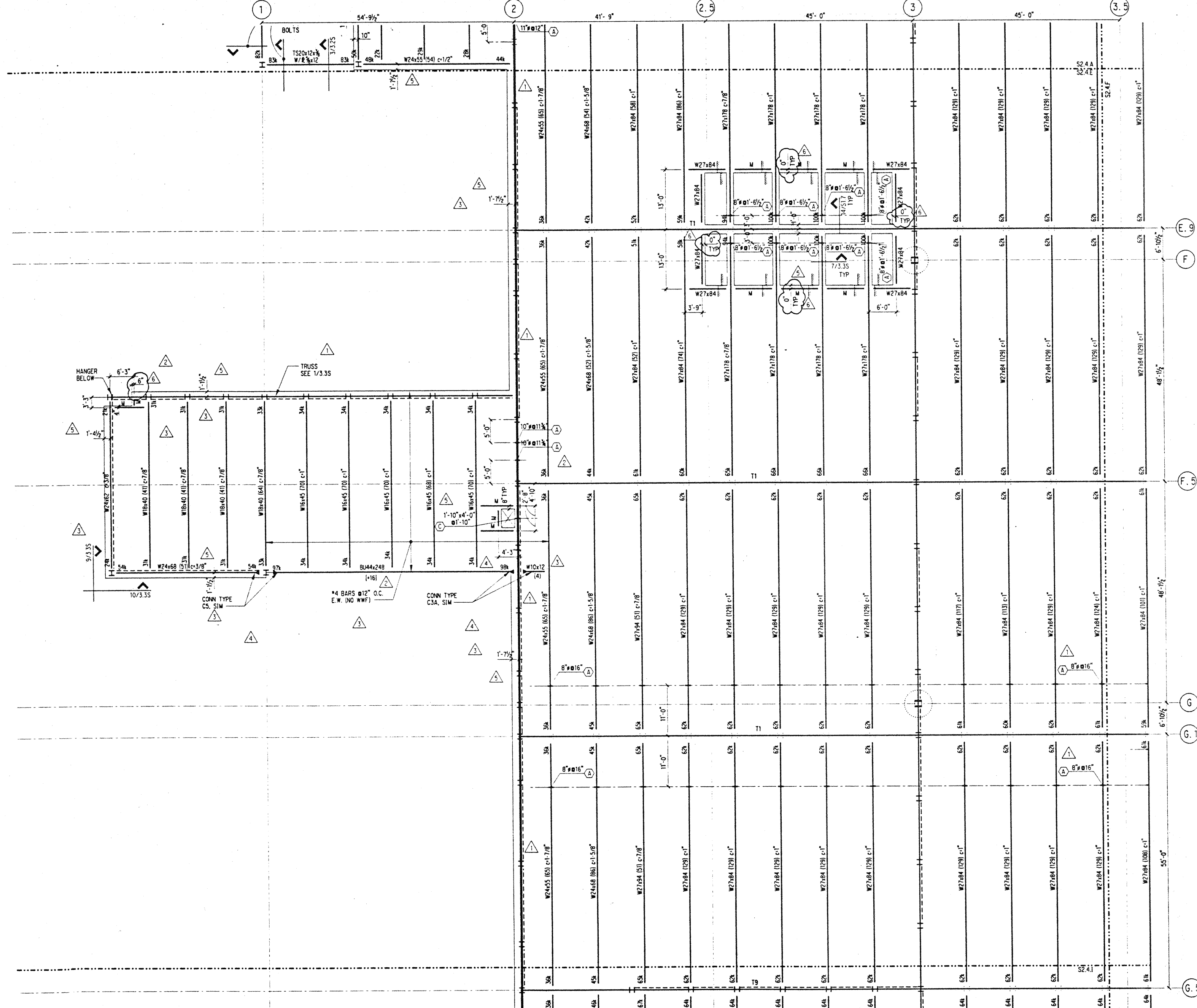
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 Date 4 / 12 / 95
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 Sheet No.

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S2.4.E



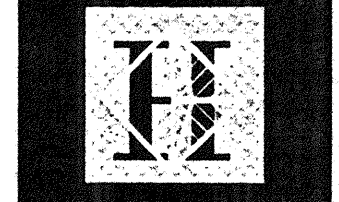
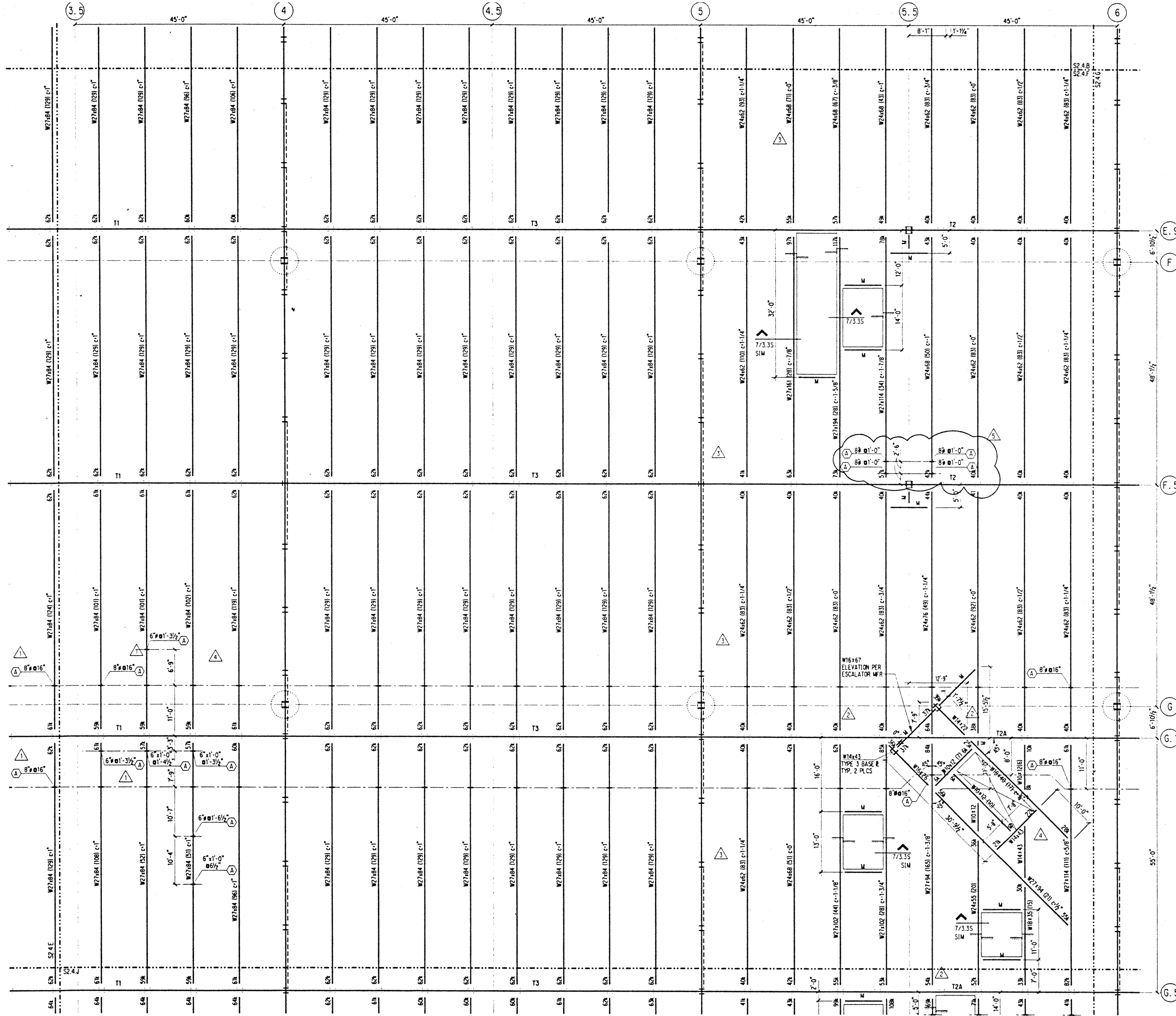
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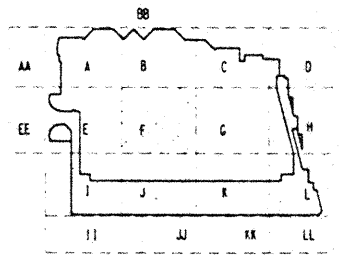
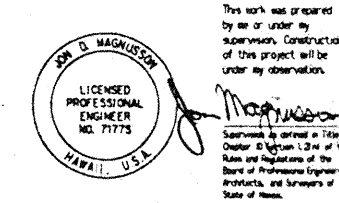
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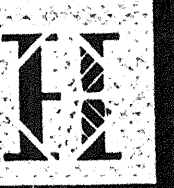
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6	5-8-95		ADDENDUM #3

MEETING FRAMING PLAN SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.4.A

S2.4.F



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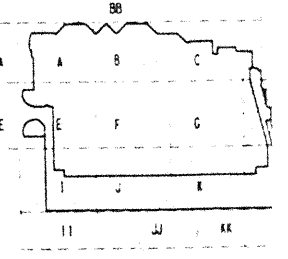
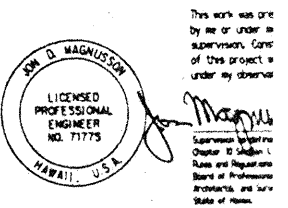
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Revisions

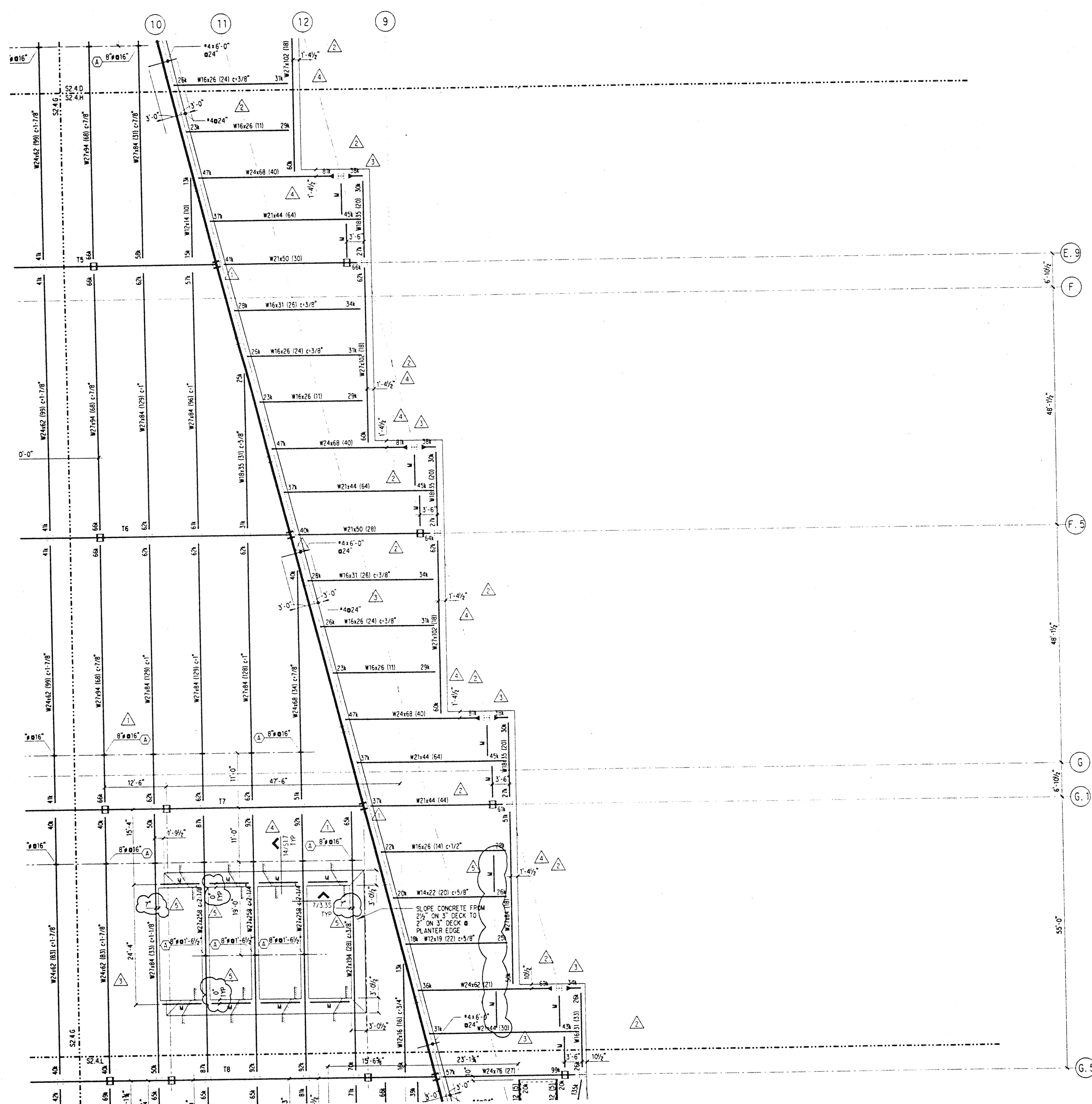
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Sheet Title

**MEETING
FRAMING PLAN
SECTOR H**

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.4.H

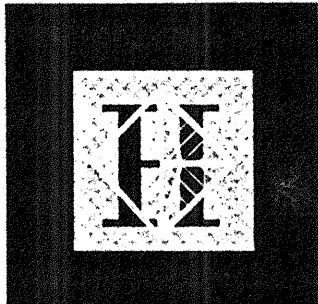


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.4.A

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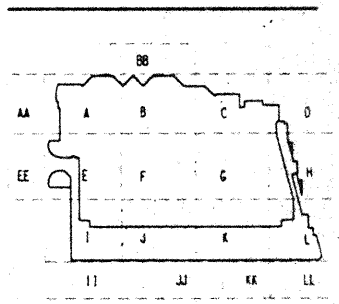
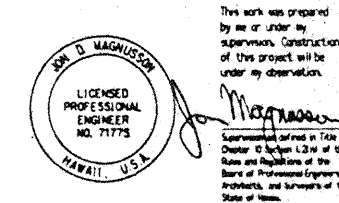
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5-8-95	ADDENDUM #3

No. Date By Description

Sheet Title

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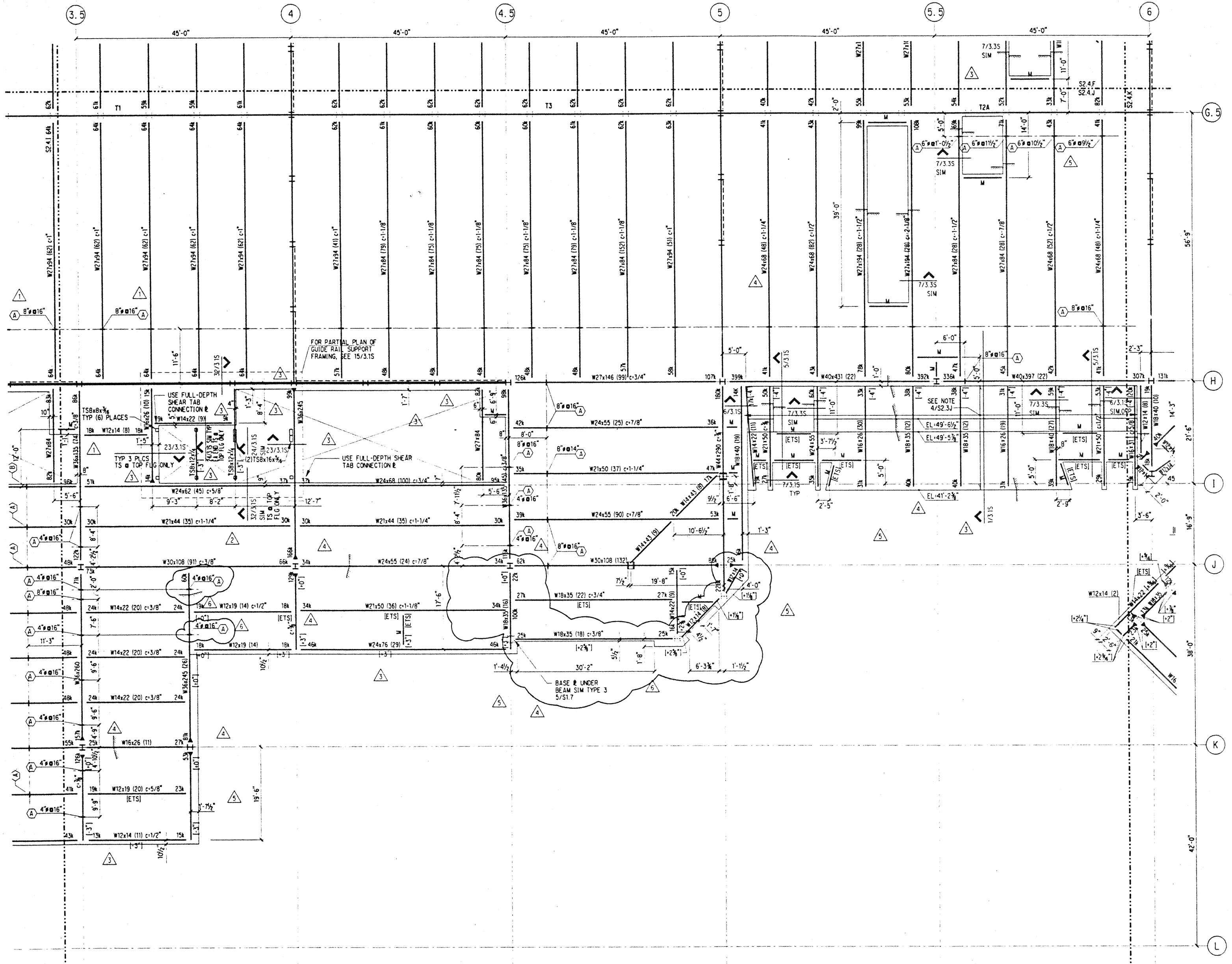
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 Job No.: 93036.02.9
 Sheet No.:

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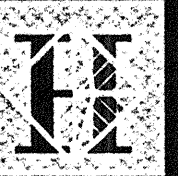
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.4.A



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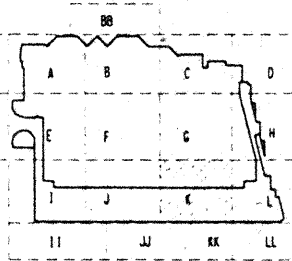
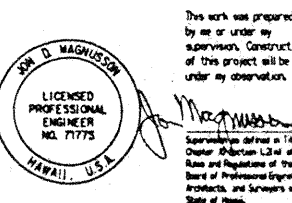
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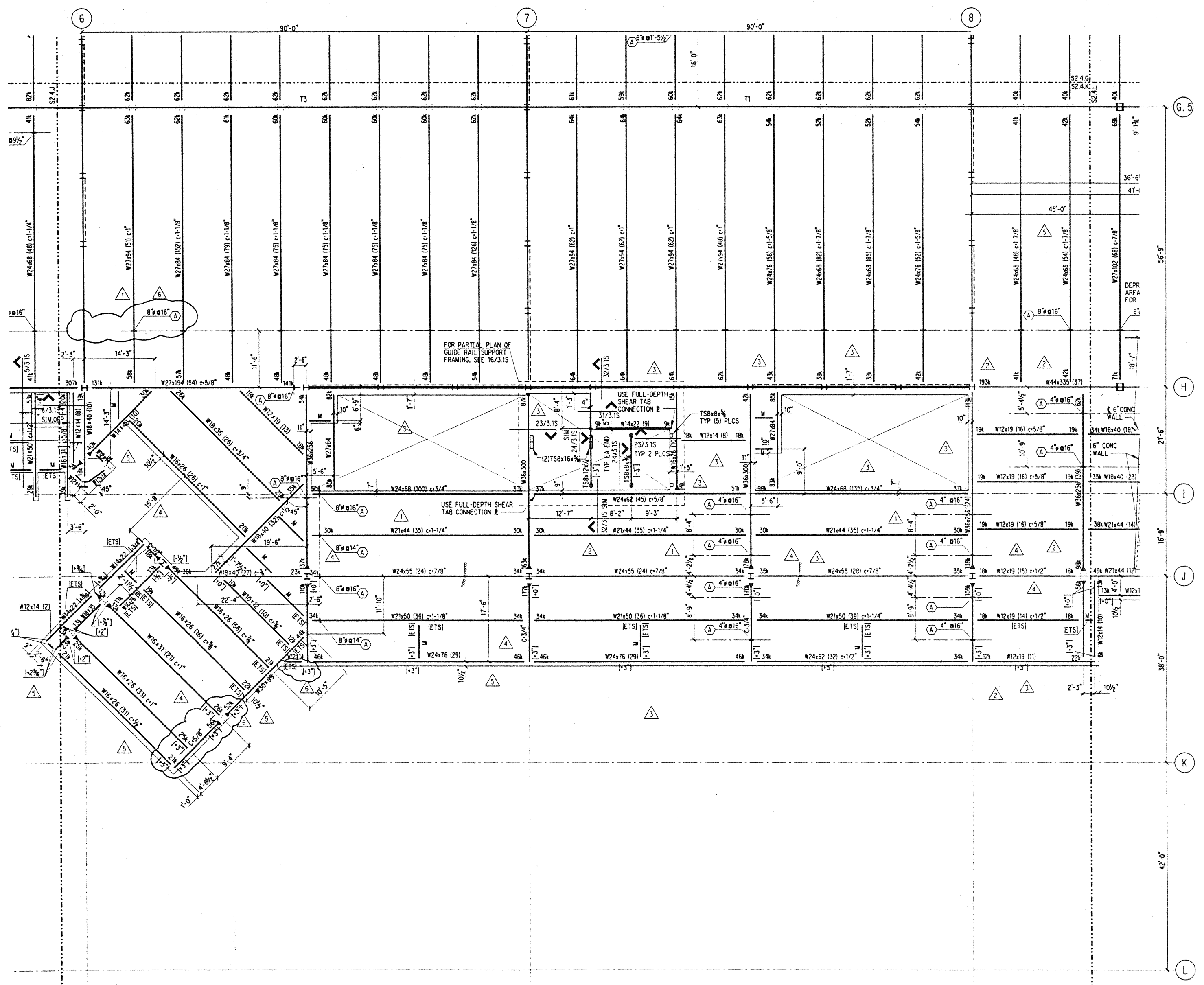
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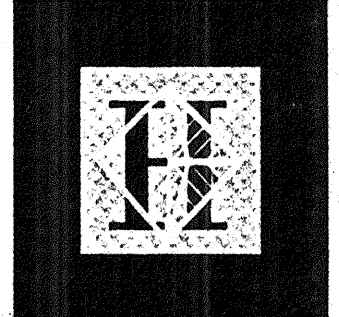
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Checked	JDM
Date	4 / 12 / 95
Job No.	93036, 02, 9
Sheet No.	

S2.4.K



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.4.A



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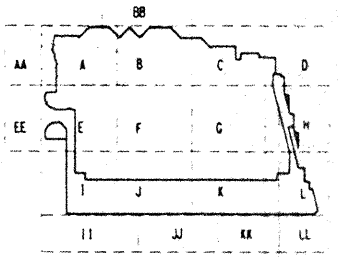
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(206) 292-1200 FAX: (206) 292-1201

This work was prepared by me or under my supervision. Construction of this project will be under my supervision.

DR. D. MAGNUSON
LICENSED PROFESSIONAL ENGINEER
NO. 71775
HAWAII, U.S.A.



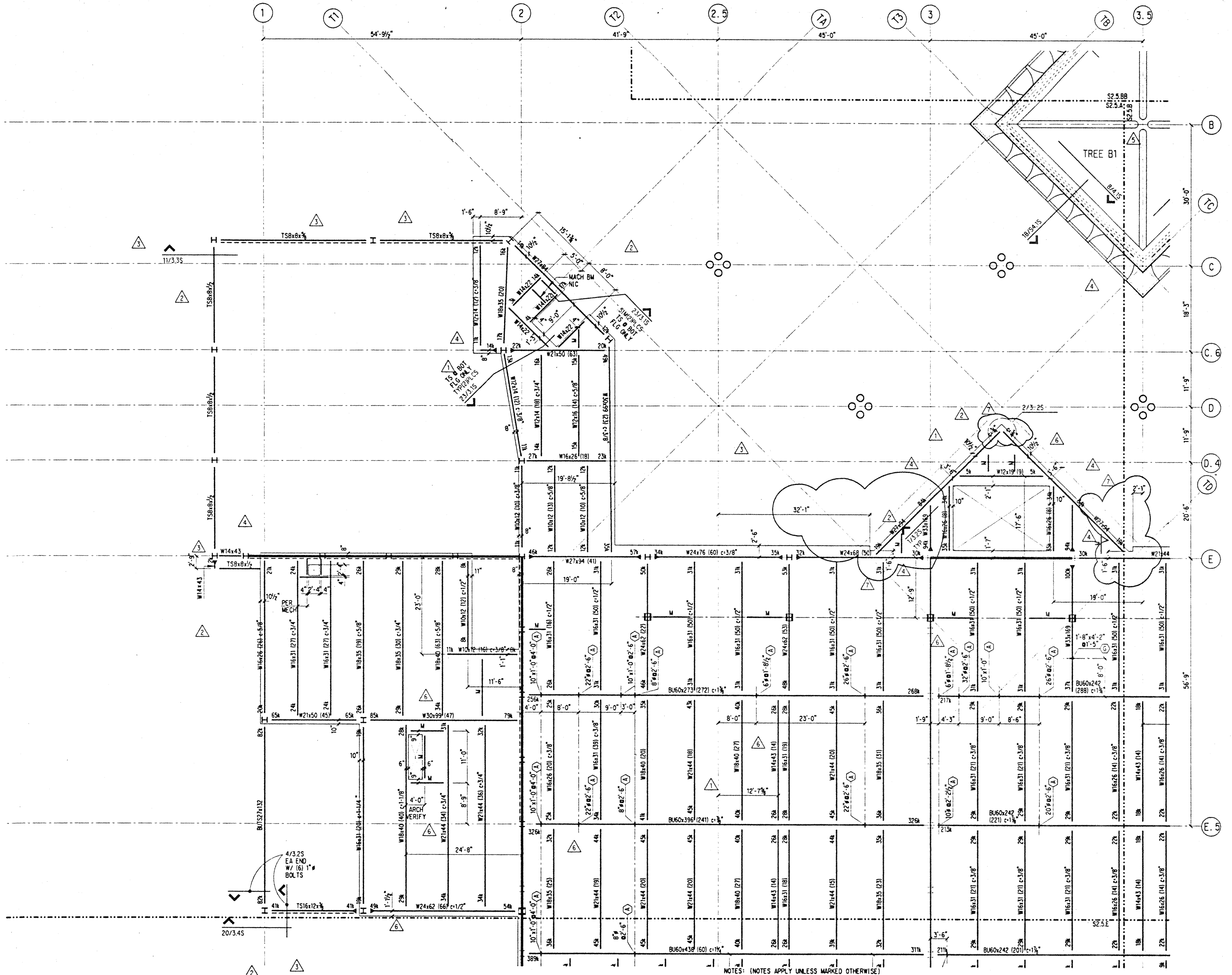
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9-20-95			BUILDING PERMIT SET
8-31-95			
8-14-95			
6-9-95			
5-8-95			ADDENDUM #3

Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR A

Designed **BJM**
Drawn **DMS**
Checked **JDM**
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

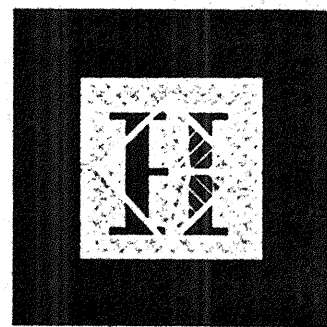


- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- REFERENCE DRAWINGS:
S0. • SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
S1. • TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
S1.20 CONCRETE SCHEDULES
S1.21 S1.22 STEEL SCHEDULES
S2. • PLANS
S3. • TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
S4. • BRACED FRAME ELEVATIONS
S5. • TRUSS ELEVATIONS
S6. • BRACED FRAME & TRUSS DETAILS
DETAIL BOOK STRUCTURAL DETAILS
 - REFERENCE FLOOR ELEVATION + 77'-6"
 - TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
 - REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION, UNO.
 - WHERE NOTED THUS (+/- 8"), TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
 - STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
 - REINFORCE SLAB WITH W# 6X6 - W2.9 X W2.9, UNO.
 - "M" DENOTES W10X12 BEAM, FY-50 KSI, MIN. STUDS.
 - INDICATES BEAM PENETRATION, SEE S1.10
 - INDICATES HAUNCHED BEAM, SEE S1.10
 - REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

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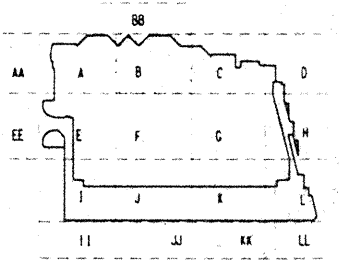
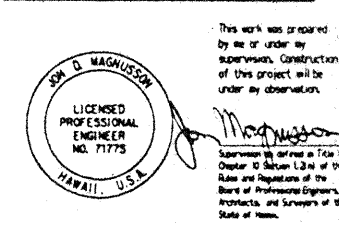
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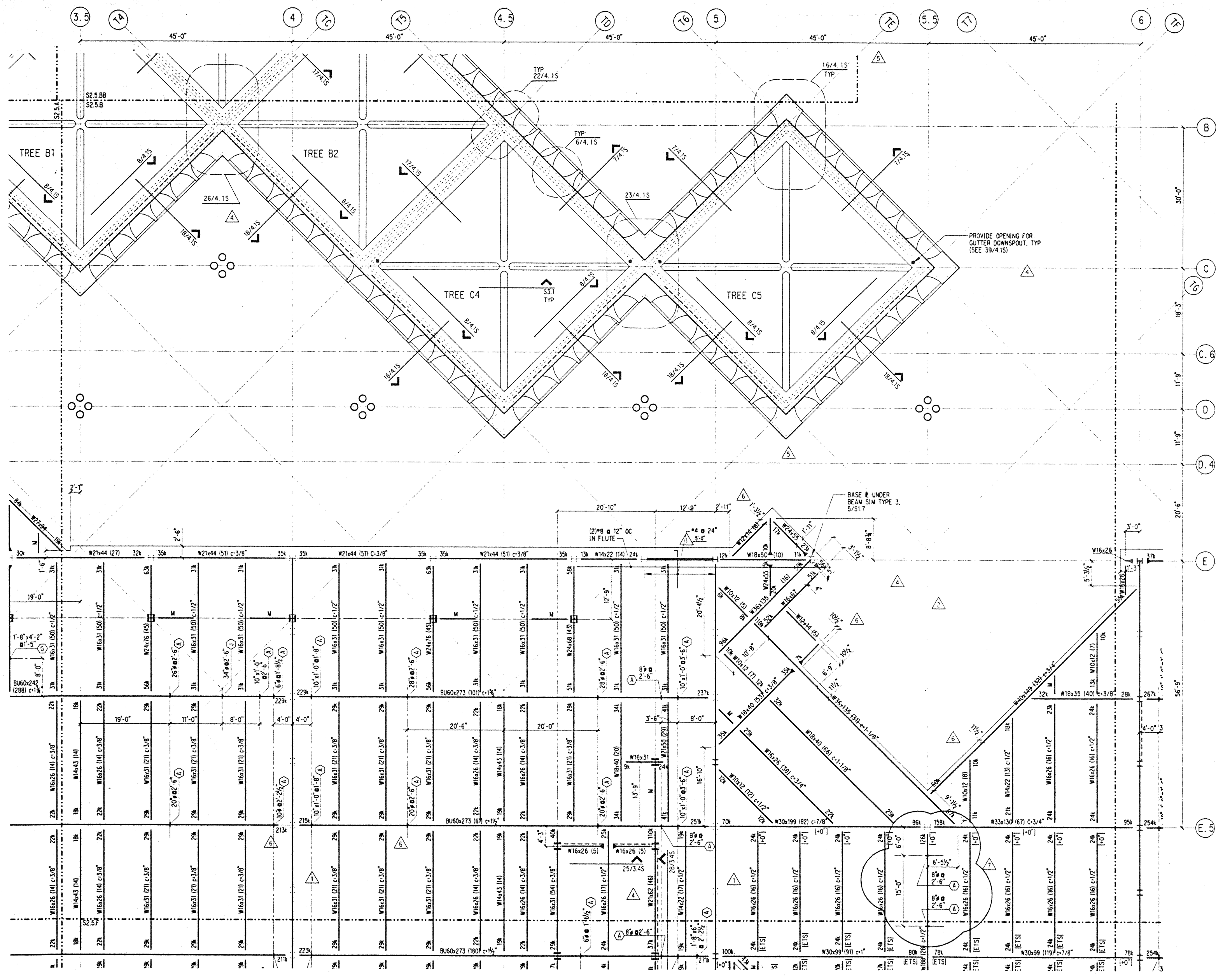


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2	11-7-95		TREE REVISIONS
3	9-20-95		BUILDING PERMIT SET
4	8-31-95		REVISION #3
5	6-19-95		REVISION #2
6	5-8-95		REVISION #1

BALLROOM/TERRACE FRAMING PLAN SECTOR B

Designed JJM
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 Checked JJM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.5.B

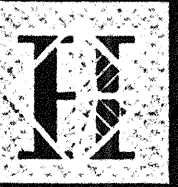


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.5.A

Thu Nov 30 11:27:46 1995

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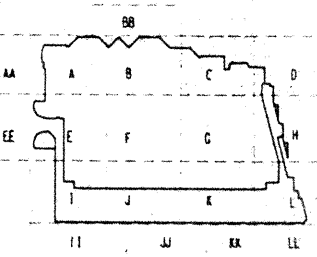
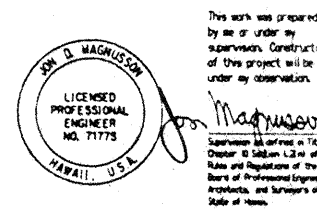
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11-7-95			BUILDING PERMIT SET
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8-31-95			
6-19-95			
5-8-95			ADDENDUM #3

Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR C

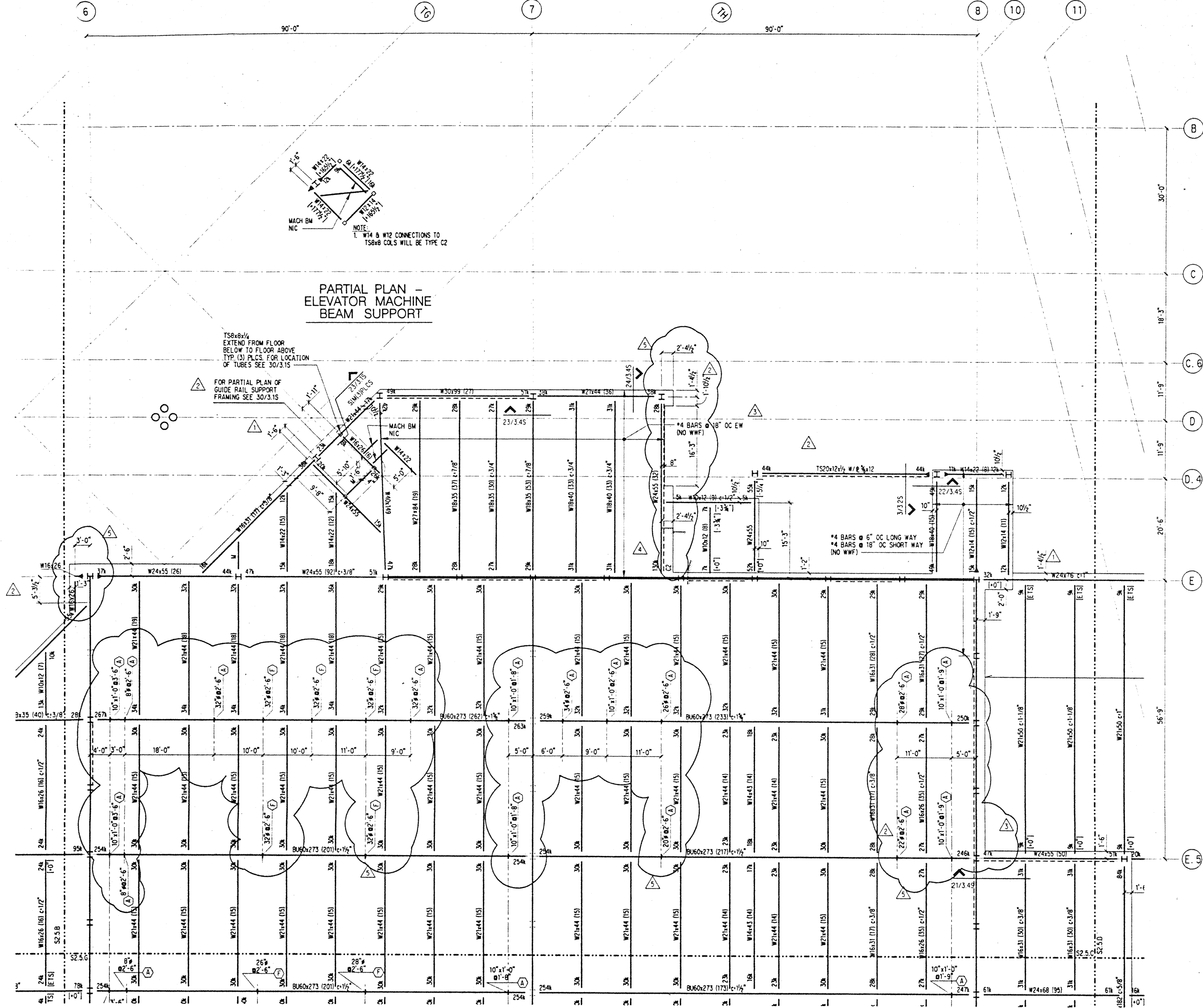
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 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036, 02, 9
 Sheet No.

S2.5.C

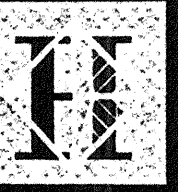
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11/30/95



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.5.A



Hawai'i Convention Center

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Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Neshom
Seattle, Washington

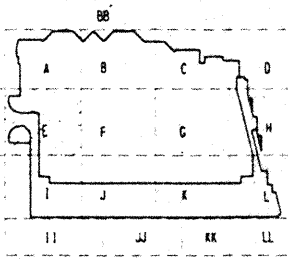
NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKELLING WARD MAGNUSON BAKERBEE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2999
(206) 292-1200 FAX: (206) 292-1201

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

SKELLING WARD MAGNUSON BAKERBEE INC.
LICENSED PROFESSIONAL ENGINEER
NO. 71778
HAWAII, U.S.A.



Revisions

No.	Date	Description
12-15-95		FINAL DOCUMENT
11-7-95		BUILDING PERMIT SET
9-15-95		
8-31-95		
8-4-95		
6-19-95		
5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR E

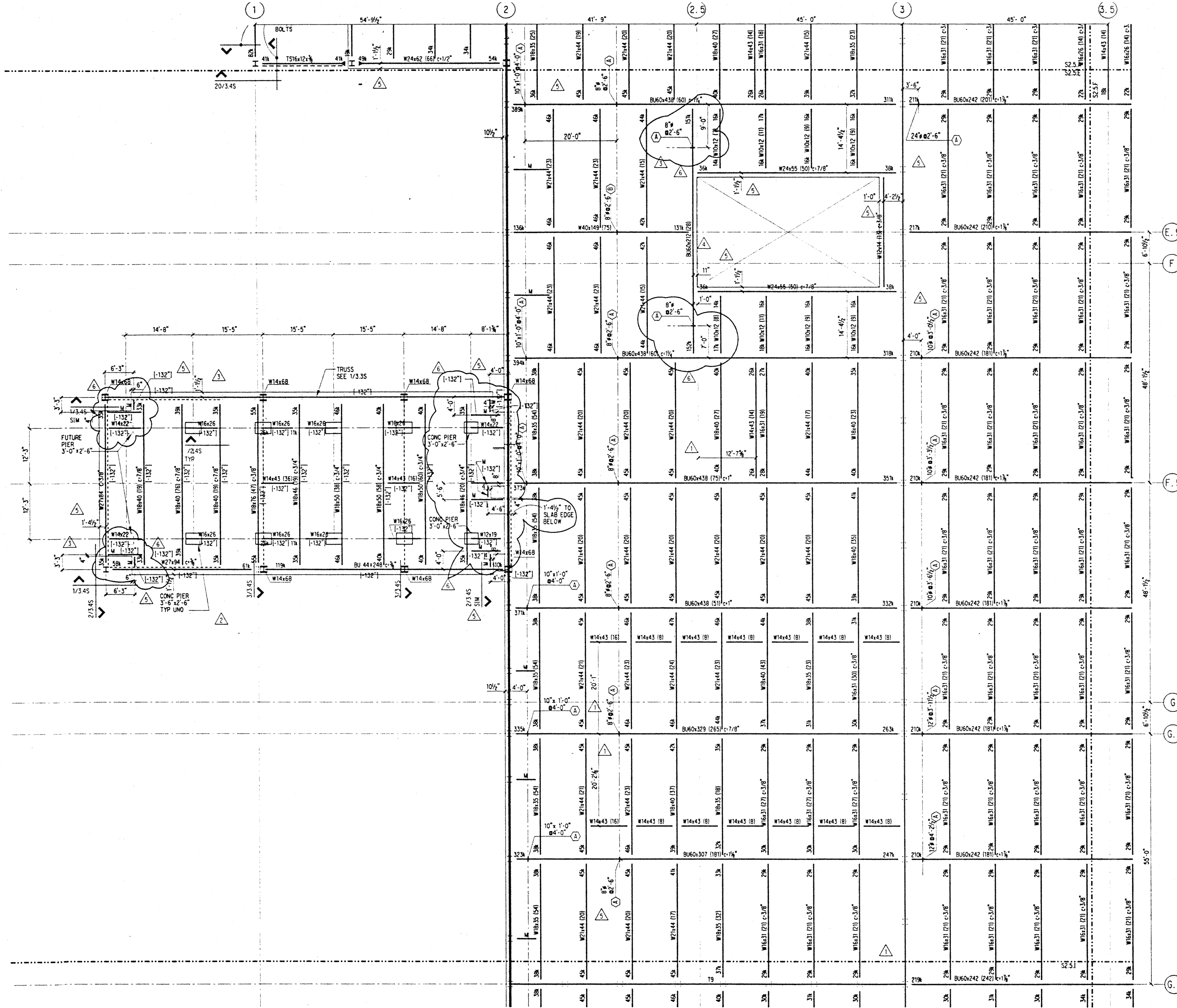
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 Date 4 / 12 / 95
 Job No. 93036, Q2.9
 Sheet No.

S2.5.E

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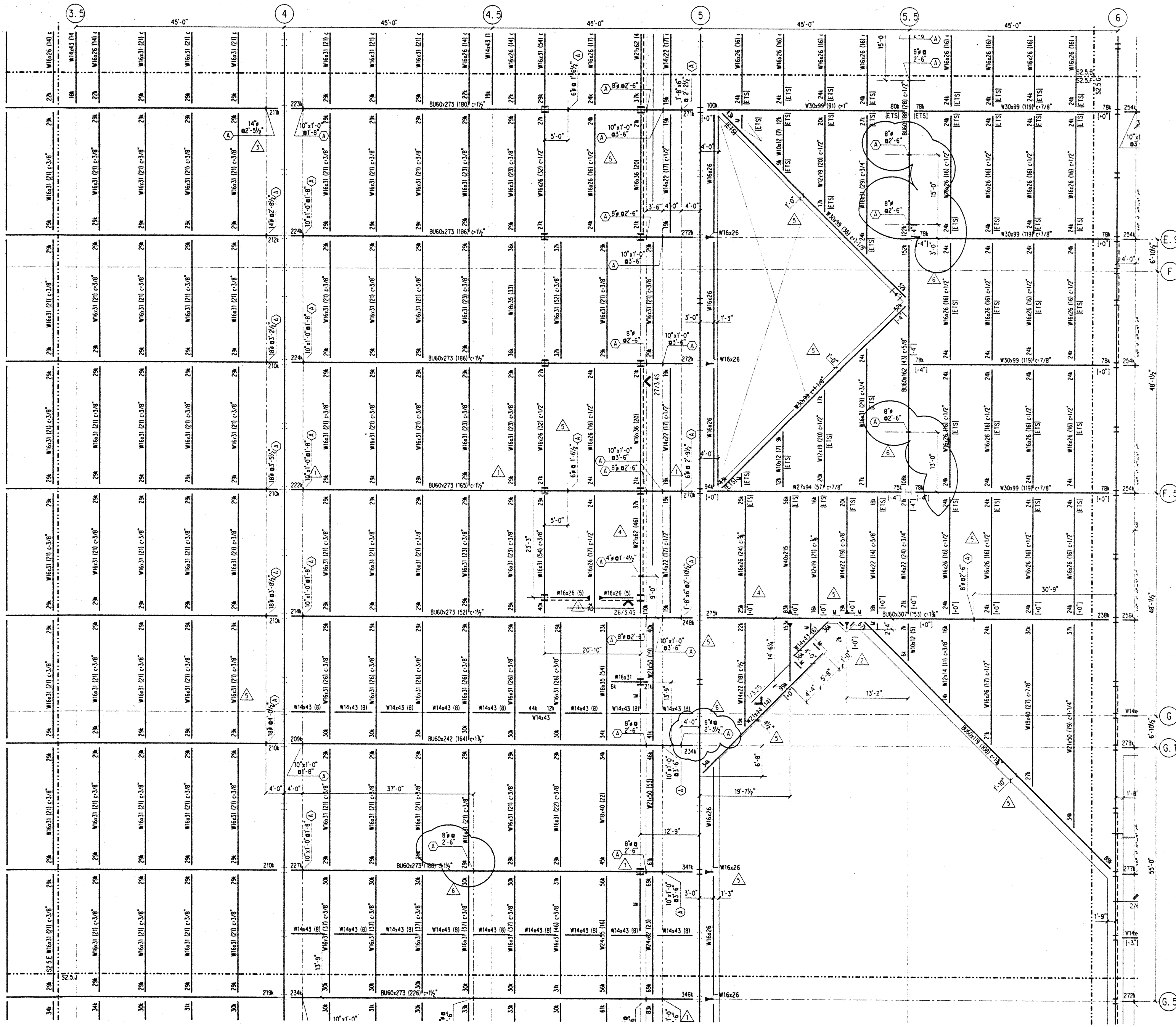
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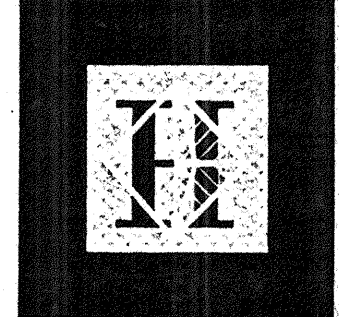
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.5.A



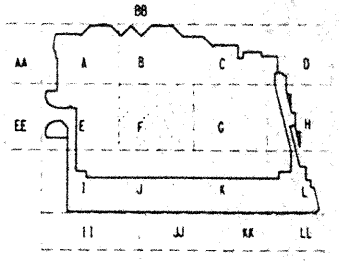
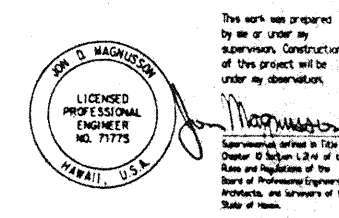
Hawai'i Convention Center

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SKIDMORE OWINGS & MERRILL LLP
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 1301 Fifth Avenue, Suite 3300 Seattle WA 98101-3599
 (206) 292-1200 FAX: (206) 292-1001

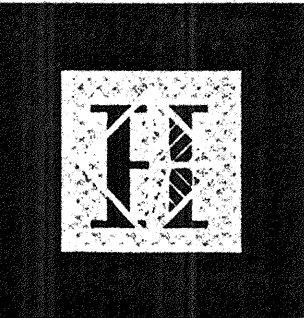


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3	9-15-95		BUILDING PERMIT SET
4	8-31-95		
5	8-4-95		
6	6-19-95		
7	5-8-95		ADDENDUM #3

BALLROOM/TERRACE FRAMING PLAN SECTOR F

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 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.5.F



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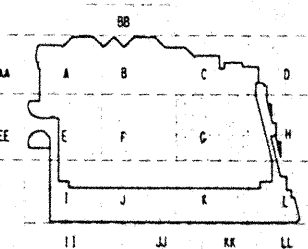
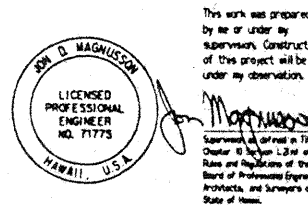
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Consulting Structural and Civil Engineers

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206 425-1200 FAX 206 425-2501



Revisions

No.	Date	By	Description
1	12-15-95	JDM	FINAL DOCUMENT
2	11-7-95	JDM	BUILDING PERMIT SET
3	8-4-95	JDM	ADDENDUM #3
4	5-8-95	JDM	ADDENDUM #3

Sheet Title

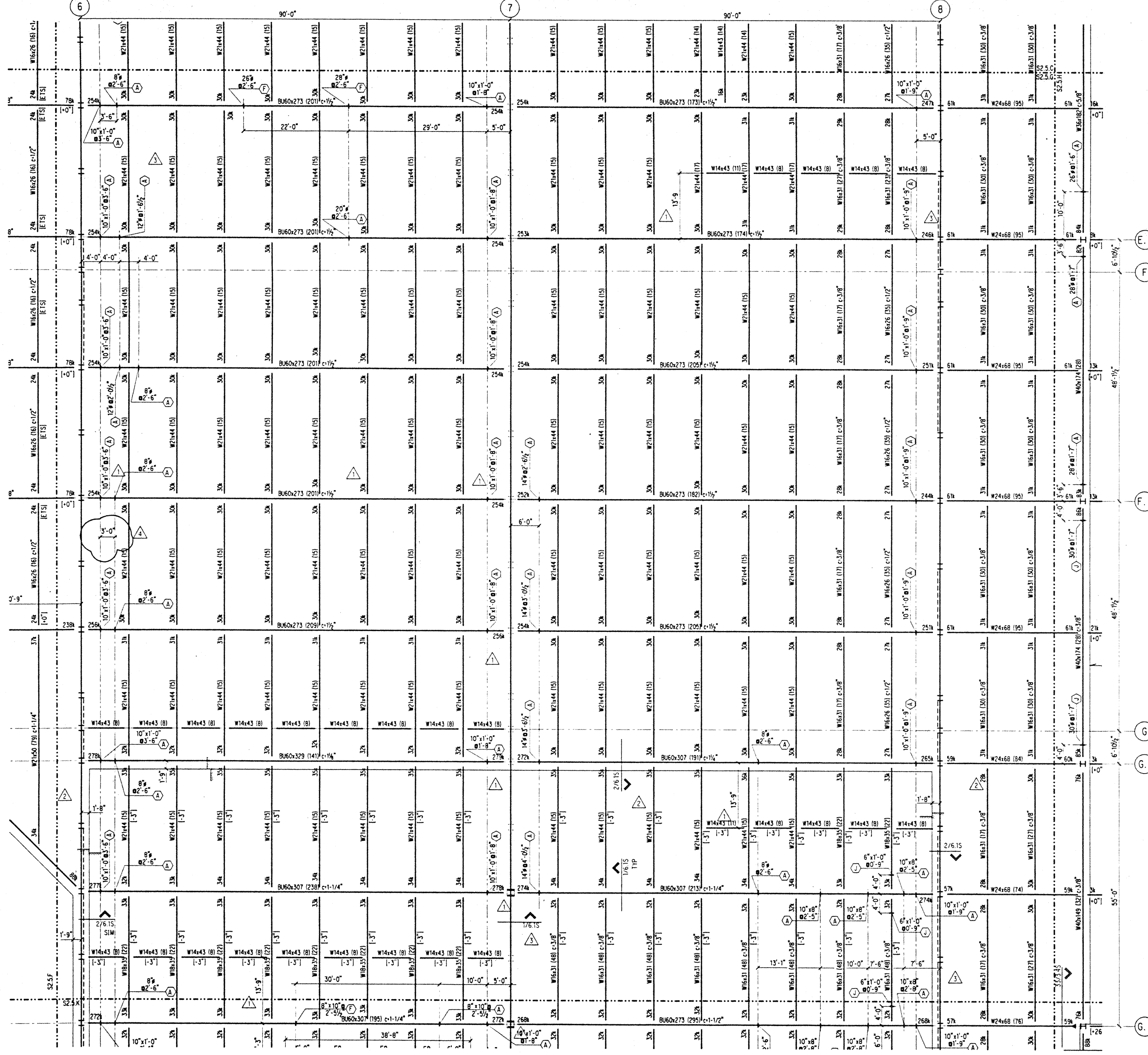
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Designed JDM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

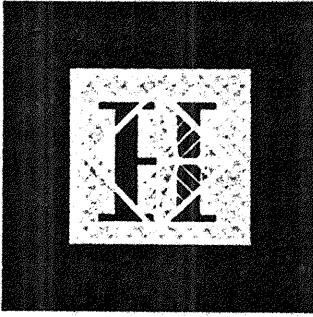
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
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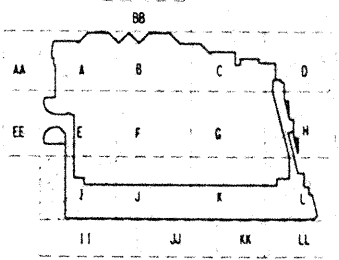
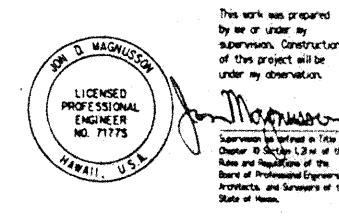
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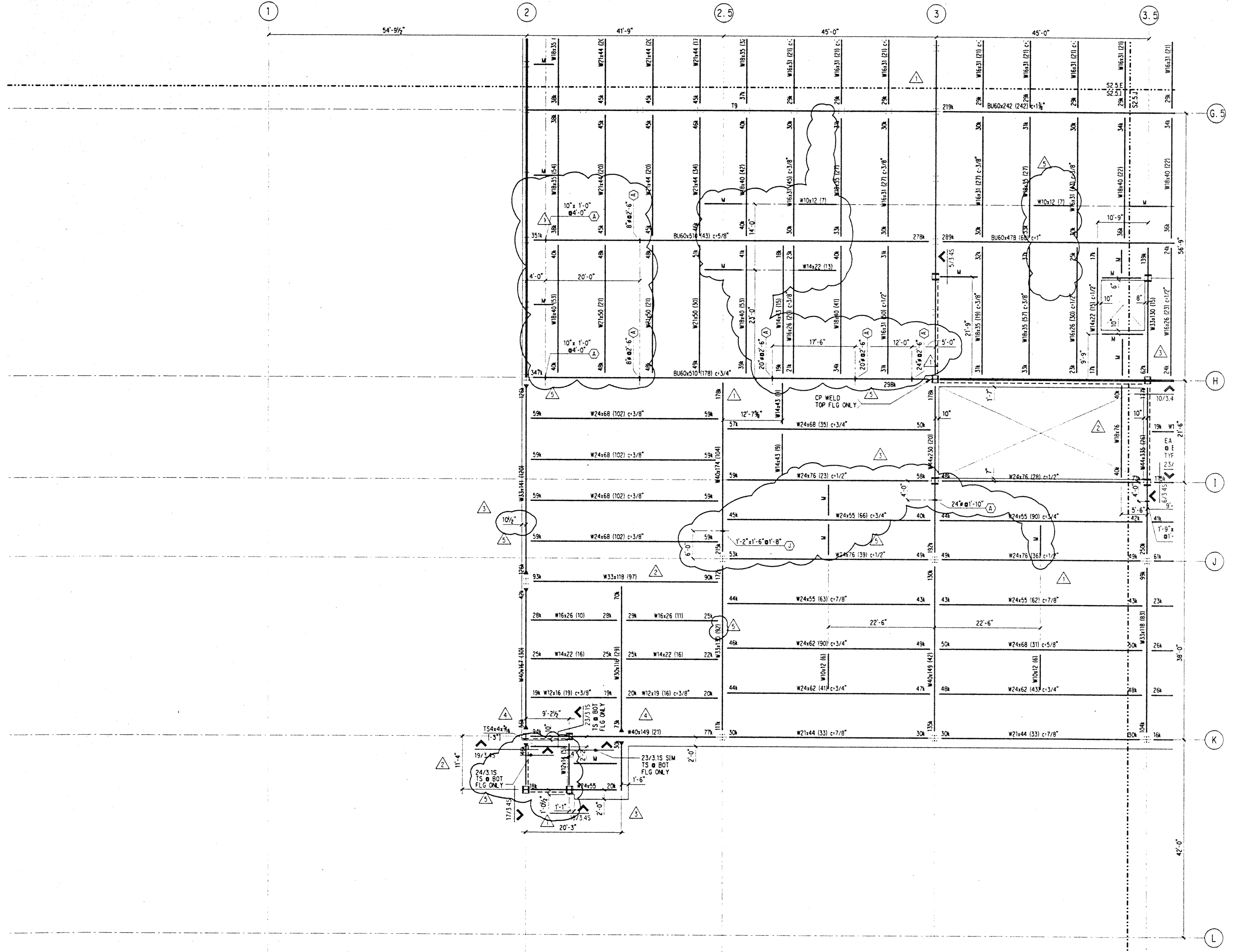


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8-4-95			
6-19-95			
5-8-95			ADDENDUM #3

BALLROOM/TERRACE FRAMING PLAN SECTOR I

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036_Q2_9
 Sheet No.

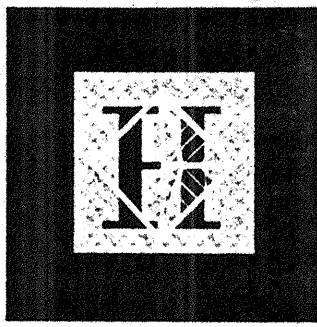


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
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11/15/95



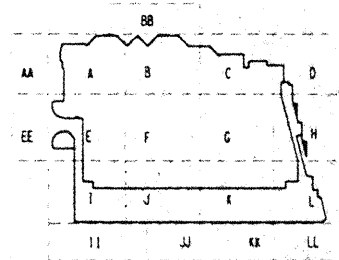
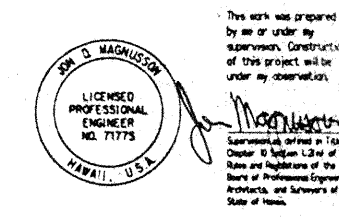
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No.	Date	By	Description

Sheet Title

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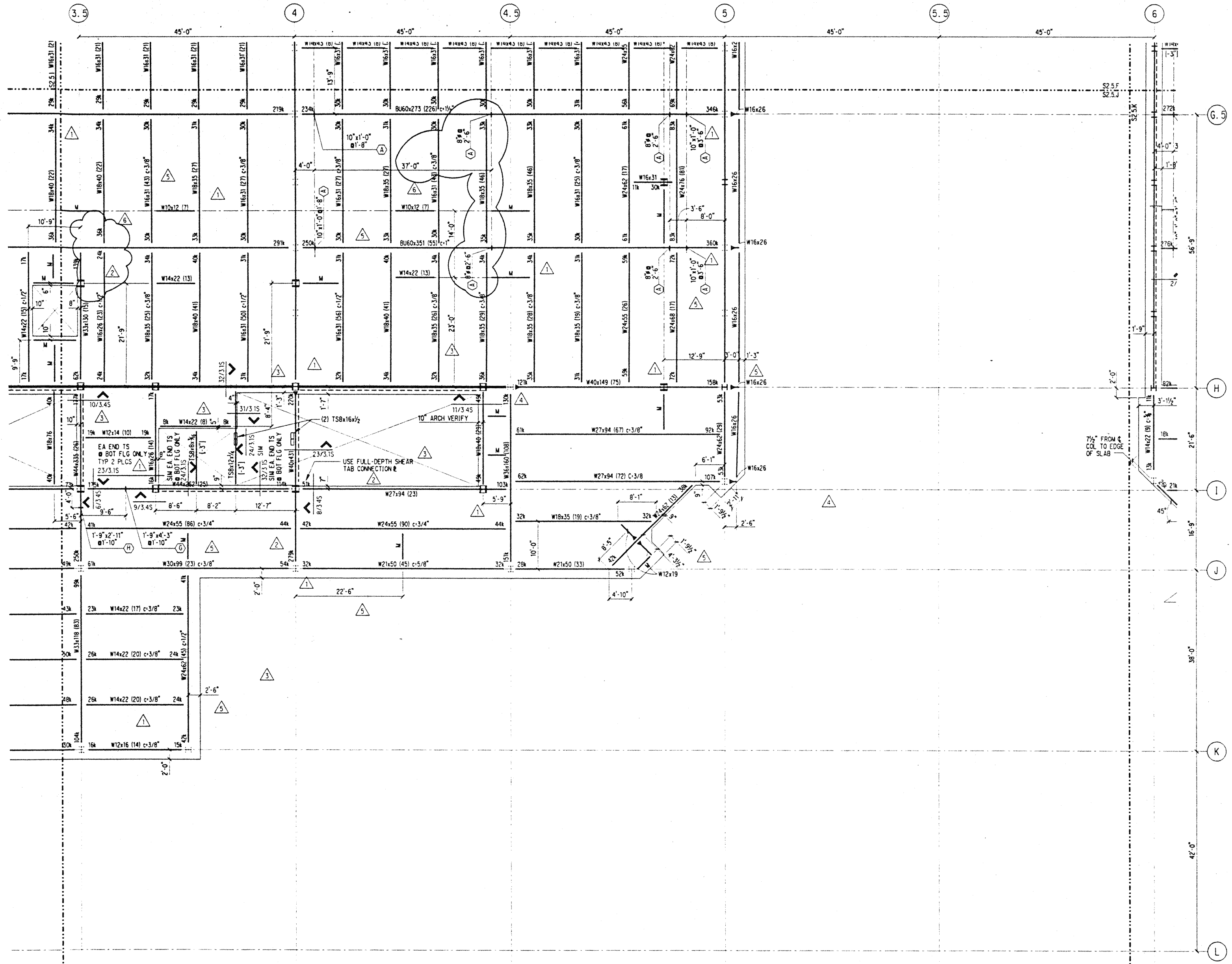
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 Checked **JDM**
 Date **4 / 12 / 95**
 Job No. **93036.02.9**
 Sheet No.

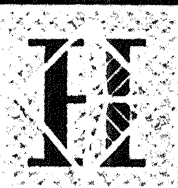
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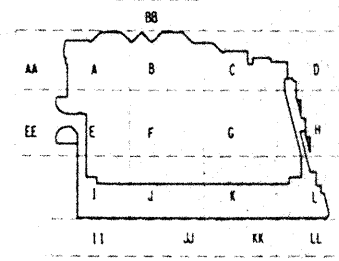
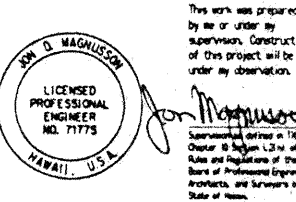
Hawai'i Convention Center

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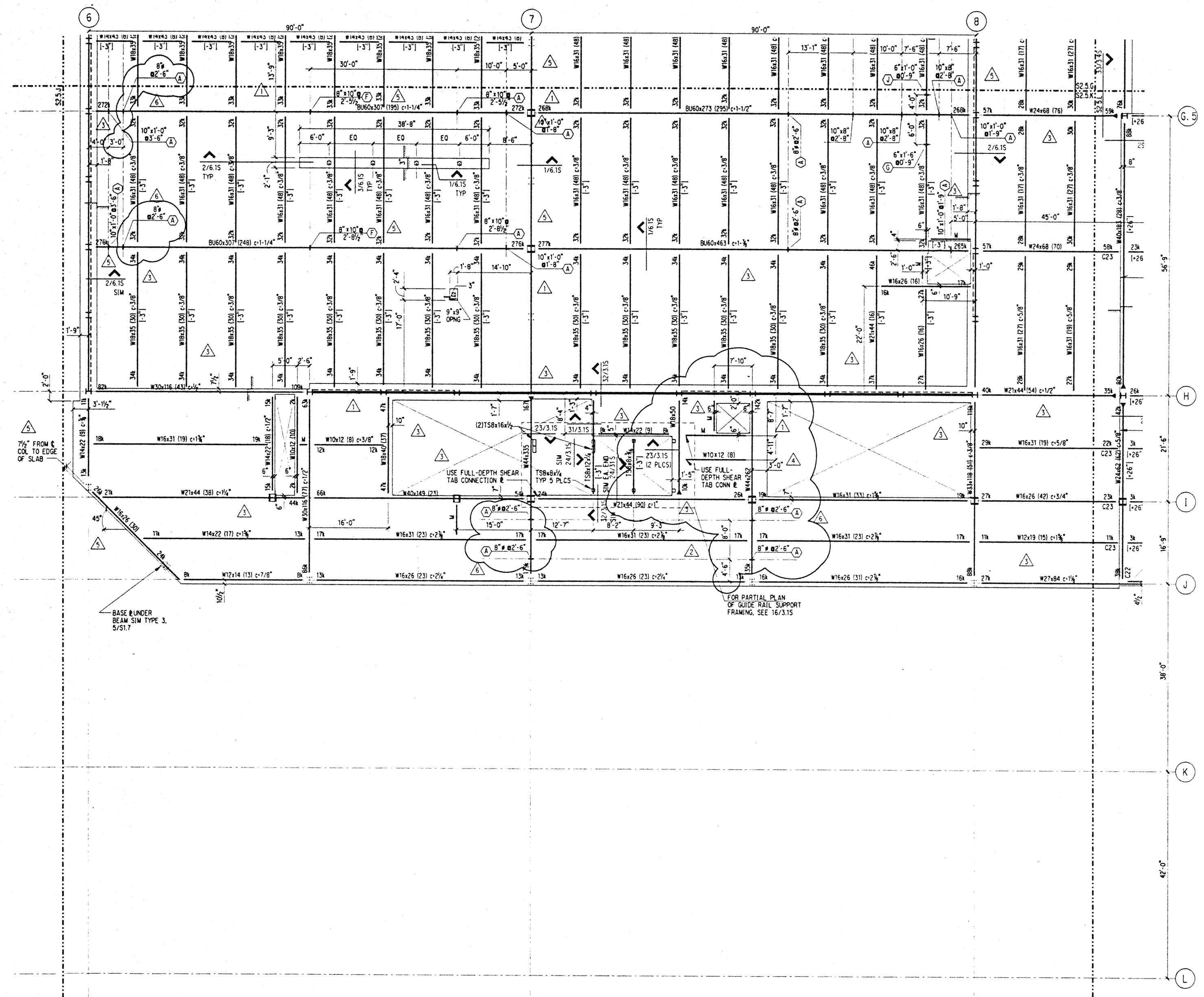
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Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR K

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 Checked JCM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.5.K

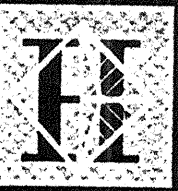


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.5.A

Thu Nov 30 11:49:47 1995

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Seattle, Washington

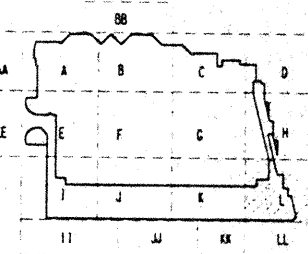
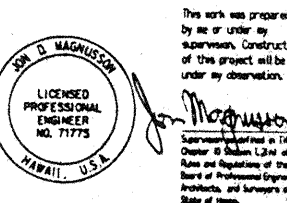
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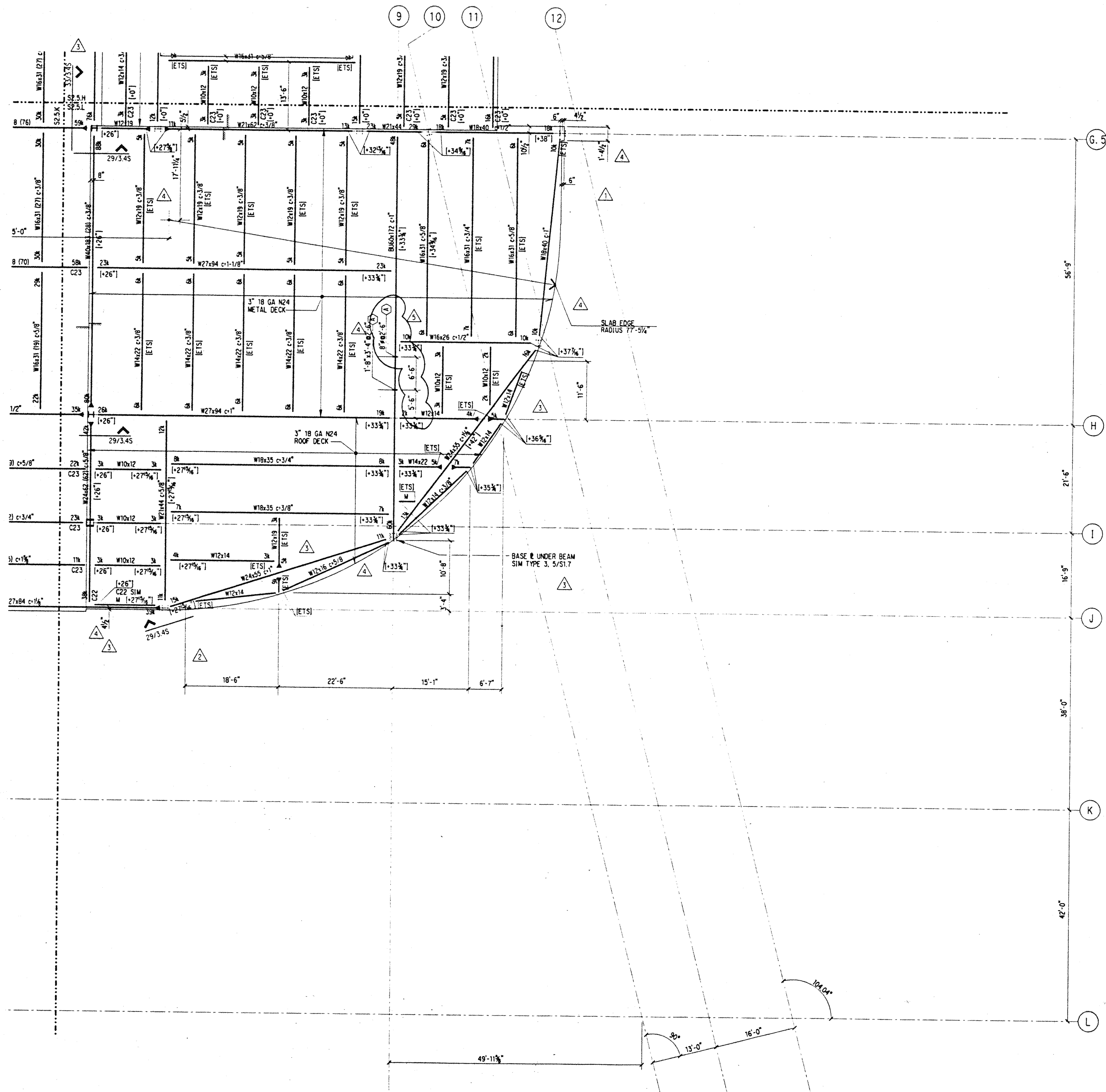
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8-4-95			
5-8-95			REVISION #3

Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR L

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.5.L

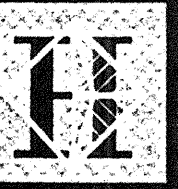


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.5.A

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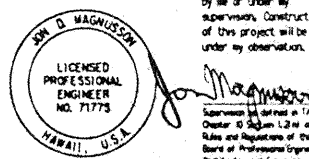
Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

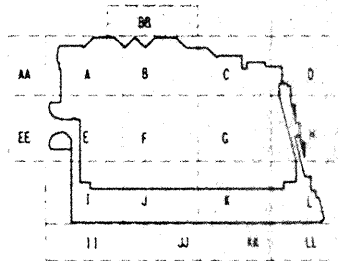
Consultants

SKILLING WARD MANNING BANKS INC.

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1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2999
(206) 292-1200 FAX (206) 292-1201



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Revisions

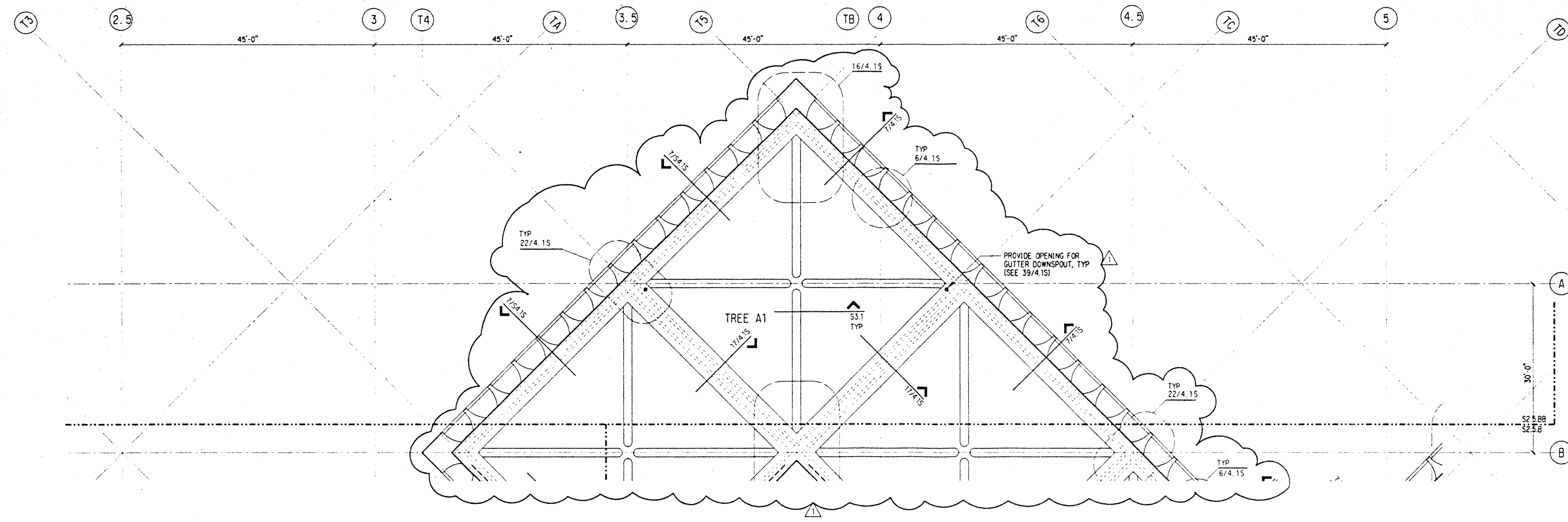
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Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR BB

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Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.5.BB

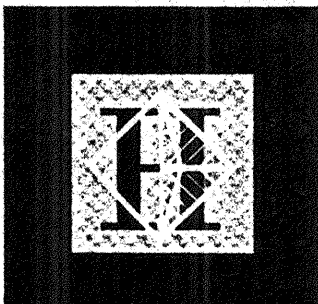


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.5.A

Thu Nov 30 17:54:41 1995

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Honolulu, Hawaii
in association with

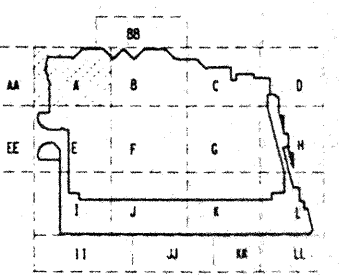
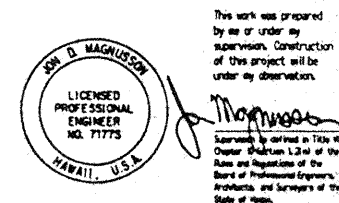
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Seattle, Washington

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(206) 392-1200 FAX: (206) 623-1201

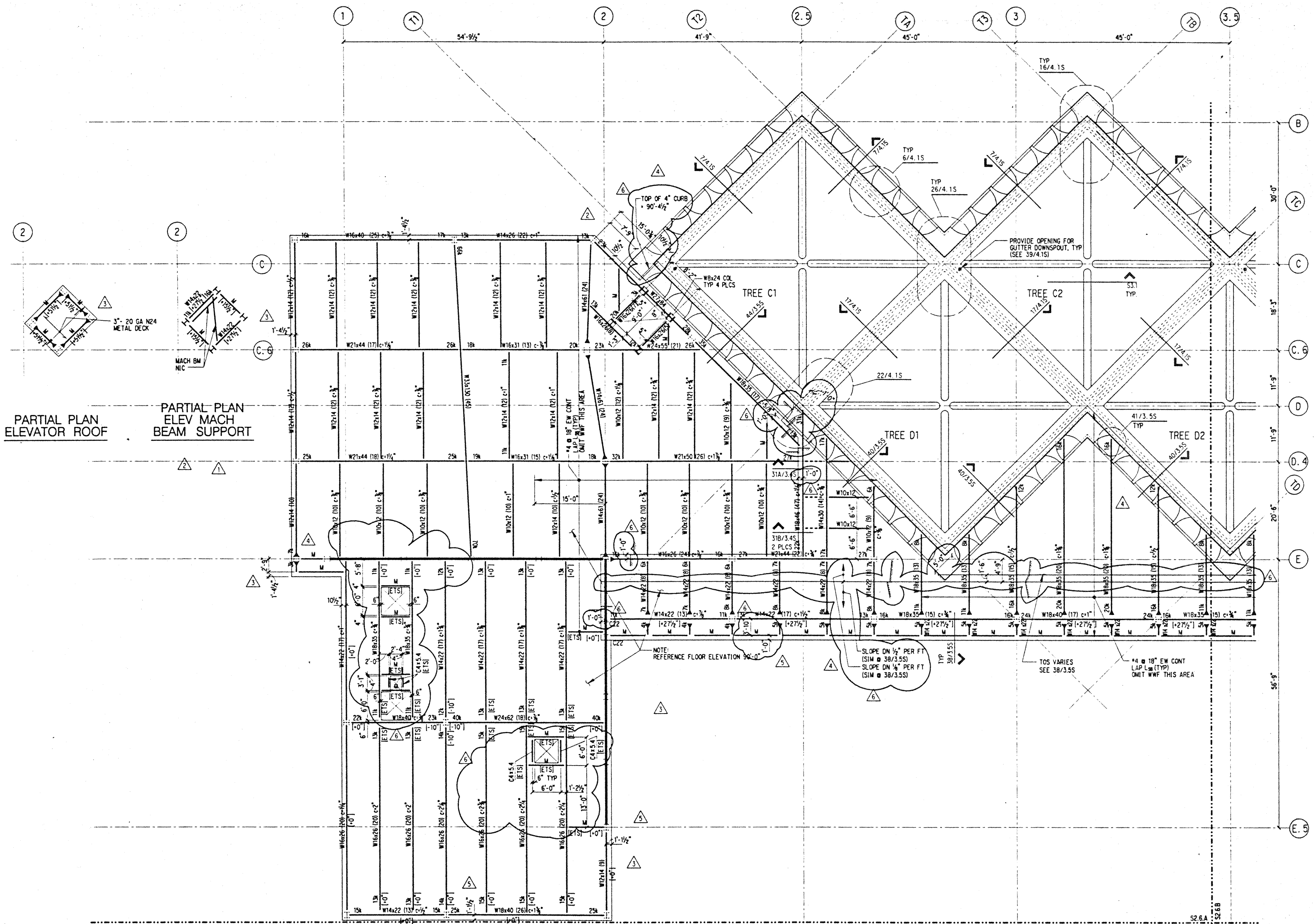


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4	8-14-95	JOM	BUILDING PERMIT SET
5	6-19-95	JOM	ADDENDUM #4
6	5-8-95	JOM	ADDENDUM #4

MECHANICAL PENTHOUSE FRAMING PLAN SECTOR A

Designed **BUM**
 Drawn **DMS**
 Checked **JOM**
 Date **4 / 12 / 95**
 Job No. **93036, 02, 9**
 Sheet No.

S2.6.A



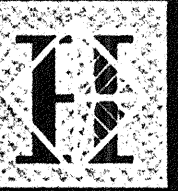
PARTIAL PLAN ELEVATOR ROOF
PARTIAL PLAN ELEV MACH BEAM SUPPORT

- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- REFERENCE DRAWINGS:
 S0. * SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
 S1. * TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
 S1.20 CONCRETE SCHEDULES
 S1.21 S1.22 STEEL SCHEDULES
 S2. * PLANS
 S3. * TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
 S4. * BRACED FRAME ELEVATIONS
 S5. * TRUSS ELEVATIONS
 S6. * BRACED FRAME & TRUSS DETAILS
 DETAIL BOOK STRUCTURAL DETAILS
 - REFERENCE FLOOR ELEVATION = 97'-0" UNO.
 - TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
 - ROOF DECK, 3" 20 GAGE, N24 METAL DECK, UNO.
 -
 - REFERENCE TOP OF STRUCTURAL STEEL IS 5'-1/2" BELOW REFERENCE ELEVATION AT SLAB AND ROOF DECK LOCATIONS, UNO.
 - WHERE NOTED THUS (+/- 8"), TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
 - STRUCTURAL SLAB IS 2'-1/2" OF HARDBOOK CONCRETE ON 3" STEEL DECK, UNO.
 - REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
 - "M" DENOTES W10X12 BEAM, Fy-50 KSI, MIN. STUDS.
 - INDICATES BEAM PENETRATION, SEE S1.10
 - INDICATES HAUNCHED BEAM, SEE S1.10
 - REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

Thu Nov 30 16:26:51 1995

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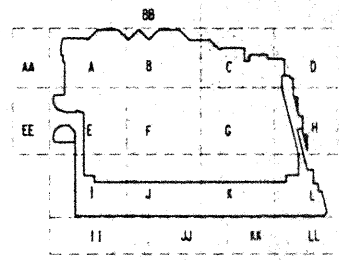
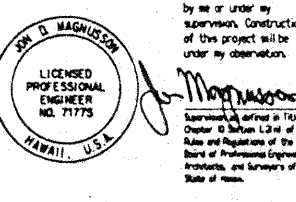
Hawai'i Convention Center

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Seattle, Washington

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Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 2200 Seattle WA 98101-2699
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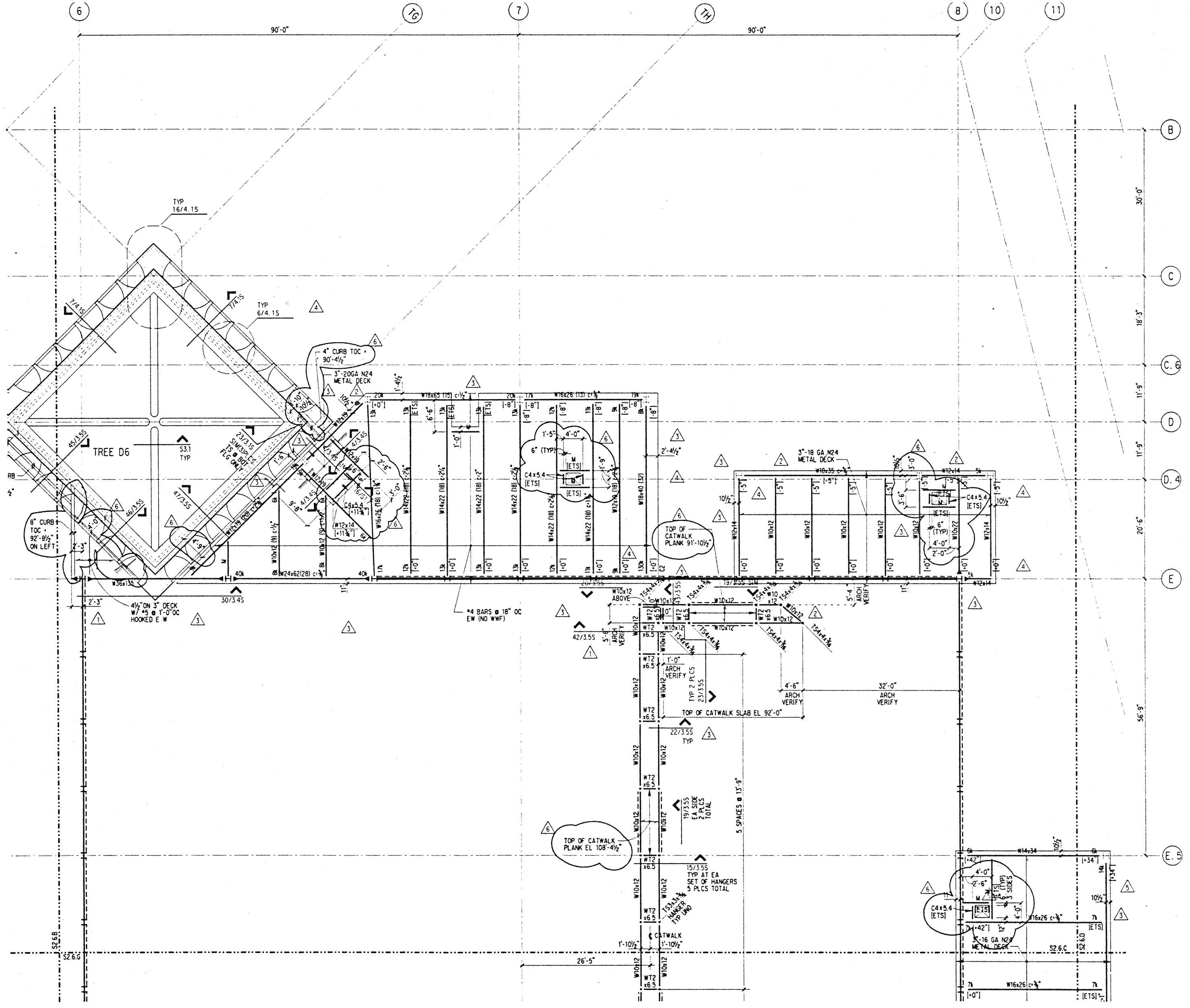
Revisions

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11-7-95		BUILDING PERMIT SET
8-31-95		
8-4-95		
6-19-95		
5-8-95		ADDENDUM #3

Sheet Title
**MECHANICAL
PENTHOUSE
FRAMING PLAN
SECTOR C**

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.6.C

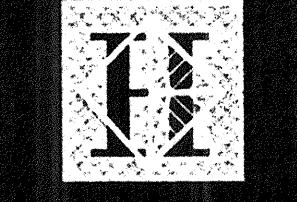


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.6.A
 2. REFERENCE FLOOR ELEVATION + 93'-6"

Thu Nov 30 16:32:20 1995

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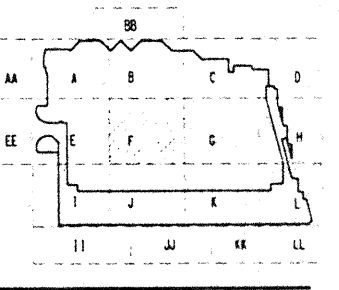
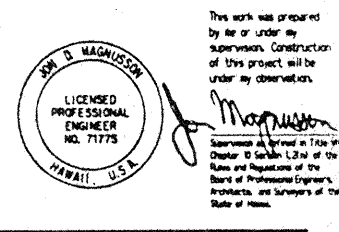
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1801 Fifth Avenue, Suite 2000 Seattle, WA 98101-2999
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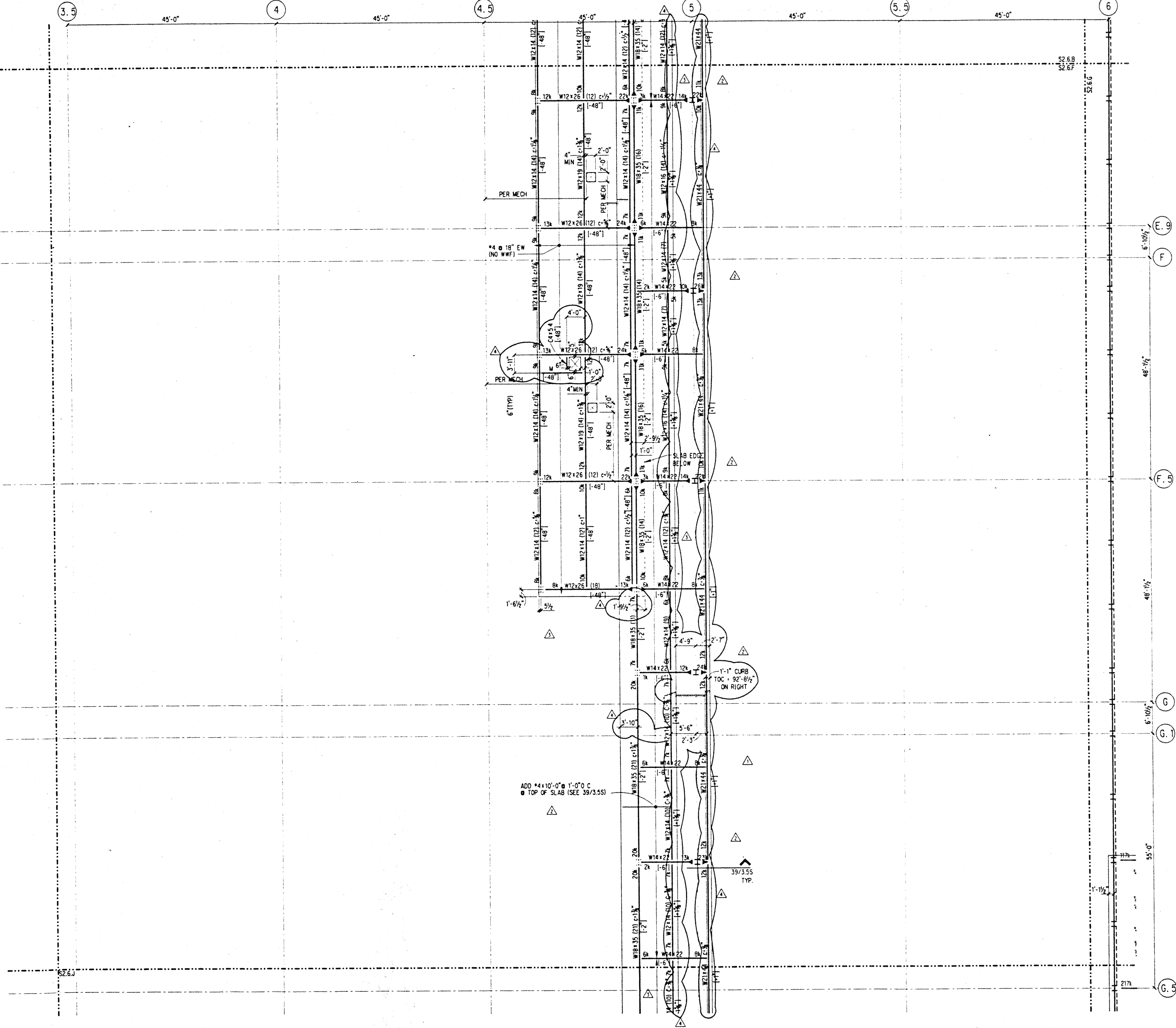
Revisions

No.	Date	Description
△	02-15-95	FINAL DOCUMENT
△	11-2-95	
△	9-15-95	BUILDING PERMIT SET
△	8-31-95	
△	8-4-95	

Sheet Title

**MECHANICAL
PENTHOUSE
FRAMING PLAN
SECTOR F**

Designed	BLM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036, 02, 9
Sheet No.	



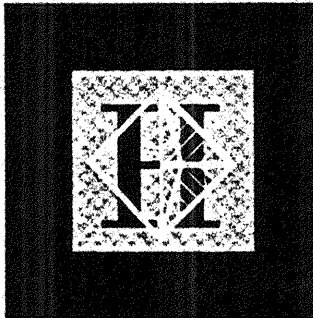
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- REFERENCE NOTES ON S2.6.A
- REFERENCE FLOOR ELEVATION = 92'-5 1/2" △

Thu Nov 30 16:34:45 1995

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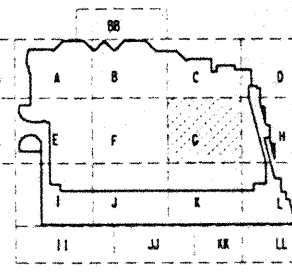
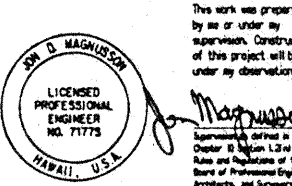
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No.	Date	By	Description
1	12-15-95		FINAL DOCUMENT
2	11-7-95		BUILDING PERMIT SET
3	9-4-95		ADDENDUM #3
4	6-19-95		
5	5-8-95		

Sheet Title
**MECHANICAL
PENTHOUSE
FRAMING PLAN
SECTOR G**

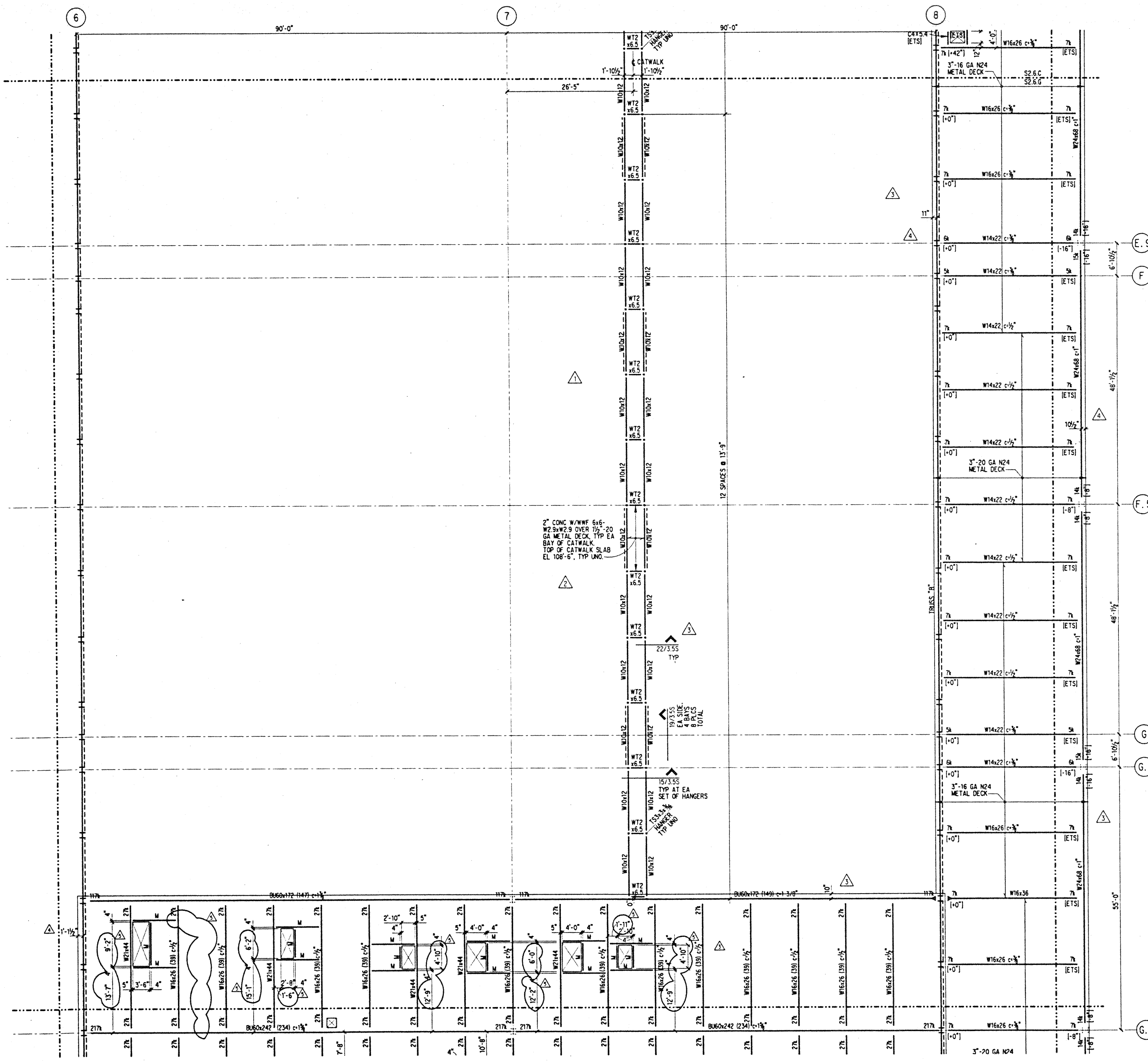
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Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.6.G

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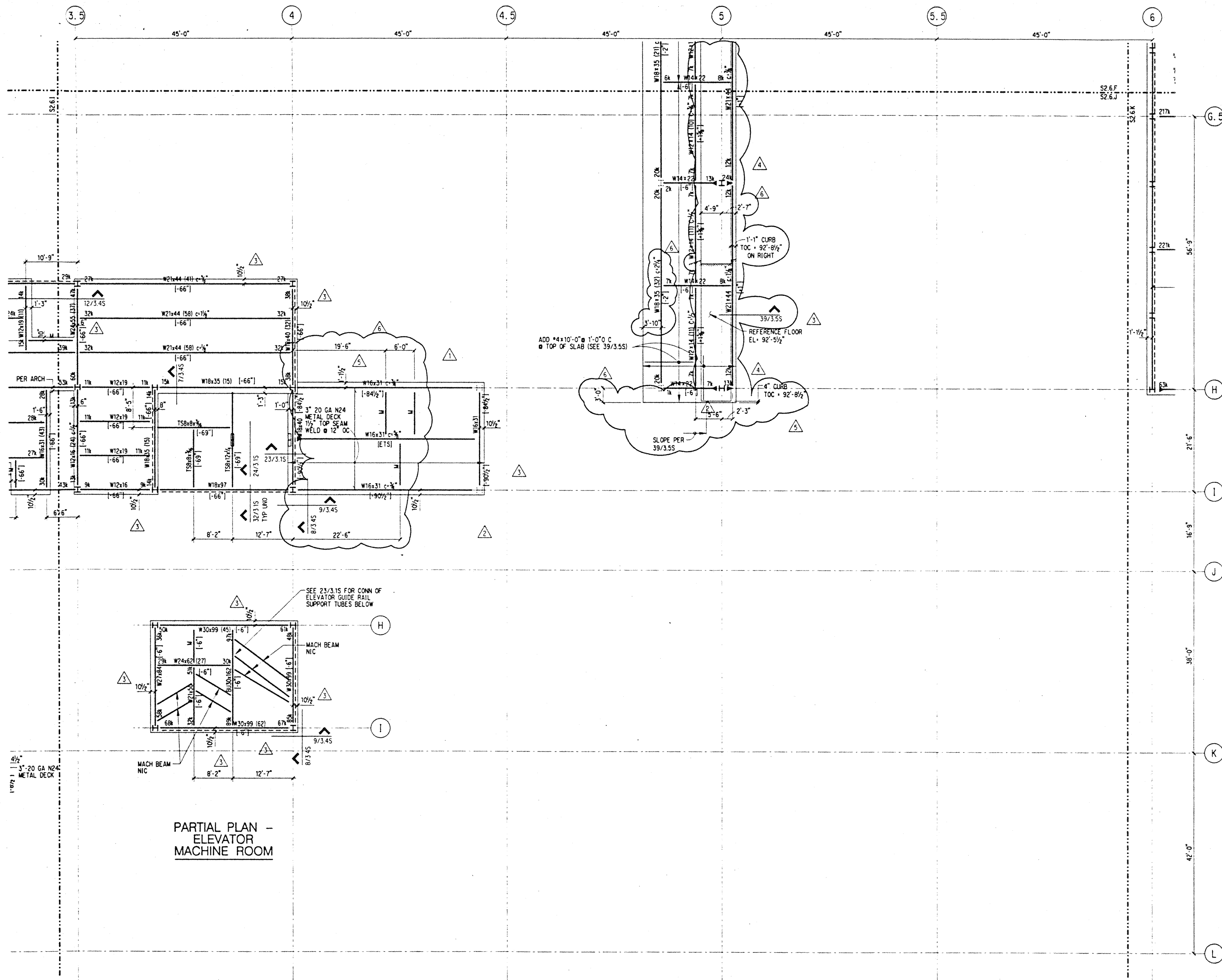


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.6.A

Thu Nov 30 16:41:45 1995

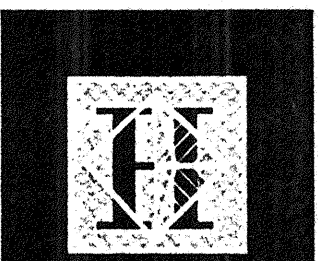
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/r, plt



**PARTIAL PLAN -
ELEVATOR
MACHINE ROOM**

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.6.A



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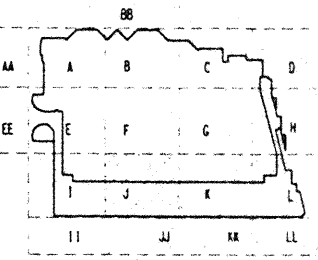
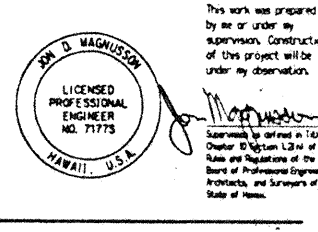
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1	12-15-95		FINAL DOCUMENT
2	11-7-95		
3	9-15-95		BUILDING PERMIT SET
4	8-31-95		
5	8-4-95		
6	6-19-95		
7	5-8-95		ADDENDUM #3

**MECHANICAL
PENTHOUSE
FRAMING PLAN
SECTOR J**

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 Date 4 / 12 / 95
 Job No. 93036.02.9
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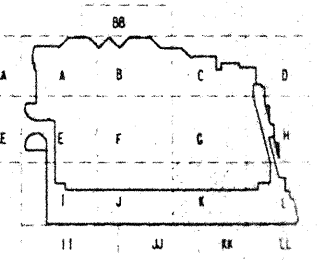
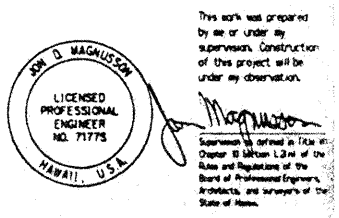
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Consulting Structural and Civil Engineers
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206 465-1200 FAX: 206 465-1001



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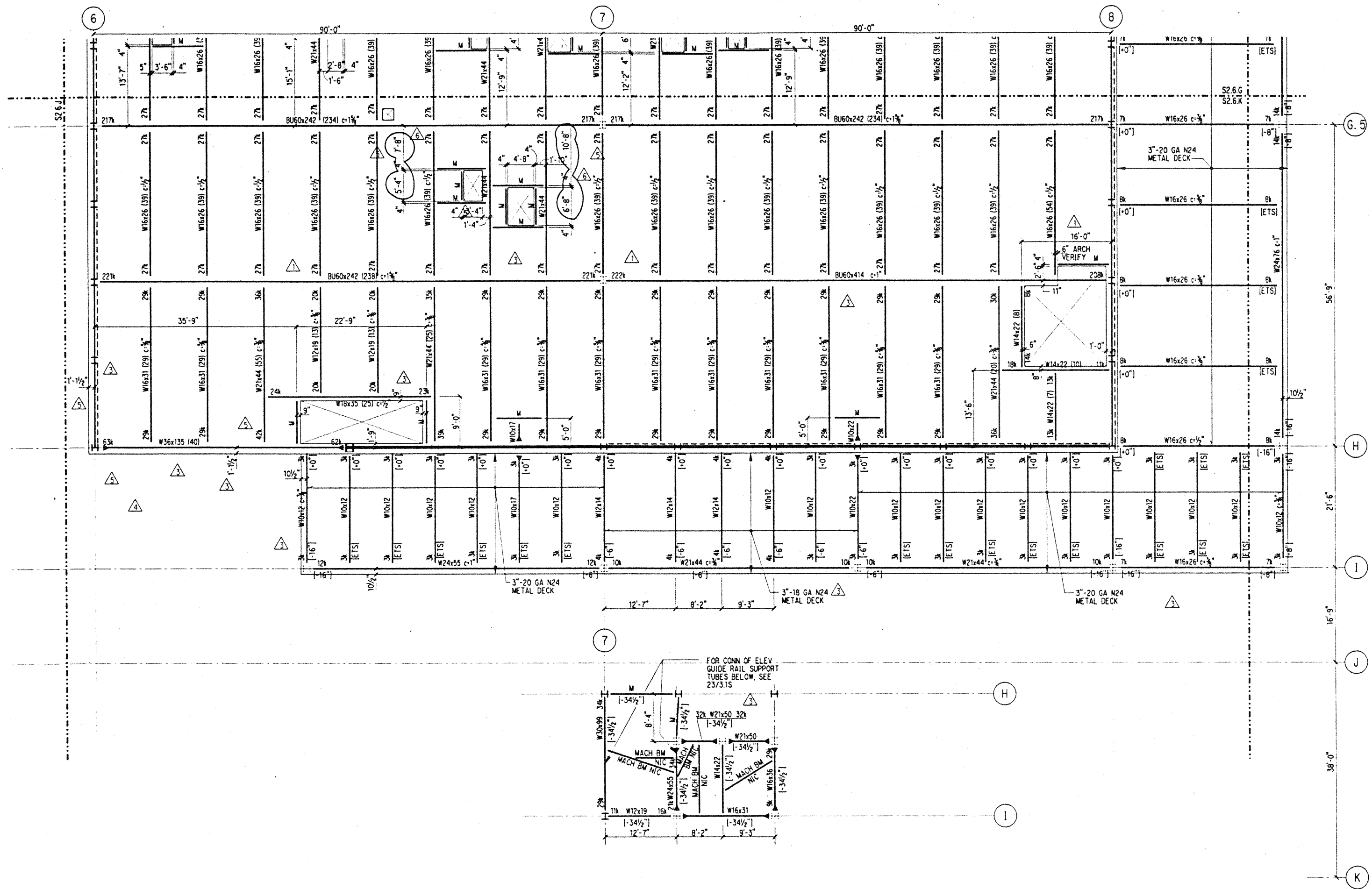
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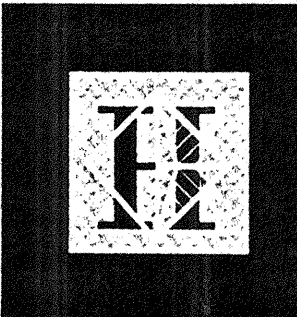
Sheet Title

MECHANICAL PENTHOUSE FRAMING PLAN SECTOR K

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.6.A





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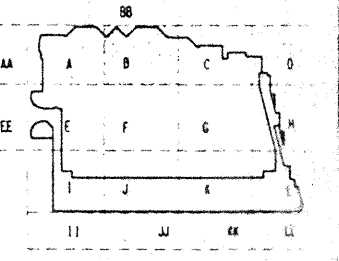
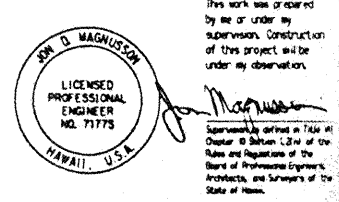
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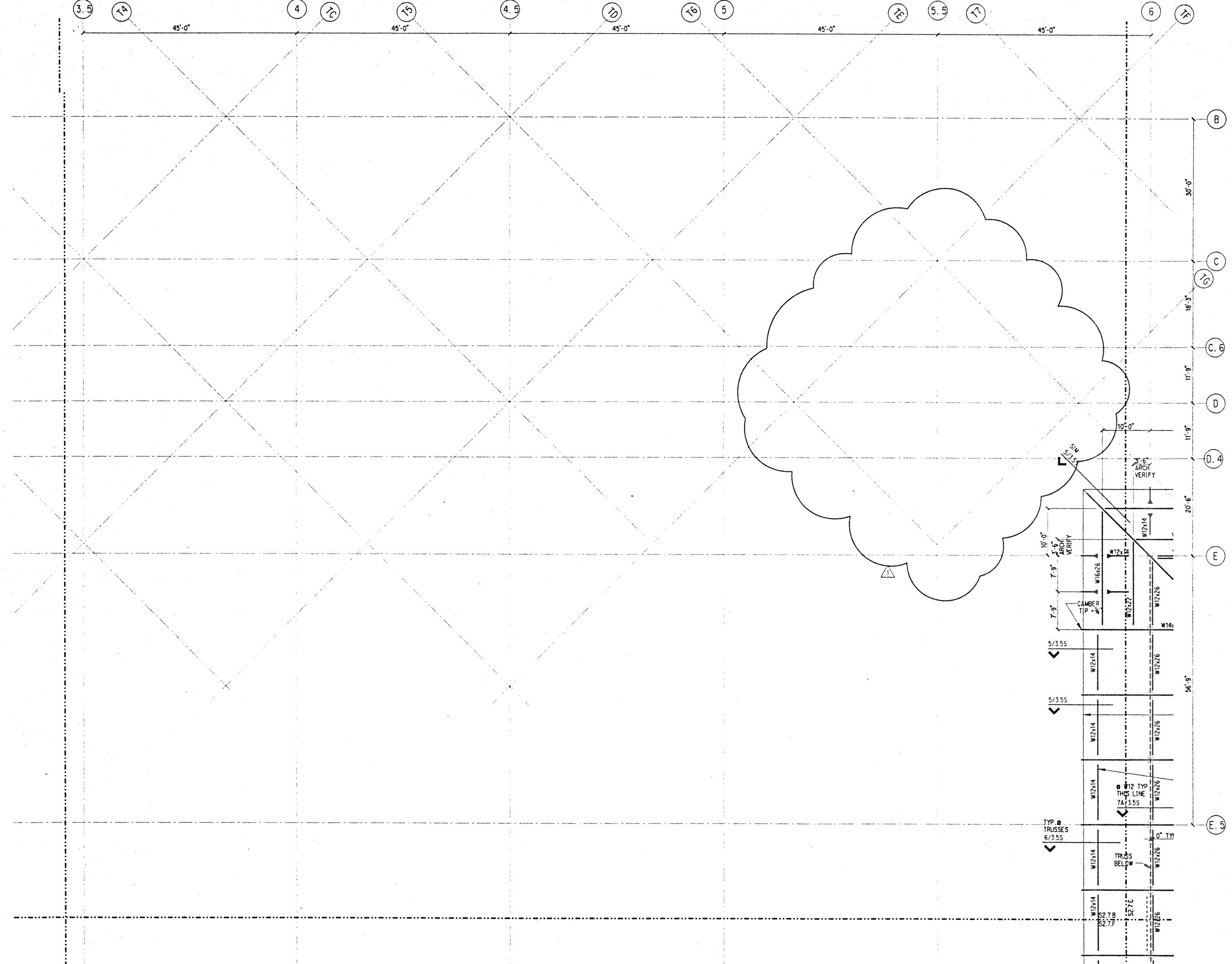
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12-15-95			FNL DOCUMENTS
9-15-95			BUILDING PERMIT SET
8-4-95			
No.	Date	By	Description

ROOF FRAMING PLAN SECTOR B

Designed BJM
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 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.

S2.7.B



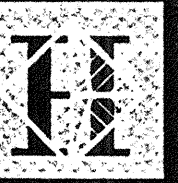
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- REFERENCE DRAWINGS:
 S0.* SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
 S1.* TYPICAL FOUNDATION, CONCRETE, STEEL AND CMJ DETAILS
 S1.20 CONCRETE SCHEDULES
 S1.21 S1.22 STEEL SCHEDULES
 S2.* PLANS
 S3.* TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
 S4.* BRACED FRAME ELEVATIONS
 S5.* TRUSS ELEVATIONS
 S6.* BRACED FRAME & TRUSS DETAILS
 DETAIL BOOK STRUCTURAL DETAILS
- REFERENCE FLOOR ELEVATION. SEE PLAN.
- ROOF DECK, 3", 20 GAGE, #24 METAL DECK, UNO.
- TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
- REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION AT SLAB AND ROOF DECK LOCATIONS, UNO.
- WHERE NOTED THIS [+/- 8"], TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
- STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
- REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
- "M" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
- INDICATES BEAM PENETRATION. SEE S1.10
- INDICATES HAUNCHED BEAM. SEE S1.10
- REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMJ WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

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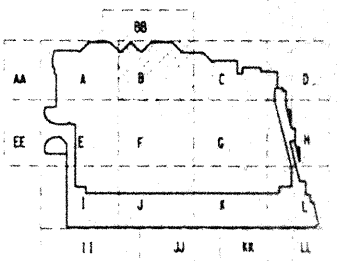
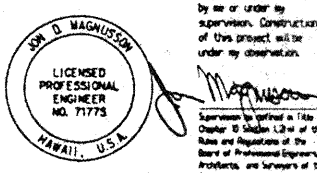
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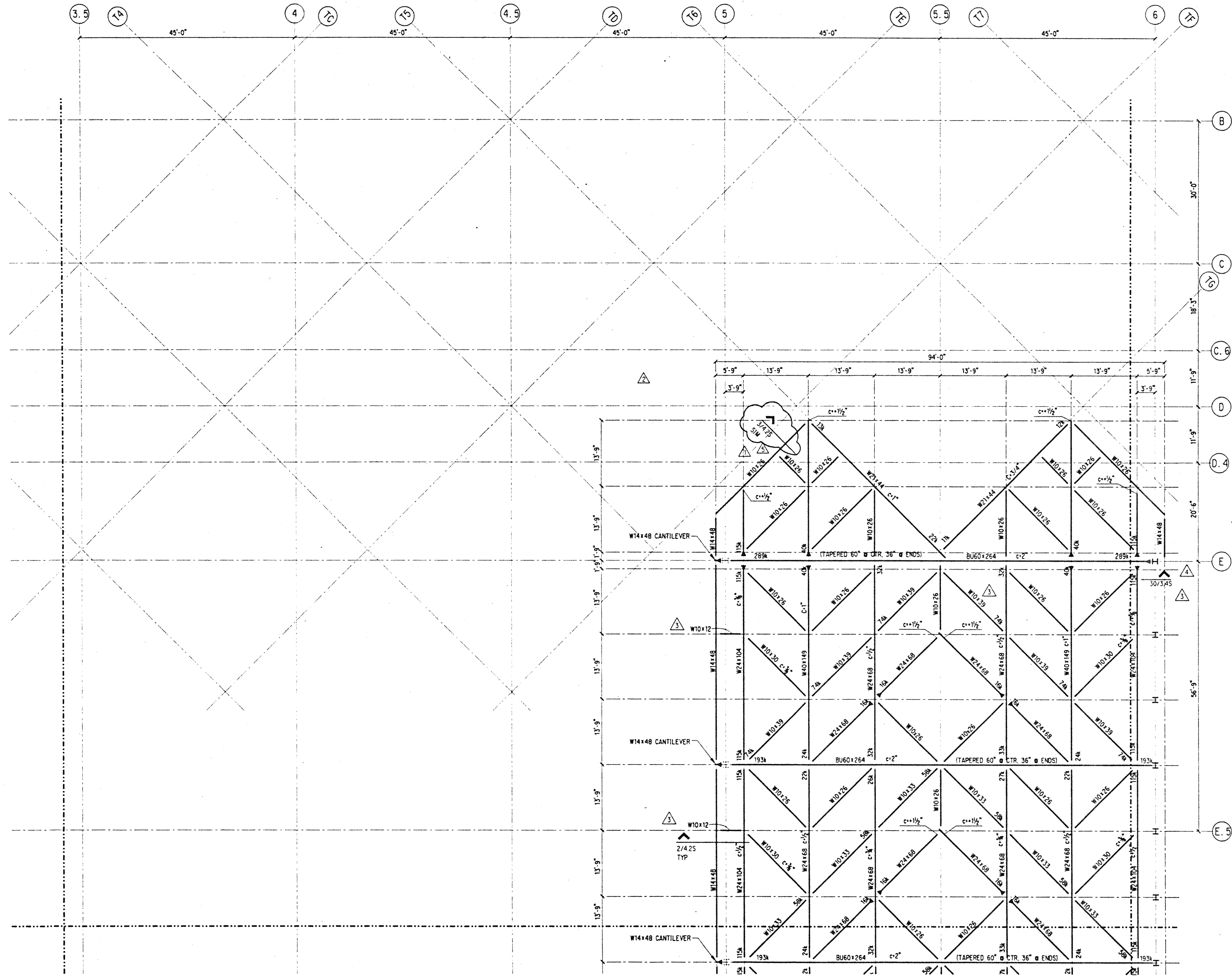
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11-7-95			BUILDING PERMIT SET
9-15-95			
8-31-95			
8-4-95			
6-19-95			

Sheet Title

ROOF TRELLIS FRAMING PLAN SECTOR B

Designed	BJM
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Checked	JDM
Date	4 / 12 / 95
Job No.	93036, 02, 9
Sheet No.	

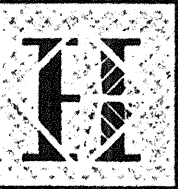


NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.7.B
 2. ALL BEAMS ON THE TRELLIS FRAMING PLAN, FRAMING INTO SLOPING MEMBERS, SHALL HAVE THE SAME TOP OF STEEL ELEVATION AT THE ENDS AS THEIR SUPPORTING BEAMS, UNO.

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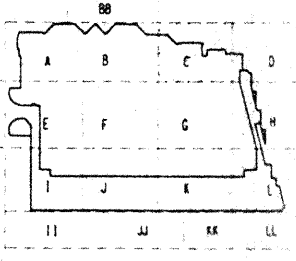
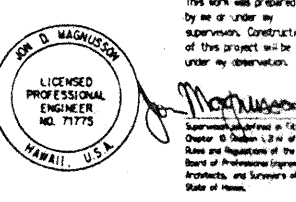
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No.	Date	By	Description
12-15-95			FINAL DOCUMENT
9-15-95			BUILDING PERMIT SET
8-4-95			ADDENDUM #3
5-8-95			ADDENDUM #3

Sheet Title

ROOF FRAMING PLAN SECTOR C

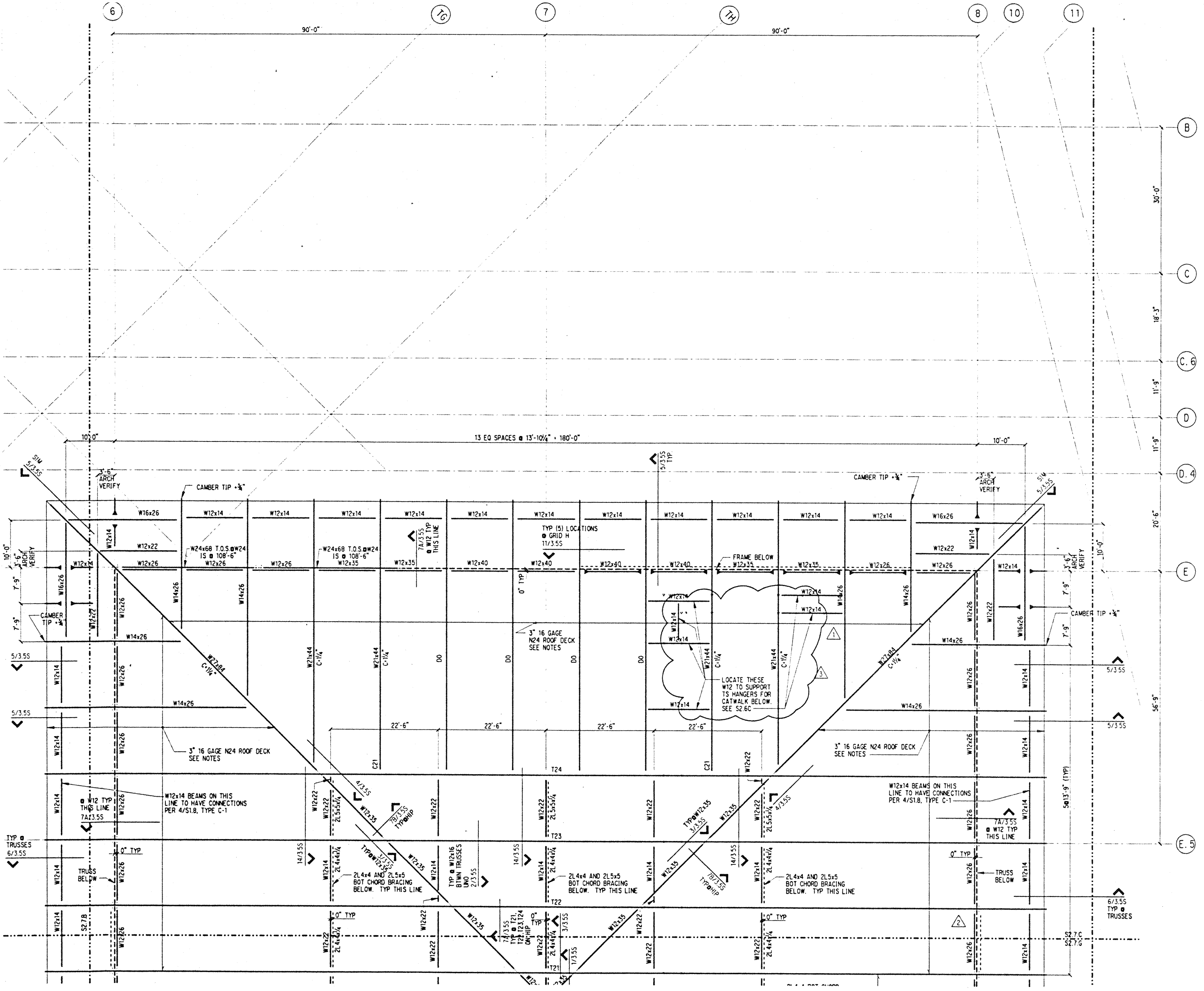
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Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.7.C

Thu Nov 30 15:11:59 1995

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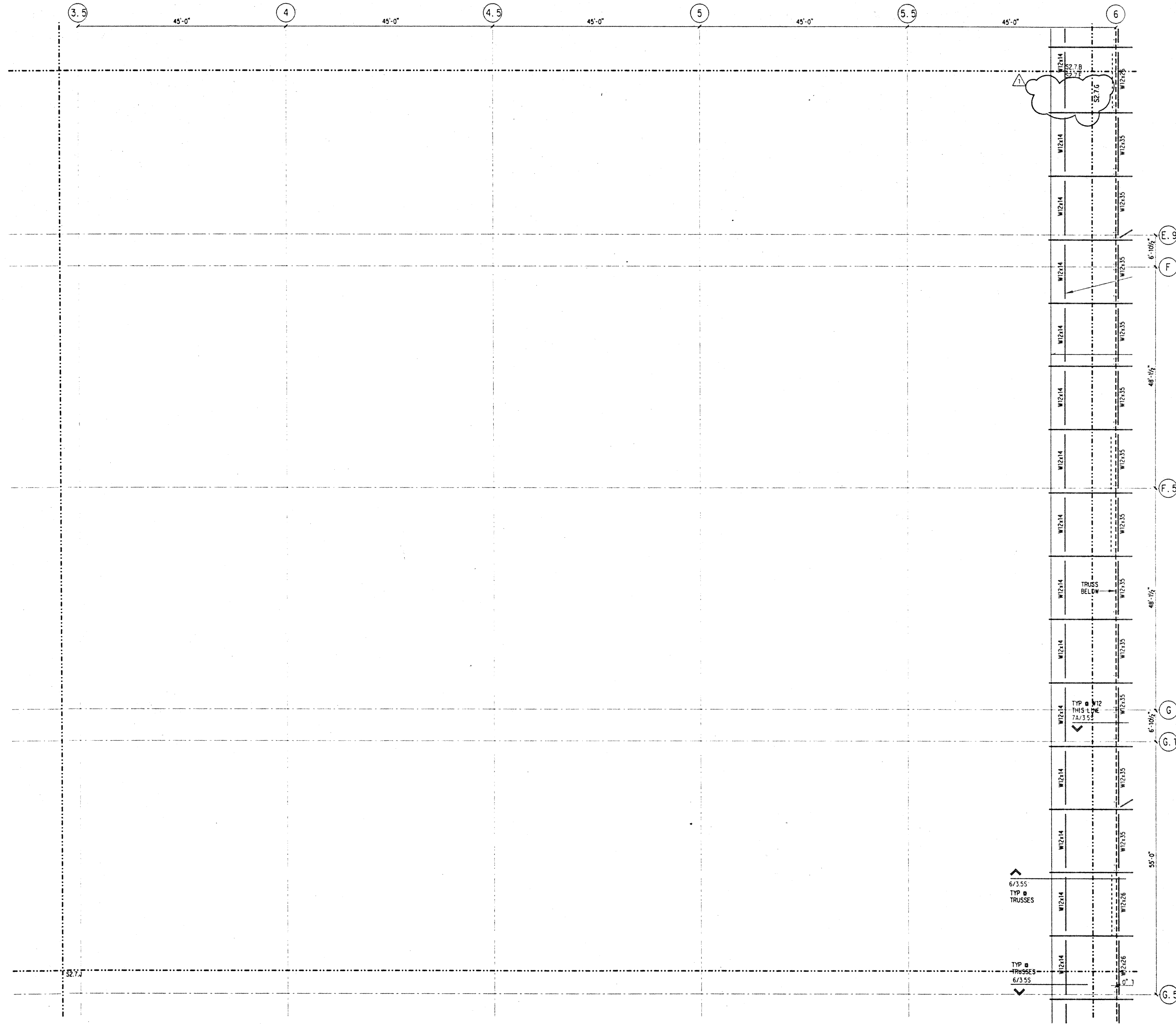


- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- REFERENCE NOTES ON S2.7.B
 - METAL ROOF DECK IS TO BE THE SIZE AND GAGE SHOWN ON THE PLAN. ALL UNITS ARE TO BE FASTENED TO EACH SUPPORT WITH (4) 3/8" PUDDLE WELDS ALONG THE WIDTH AND 1/2" TOP SEAM WELDS AT 1'-0" ON SIDE BOUNDARIES.
 - FOR TOP OF STEEL ELEVATIONS AT TRUSSES REFER TO TRUSS ELEVATIONS ON S5.9.
 - FOR TOP OF STEEL ELEVATIONS @ BEAMS REFER TO DETAILS.
 - FOR LOCATION OF CATWALK HANGERS & BRACES HUNG FROM BOTTOM CHORD OF TRUSSES REFER TO S2.6C AND S2.6G.

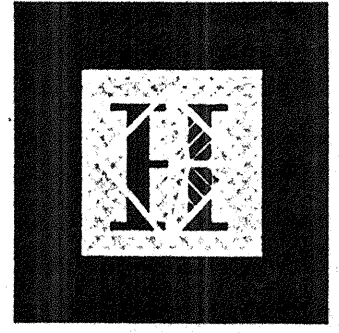
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
 1. REFERENCE NOTES ON S2.7.B



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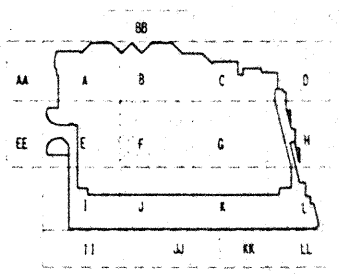
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 1301 Fifth Avenue, Suite 3700 Seattle WA 98101-2699
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This work was prepared by me or under my supervision. Construction of this project will be under my observation.

[Signature]
 SKELLING WARD MAGNUSON BARRISBERG INC.
 LICENSED PROFESSIONAL ENGINEER
 NO. 71775
 HAWAII, U.S.A.

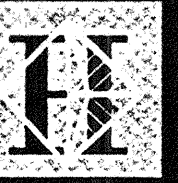


Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
8-4-95			

ROOF FRAMING PLAN SECTOR F

Designed **BJM**
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 Checked **JDM**
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



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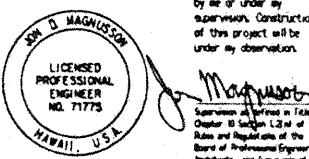
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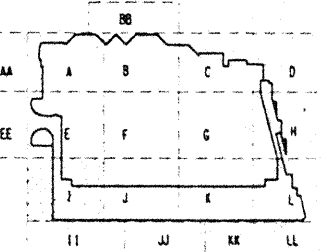
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Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
8-4-95			ADDENDUM #3
5-8-95			ADDENDUM #3

Sheet Title

ROOF FRAMING PLAN SECTOR I

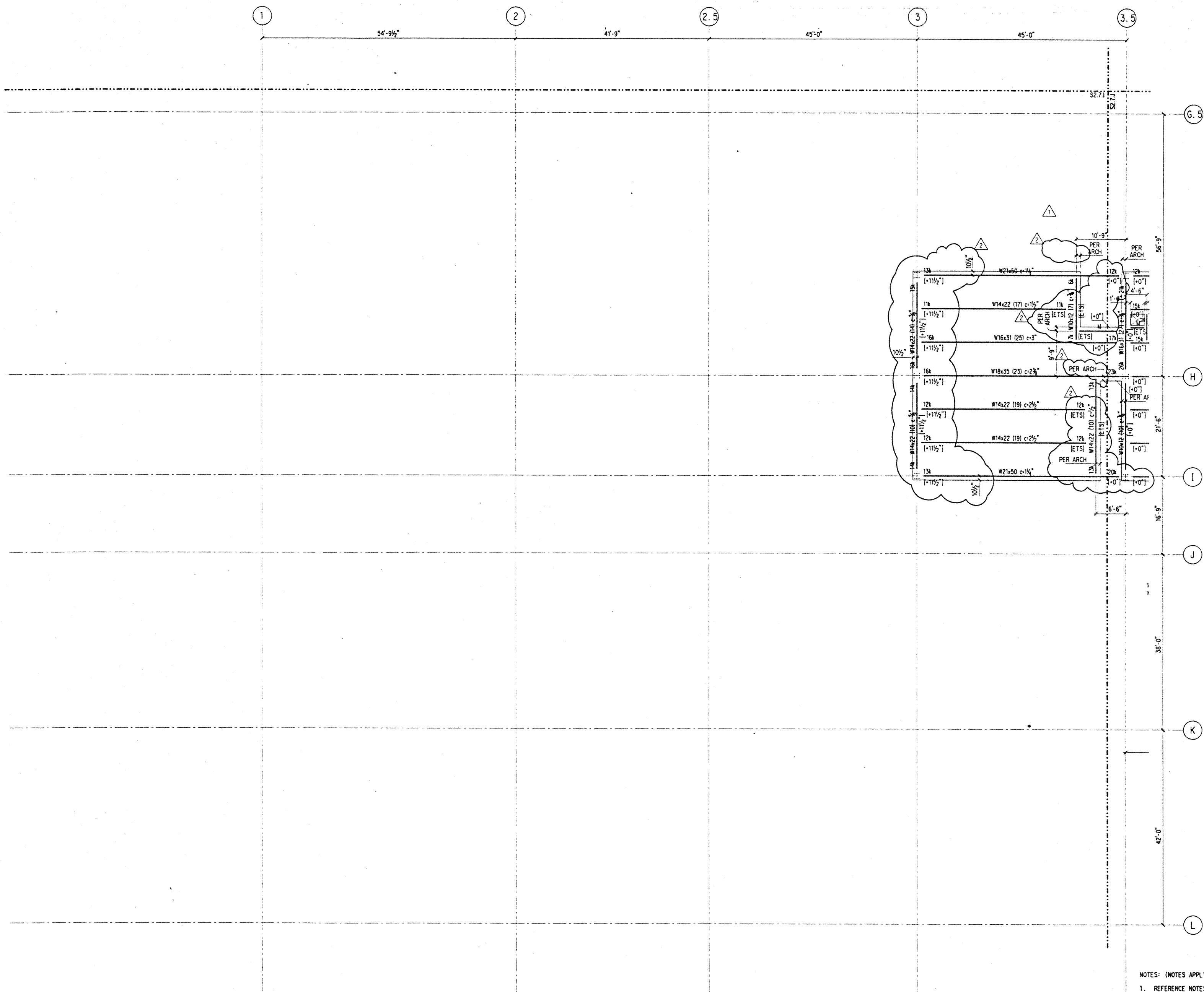
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Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

S2.7.1

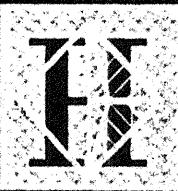
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EMS pit



- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.7.8
 2. REFERENCE FLOOR ELEVATION = 102'-6".



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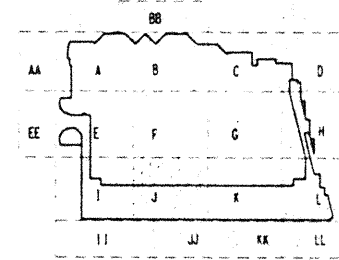
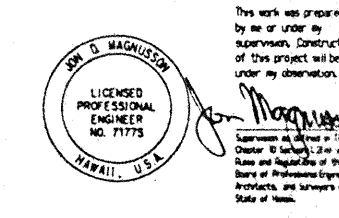
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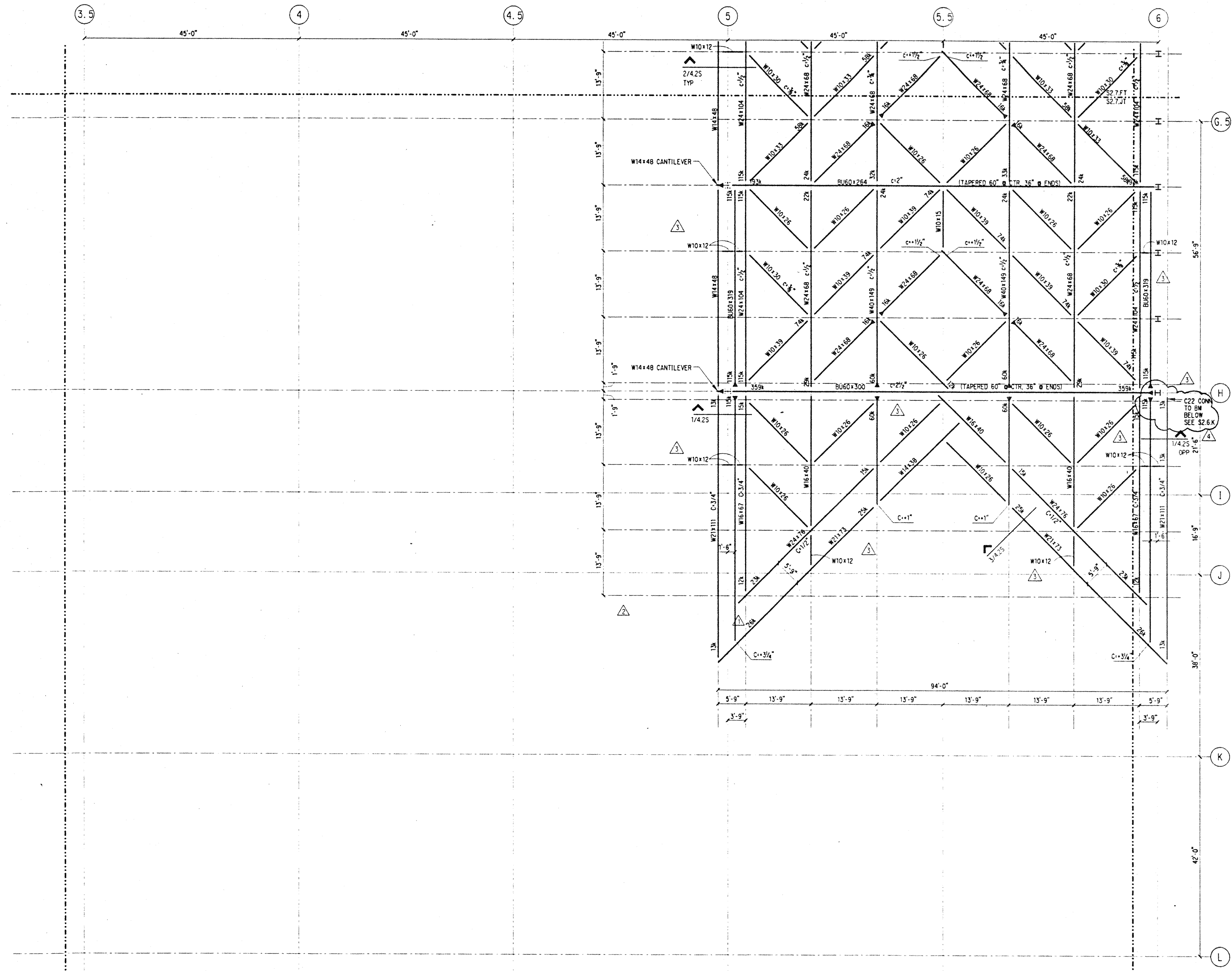


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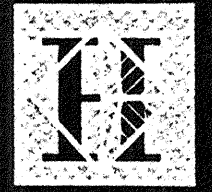
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6-19-95			

Sheet Title
ROOF TRELLIS FRAMING PLAN SECTOR J

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Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
1. REFERENCE NOTES ON S2.7.B
2. SEE NOTE 2/S2.7.BT



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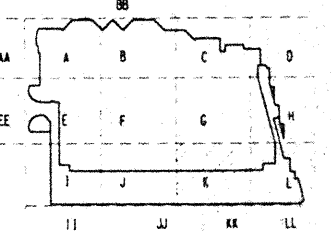
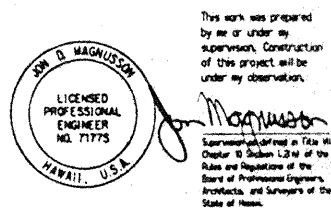
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No.	Date	By	Description
2-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
8-4-95			

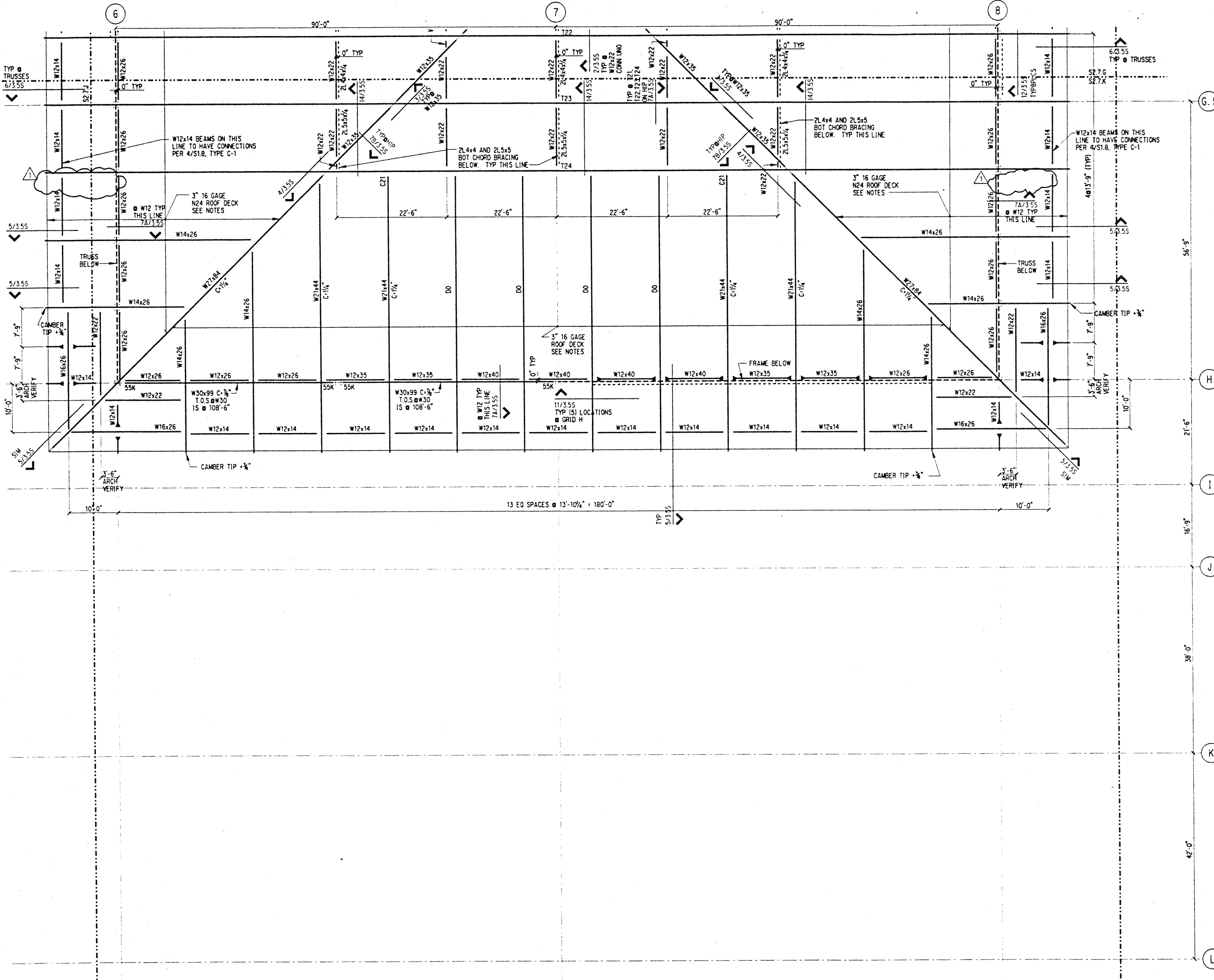
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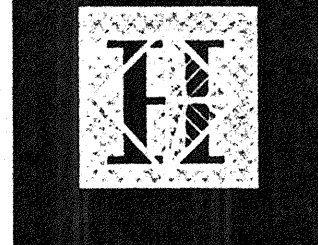
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Date	4 / 12 / 95
Job No.	93036.02.9
Sheet No.	

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- REFERENCE NOTES ON S2.7.B
- METAL ROOF DECK IS TO BE THE SIZE AND GAGE SHOWN ON THE PLAN. ALL UNITS ARE TO BE FASTENED TO EACH SUPPORT WITH (4) 3/8" PUDDLE WELDS ALONG THE WIDTH AND 1/2" TOP SEAM WELDS AT 1'-0" ON SIDE BOUNDARIES.
- FOR TOP OF STEEL ELEVATIONS AT TRUSSES REFER TO TRUSS ELEVATIONS ON S5.9.
- FOR TOP OF STEEL ELEVATIONS @ BEAMS REFER TO DETAILS.



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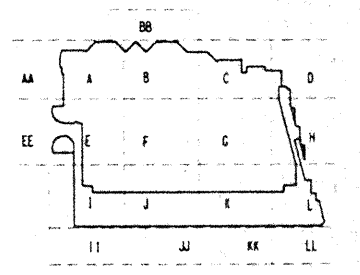
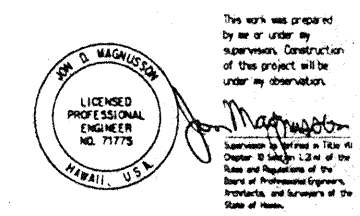
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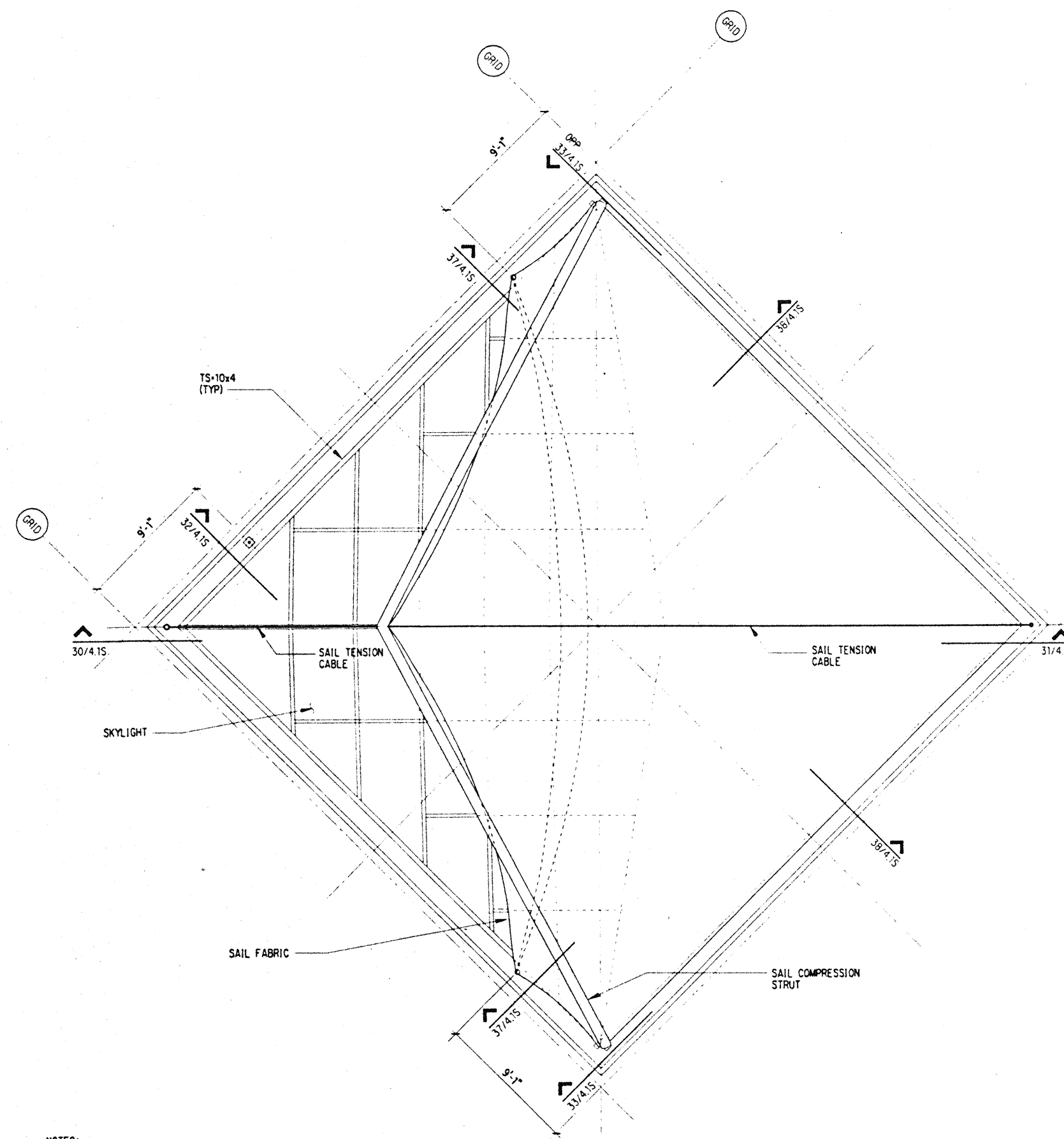
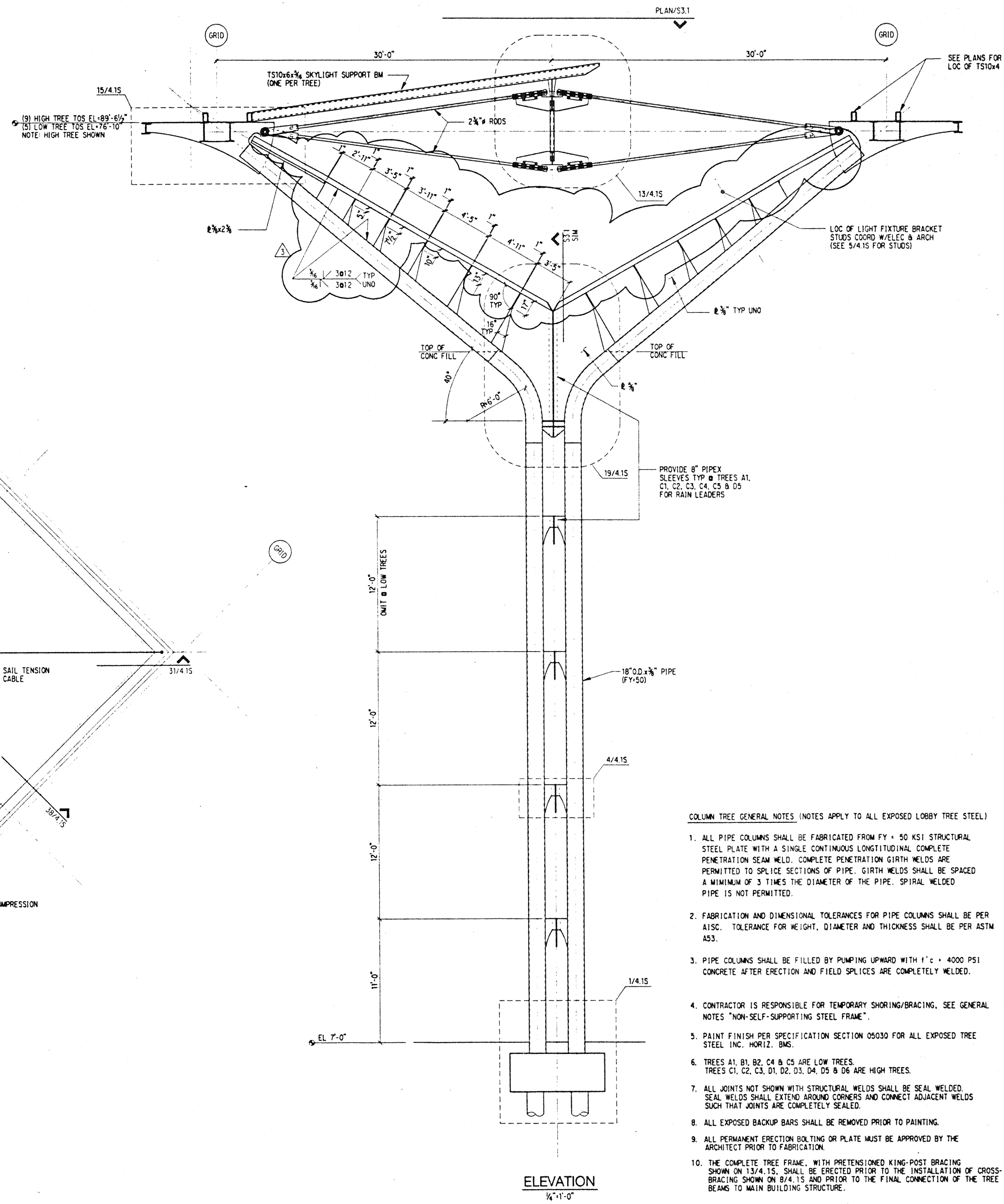
Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
11-7-95			TREE REVISIONS
9-20-95			BUILDING PERMIT SET
9-15-95			
8-31-95			

Sheet Title

TYPICAL STEEL COLUMN TREE ELEVATION & PLAN

Designed BJM
Drawn DNS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.



TYPICAL PLAN OF SAIL & SKYLIGHT (NOT SHOWN ON ELEVATION)

NOTES:
1. TREE FRAMING NOT SHOWN FOR CLARITY
2. CONTRACTOR SHALL REVIEW ALL ELEMENTS SHOWN FOR COMPATIBILITY W/ SAIL & SKYLIGHT.

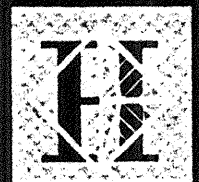
COLUMN TREE GENERAL NOTES (NOTES APPLY TO ALL EXPOSED LOBBY TREE STEEL)

- ALL PIPE COLUMNS SHALL BE FABRICATED FROM FY = 50 KSI STRUCTURAL STEEL PLATE WITH A SINGLE CONTINUOUS LONGITUDINAL COMPLETE PENETRATION SEAM WELD. COMPLETE PENETRATION GIRTH WELDS ARE PERMITTED TO SPLICE SECTIONS OF PIPE. GIRTH WELDS SHALL BE SPACED A MINIMUM OF 3 TIMES THE DIAMETER OF THE PIPE. SPIRAL WELDED PIPE IS NOT PERMITTED.
- FABRICATION AND DIMENSIONAL TOLERANCES FOR PIPE COLUMNS SHALL BE PER AISC. TOLERANCE FOR WEIGHT, DIAMETER AND THICKNESS SHALL BE PER ASTM A53.
- PIPE COLUMNS SHALL BE FILLED BY PUMPING UPWARD WITH 1'c + 4000 PSI CONCRETE AFTER ERECTION AND FIELD SPLICES ARE COMPLETELY WELDED.
- CONTRACTOR IS RESPONSIBLE FOR TEMPORARY SHORING/BRACING. SEE GENERAL NOTES "NON-SELF-SUPPORTING STEEL FRAME".
- PAINT FINISH PER SPECIFICATION SECTION 05030 FOR ALL EXPOSED TREE STEEL INC. HORIZ. BMS.
- TREES A1, B1, B2, C4 & C5 ARE LOW TREES. TREES C1, C2, C3, D1, D2, D3, D4, D5 & D6 ARE HIGH TREES.
- ALL JOINTS NOT SHOWN WITH STRUCTURAL WELDS SHALL BE SEAL WELDED. SEAL WELDS SHALL EXTEND AROUND CORNERS AND CONNECT ADJACENT WELDS SUCH THAT JOINTS ARE COMPLETELY SEALED.
- ALL EXPOSED BACKUP BARS SHALL BE REMOVED PRIOR TO PAINTING.
- ALL PERMANENT ERECTION BOLTING OR PLATE MUST BE APPROVED BY THE ARCHITECT PRIOR TO FABRICATION.
- THE COMPLETE TREE FRAME, WITH PRETENSIONED KING-POST BRACING SHOWN ON 13/4.15, SHALL BE ERECTED PRIOR TO THE INSTALLATION OF CROSS-BRACING SHOWN ON 8/4.15 AND PRIOR TO THE FINAL CONNECTION OF THE TREE BEAMS TO MAIN BUILDING STRUCTURE.

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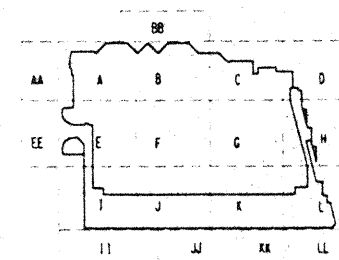
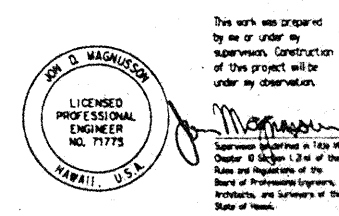
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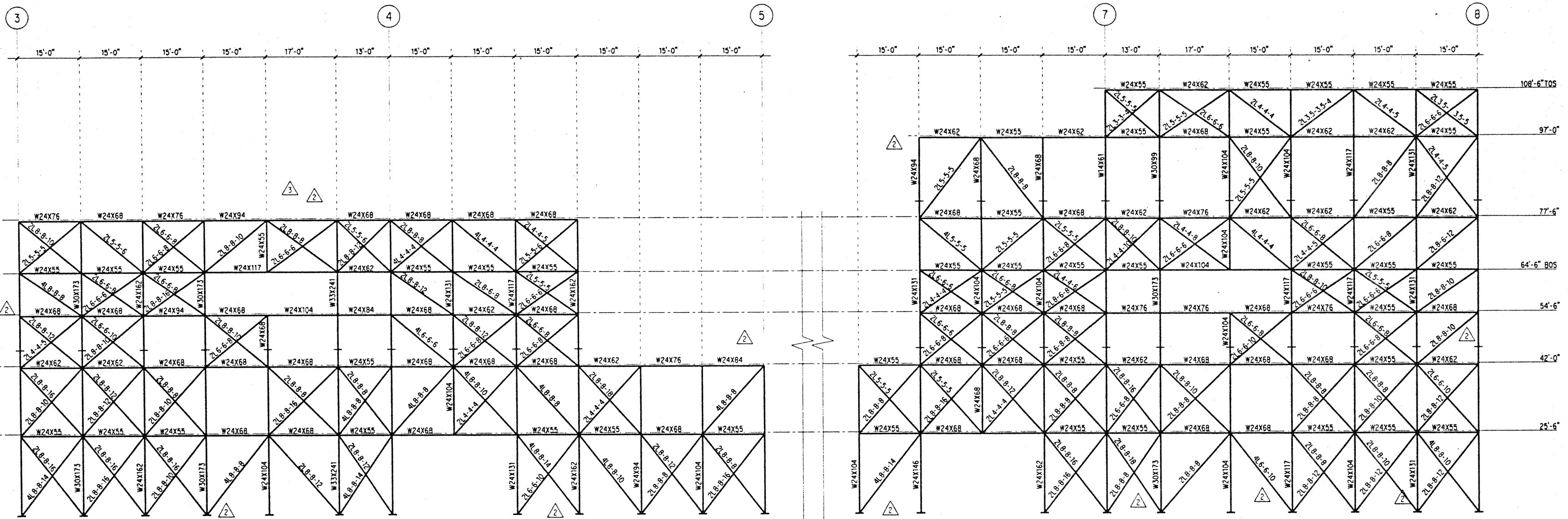


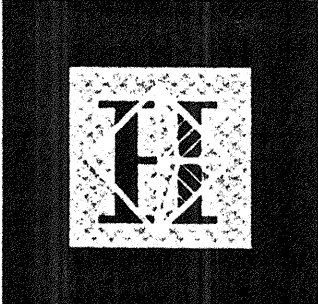
Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
11-7-95			BUILDING PERMIT SET
9-15-95			
8-4-95			
6-19-95			REVISION #3
5-8-95			

Sheet Title
BRACED FRAME ELEVATIONS - GRIDS H & 2

Designed: BJM
Drawn: DMS
Checked: JDM
Date: 4 / 12 / 95
Job No.: 93036.02.9
Sheet No.:





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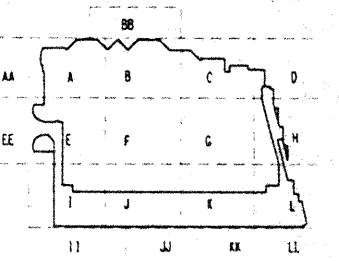
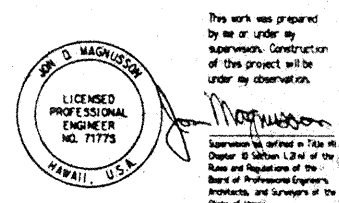
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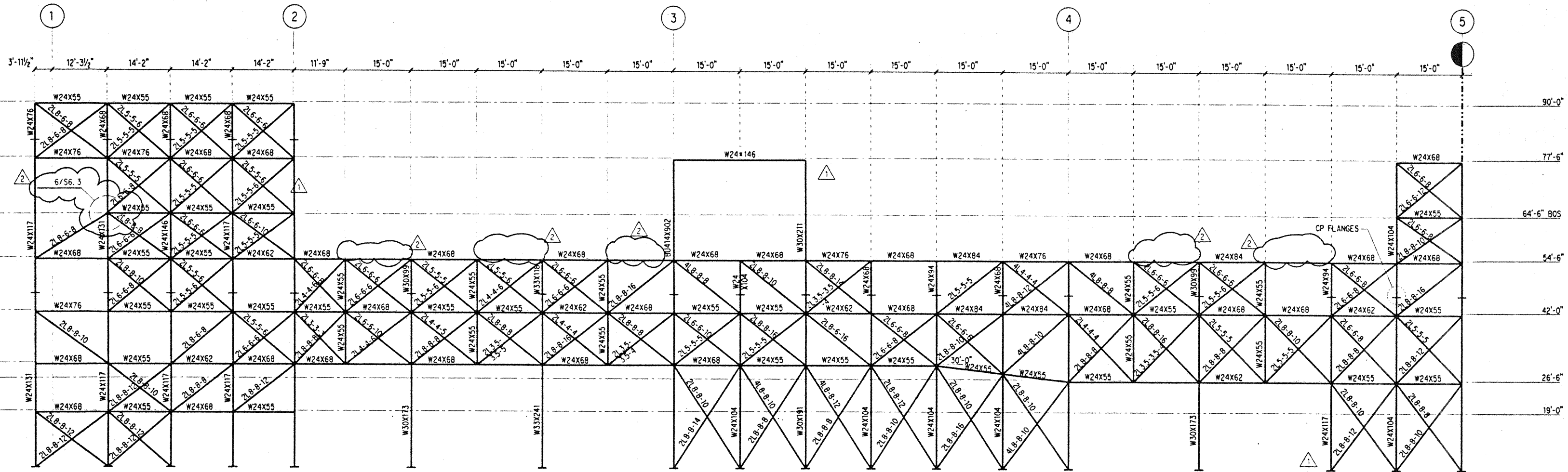
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9-15-95			BUILDING PERMIT SET
8-4-95			
5-19-95			

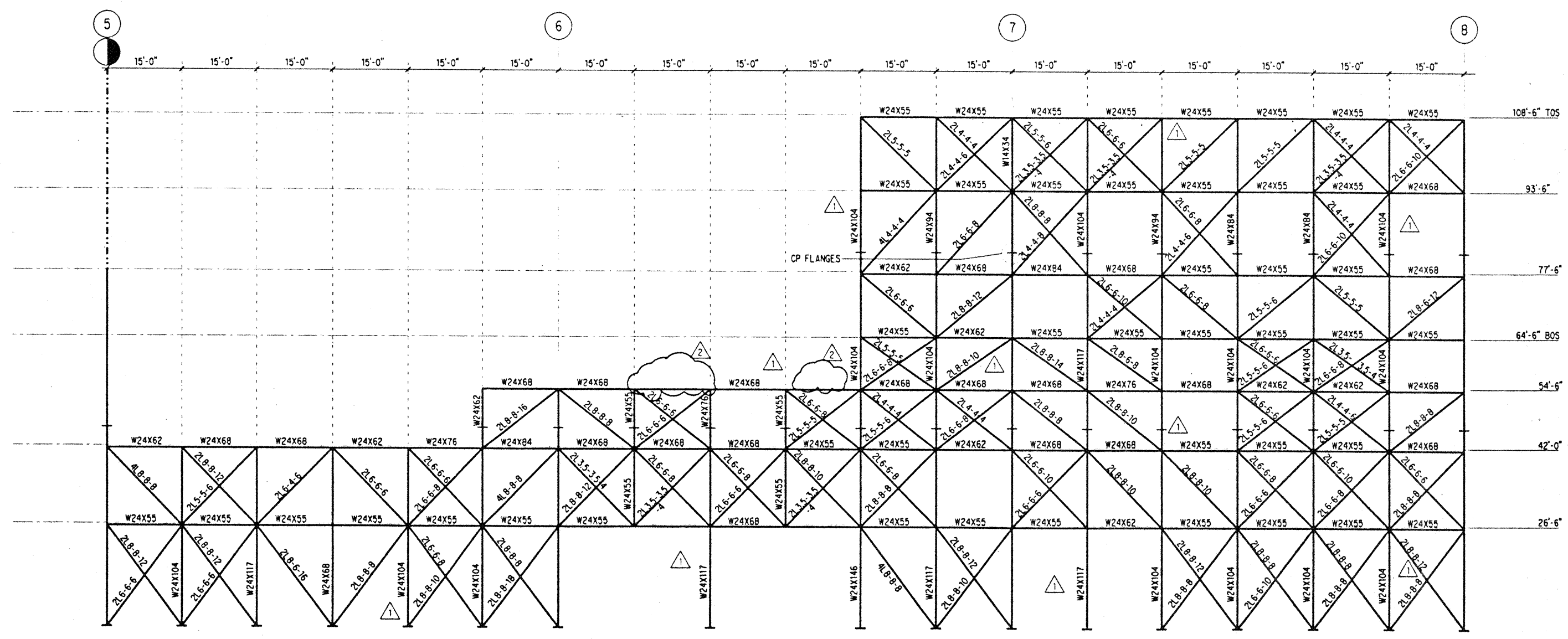
Sheet Title

BRACED FRAME ELEVATION - GRID E

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



FRAME ON GRID E
SCALE: 3/8" = 1'-0"



FRAME ON GRID E
SCALE: 3/8" = 1'-0"

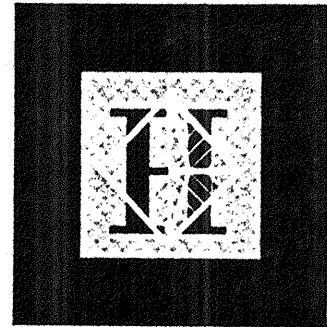
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

- ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
- SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/56.1.
- TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C + %. CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
- + DENOTES BEAM SPLICE LOCATIONS PER 3/56.2.
- + DENOTES COLUMN SPLICE LOCATIONS PER 20/56.4.
- SEE 56.4 FOR BRACED FRAME COLUMN BASE PLATE SCHEDULE AND DETAILS.

Thu Nov 30 11:02:57 1995

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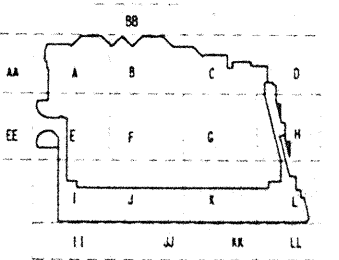
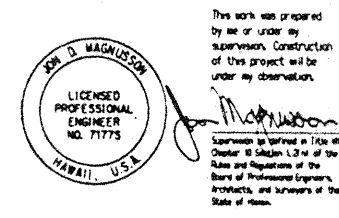
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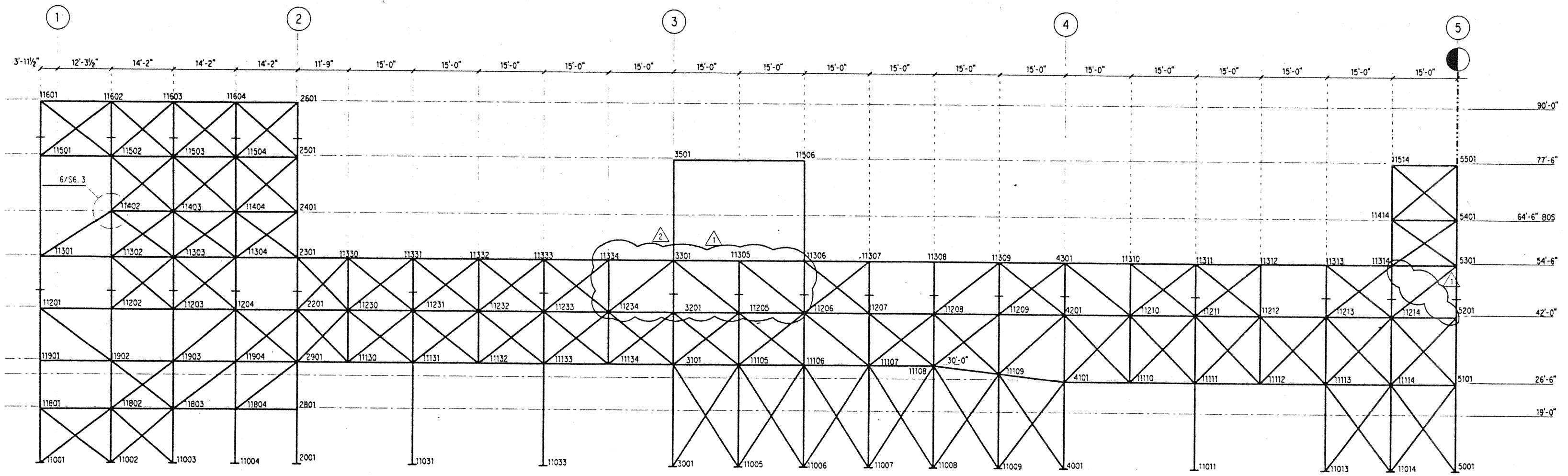
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9-15-95			BUILDING PERMIT SET
6-19-95			

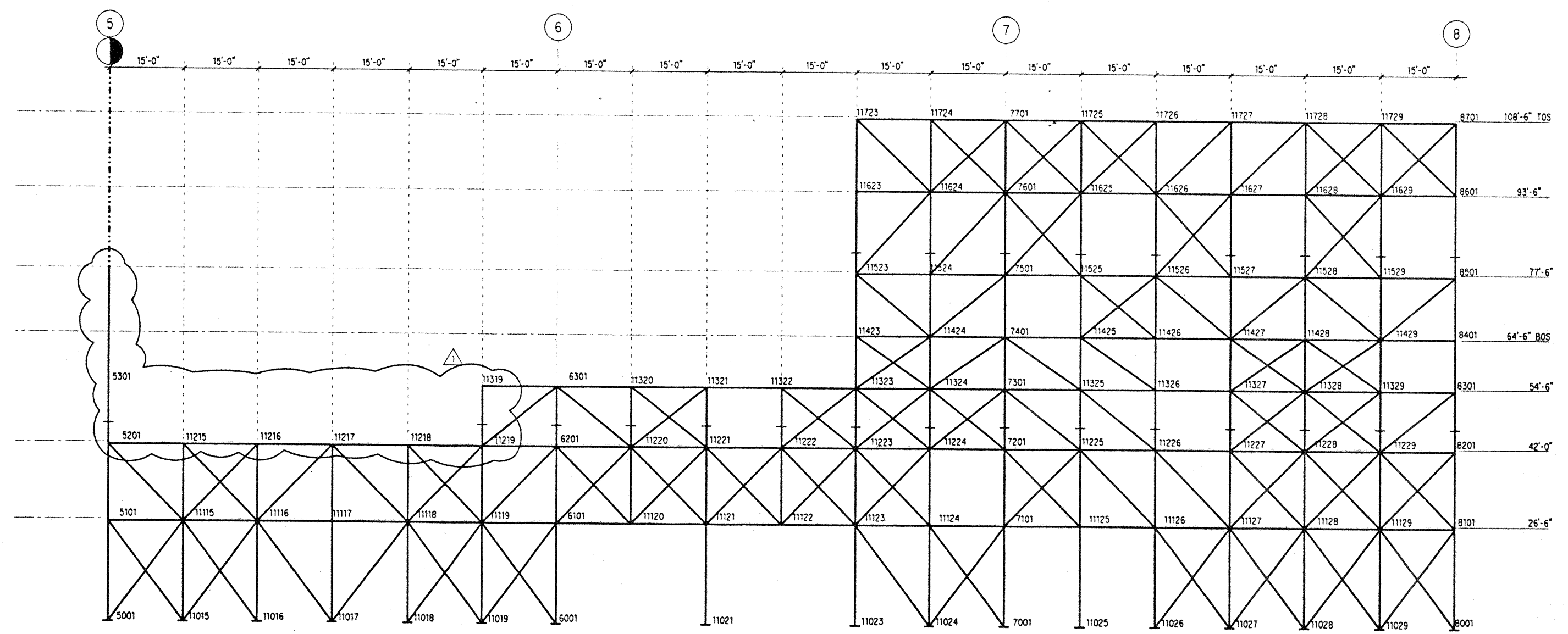
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BRACED FRAME ELEVATION - GRID E

Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



FRAME ON GRID E
 SCALE: 3/32" = 1'-0"



FRAME ON GRID E
 SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

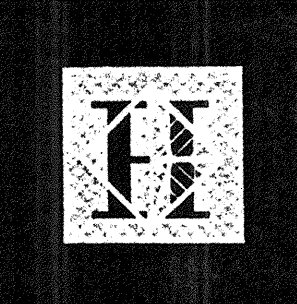
- ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
- SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
- TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C-1/8". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS, EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.

- + DENOTES BEAM SPLICE LOCATIONS PER 20/S6.2.
- + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

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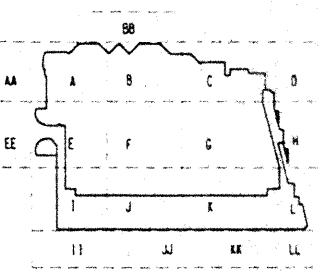
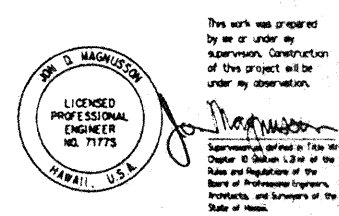
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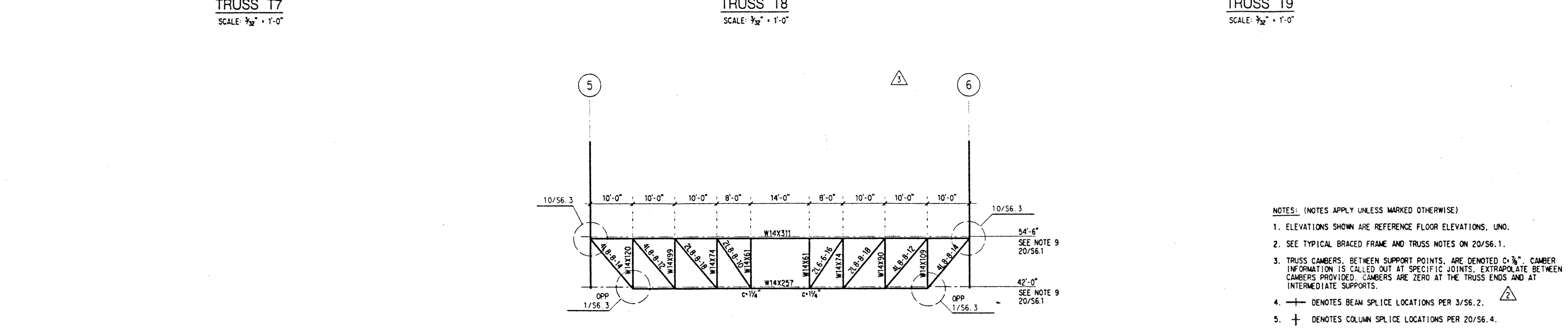
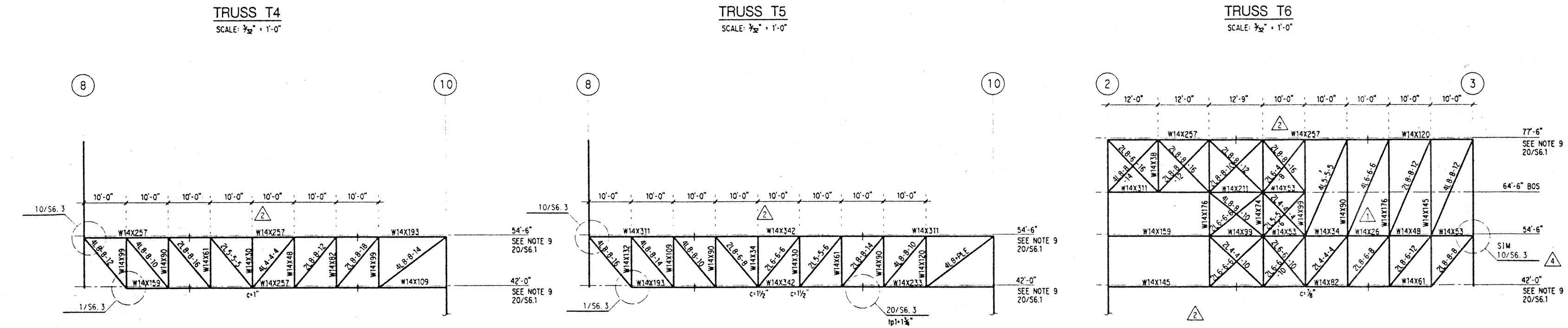
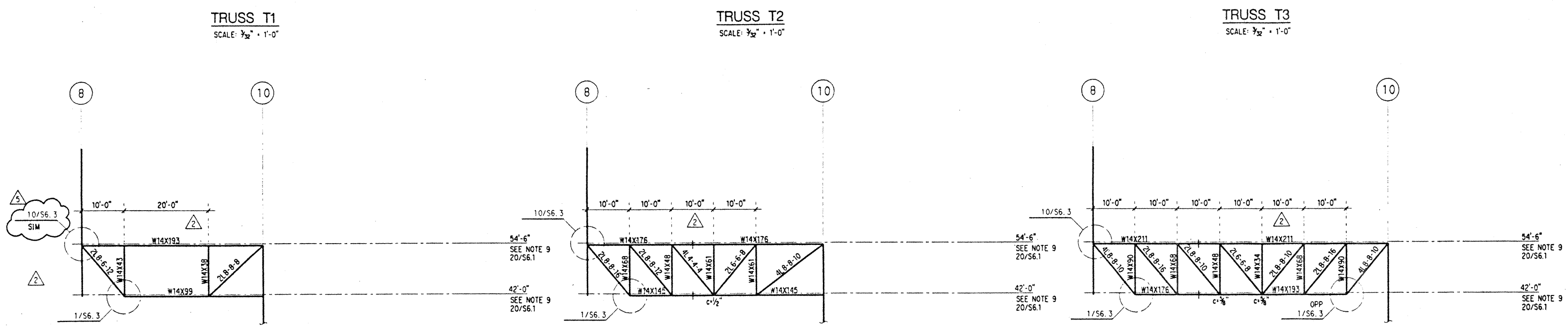
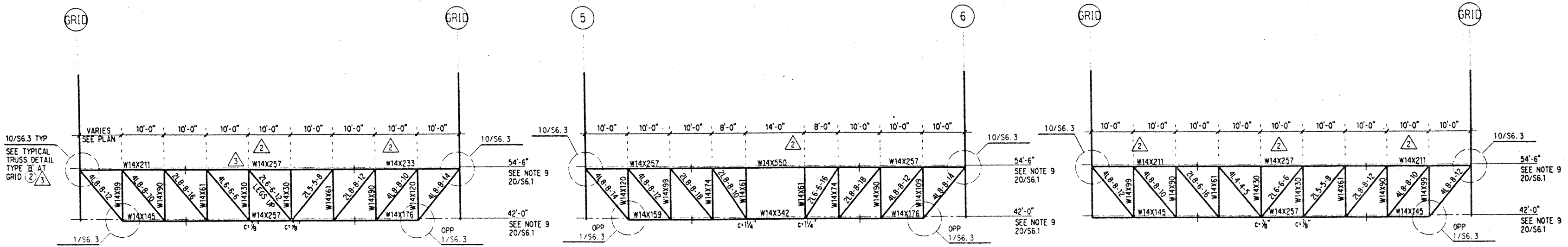


No.	Date	By	Description
12-15-95			FPM DOCUMENTS
11-7-95			
9-15-95			BUILDING PERMIT SET
8-31-95			
8-4-95			
6-19-95			
5-8-95			ADDENDUM #3

Sheet Title

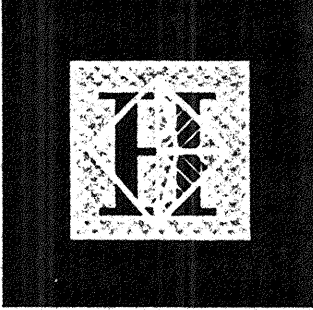
TRUSS ELEVATIONS - PARKING

Designed: BJM
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 Date: 4 / 12 / 95
 Job No.: 93036.02.9
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- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
 - SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
 - TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C-3/8". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
 - + DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
 - + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

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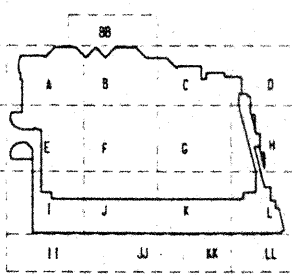
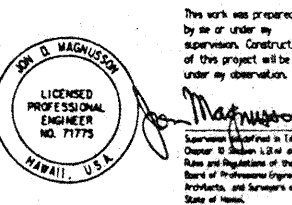
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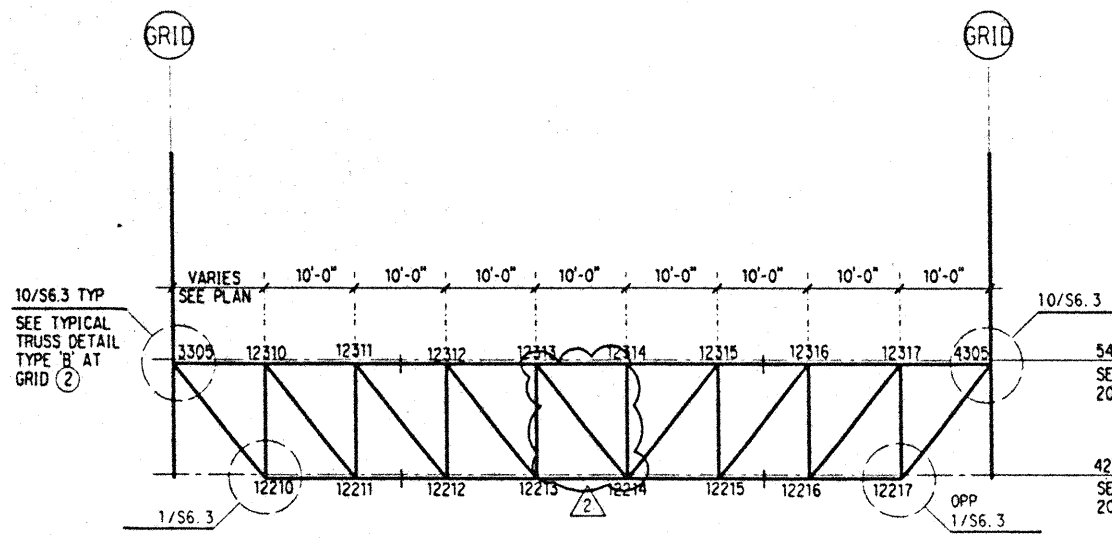
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12-15-95			FINAL DOCUMENTS
9-12-95			BUILDING PERMIT SET
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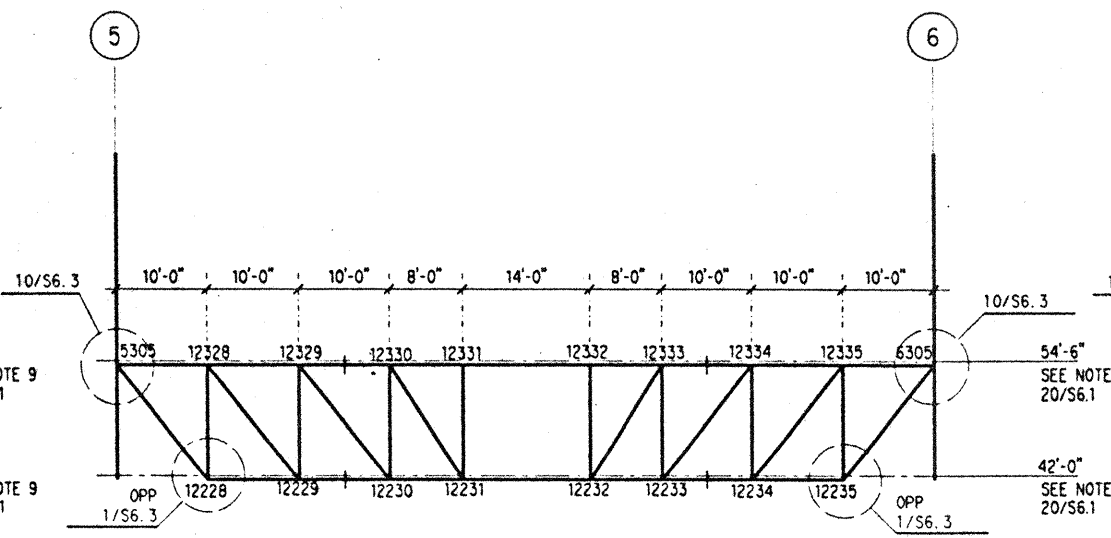
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TRUSS ELEVATIONS - PARKING

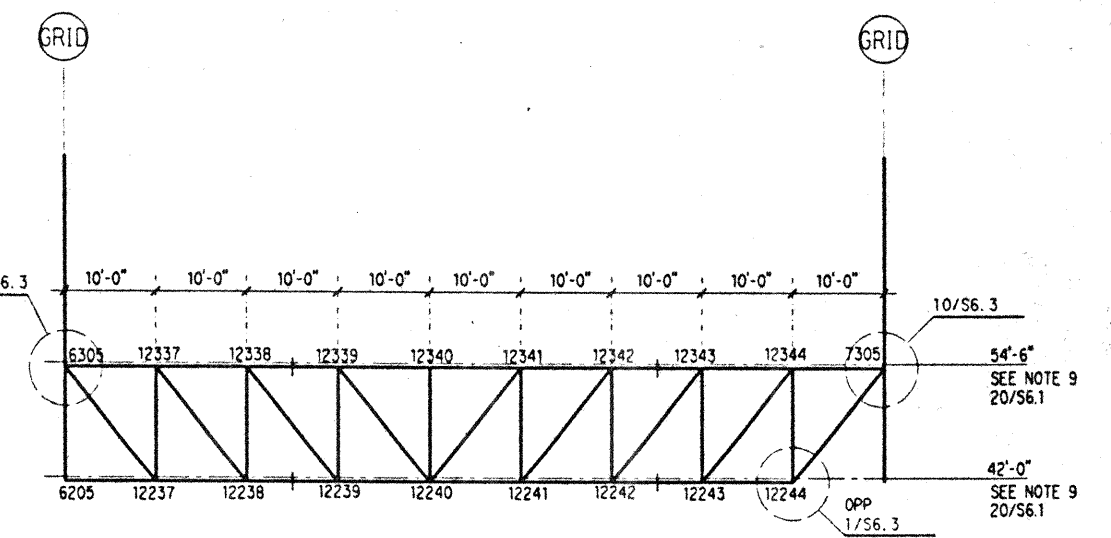
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 Date **4 / 12 / 95**
 Job No. **93036.02.9**
 Sheet No.



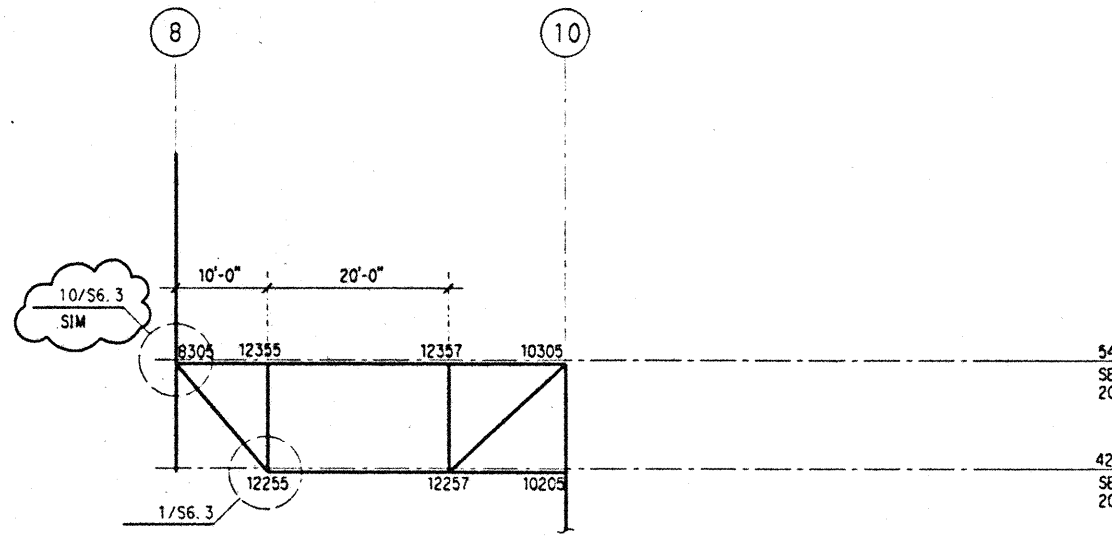
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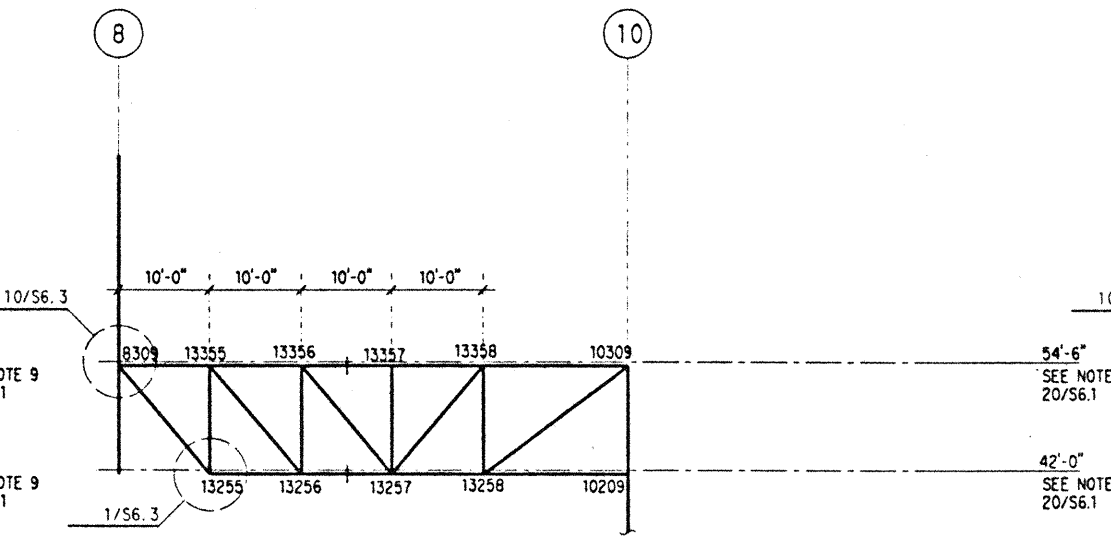
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SCALE: 3/32" = 1'-0"



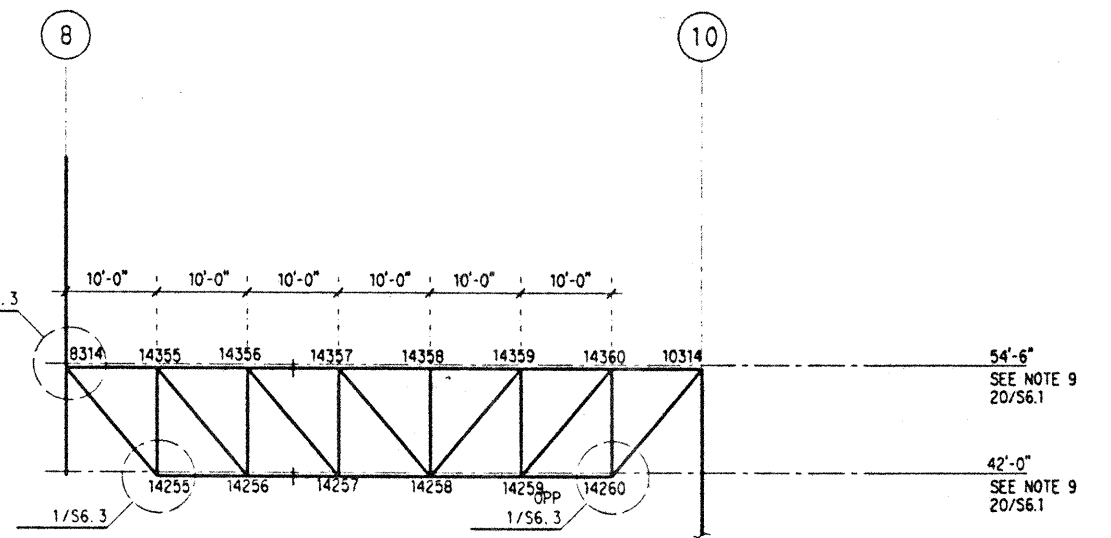
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SCALE: 3/32" = 1'-0"



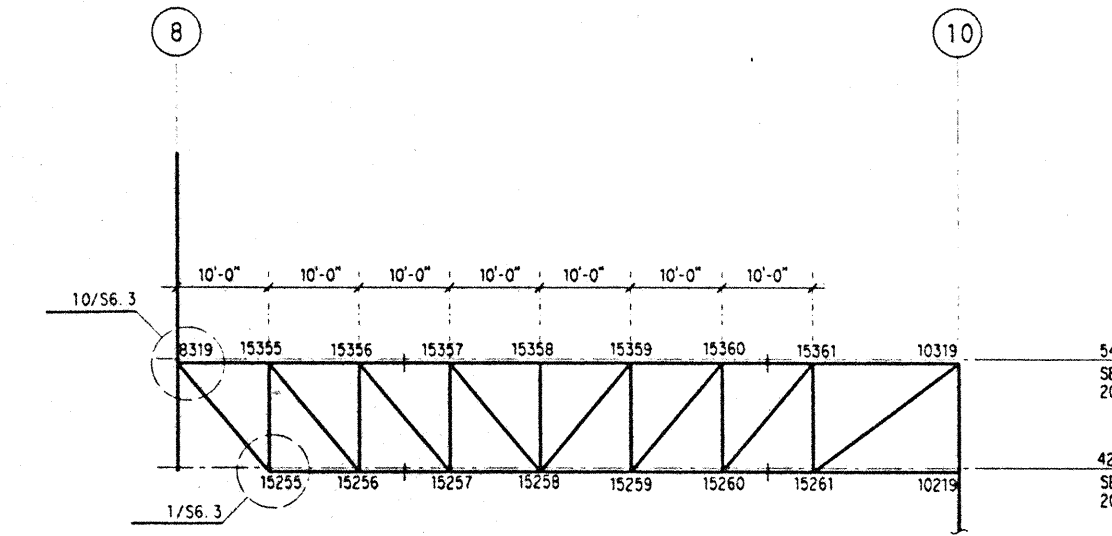
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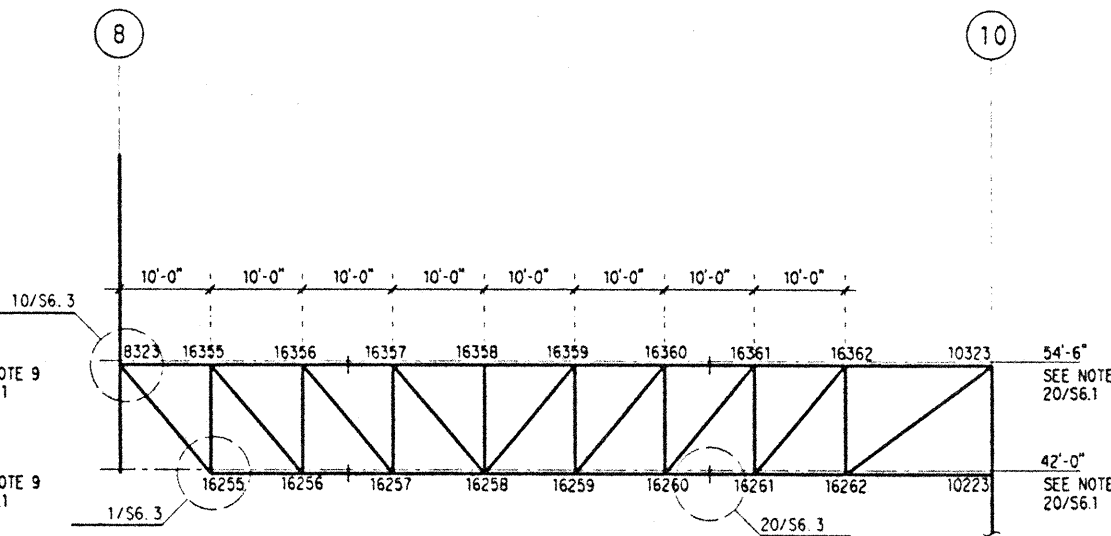
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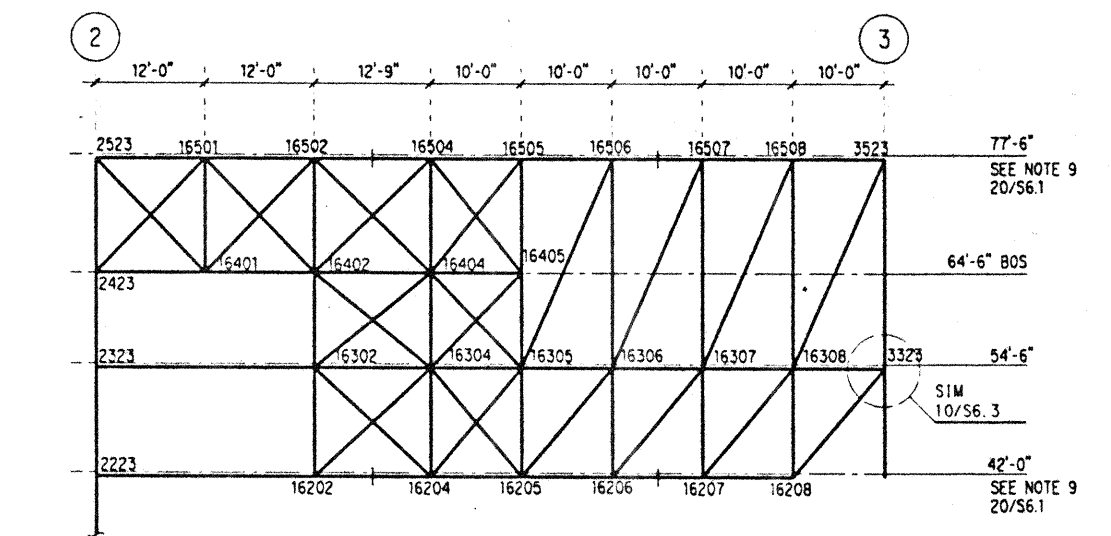
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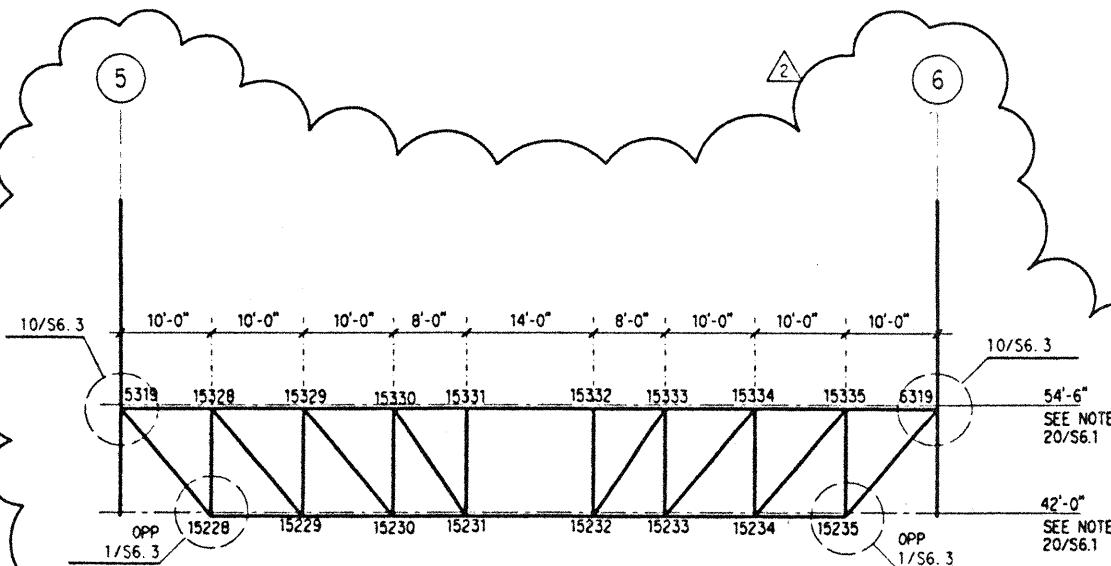
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TRUSS T8
SCALE: 3/32" = 1'-0"



TRUSS T9
SCALE: 3/32" = 1'-0"



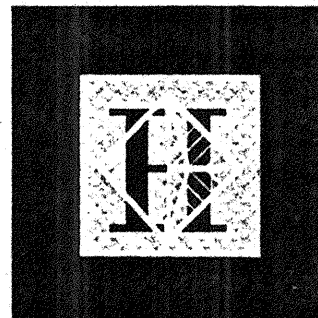
TRUSS T2A
SCALE: 3/32" = 1'-0"

- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
 - SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
 - TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C-3/4". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS, EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
 - DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
 - + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

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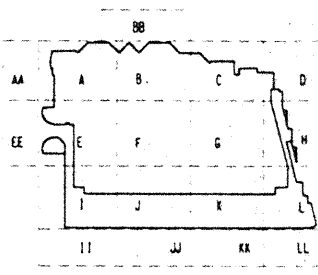
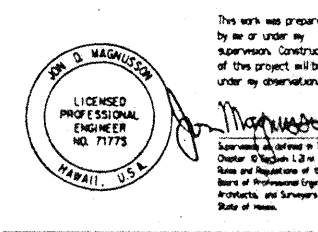
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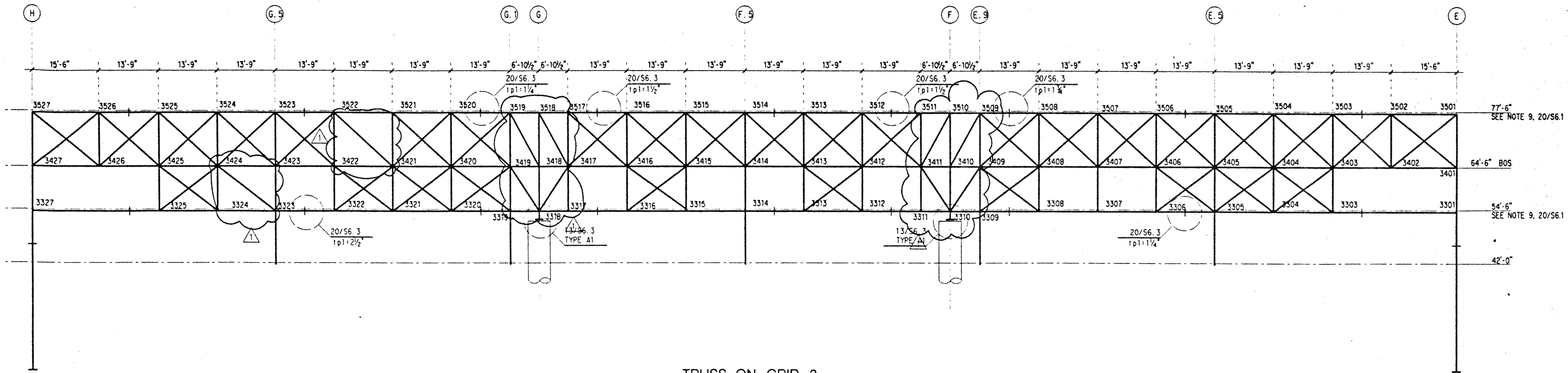


Revisions	
12-15-95	FINAL DOCUMENTS
9-15-95	BUILDING PERMIT SET
6-19-95	
No.	Date By Description

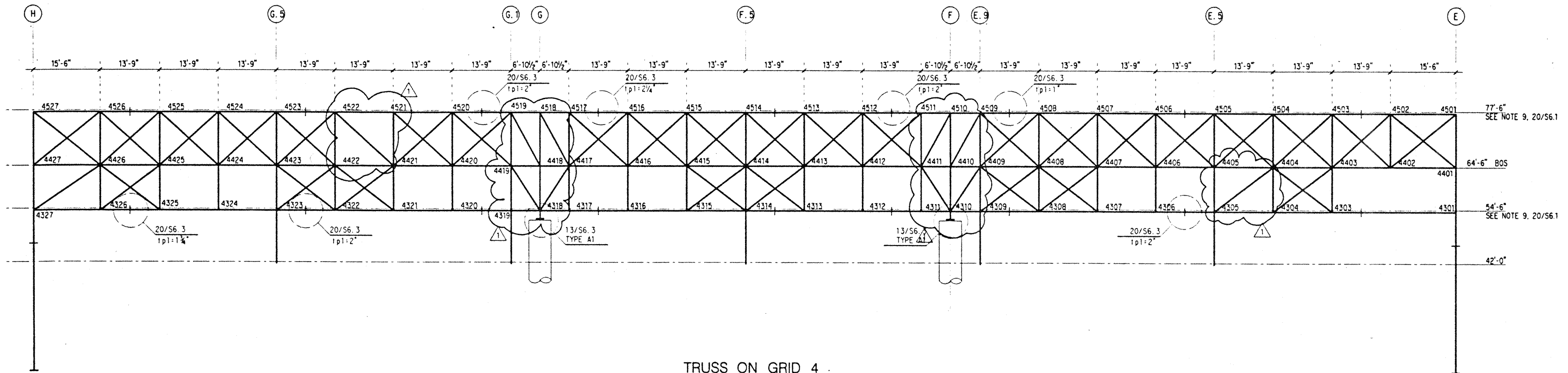
Sheet Title

TRUSS ELEVATIONS - GRIDS 3, 4, 5

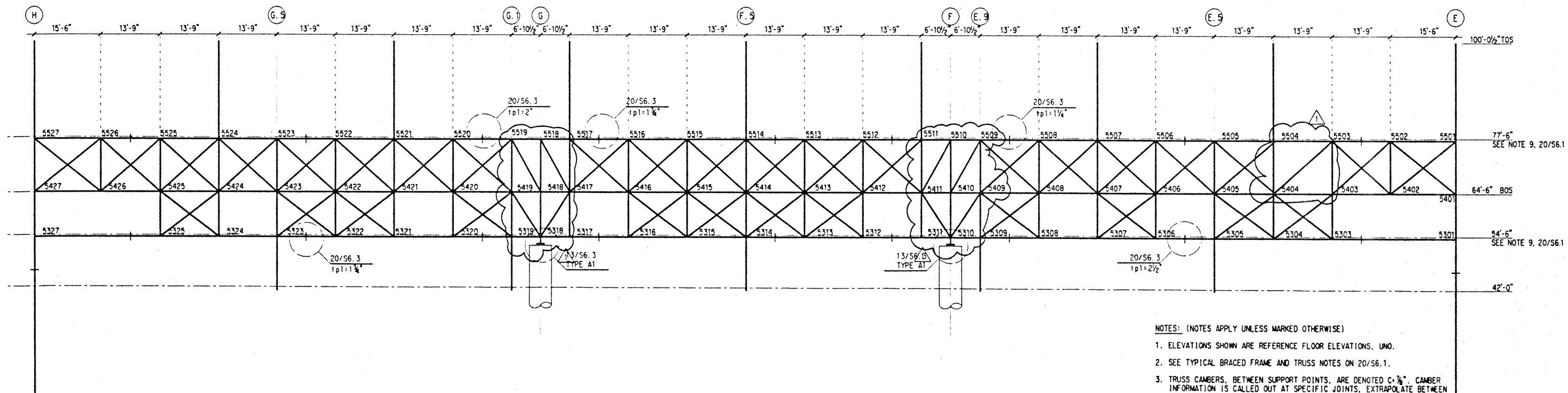
Designed BJM
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Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.



TRUSS ON GRID 3
SCALE: 3/32" = 1'-0"



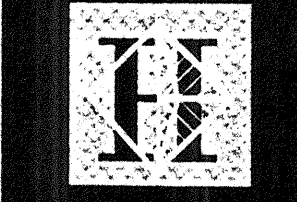
TRUSS ON GRID 4
SCALE: 3/32" = 1'-0"



TRUSS ON GRID 5
SCALE: 3/32" = 1'-0"

- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS. UNO.
 - SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
 - TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C + 3/8". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
 - + DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
 - + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

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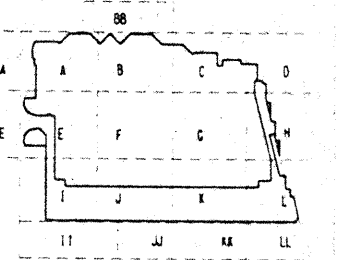
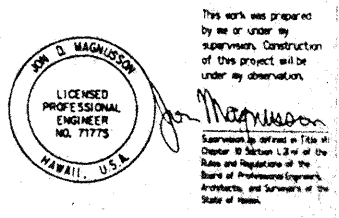
Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with
Loschky Marquardt & Nesheim
Seattle, Washington

NORDIC / PCL
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Honolulu, HI

Consultants
SKILLING WARD MAGNUSON BARRERE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 623-1001



Revisions

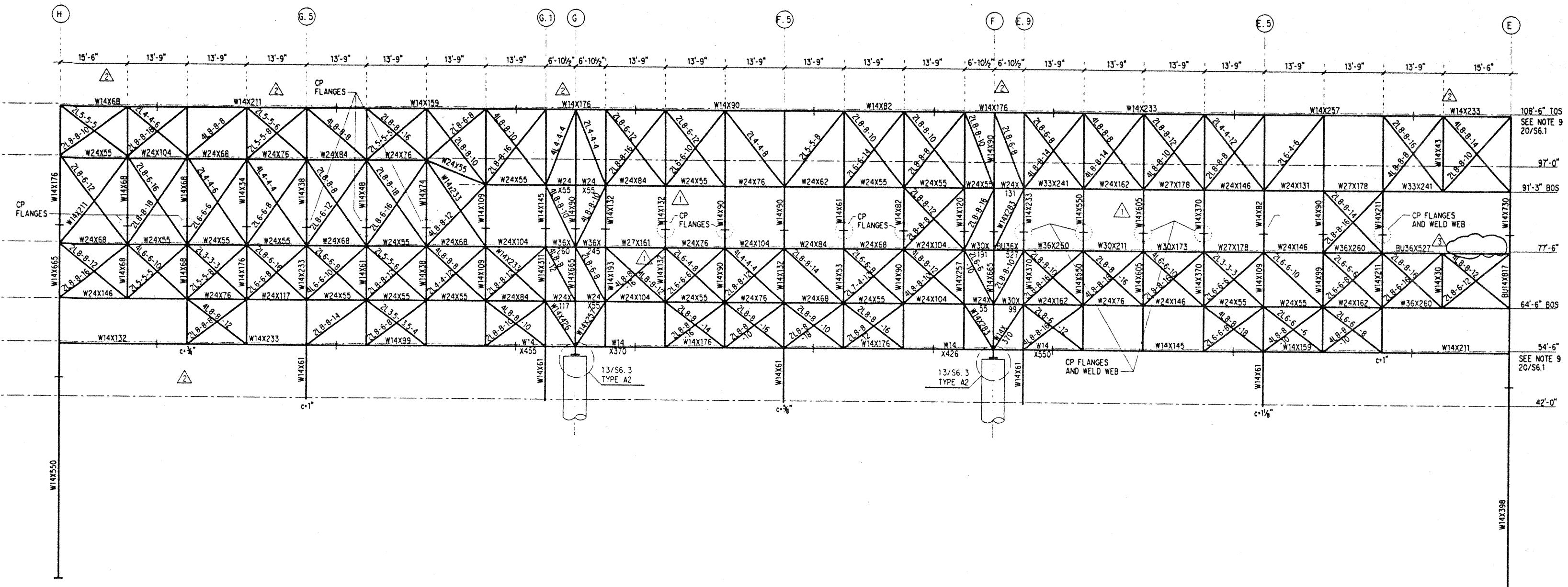
No.	Date	By	Description
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8-4-95			BUILDING PERMIT SET
6-19-95			
5-8-95			REVISION #3

No. Date By Description

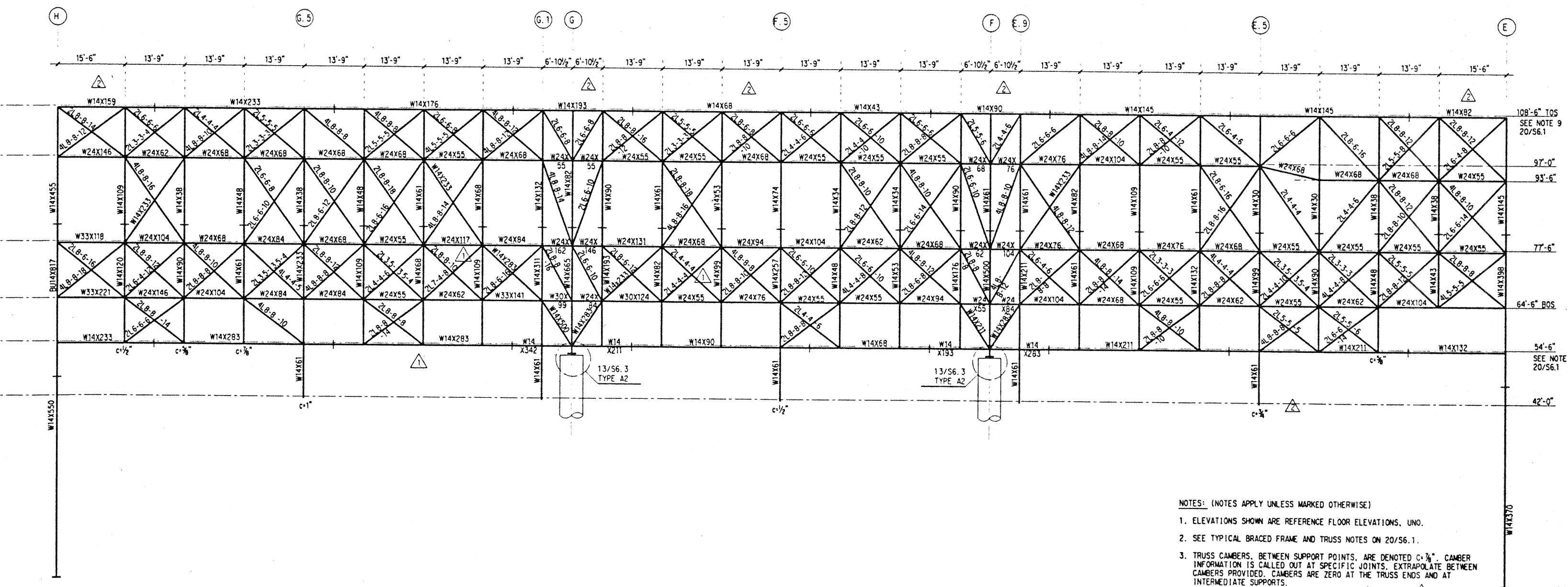
Sheet Title

TRUSS ELEVATIONS - GRIDS 6 & 8

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.



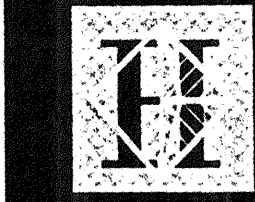
TRUSS ON GRID 6
SCALE: 3/32" = 1'-0"



TRUSS ON GRID 8
SCALE: 3/32" = 1'-0"

- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
 - SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/56.1.
 - TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C-1/8". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
 - + DENOTES BEAM SPLICE LOCATIONS PER 3/56.2.
 - + DENOTES COLUMN SPLICE LOCATIONS PER 20/56.4.

I:\PCC\TRUSS.7.DWG Thu Nov 30 13:24:52 1995



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Convention Center Authority
Honolulu, Hawaii

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Honolulu, Hawaii

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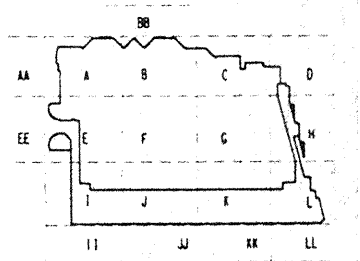
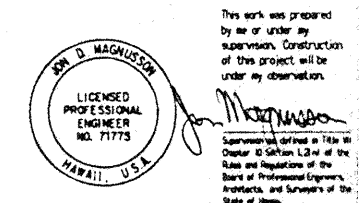
Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

Consultants

SKELLING WARD MAGNUSON BULLSHIRE INC.

Consulting Structural and Civil Engineers
1301 Firm Avenue, Suite 3300 Seattle WA 98101-2999
206/292-1000 FAX: 206/433-1001

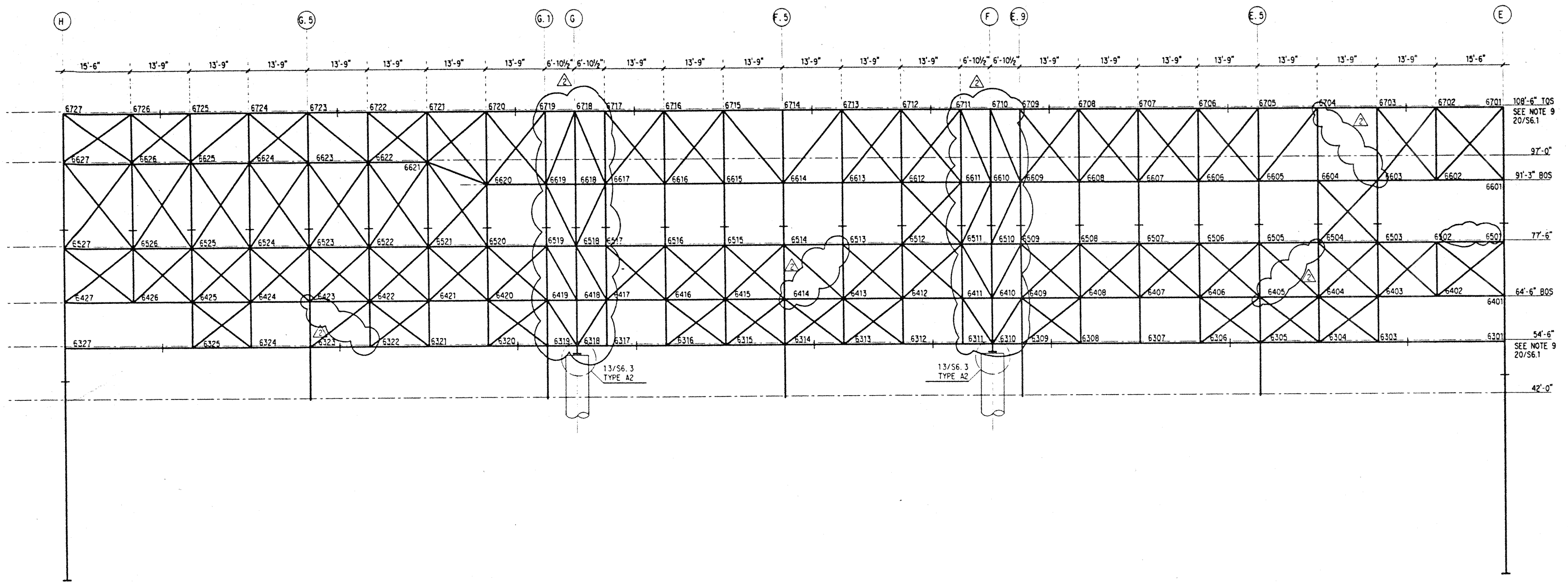


No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
6-19-95			
5-8-95			ADDENDUM #3

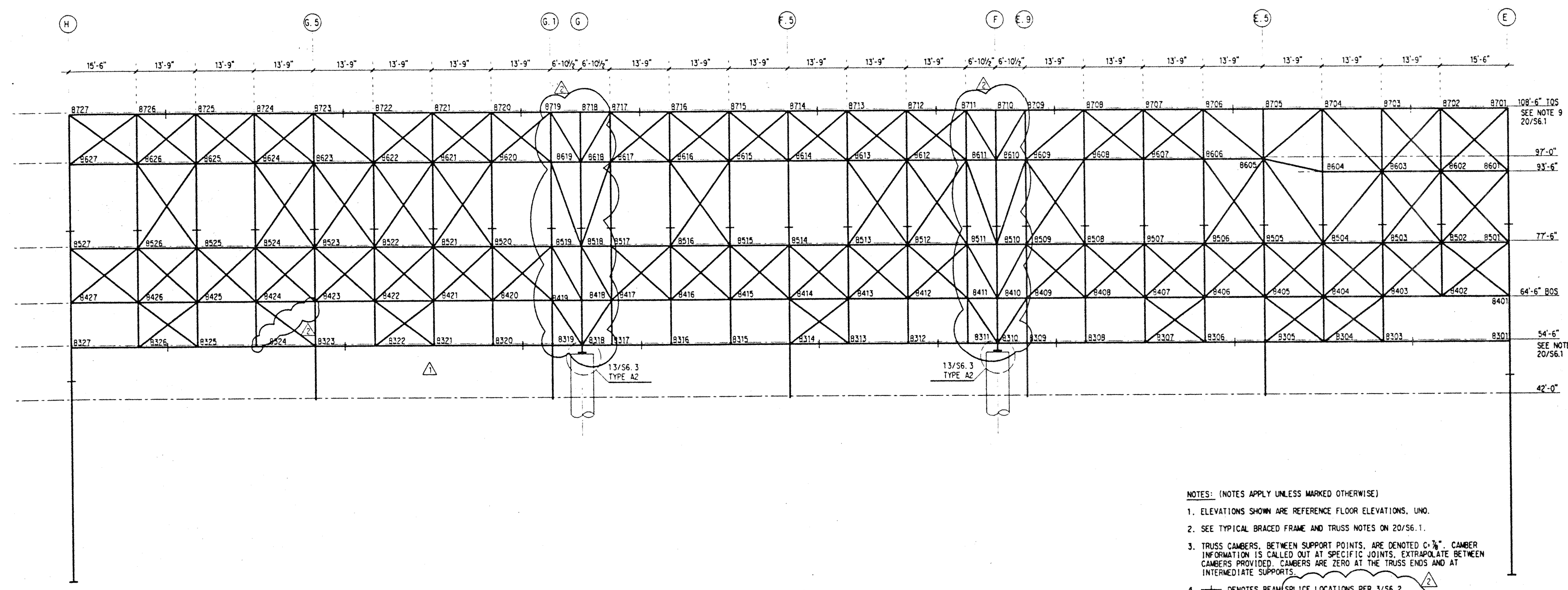
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TRUSS ELEVATIONS - GRIDS 6 & 8

Designed **BJM**
 Drawn **DMS**
 Checked **JDM**
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



TRUSS ON GRID 6
SCALE: 3/32" = 1'-0"



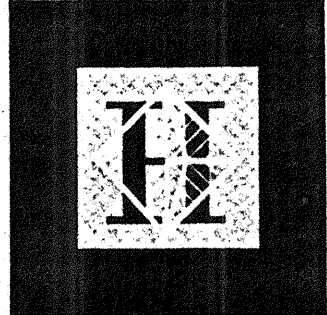
TRUSS ON GRID 8
SCALE: 3/32" = 1'-0"

- NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)
- ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
 - SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
 - TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C-7/8". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
 - + DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
 - + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

Thu Nov 30 13:27:03 1995

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9/5 pit



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Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

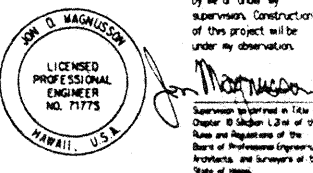
Loschky Marquardt & Nesholm
Seattle, Washington

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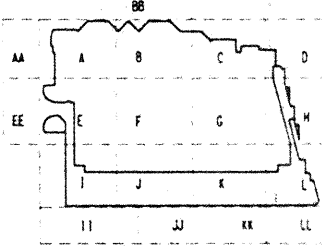
Consultants

STELLING WARD MAGNUSON BAKSHISE INC.

Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 2000 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 823-1201



This work was prepared by me or under my supervision. Construction of this project will be under my observation.
S.W. Magnuson
S.W. Magnuson is a Licensed Professional Engineer in the State of Hawaii, License No. 71778. He is also a Licensed Professional Engineer in the State of Washington, License No. 12484. He is a member of the American Institute of Steel Construction, Inc. and the American Institute of Architects.



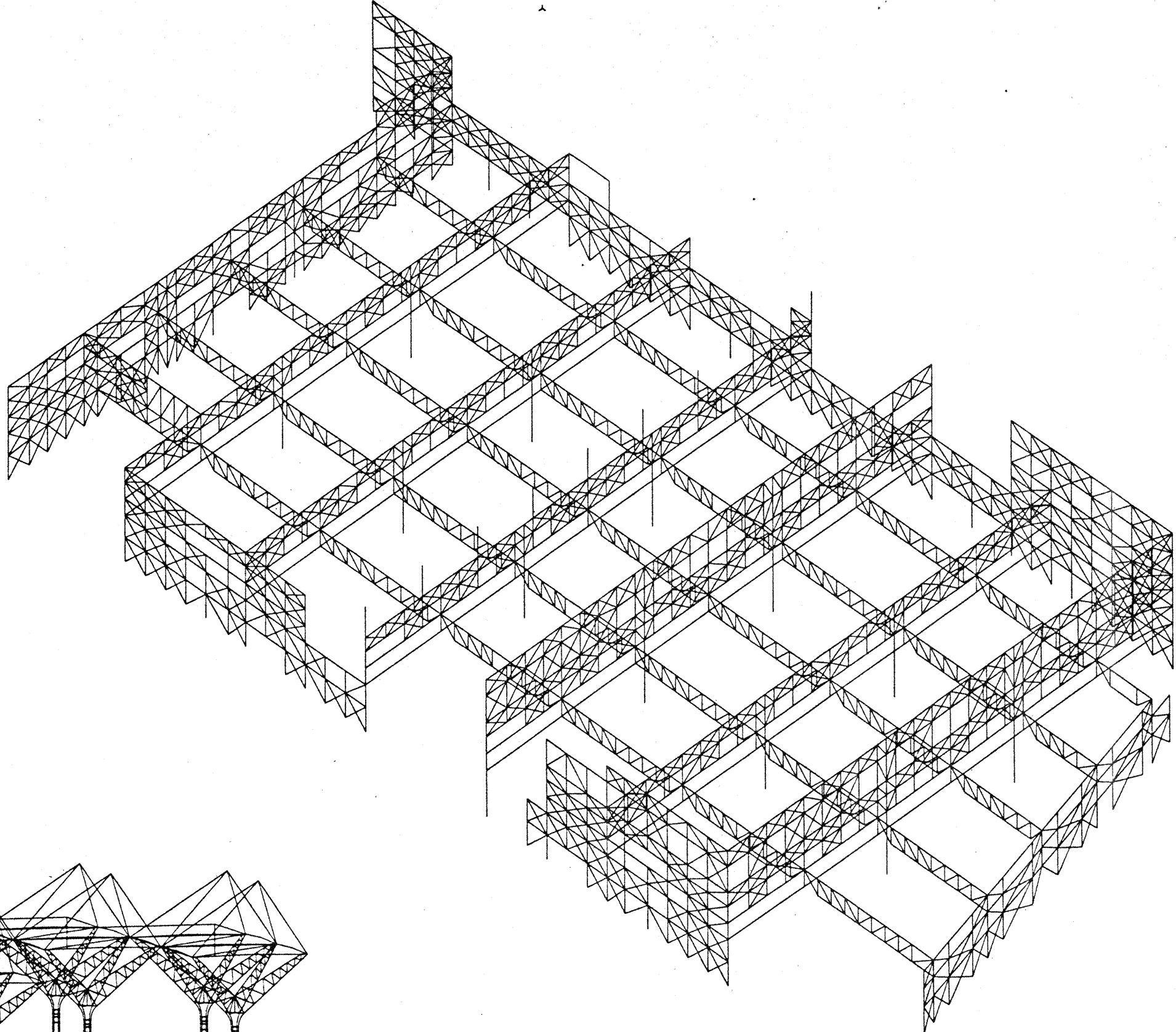
No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
6-19-95			

Sheet Title

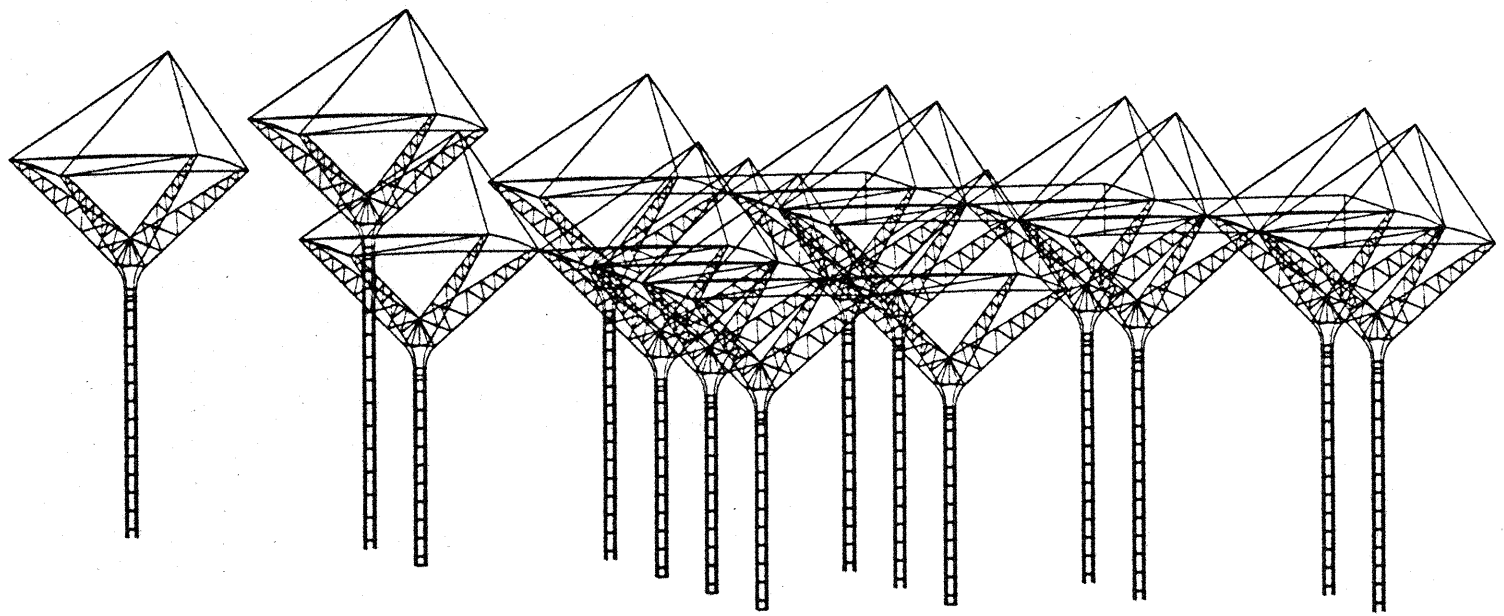
STRUCTURAL FRAME ISOMETRIC

Designed BJM
 Drawn DMS
 Checked JDM
 Date 6 / 19 / 95
 Job No. 93036.02.9
 Sheet No.

S6.0



STRUCTURAL FRAME ISOMETRIC

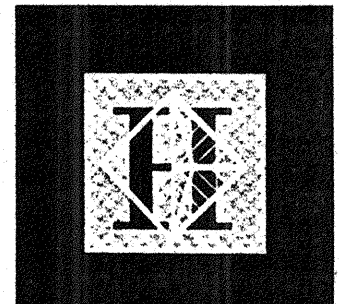


LOBBY ISOMETRIC

Thu Nov 30 13:31:51 1995

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lms pit



Hawai'i Convention Center

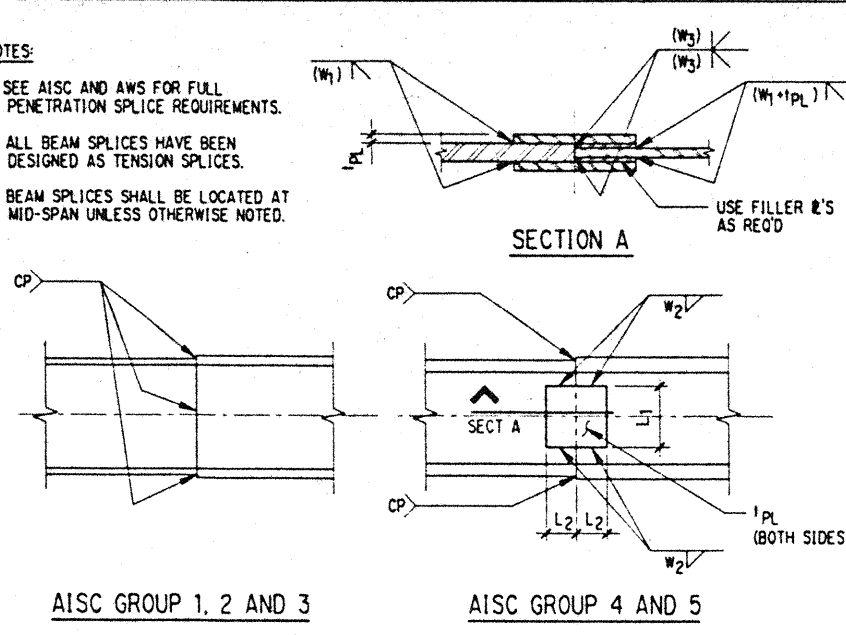
Convention Center Authority
Honolulu, Hawaii

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SKILLING WARD MAGNUSON BUILDERS INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 823-1201

- NOTES:
1. SEE AISC AND AWS FOR FULL PENETRATION SPLICE REQUIREMENTS.
 2. ALL BEAM SPLICES HAVE BEEN DESIGNED AS TENSION SPLICES.
 3. BEAM SPLICES SHALL BE LOCATED AT MID-SPAN UNLESS OTHERWISE NOTED.



BEAM SPLICE SCHEDULE						
BEAM SIZE	L1 (IN)	L2 (IN)	1 PL (IN)	W1 (IN)	W2 (IN)	W3 (IN)
W14x730	10	5	1/2	1/4	1/2	1/2
W14x665	10	5	1/2	1/4	1/2	1/2
W14x605	10	5	1/2	1/4	1/2	1/2
W14x550	10	5	1/4	1	1/2	1/2
W14x500	10	5	1/4	1	1/2	1/2
W14x455	10	5	1	3/8	3/8	1/2
W14x426	10	5	1	3/8	3/8	1/2
W14x398	10	5	1	3/8	3/8	1/2
W14x370	10	5	1	3/8	3/8	1/2
W14x342	10	5	3/8	1/2	1/4	1/2
W14x311	10	5	3/8	1/2	1/4	1/2
W14x283	10	5	1/2	3/8	1/4	3/8
W14x257	10	5	1/2	3/8	1/4	3/8
W14x233	10	5	1/2	3/8	1/4	3/8
W12x252	8	5	1/2	3/8	1/4	3/8

BEAM SPLICE DETAILS

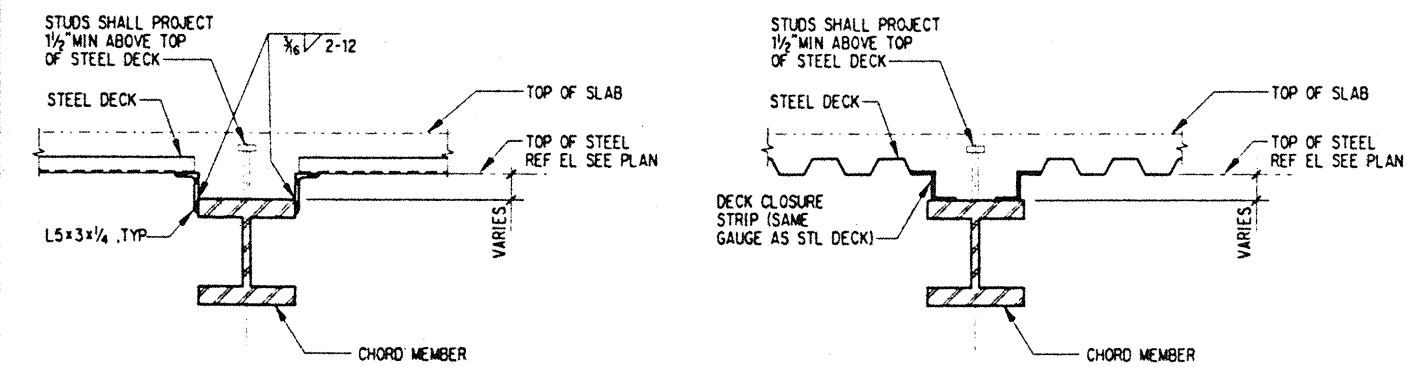
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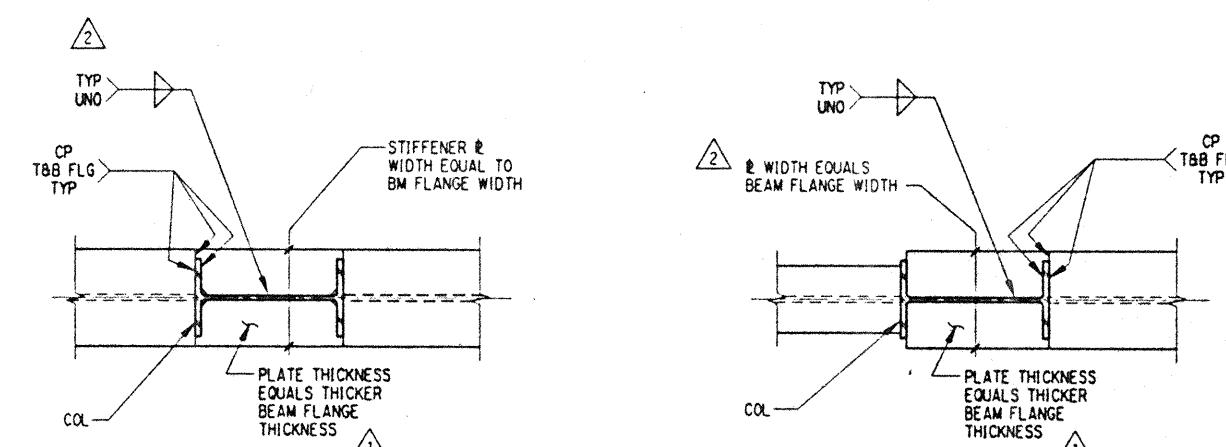
SECTION	MEMBER SIZE	b1 (IN)	1/2 (IN)	W2 (IN)
BU14x1074	W14x730	17.89	4	3/8
BU14x988	W14x730	17.89	3	1/2
BU14x902	W14x730	17.89	2	1/2
BU14x817	W14x730	17.89	1	1/2
BU36x527	W36x300	--	1	1/2
BU36x641	W36x300	16.65	1/2	1/2

NOTE: ADDED PLATES EXTEND 3'-0" ABOVE TOP OF STEEL @ BU14 UNLESS NOTED OTHERWISE.



DECK SPAN NORMAL TO TRUSS CHORD

DECK SPAN PARALLEL TO TRUSS CHORD



BEAM FLANGES WIDER THAN COLUMN FLANGES BOTH SIDES

BEAM FLANGES WIDER THAN COLUMN FLANGES ON ONE SIDE

NOTE: BRACE GUSSET & S NOT SHOWN FOR CLARITY.

BUILT-UP MEMBER

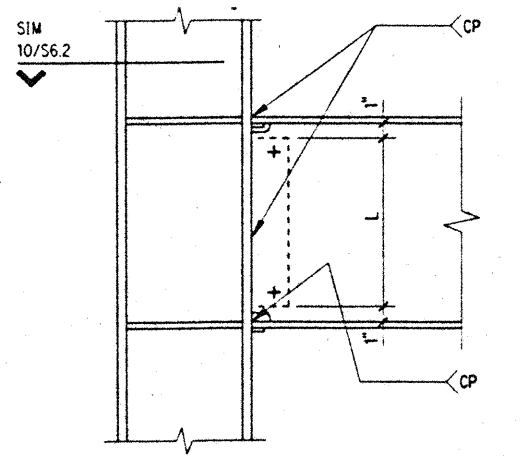
6

TYPICAL STEEL DECK SUPPORT AT CHORD MEMBERS

8

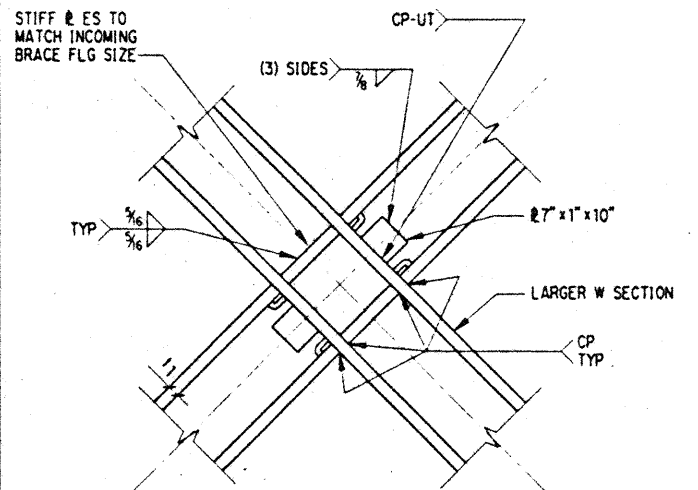
BEAM FLANGE DETAILS - TYPE A & C

10



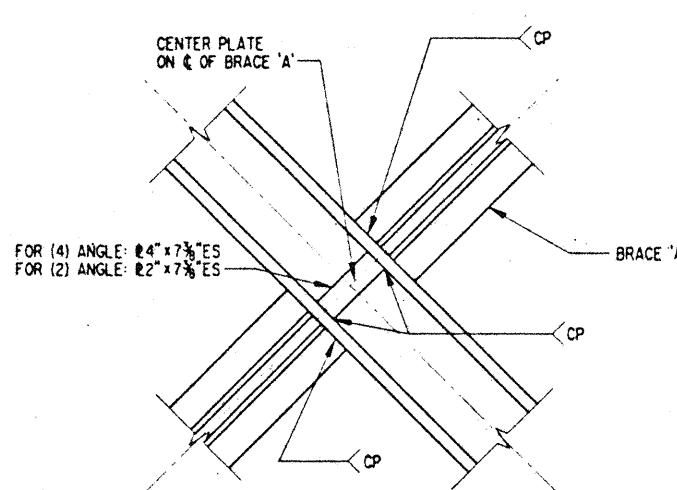
NOTE: PROVIDE 3/4x5 L ERECTION & W/ (2) 1/4 A490 BOLTS FOR W14'S OR (3) 1/4 A490 BOLTS FOR LARGER BEAMS. WELD ERECTION & TO COL SECTION W/ 3/16 FILLET WELD BOTH SIDES.

DETAIL KEY FOR TYPE A & C



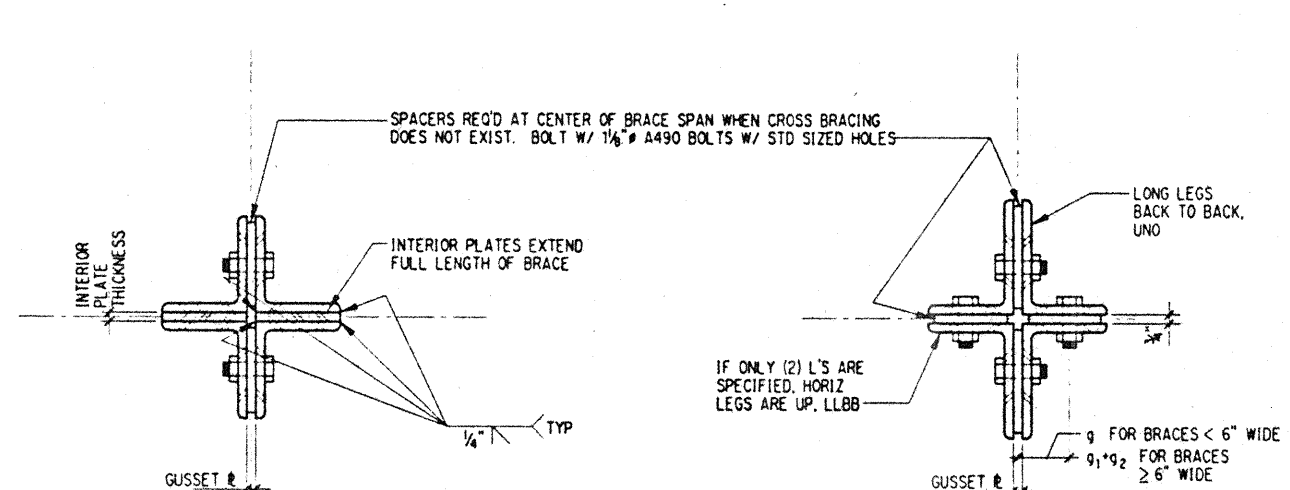
NOTE: THE LARGEST WIDE FLANGE BRACE SHALL BE CONTINUOUS THRU JOINT.

DOUBLE W-SECTION



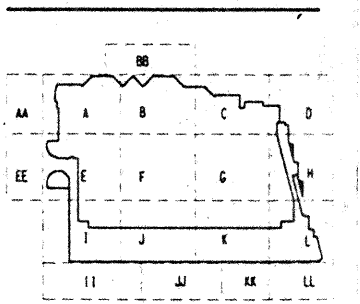
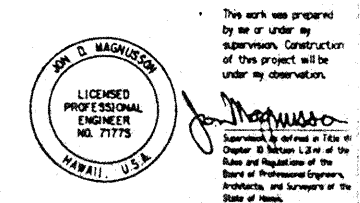
NOTE: BRACE 'A' SHALL HAVE A COMPLETE PENETRATION WELD TO THE W-SECTION.

W-SECTION



WITH PLATES

WITHOUT PLATES



Revisions

MOMENT CONNECTIONS

11

BRACE CONNECTION DETAIL

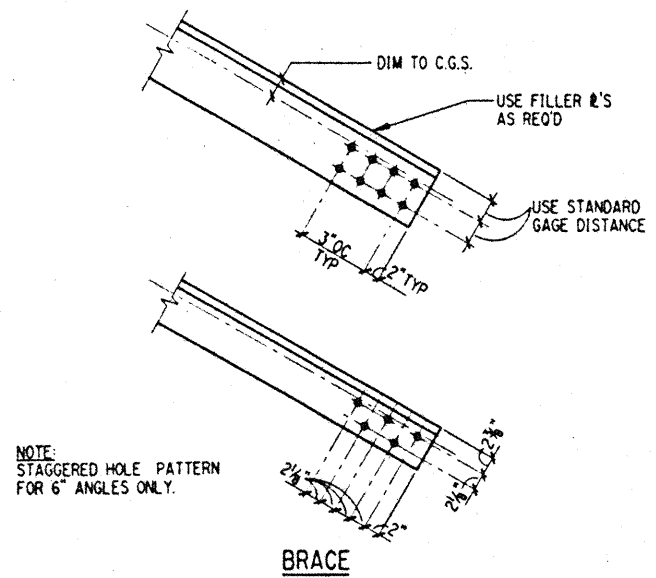
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BRACE CONNECTION DETAIL

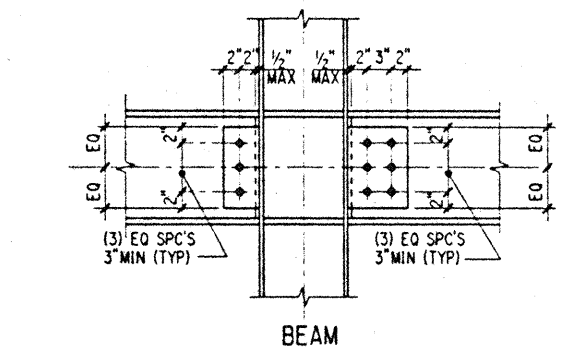
13

ANGLE BRACING DETAILS

15



NOTE: STAGGERED HOLE PATTERN FOR 6" ANGLES ONLY.



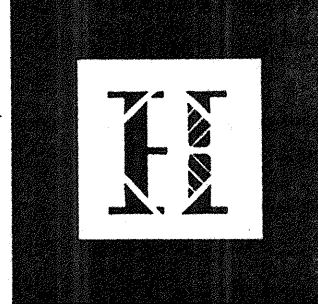
STANDARD GAGE DISTANCE							
LEG LENGTH	8"	7"	6"	5"	4"	3 1/2"	3"
g	4.5	4.0	3.5	3.0	2.5	2.0	1.75
g1	3.0	2.5	2.375	-	-	-	-
g2	3.0	3.0	2.125	-	-	-	-

NOTE: ALL BOLT HOLES IN BRACES ARE STANDARD SIZE HOLES.

EL BRACE MARK	ACTUAL BUILT-UP SHAPE	EL BRACE MARK	ACTUAL BUILT-UP SHAPE
4L8-8-18-PLA	(4) L8x8x1-1/8 with (2) 1-1/2x8 PLS	2L6-6-16	(2) L6x6x1
4L8-8-18-PLB	(4) L8x8x1-1/8 with (2) 1-1/4x8 PLS	2L6-6-14	(2) L6x6x7/8
4L8-8-18-PLC	(4) L8x8x1-1/8 with (2) 1x8 PLS	2L6-6-12	(2) L6x6x3/4
4L8-8-18-PLD	(4) L8x8x1-1/8 with (2) 3/4x8 PLS	2L5-6-10	(2) L6x6x5/8
4L8-8-18-PL E	(4) L8x8x1-1/8 with (2) 1/2x8 PLS	2L5-6-8	(2) L6x6x1/2
4L8-8-18	(4) L8x8x1-1/8	2L5-6-6	(2) L6x6x3/8
4L8-8-16	(4) L8x8x1	2L6-4-12	(2) L6x4x3/4
4L8-8-14	(4) L8x8x7/8	2L6-4-8	(2) L6x4x3/8
4L8-8-12	(4) L8x8x3/4	2L5-4-8	(2) L6x4x1/2
4L8-8-10	(4) L8x8x5/8	2L5-4-6	(2) L6x4x3/8
4L8-8-8	(4) L8x8x1/2	2L5-5-8	(2) L5x5x1/2
4L6-6-10	(4) L6x6x5/8	2L5-5-5	(2) L5x5x5/16
4L6-6-6	(4) L6x6x3/8	2L4-4-12	(2) L4x4x3/4
4L5-5-5	(4) L5x5x5/16	2L4-4-10	(2) L4x4x5/8
4L4-4-8	(4) L4x4x1/2	2L4-4-8	(2) L4x4x1/2
4L4-4-5	(4) L4x4x5/16	2L4-4-6	(2) L4x4x3/8
4L4-4-4	(4) L4x4x1/4	2L4-4-5	(2) L4x4x5/16
2L8-8-18	(2) L8x8x1-1/8	2L4-4-4	(2) L4x4x1/4
2L8-8-16	(2) L8x8x1	2L3-5-3-5-6	(2) L3-1/2x3-1/2x3/8
2L8-8-14	(2) L8x8x7/8	2L3-5-3-5-5	(2) L3-1/2x3-1/2x5/16
2L8-8-12	(2) L8x8x3/4	2L3-3-3-5-4	(2) L3x3x1/2
2L8-8-10	(2) L8x8x5/8	2L3-3-8	(2) L3x3x1/2
2L8-8-8	(2) L8x8x1/2	2L3-3-6	(2) L3x3x3/8
2L8-6-16	(2) L8x6x1	2L3-3-5	(2) L3x3x5/16
2L8-6-12	(2) L8x6x3/4	2L3-3-4	(2) L3x3x1/4
2L8-6-8	(2) L8x6x1/2		
2L7-4-12	(2) L7x4x3/4		
2L7-4-10	(2) L7x4x5/8		
2L7-4-8	(2) L7x4x1/2		

ANGLE BRACE CONVERSION TABLE

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 Drawn **DMS**
 Checked **JDM**
 Date **4 / 12 / 95**
 Job No. **93036.02.9**
 Sheet No.



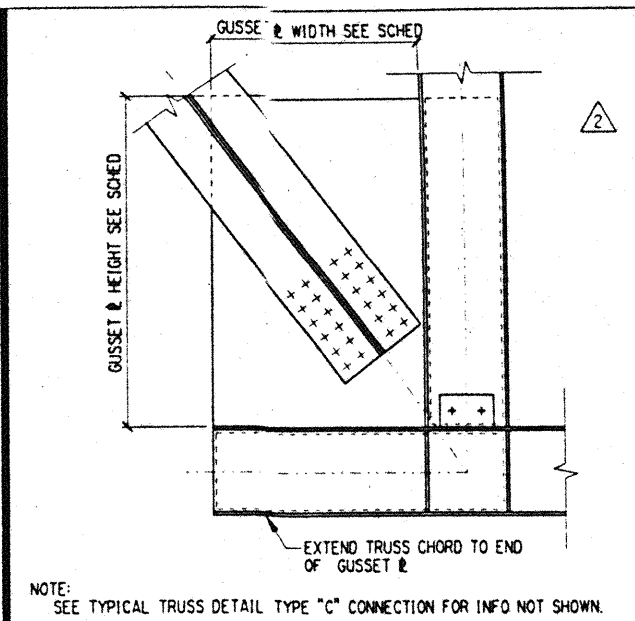
Hawaii Convention Center

Convention Center Authority
Honolulu, Hawaii

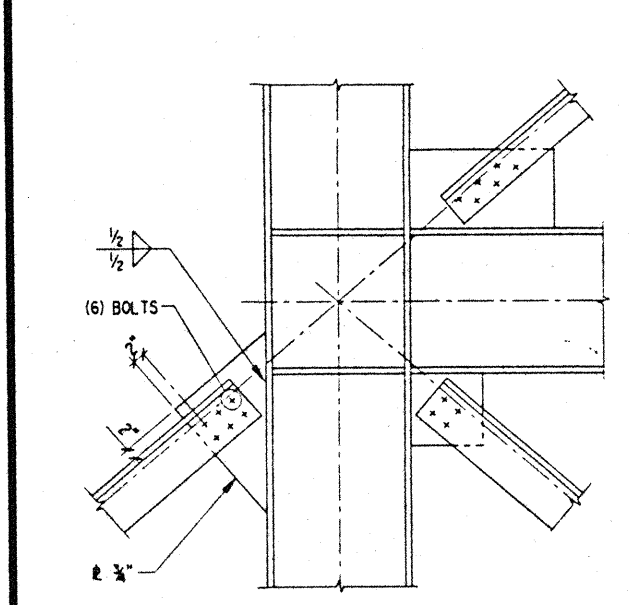
Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
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Seattle, Washington

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Honolulu, HI

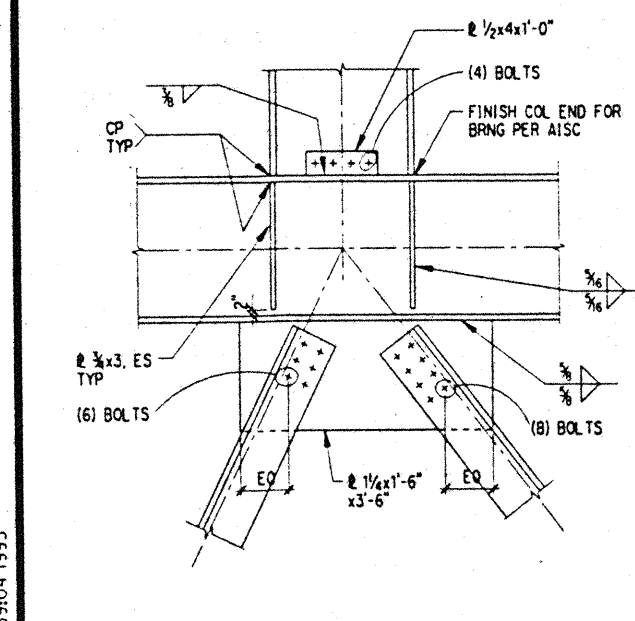
Consultants
SKELING WARD MAGNUSON BARRKNEBE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2698
(206) 282-1200 FAX: (206) 823-1201



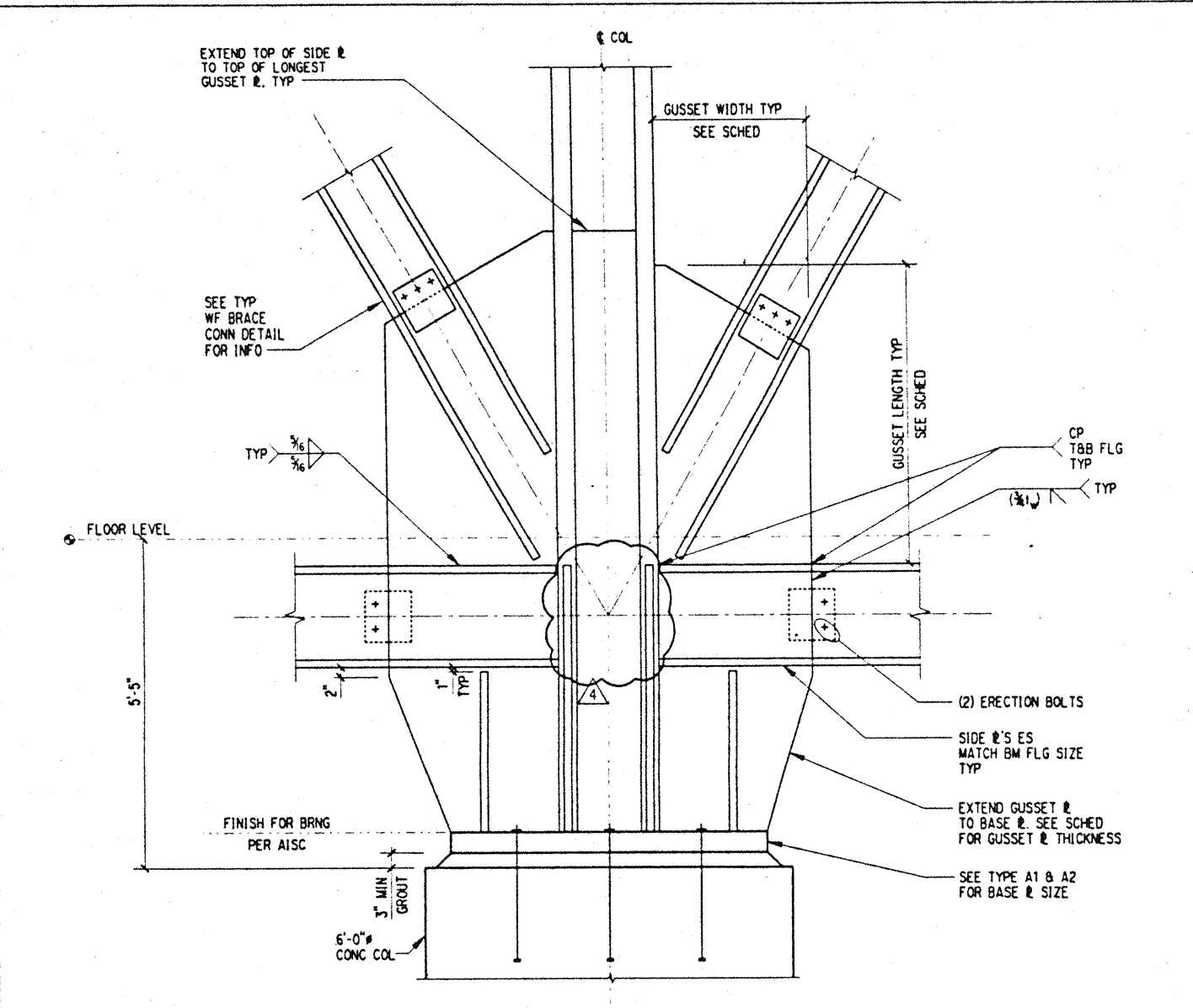
DETAIL 1



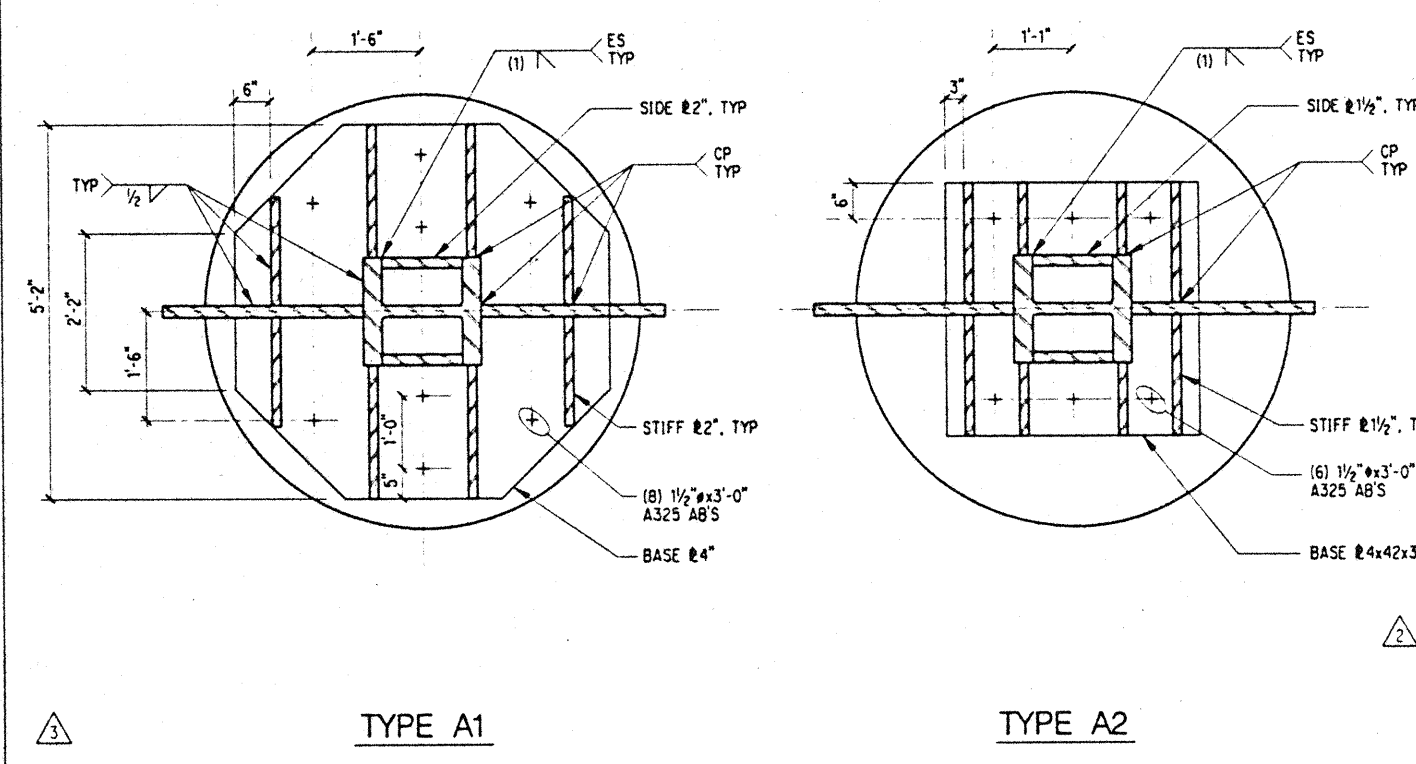
DETAIL 6



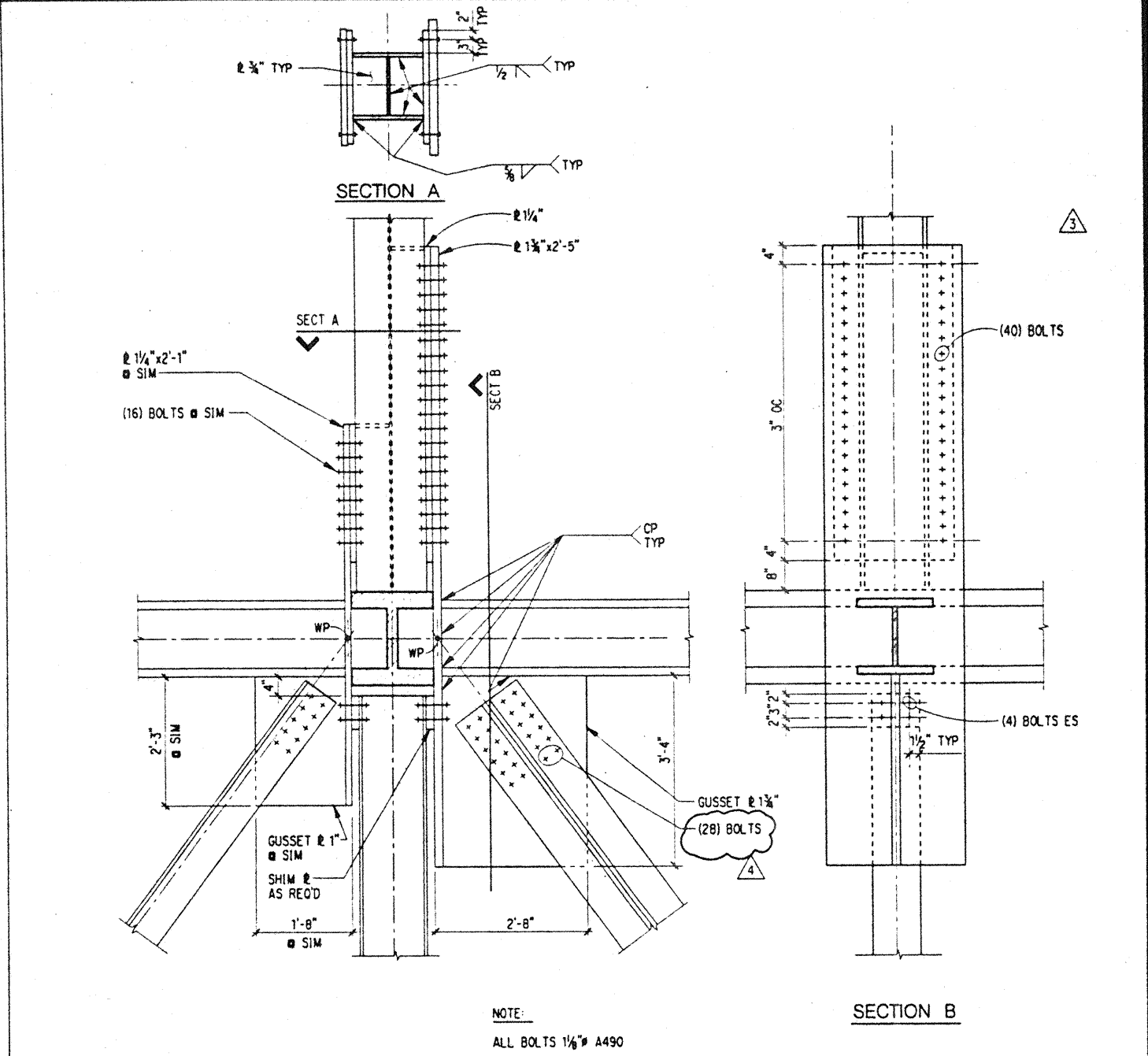
DETAIL 11



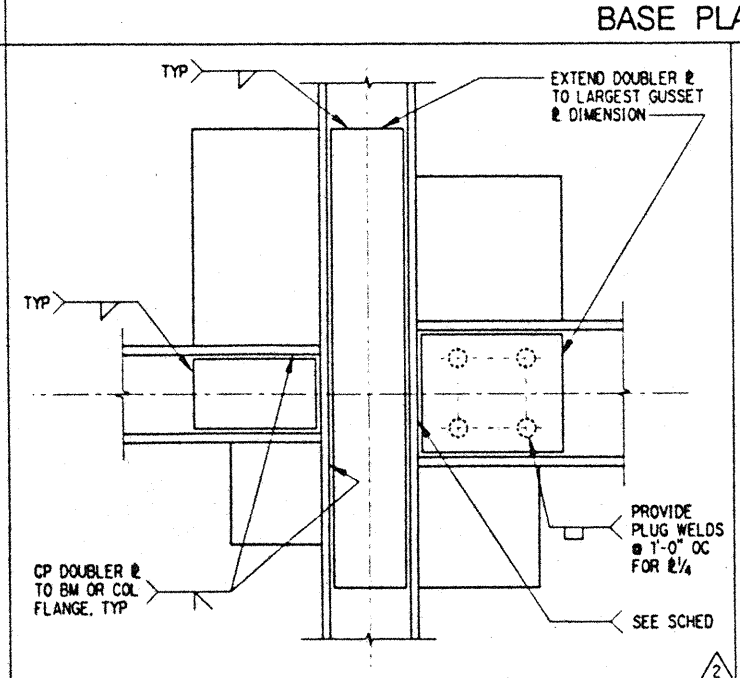
BASE PLATE - TYPE A



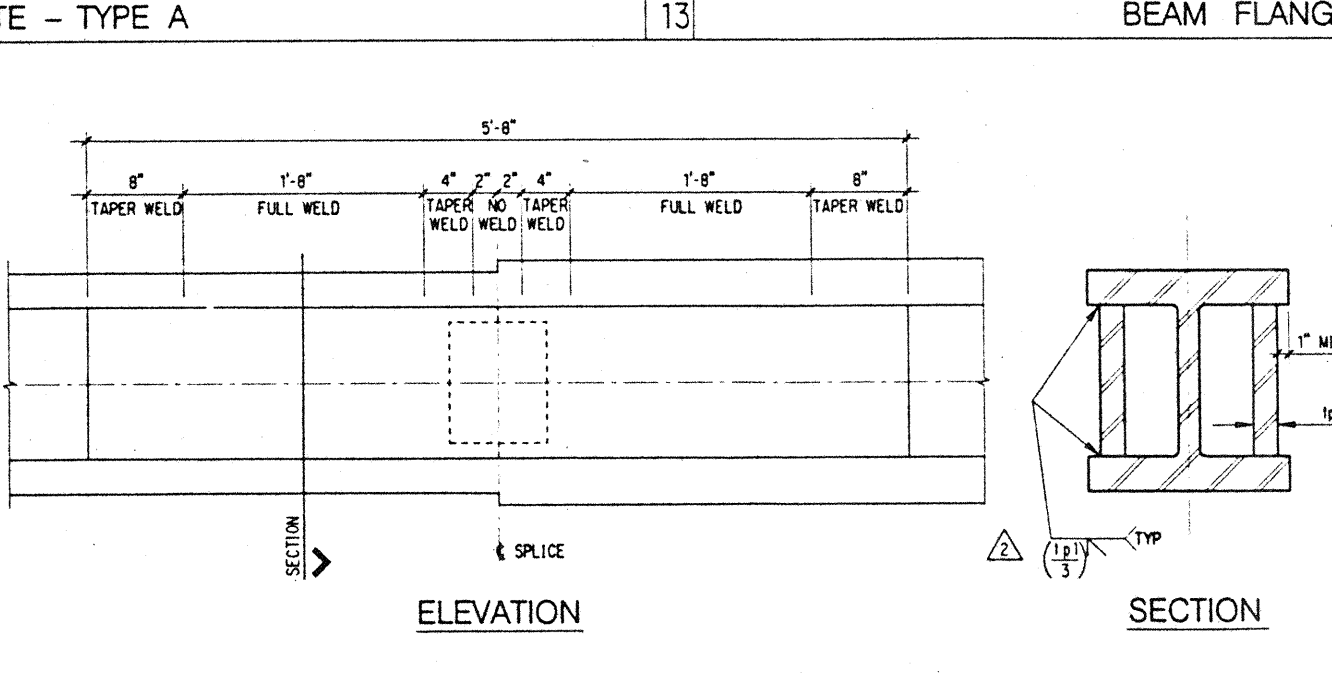
TYPE A1 TYPE A2



BEAM FLANGE DETAILS - TYPE B

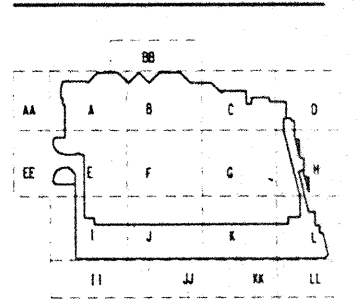
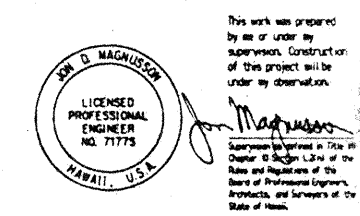


TYPICAL DOUBLER PLATE DETAIL



TENSION SPLICE PLATES

- NOTES:
- SPLICE PLATES SHALL BE INSTALLED AFTER BEAM SPLICE DETAIL 3/56.2 IS COMPLETE AND HAS BEEN FULLY INSPECTED.
 - ALL PLATES SHALL BE Fy=50 KSI, ASTM A572 GR 50, A441 Fy=50 OR A588 Fy=50.
 - ALL PLATE MATERIAL TO BE TESTED FOR CHARPY V-NOTCH TESTING IN ACCORDANCE WITH ASTM A6, SUPPLEMENTARY REQUIREMENT S5.
 - ALL BEVELS SHALL HAVE A MILLED FACE.
 - PARTIAL PENETRATION WELD SIZES ARE CALLED OUT AS EFFECTIVE THROAT THICKNESS.
 - ALL PLATES SHALL BE ULTRASONICALLY TESTED FOR LAMINAR DEFECTS USING ASTM A578 - LEVEL 1.
 - ALL WELDS SHALL BE M-T TESTED AFTER THEY ARE COMPLETE.
 - SEE FRAME AND TRUSS ELEVATIONS FOR LOCATIONS AND SIZE OF TENSION SPLICE PLATES.



No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
8-31-95			
8-4-95			
6-19-95			
5-8-95			ADDENDUM #3

Revisions

Sheet Title

TYPICAL BRACED FRAME & TRUSS DETAILS

Designed: BJM
 Drawn: DMS
 Checked: JDM
 Date: 4 / 12 / 95
 Job No.: 93036.02.9
 Sheet No.:

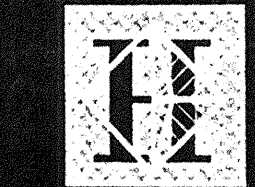
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jwccbrfrat_3.dgn
LWS: pit

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL OT	LEFT BEAM SHEAR PLATE				RIGHT BEAM SHEAR PLATE				UPPER COLUMN SHEAR PLATE				LOWER COLUMN SHEAR PLATE				BRACE BOLTS			UP-LEFT GUSSET PLATE			UP-RIGHT GUSSET PLATE			DN-LEFT GUSSET PLATE			DN-RIGHT GUSSET PLATE			DOUBLER PLATES					
	NBL	SPTL	SPWL	SPLL	NBR	SPTR	SPWR	SPLR	NBU	SPTU	SPWU	SPLU	NBD	SPTD	SPWD	SPLD	BUL	BUR	BLR	GLUL	GLUL	GLUL	GLUR	GLUR	GLUR	GLLL	GLLL	GLLL	GLLR	GLLR	GLLR	GBRL	GBRR	DBRC			
7309 YZ C	3	.375	4.63	26.00	3	.375	4.63	32.00	2	.375	4.63	11.00	2	.375	4.63	11.00	3	6	6				1.25	51.0	28.0	1.75	33.0	52.0	1.75	26.0	44.0	1.75	54.0	34.0	1.25	1.00	.75
7409 YZ A																																					
7509 YZ C																																					
7310 YZ A	2	.375	4.63	11.00	2	.375	4.63	11.00									3	6	6	2.00	30.0	54.0	2.25	33.0	58.0							1.00	1.00	1.00			
7410 YZ A	3	.375	4.63	29.00	3	.375	4.63	26.00																													
7510 YZ C													3	.500	4.63	10.00							1.50	26.0	40.0	.50	10.0	23.0									
7311 YZ C	3	.375	4.63	31.00	3	.375	4.63	29.00	3	.500	4.63	10.00					3	3	3	1.75	51.0	28.0	1.25	15.0	11.0	.313	.313		1.75	30.0	51.0	.75	.75	1.00			
7411 YZ A																																					
7511 YZ C													2	.375	4.63	11.00							1.50	39.0	28.0												
7312 YZ C	11	.750	7.63	25.00	13	.750	7.63	31.00	2	.375	4.63	11.00					12	28	28	1.50	35.0	18.0	1.50	51.0	28.0				1.75	42.0	28.0			1.00			
7412 YZ A																																					
7512 YZ C																																					
7313 YZ C	3	.375	4.63	21.00	3	.375	4.63	26.00	2	.375	4.63	11.00					12	28	32	.75	26.0	20.0	1.75	46.0	28.0				1.50	45.0	30.0			1.00			
7413 YZ A																																					
7513 YZ C													2	.375	4.63	11.00							1.50	37.0	30.0	.75	17.0	13.0	.250	.250							
7314 YZ C	3	.375	4.63	21.00	3	.375	4.63	21.00	2	.375	4.63	11.00					36	32	32	1.50	40.0	32.0	1.50	37.0	30.0				1.00	22.0	18.0			.25			
7414 YZ A																	3	4	10	.75	22.0	10.0	.75	24.0	12.0						.25						
7514 YZ C													3	.500	4.63	10.00																					
7315 YZ C	3	.375	4.63	21.00	3	.375	4.63	21.00	2	.375	4.63	11.00					10	10	10	1.50	56.0	32.0	1.00	23.0	18.0									.25			
7415 YZ A																	36	36	36										1.25	22.0	17.0			1.00			
7515 YZ C													2	.375	4.63	11.00							1.50	56.0	32.0				1.25	22.0	17.0			.250			
7316 YZ C	3	.375	4.63	31.00	7	.500	7.63	19.00	2	.375	4.63	11.00					32	10	10	1.25	55.0	30.0	1.25	28.0	16.0				1.50	50.0	32.0			.25			
7416 YZ A																															1.25						
7516 YZ C													2	.375	4.63	11.00							1.25	28.0	16.0				1.50	50.0	32.0			1.25			
7317 YZ C	3	.375	4.63	31.00	3	.375	4.63	31.00	3	.500	4.63	10.00					3	3	3	1.00	18.0	11.0	1.25	55.0	32.0				1.25	30.0	46.0			.25			
7417 YZ A																															1.00						
7517 YZ C													2	.375	4.63	11.00							1.25	55.0	32.0				1.25	42.0	30.0			.75			
7318 YZ A	2	.375	4.63	11.00	2	.375	4.63	11.00									3	3	3	3.00	34.0	61.0	1.75	30.0	54.0							1.50	1.00	1.50			
7418 YZ A	3	.375	4.63	31.00	3	.375	4.63	31.00									10	10	10	1.25	19.0	21.0															
7518 YZ C													3	.500	4.63	10.00																					
7319 YZ C	3	.375	4.63	31.00	3	.375	4.63	31.00	2	.375	4.63	11.00					3	3	3	1.50	55.0	33.0							2.75	34.0	60.0			.75			
7419 YZ A																													1.25	16.0	28.0			1.25			
7519 YZ C													2	.375	4.63	11.00							1.75	54.0	31.0				1.25	16.0	28.0			1.25			
7320 YZ C	3	.375	4.63	31.00	3	.375	4.63	31.00	2	.375	4.63	11.00					3	36	36	1.75	53.0	32.0	1.75	54.0	31.0				1.75	52.0	33.0			.50			
7420 YZ A																													1.50	52.0	33.0			1.00			
7520 YZ C													2	.375	4.63	11.00							1.75	53.0	32.0				1.75	52.0	33.0			1.00			
7321 YZ C	3	.375	4.63	26.00	3	.375	4.63	31.00	2	.375	4.63	11.00					10	9	9	1.00	24.0	17.0	1.75	56.0	37.0				1.75	56.0	37.0			1.00			
7421 YZ A																	4	9	9	1.75	29.0	13.0	1.75	58.0	33.0				1.00	16.0	10.0	.313	.313	1.25			
7521 YZ C													2	.375	4.63	11.00							1.75	58.0	33.0				1.00	16.0	10.0	.313	.313	1.25			
7322 YZ C	3	.375	4.63	21.00	3	.375	4.63	26.00	2	.375	4.63	11.00					16	9	10	1.50	31.0	22.0	1.75	45.0	37.0				1.25	46.0	32.0			.25			
7422 YZ A																													1.25	23.0	18.0			.25			
7522 YZ C													2	.375	4.63	11.00							1.25	28.0	10.0	.313	.313		1.25	19.0	13.0			.438			
7323 YZ C	3	.375	4.63	21.00	3	.375	4.63	21.00	2	.375	4.63	11.00					36	16	16	.75	40.0	18.0	1.25	44.0	32.0				.50	25.0	12.0	.313	.313	1.50			
7423 YZ A													3	.500	4.63	10.00													1.50	31.0	22.0			1.50			
7523 YZ C													2	.375	4.63	11.00							1.25	44.0	32.0				.50	25.0	12.0	.313	.313	1.50			
7324 YZ C	4	.500	4.63	13.00	9	.750	7.63	19.00	2	.375	4.63	11.00					4	16	12	1.50	29.0	12.0	1.50	33.0	22.0				1.50	45.0	28.0			.500			
7424 YZ A																															.500						
7524 YZ C													2	.375	4.63	11.00							1.50	33.0	22.0				1.50	45.0	28.0			.500			
7325 YZ C	3	.375	4.63	21.00	3	.375	4.63	21.00	2	.375	4.63	11.00					36	28	20	1.50	44.0	32.0	1.75	40.0	28.0				1.50	38.0	24.0			.50			
7425 YZ A																	3	28	20	1.50	21.0	10.0	1.50	40.0	28.0				1.25	30.0	22.0			1.50			
7525 YZ C													2	.375	4.63	11.00							1.50	40.0	28.0				1.25	30.0	22.0			1.50			
7326 YZ C	3	.375	4.63	21.00	3	.375	4.63	21.00	2	.375	4.63	11.00					14	16	36	1.50	49.0	19.0	1.50	32.0	24.0				1.50	45.0	32.0			.50			
7426 YZ A																	16	3	36	1.50	33.0	22.0	1.50	33.0	22.0				1.50	48.0	32.0			1.25			
7526 YZ C																																					

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT	PL	OT	LEFT BEAM SHEAR PLATE				RIGHT BEAM SHEAR PLATE				UPPER COLUMN SHEAR PLATE				LOWER COLUMN SHEAR PLATE				BRACE BOLTS				UP-LEFT GUSSET PLATE				UP-RIGHT GUSSET PLATE				DN-LEFT GUSSET PLATE				DN-RIGHT GUSSET PLATE				DOUBLER PLATES																					
			NBL	SPTL	SPWL	SPLL	NBR	SPTB	SPWB	SPLB	NBU	SPTU	SPWU	SPLU	NBD	SPTD	SPWD	SPLD	BUL	BUR	BLL	BUR	GLUL	GLUL	GLUL	GLUL	GLUR	GLUR	GLUR	GLUR	GTLL	GTLL	GTLL	GTLL	DN-LEFT	DN-LEFT	DN-LEFT	DN-LEFT	DN-RIGHT	DN-RIGHT	DN-RIGHT	DN-RIGHT	DOUBLER	DOUBLER	DOUBLER	DOUBLER														
15355	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	32.0	41.0	CP	CP	CP	CP	.25	-	-	-	-												
15356	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	24.0	31.0	CP	CP	CP	CP	.25	-	-	-	-												
15357	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	14.0	18.0	CP	CP	CP	CP	.188	-	-	-	-												
15358	XY	C	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-												
15359	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-												
15360	XY	C	-	-	-	-	-	-	-	-	4	.625	4.63	10.00	.375	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	13.0	17.0	CP	CP	CP	CP	.250	.313	.313	.313	.313	-	-	-	-												
15361	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	25.0	33.0	CP	CP	CP	CP	.188	-	-	-	-																
10319	XY	B	2	1.625	-	-	1.125	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	1.50	39.0	31.0	CP	CP	CP	CP	.188	-	-	-	-	.25	.25	.25	.25	-	-	-	-								
TRUSS T8																																																												
16255	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	1.50	30.0	40.0	CP	CP	CP	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	.50	.50	.50	-	-	-	-				
16256	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	1.50	30.0	40.0	CP	CP	CP	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	.50	.50	.50	-	-	-	-				
16257	XY	C	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313	CP	-	-	-	-	16	4	-	-	1.00	32.0	40.0	CP	CP	CP	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	.50	.50	.50	-	-	-	-				
16258	XY	C	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313	CP	-	-	-	-	16	4	-	-	1.50	24.0	30.0	CP	.375	.438	-	-	.50	14.0	14.0	CP	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-
16259	XY	C	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250	CP	-	-	-	-	6	-	-	-	.50	18.0	22.0	CP	.313	.375	-	-	.50	18.0	22.0	CP	.313	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-
16260	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	16	-	-	-	1.00	24.0	30.0	CP	CP	CP	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
16261	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	32	-	-	-	1.00	30.0	39.0	CP	CP	CP	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
16262	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	32	-	-	-	1.00	30.0	39.0	CP	CP	CP	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
10223	XY	B	3	.500	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	40	-	-	-	-	-	-	-	1.75	51.0	33.0	CP	CP	CP	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
16355	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.50	30.0	38.0	CP	CP	CP	CP	.25	-	-	-	-												
16356	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	32.0	40.0	CP	CP	CP	CP	.25	-	-	-	-												
16357	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	24.0	30.0	CP	.375	.438	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
16358	XY	C	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-												
16359	XY	C	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	18.0	CP	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
16360	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	18.0	22.0	CP	.313	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
16361	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	24.0	30.0	CP	CP	CP	CP	.25	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
16362	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	30.0	38.0	CP	CP	CP	CP	.25	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
10323	XY	B	2	2.125	-	-	1.438	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	1.75	51.0	33.0	CP	CP	CP	CP	.188	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
TRUSS T9																																																												
2223	XY	B	-	-	-	-	3	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-						
16202	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	4	4	-	-	75	15.0	14.0	CP	.500	.500	-	-	.75	14.0	15.0	CP	.500	.500	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
16204	XY	C	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313	CP	-	-	-	-	4	4	-	-	75	17.0	20.0	CP	CP	CP	-	-	.50	13.0	15.0	CP	.250	.250	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
16205	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-								
16206	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-								
16207	XY	C	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	14	-	-	-	1.00	21.0	25.0	CP	CP	CP	-	-	1.00	21.0	25.0	CP	CP	CP	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-								
16208	XY	C	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	CP	-	-	-	-	8	-	-	-	1.00	16.0	16.0	CP	.500	.500	-	-	.50	16.0	16.0	CP	.500	.500	-	-	-	-	-	-	-	-	-	-	.25	.25	.25	.25	-	-	-	-				
2323	XY	B	-	-	-	-	3	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-										
16302	XY	A	4	.500	7.63	10.00	.375	3	.500	4.63	10.00	.250	-	-	-	-	-	-	-	-	4	4	4	-	1.00	37.0	32.0	CP	CP	CP	-	-	.50	17.0	16.0	CP	.250																							



Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tang & Goo, Inc.
Honolulu, Hawaii
in association with

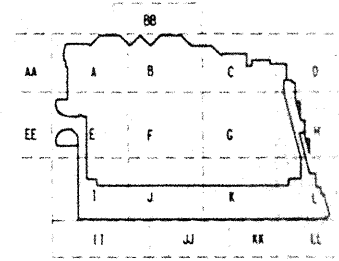
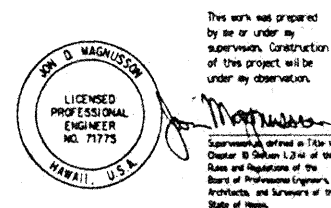
Loschky Marquardt & Nestheim
Seattle, Washington

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Honolulu, HI

Consultants

SKILLING WARD MAGNUSON BARRBERG INC.

Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle, WA 98101-2699
(206) 292-1200 FAX: (206) 623-1201



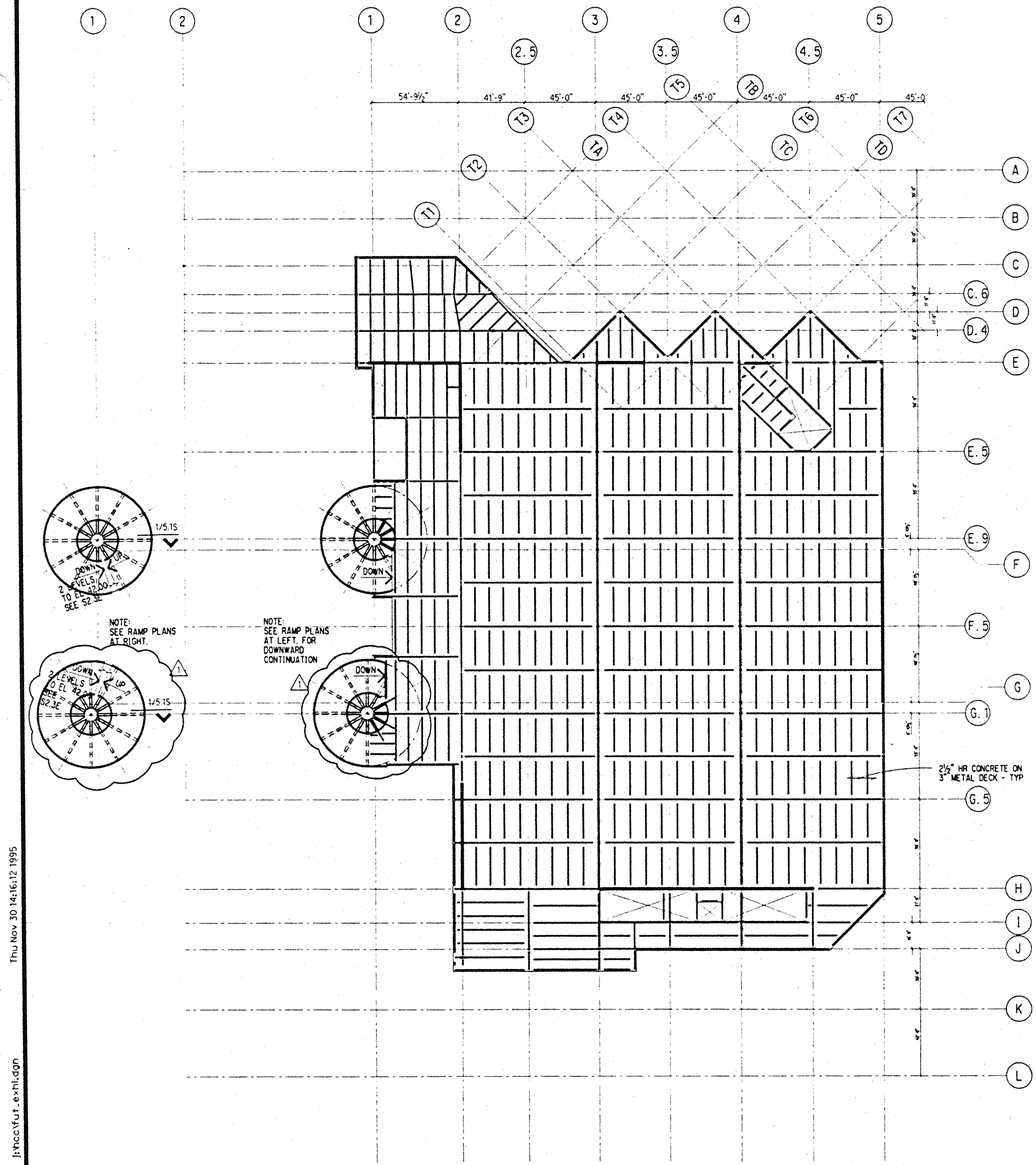
Revisions

No.	Date	By	Description
12-15-95			FINAL DOCUMENTS
9-15-95			BUILDING PERMIT SET
8-4-95			

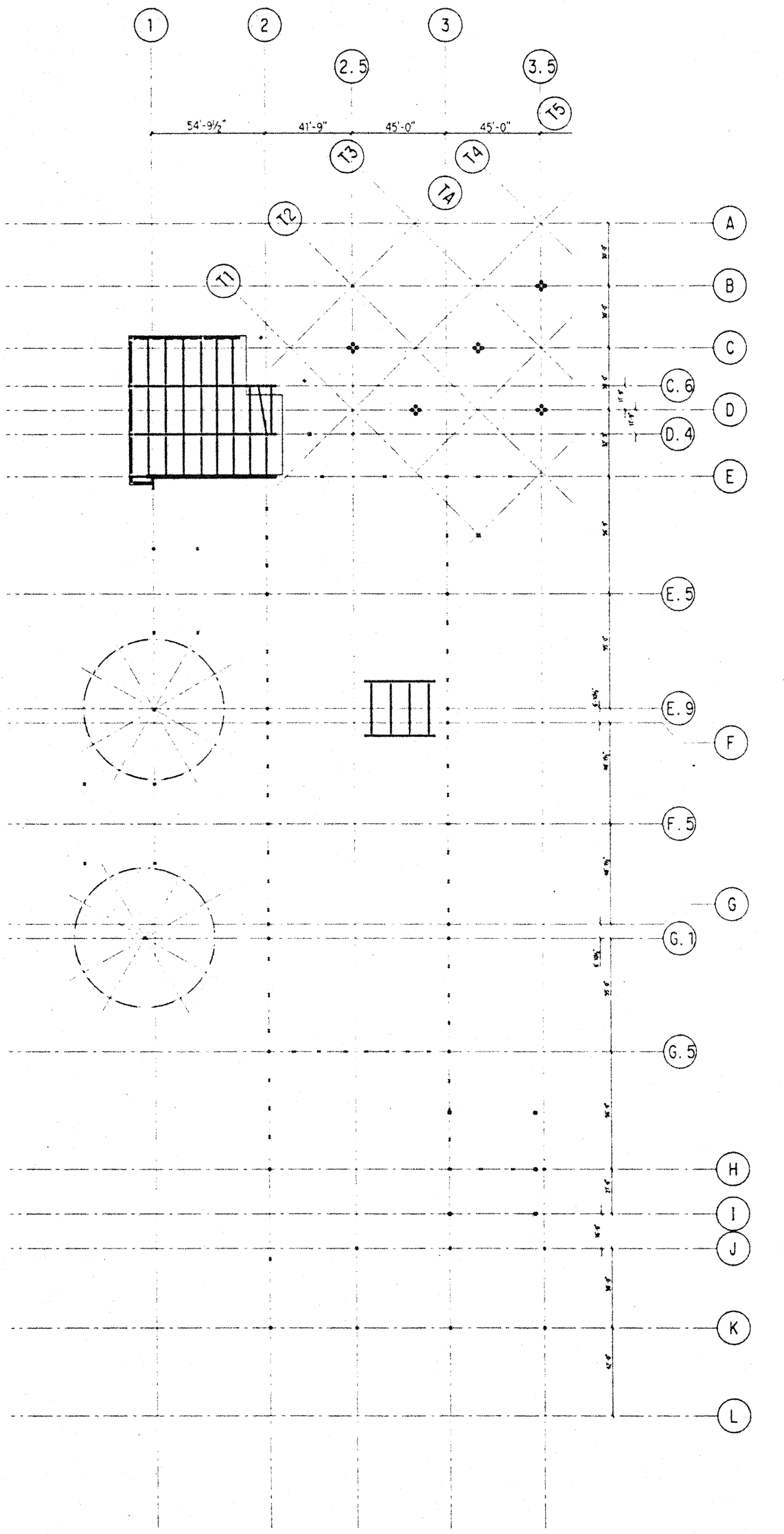
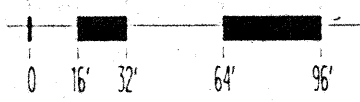
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FUTURE EXHIBIT HALL FRAMING PLAN (NIC)

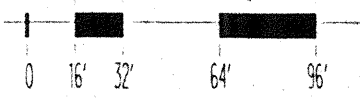
Designed BJM
 Drawn DMS
 Checked JDM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



FUTURE EXHIBIT HALL FRAMING PLAN (NIC) EL 100'-6"



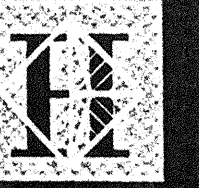
FUTURE MEETING FRAMING PLAN (NIC) EL 77'-6"



Thu Nov 30 14:16:12 1995

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DWS.plt



Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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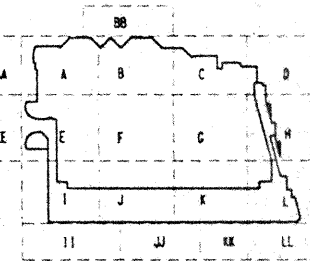
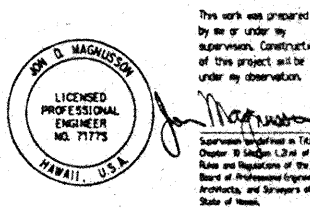
Consultants

SCULLY WARD MAGNUSON BAKER INC.

Consulting Structural and Civil Engineers

1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2899

(206) 282-1200 FAX: (206) 623-1201



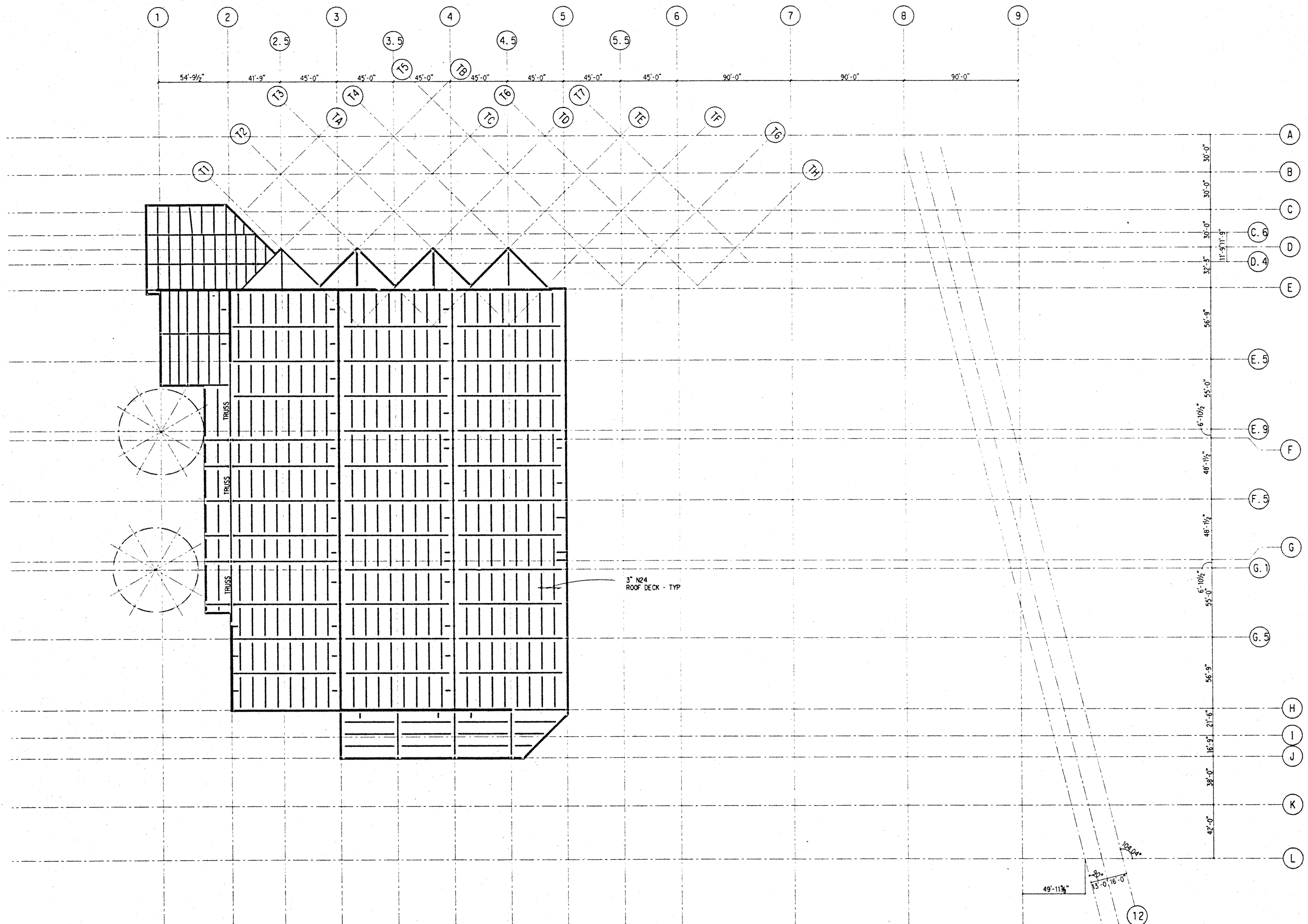
Revisions

No.	Date	By	Description
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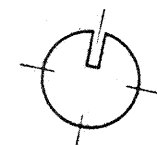
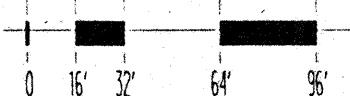
Sheet Title

FUTURE ROOF FRAMING PLAN (NIC)

Designed BJM
 Drawn DMS
 Checked JCM
 Date 4 / 12 / 95
 Job No. 93036.02.9
 Sheet No.



FUTURE ROOF FRAMING PLAN (NIC) EL 140'-6"



Thu Nov 30 14:18:27 1995

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DMS.plt

