Hawai'i Convention Center

Final Documents

DESIGN BUILDER :

NORDIC / PCL
A Joint Venture
Honolulu, HI

ARCHITECTS :

WIMBERLY ALLISON TONG & GOO, INC.
Honolulu, HI

LOSCHKY MARQUARDT & NESHOLM
Seattle, WA

STRUCTURAL CONSULTANT :

SKILLING WARD MAGNUSSON BARKSHIRE, INC.
Seattle, WA

CIVIL / ENVIRONMENTAL :

WILSON OKAMOTO & ASSOCIATES, INC.
Honolulu, HI

LANDSCAPE ARCHITECT :

WALTERS KIMURA MOTODA
Honolulu, HI

LIGHTING :

HORTON-LEES LIGHTING DESIGN, INC.
San Francisco, CA

V.A.C. / ELECTRICAL / PLUMBING :

SYSKA & HENNESSY
Los Angeles, CA

V.A.C. DESIGN BUILDER:

CRITCHFIELD MECHANICAL
Menlo Park, CA

ELECTRICAL DESIGN BUILDER:

LUDVIK ELECTRIC COMPANY
Englewood, CO

PLUMBING DESIGN BUILDER:

SCOTT COMPANY
San Leandro, CA

AV / ACOUSTICAL :

McKAY CONANT BROOK, INC.
Tarzana, CA

ENERGY CODE :

DOUGLAS V. MACMAHON
Honolulu, HI

FOOD SERVICE :

FSA DESIGN
Los Angeles, CA

SECURITY / TELECOMMUNICATIONS :

CON-TECH CONSULTANTS
Dublin, CA

LIFE SAFETY :

ROBERT J. PIELOW ASSOCIATES
Olathe, KS

VERTICAL TRANSPORTATION :

SYSKA & HENNESSY
San Francisco, CA

Structural

CODE INFORMATION

SITE INFORMATION

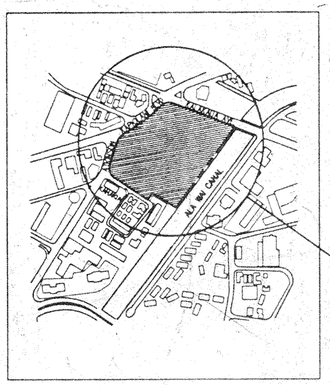
TAX MAP KEY: 2-3-35 : 1

LAND AREA OF PROJECT: 9.7 ACRES

CITY AND COUNTY OF HONOLULU ZONING DESIGNATION: BMX-3

BUILDING INFORMATION

BUILDING CODE:	UNIFORM BUILDING CODE 1991 EDITION AND CHAPTER 5 (AMENDMENTS)
TYPE OF CONSTRUCTION:	TYPE 1 - FIRE-RESISTIVE
BUILDING HEIGHT:	TOP FLOOR HEIGHT = 70'-6"
NUMBER OF STORIES:	4
PARKING:	800 SPACES
OCCUPANCY TYPES:	A-2.1, A-3, B-2, & B-3



VICINITY MAP

Hawai'i Convention Center

Convention Center Authority
Honolulu, HI

Wimberly Allison Tong & Goo, Inc.
Honolulu, HI

in association with

Loschky Marquardt & Nesholm
Seattle, WA

NORDIC / PCL
A Joint Venture
Honolulu, HI

Consultants

This work was prepared by me or under my supervision and construction of this project will be under my observation.

Revisions

No.	Date	By	Description
	12/15/95		FINAL DOCUMENTS
	10/17/95		OCTOBER COORDINATION
	9/15/95		BUILDING PERMIT SET

Sheet Title

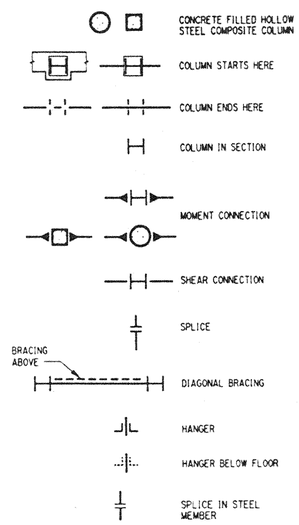
COVER SHEET
STRUCTURAL

Designed
Drawn
Checked
Date

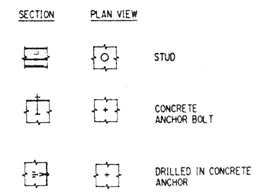
ABBREVIATIONS

AB	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
ADD	ADDITIONAL
ADJ	ADJACENT
AGGR	AGGREGATE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
ALT	ALTERNATE
ALUM	ALUMINUM
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
APA	AMERICAN PLYWOOD ASSOCIATION
APPD	APPROVED
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL: ARCHITECT
ASPT	ASPHALT
ASSY	ASSEMBLY
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
AWS	AMERICAN WELDING SOCIETY
&	AND
L	ANGLE
BD	BOARD
BLDG	BUILDING
BLK	BLOCK: BLOCKING
BM	BEAM
BOT	BOTTOM
BRG	BEARING
BRKT	BRACKET
BT	BENT
BTW	BETWEEN
C	STANDARD CHANNEL
CC	CENTER TO CENTER
CG	CENTER OF GRAVITY
CIP	CAST-IN-PLACE
CJ	CONSTRUCTION JOINT: CONTROL JOINT
CL	CENTERLINE
CLR	CLEARANCE: CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
COMP	COMPRESSION
CONC	CONCRETE
CONN	CONNECTION: CONNECT
CONST	CONSTRUCTION
CONT	CONTINUE: CONTINUOUS
CONTR	CONTRACTOR
CORR	CORRUGATED
CP	COMPLETE PENETRATION WELD-ULTRASONIC TEST
CTR	CENTER
CTSK	COUNTERSINK: COUNTERSUNK
CU FT	CUBIC FOOT
CU IN	CUBIC INCH
CY	CUBIC YARD
d	PENNY (NAILS)
DBL	DOUBLE
DEPT	DEPARTMENT
DET	DETAIL
DIA OR ø	DIAMETER
DIAG	DIAGONAL
DIAPH	DIAPHRAGM
DICA	DRILLED-IN CONCRETE ANCHOR
DIM	DIMENSION
DISC	DISCONTINUED
DN	DOWN
DO	DITTO
DWG	DRAWING
DWL	DOWELS
EA	EACH
EE	EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
ENGR	ENGINEER
EQ	EARTHQUAKE
EQUIP	EQUIPMENT
ETC	ET CETERA
EW	EACH WAY
EXIST	EXISTING
EXT	EXTERIOR
EXTD	EXTEND or EXTENDED
FB	FLAT BAR
FD	FLOOR DRAIN
FDN	FOUNDATION
FF	FAR FACE
FIN	FINISH
FL	FLOOR: FLOOR LINE
FLG	FLANGE
FOS	FACE OF STUD
FP	FIREPROOF: FULL PENETRATION
FT	FOOT: FEET
FTG	FOOTING
GA	GAUGE
GALV	GALVANIZED
GL	GLUE LAMINATED
GRND	GROUND
H or HORIZ	HORIZONTAL
HOSP	HOSPITAL
HP	HP SHAPES: HIGH POINT
HS	HIGH STRENGTH
HT	HEIGHT
ID	INSIDE DIAMETER
I.E.	THAT IS
IF	INSIDE FACE
IN	INCH
INCL	INCLUDE
INFO	INFORMATION
J	JOIST
JT	JOINT
KIP or K	1,000 POUNDS
KO	KNOCK-OUT
KSI	KIPS PER SQUARE INCH
LAB	LABORATORY
LB	POUND
LF	LINEAL FOOT
LIN	LINEAL: LINEAR
LLBB	LONG LEG(S) BACK-TO-BACK
LLV	LONG LEG(S) VERTICAL
LP	LOW POINT
LSL	LONG SLOTTED HOLES
LT WT	LIGHTWEIGHT
M	MISC SHAPE
MACH	MACHINE
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MC	MISCELLANEOUS CHANNEL
MECH	MECHANICAL
MEMB	MEMBRANE
MFR	MANUFACTURE
MIN	MINIMUM: MINUTE
MISC	MISCELLANEOUS
N	NORTH
NF	NEAR FACE
NFPA	NATIONAL FOREST PRODUCTS ASSOC.
NIC	NOT IN CONTRACT
NO. or #	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OPNG	OPENING
OVS	OVER SIZED HOLES
PC	PRECAST
PERP	PERPENDICULAR
PIPEX	PIPE - EXTRA STRONG
PIPEXX	PIPE - DOUBLE EXTRA STRONG
PL	PLATE
PLWD	PLYWOOD
PP	PARTIAL PENETRATION
PREFAB	PREFABRICATED
PS	PRESTRESSED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PVC	POLYVINYL CHLORIDE
#	POUND
RB	RAISER BAR
REINF	REINFORCE: REINFORCING
REQD	REQUIRED
REQMTS	REQUIREMENTS
#	ROUND
S	AMERICAN STANDARD SHAPE
SB	SPACER BAR or SUPPORT BAR
SCHED	SCHEDULE
SECT	SECTION
SHTG	SHEETING
SIM	SIMILAR
SLBB	SHORT LEGS BACK-TO-BACK
SP	SPIRAL
SPCG	SPACING
SPECS	SPECIFICATIONS
SQ or ☐	SQUARE
SSL	SHORT SLOTTED HOLES
STD	STANDARD
STIFF	STIFFENER
STIRR	STIRRUP
STL	STEEL
STR	STRAIGHT
STRUC	STRUCTURAL
S4S	SURFACED FOUR SIDES
S2S	SURFACED TWO SIDES
S1S	SURFACED ONE SIDE
T	TOP
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
TEMP	TEMPERATURE
THRU	THROUGH
TOC	TOP OF CURB: TOP OF CONCRETE
TOS	TOP OF STEEL
TOW	TOP OF WALL
TS	STRUCTURAL TUBING
TYP	TYPICAL
UL	UNDERWRITERS' LABORATORY, INC.
UBC	UNIFORM BUILDING CODE
UNO	UNLESS NOTED OTHERWISE
UON	UNLESS OTHERWISE NOTED
UT	ULTRA-SONIC TEST
VERT or V	VERTICAL
VG	VERTICAL GRAIN
W	WIDE FLANGE (STEEL)
W/	WITH
W/O	WITHOUT
WH	WEEP HOLE
WP	WORK POINT
WPJ	WEAKENED PLANE JOINT
WT	WEIGHT: STRUCTURAL TEE CUT FROM W SHAPE
WWF	WELDED WIRE FABRIC

SYMBOLS



STEEL COLUMNS, BEAMS, HANGERS & BRACING



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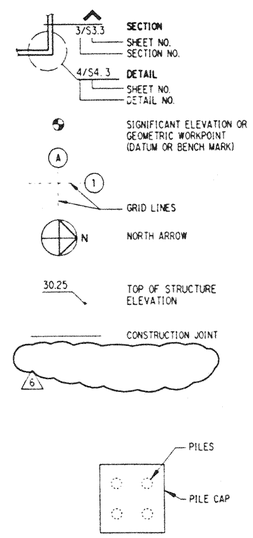

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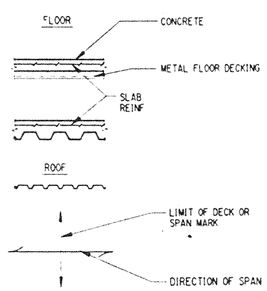
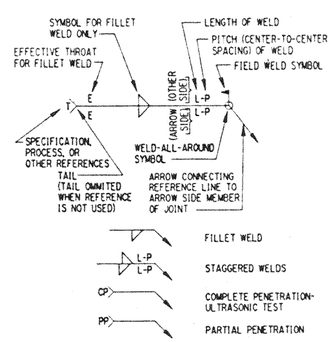


RIBBED FLOOR



WELDING SYMBOLS

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S2.1.D	EXHIBIT HALL SLAB PLAN SECTOR D
S2.1.E	EXHIBIT HALL SLAB PLAN SECTOR E
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S2.1.G	EXHIBIT HALL SLAB PLAN SECTOR G
S2.1.H	EXHIBIT HALL SLAB PLAN SECTOR H
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S2.2.C	INTERMEDIATE LEVEL FRAMING PLAN SECTOR C
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S2.2.H	INTERMEDIATE LEVEL FRAMING PLAN SECTOR H
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S2.2.J	INTERMEDIATE LEVEL FRAMING PLAN SECTOR J
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S2.4.C	MEETING FRAMING PLAN SECTOR C
S2.4.D	MEETING FRAMING PLAN SECTOR D
S2.4.E	MEETING FRAMING PLAN SECTOR E
S2.4.F	MEETING FRAMING PLAN SECTOR F
S2.4.G	MEETING FRAMING PLAN SECTOR G
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S2.5.J	BALLROOM/TERRACE FRAMING PLAN SECTOR J
S2.5.K	BALLROOM/TERRACE FRAMING PLAN SECTOR K
S2.5.L	BALLROOM/TERRACE FRAMING PLAN SECTOR L
S2.5.BB	BALLROOM/TERRACE FRAMING PLAN SECTOR BB
S2.6.A	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR A
S2.6.B	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR B
S2.6.C	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR C
S2.6.F	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR F
S2.6.G	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR G
S2.6.I	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR I
S2.6.J	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR J
S2.6.K	MECHANICAL PENTHOUSE FRAMING PLAN SECTOR K
S2.7.B	ROOF FRAMING PLAN SECTOR B
S2.7.BT	ROOF TRELLIS FRAMING PLAN SECTOR B
S2.7.C	ROOF FRAMING PLAN SECTOR C
S2.7.F	ROOF FRAMING PLAN SECTOR F
S2.7.FT	ROOF TRELLIS FRAMING PLAN SECTOR F
S2.7.G	ROOF FRAMING PLAN SECTOR G
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S2.7.JT	ROOF TRELLIS FRAMING PLAN SECTOR J
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Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

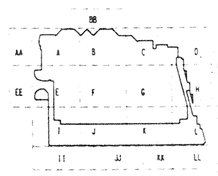
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-2200 FAX: (206) 292-2201

LICENSED PROFESSIONAL ENGINEER NO. 7575 HAWAII U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 10 Section 1.3 in of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
6	12-15-95		FINAL DOCUMENT
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

SYMBOLS, INDEX & ABBREVIATIONS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S0

Mon Dec 4 13:56:28 1995
j:\hccv\index.dgn
us plt

GENERAL NOTES

GENERAL:

ALL TYPICAL DETAILS AND NOTES SHOWN ON DRAWINGS SHALL APPLY, UNLESS NOTED OTHERWISE.

BUILDING CODE:

ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. THE PUBLICATIONS LISTED BELOW ARE THE GOVERNING CODES AND STANDARDS AND ARE REFERENCED BY THE BASIC DESIGNATION. IN THE CASE OF CONFLICTING REQUIREMENTS, THE BUILDING CODE SHALL GOVERN.

APPLICABLE CODES AND STANDARDS:

BUILDING CODE	UNIFORM BUILDING CODE, 1991 EDITION, INCLUDING THE 1993 BUILDING CODE OF THE CITY AND COUNTY OF HONOLULU AMENDMENTS.
ACI	AMERICAN CONCRETE INSTITUTE BUILDING CODE AND REQUIREMENTS FOR REINFORCED CONCRETE, 1989 EDITION
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," JUNE 1, 1989.
AISC SPEC. FOR STRUCT. JOINTS	AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 A490 BOLTS", NOVEMBER 13, 1985
AWS D1.1	AMERICAN WELDING SOCIETY D1.1-90, "STRUCTURAL WELDING CODE -STEEL"
AWS D1.3	AMERICAN WELDING SOCIETY D1.3-89, "STRUCTURAL WELDING CODE -SHEET STEEL"
AWS A2.4	AMERICAN WELDING SOCIETY A2.4-86, "SYMBOLS FOR WELDING AND NONDESTRUCTIVE TESTING"
AISI	AMERICAN IRON AND STEEL INSTITUTE, "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS," AUGUST 19, 1986
ASTM	AMERICAN SOCIETY OF TESTING AND MATERIALS

CONCRETE:

MIXING AND PLACING OF ALL CONCRETE AND SELECTION OF MATERIALS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. PROPORTIONS OF AGGREGATE TO CEMENT SHALL BE SUCH AS TO PRODUCE A DENSE, WORKABLE MIX WHICH CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFACE WATER. ALL CONCRETE, INCLUDING SLABS ON GROUND, SHALL HAVE AN ACCEPTABLE WATER-REDUCING ADMIXTURE ADDED IN ACCORDANCE WITH MANUFACTURER'S DIRECTIONS.

MAXIMUM SIZE OF AGGREGATE SHALL BE 1-1/2 INCHES, BUT MAXIMUM SIZE OF AGGREGATE SHALL NOT BE MORE THAN THREE QUARTERS OF THE CLEAR DISTANCE BETWEEN REINFORCING BARS. MAXIMUM SIZE OF AGGREGATE FOR SLABS ON GROUND SHALL BE ONE-THIRD THE THICKNESS OF THE SLAB.

MIX DESIGNS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR ACCEPTANCE PRIOR TO USE. MAXIMUM WATER-CEMENT RATIO AND SLUMP SHALL BE AS FOLLOWS FOR VARIOUS CONCRETE STRENGTHS (f'c) BASED ON STANDARD 28-DAY CYLINDER TESTS.

WATER-CEMENT RATIO BY WEIGHT

f'c	NON AIR-ENTRAINED	MAXIMUM SLUMP	LOCATION
2,500psi	0.58	6	STAIR PANS
3,000 psi	0.58	4	CURBS AND SIDEWALKS
3,500 psi	0.51	4	ARCH TOPPING
4,000 psi	0.44	4	ALL CONCRETE UNLESS OTHERWISE NOTED
5,000 psi	*	*	PRECAST ELEMENTS
7,500 PSI	*	*	COLUMNS AS NOTED

* APPROVED DESIGN BY APPROVED LAB.

FORM CAMBER:

CONCRETE FORMWORK SHALL HAVE BUILT-IN CAMBER TO COMPENSATE FOR FORM SAG UNDER WET CONCRETE LOAD, IN ADDITION TO STRUCTURAL CAMBER NOTED. ONE-WAY SLABS SHALL BE CAMBERED 1/1000 OF THE SPAN, UNLESS OTHERWISE NOTED ON THE DRAWINGS. BEAM STRUCTURAL CAMBERS ARE NOTED ON THE DRAWINGS. CAMBERS OF LESS THAN 1/8 INCH MAY BE NEGLECTED.

CONSTRUCTION JOINTS:

ALL CONSTRUCTION JOINTS IN WALLS SHALL BE KEYED IN ACCORDANCE WITH THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS OR, AT THE CONTRACTOR'S OPTION, SHALL BE INTENTIONALLY ROUGHENED CONSTRUCTION JOINTS IN ACCORDANCE WITH THE FOLLOWING: THE SURFACE OF ROUGHENED JOINTS SHALL BE SAND BLASTED OR ROUGHENED WITH A CHIPPING HAMMER TO EXPOSE THE AGGREGATE EMBEDDED IN THE PREVIOUS POUR. THE EXPOSED AGGREGATE SHALL PROTRUDE A MINIMUM OF 1/4 INCH. ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED AND LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, ALL CONSTRUCTION JOINTS SHALL BE WETTED AND STANDING WATER REMOVED. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL BE HELD TO A MAXIMUM SPACING OF 40'-0". ALL CONSTRUCTION JOINTS IN SLABS, BEAMS, AND GIRDERS SHALL BE OFFSET A DISTANCE EQUAL TO TWICE THE WIDTH OF THE BEAM. THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS TO THE ARCHITECT/ ENGINEER FOR ACCEPTANCE BEFORE STARTING CONSTRUCTION.

ALL CONSTRUCTION JOINTS FOR SLABS ON DECK SHALL BE IN ACCORDANCE WITH THE TYPICAL SLAB ON DECK CONSTRUCTION JOINT DETAIL SHOWN ON THE STRUCTURAL DRAWINGS.

SLEEVES:

EXCEPT AS DETAILED ON STRUCTURAL DRAWINGS, NO BEAMS, OR GIRDERS SHALL BE SLEEVED FOR PIPING OR DUCTS, UNLESS APPROVED BY THE ARCHITECT/ ENGINEER.

ELECTRICAL CONDUIT:

IF CONDUIT IS TO RUN IN SLABS, THE CONDUIT SHALL BE RUN AT MID-DEPTH OF SLAB AND CONDUIT SIZE SHALL NOT EXCEED 33 PERCENT OF THE SLAB DEPTH. NO CONDUIT SHALL BE PLACED IN SLABS WITH ACTUAL CONCRETE THICKNESS LESS THAN 3 INCHES. THERE SHALL BE A MINIMUM OF 3 INCHES OF CLEAR SPACE BETWEEN CONDUITS. ALUMINUM CONDUIT IS PROHIBITED. PLACE #3 AT 12 INCHES ADDITIONAL REINFORCING ABOVE AND BELOW, PERPENDICULAR TO THE CONDUIT. THE ADDED REINFORCING SHALL EXTEND 1'-0" PAST THE CONDUITS ON BOTH SIDES. SEE SECTION 15/S1.6 FOR ALLOWABLE CONDUIT IN SLABS ON METAL DECK.

REINFORCING STEEL:

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ALL REINFORCING SHALL BE NEW BILLET STOCK ASTM A 615, GRADE 60.
REINFORCING STEEL TO BE WELDED TO STRUCT STEEL SHALL BE ASTM A706. BARS SHALL BE SECURELY TIED IN PLACE WITH #16 DOUBLE-ANNEALED IRON WIRE. BARS SHALL BE SUPPORTED ON ACCEPTABLE CHAIRS. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING OF REINFORCED CONCRETE STRUCTURES." CONTRACTOR SHALL COORDINATE REINFORCING STEEL PLACEMENT DETAILS AND PROVIDE TEMPLATES FOR PLACING STEEL IN CONGESTED AREAS AS NECESSARY. SHOP DRAWINGS (INCLUDING PLACING PLANS AND ELEVATIONS) SHALL BE SUBMITTED TO AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE STARTING FABRICATION.

NO BENDING OR STRAIGHTENING OF REINFORCEMENT WILL BE PERMITTED AFTER PARTIAL EMBEDMENT IN CONCRETE. FOOTING AND PIER DOWELS MUST BE PROTECTED FROM DAMAGE BY EQUIPMENT.

NO REINFORCING BARS SHALL BE WELDED, EXCEPT AS FOLLOWS: CERTAIN VERTICAL COLUMN BARS AND ALL #14 AND #18 BARS SHALL HAVE FULL-PENETRATION BUTT WELDS (AS NOTED ON THE STRUCTURAL DRAWINGS). AT THE CONTRACTOR'S OPTION, MECHANICAL BUTT SPLICING USING AN EXOTHERMIC WELDING PROCESS AND HIGH-STRENGTH SLEEVES OR THREADED SPLICING MAY BE USED, PROVIDED THAT THE MECHANICAL SPLICES DEVELOP IN TENSION AT LEAST 125 PERCENT OF THE SPECIFIED YIELD STRENGTH (FY) OF THE BAR. ACCEPTABLE THREADED SPLICES SHALL BE THE LENTON REBAR SPLICE (ICBO #3967) OR THE DAYTON BAR GRIP (ICBO #3848). MECHANICAL SPLICES CONSIDERED SUSPECT BY THE INSPECTOR SHALL BE REMOVED, REPLACED AND TESTED AT THE CONTRACTOR'S EXPENSE. REINFORCING BARS SHALL BE LAP SPLICED FOR TENSION (LSB) UNLESS OTHERWISE NOTED ON THE DRAWINGS. NOTE: #14 AND #18 BARS SHALL NOT BE LAP SPLICED. WELDING OR TACK WELDING OF REINFORCING BARS TO OTHER BARS OR TO PLATES, ANGLES, ETC., IS PROHIBITED, EXCEPT WHERE SPECIFICALLY APPROVED BY THE ENGINEER. WHERE WELDING IS APPROVED, IT SHALL BE DONE BY AWS-CERTIFIED WELDERS USING E9018 OR APPROVED ELECTRODES. WELDING PROCEDURES SHALL CONFORM TO THE REQUIREMENTS OF AWS-D1.4.

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CAST-IN-PLACE CONCRETE COVER OVER REINFORCING STEEL, UNLESS OTHERWISE NOTED, SHALL BE AS FOLLOWS:

1. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 INCHES
2. CONCRETE EXPOSED TO EARTH OR WEATHER: 1-1/2 INCHES FOR #5 BAR OR SMALLER, 2 INCHES FOR #6 BAR OR LARGER
3. OTHER CONCRETE:
   WALLS - INTERIOR FACE: #14 AND #18 BARS - 1-1/2 INCHES, #11 BARS AND SMALLER - 3/4 INCH.
   SLABS AND JOISTS: #14 AND #18 BARS - 1-1/2 INCHES, #11 BARS AND SMALLER - 1 INCH
   BEAMS AND COLUMNS - TIES, STIRRUPS, SPIRALS: INTERIOR FRAMES - 1-1/2 INCHES, EXTERIOR FRAMES - 2 INCHES.

WELDED WIRE FABRIC:

WELDED WIRE FABRIC (WWF) SHALL BE ELECTRICALLY WELDED AND CONFORM TO ASTM A 185. AN 8-INCH MINIMUM LAP SHALL BE PROVIDED FOR SIDE AND END LAPS. WELDED WIRE FABRIC SHALL BE SUPPORTED ON APPROVED CHAIRS.

CURTAIN WALL INSTALLATION

THE CURTAIN WALL CONNECTIONS SHALL ACCOUNT FOR CONSTRUCTION TOLERANCES, COLUMN SHORTENING, AND BEAM DEFLECTIONS. IN ADDITION, THE CURTAIN WALL DESIGN SHALL ACCOMMODATE A TYPICAL VERTICAL MOVEMENT AT EACH FLOOR OF 3/4 INCH DUE TO VARIABLE LIVE LOADING. THIS DISPLACEMENT WILL OCCUR AT THE FREE END OF CANTILEVER BEAMS AND AT MIDSPAN OF SIMPLE SPAN EDGE BEAMS. THE CURTAIN WALL SHALL ACCOMMODATE TYPICAL LATERAL MOVEMENTS OF .004* STORY HEIGHT (IN) BETWEEN ADJACENT FLOORS PERPENDICULAR AND/OR PARALLEL TO THE CURTAIN WALL.

THE EXTERIOR BEAMS HAVE BEEN DESIGNED FOR UNIFORM LOADS REPRESENTING CURTAIN WALL REACTIONS. CANTILEVERED SLABS OVER EDGE BEAMS WERE DESIGNED WITH THE SAME WALL LOAD APPLIED AT THE FREE EDGE OF THE SLAB.

WALL CLADDING ATTACHMENTS SHALL NOT APPLY ANY LATERAL LOADS TO THE BOTTOM FLANGE OF THE BEAMS. IF ATTACHMENT IS MADE TO THE BOTTOM OF BEAMS, ADDITIONAL INCLINED STRUTS BRACING THE BOTTOM FLANGE OR OTHER EQUIVALENT MEANS TO COUNTERACT THIS FORCE SHALL BE PROVIDED BY THE WALL CLADDING SUPPLIER.

DRILLED-IN CONCRETE ANCHORS (DICA):

ACCEPTABLE DRILLED-IN CONCRETE ANCHORS, OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE DRAWINGS, SHALL BE AS FOLLOWS: HILTI "KWIK-BOLT-II" (ICBO #4627), "WEJ-IT ANCHOR BOLT" (ICBO #1821) OR "ITW RAMSET/RED HEAD TRUBOLT WEDGE ANCHORS" (ICBO #1372) OR APPROVED EQUAL. MINIMUM EMBEDMENT DEPTH SHALL BE 4.5 BOLT DIAMETERS UNLESS OTHERWISE NOTED ON DRAWINGS. ACCEPTABLE HEAVY DUTY DRILLED-IN CONCRETE ANCHORS, OF SIZE, NUMBER, AND SPACING AS SHOWN ON THE DRAWINGS SHALL BE AS FOLLOWS: "HILTI HSL METRIC ANCHOR" (ICBO #3987) OR APPROVED EQUAL.

NONSHRINK GROUT FOR BASE PLATES:

GROUT SHALL BE AN APPROVED NONSHRINK CEMENTITIOUS GROUT CONTAINING NATURAL AGGREGATES DELIVERED TO THE JOB SITE IN FACTORY PREPACKAGED CONTAINERS REQUIRING ONLY THE ADDITION OF WATER. THE MINIMUM 28-DAY COMPRESSIVE STRENGTH SHALL BE AT LEAST 1,000 PSI HIGHER THAN THE SUPPORTING CONCRETE STRENGTH, UNLESS OTHERWISE NOTED. APPROVED GROUTS INCLUDE: MASTER BUILDERS' "MASTER FLOW 713," SIKA CORPORATION'S "SIKAGROUT 212," BURKE COMPANY'S "NONFERROUS NONSHRINK GROUT," OR APPROVED EQUAL. GROUT SHALL BE MIXED, APPLIED, AND CURED STRICTLY IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTRUCTIONS.

EPOXY GROUT:

"EPOXY GROUT" SHALL BE A 100 PERCENT SOLIDS PASTE DESIGNED FOR STRUCTURAL BONDING OF DOWELS IN HARDENED CONCRETE. WHEN MIXED AND CURED ACCORDING TO THE MANUFACTURER'S WRITTEN INSTRUCTIONS, EPOXY MATERIALS SHALL PRODUCE THE FOLLOWING PROPERTIES: COMPRESSIVE STRENGTH (ASTM D695) * 10,000 PSI, TENSILE STRENGTH (ASTM D638) * 3,500 PSI.

"EPOXY GROUT" SHALL BE USED FOR ALL DRILLED AND GROUTED REINFORCING BARS OR BOLTS UNLESS NOTED OTHERWISE. EPOXY GROUT SHALL BE "CONCRESIVE 1438" AS MANUFACTURED BY ADHESIVE ENGINEERING COMPANY, "SIKADUR 31 HI MOD GEL" AS MANUFACTURED BY SIKA CORPORATION, OR APPROVED EQUAL. SUBMIT MANUFACTURER'S LITERATURE TO THE ARCHITECT / ENGINEER FOR APPROVAL.

EPOXY MATERIAL SHALL BE MIXED, APPLIED AND CURED IN STRICT ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS. ALL PLACEMENT AND CURING SHALL BE CONDUCTED WITH CONCRETE AND AIR TEMPERATURES ABOVE 50 DEGREES. APPLY EPOXY MATERIAL ONLY TO CLEAN, DRY CONCRETE.

GROUTING REINFORCING BARS AND BOLTS:

REINFORCING BARS AND BOLTS EMBEDDED IN EXISTING CONCRETE SHALL BE GROUTED INTO HOLES DRILLED INTO THE EXISTING CONCRETE. HILTI HIT ADHESIVE ANCHOR (ICBO#4419) OR EQUAL SHALL BE USED. PROVIDE POSITIVE PROTECTION SO DOWELS ARE NOT DISTURBED DURING CURING.

STRUCTURAL STEEL:

ALL STEEL SHALL CONFORM TO THE FOLLOWING:

ALL STEEL, UNLESS OTHERWISE NOTED * ASTM A572, GRADE 50, A441, FY * 50 KSI OR A588 FY * 50 KSI

MATERIAL CALLED OUT ON PLANS AS (FY * 36) ASTM A36, FY * 36 KSI

ALL PLATE AND ANGLES, UNLESS OTHERWISE NOTED, ARE ASTM A36, FY * 36 KSI

SQUARE OR RECTANGULAR STRUCTURAL TUBE * ASTM A500, GRADE B, FY * 46 KSI

STEEL PIPE DIAMETER LESS THAN OR EQUAL TO 12 INCHES * ASTM A53, TYPE E OR S, GRADE B, FY * 35 KSI

ALL WORK SHALL BE IN ACCORDANCE WITH THE AISC SPECIFICATION. SHOP DRAWINGS SHALL BE SUBMITTED AND REVIEWED BY THE ARCHITECT/ENGINEER BEFORE COMMENCING FABRICATION. ALL STEEL ANCHORS AND TIES AND OTHER MEMBERS EMBEDDED IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED. DIMENSIONAL TOLERANCE FOR BUILT-UP MEMBERS SHALL BE PER AWS D1.1.

ASTM A6 GROUP 4 AND 5 ROLLED SHAPES AND PLATES EXCEEDING 2-INCH THICKNESS USED FOR BUILT-UP MEMBERS REQUIRE SHARPY V-NOTCH TESTING IN ACCORDANCE WITH ASTM A6 SUPPLEMENTARY REQUIREMENT S5, WHERE FULL-PENETRATION WELDS ARE CALLED OUT AT EITHER END OF THE MEMBER ON THE DRAWINGS. REFER TO AISC SPECIFICATION, PARAGRAPH A3.C FOR TESTING STANDARDS.

SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. ALL WELDING SHALL BE DONE BY AWS CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS D1.1. WELDS SHOWN ON THE DRAWINGS ARE THE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES, BASED ON PLATE THICKNESS. MINIMUM WELDING SHALL BE 3/16 INCH. ALL PARTIAL PENETRATION GROOVE WELD SIZES SHOWN ON THE DRAWINGS REFER TO EFFECTIVE THROAT THICKNESS. ALL WELDS SHALL BE MADE USING LOW-HYDROGEN ELECTRODES WITH MINIMUM TENSILE STRENGTH * 70 KSI. WELDING PROCEDURES SHALL BE SUBMITTED TO THE TESTING AGENCY FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. ALL COMPLETE-PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED. REDUCTION IN TESTING MAY BE MADE IN ACCORDANCE WITH THE BUILDING CODE WITH APPROVAL OF ARCHITECT / ENGINEER.

STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS. MINIMUM CONNECTIONS SHALL BE A TWO-BOLT CONNECTION USING 7/8-INCH DIAMETER A325 BOLTS IN SINGLE SHEAR. ALL HIGH-STRENGTH BOLTS SHALL BE INSTALLED, TIGHTENED AND INSPECTED IN ACCORDANCE WITH THE AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. THE CRITERIA FOR SLIP-CRITICAL CONNECTIONS SHALL APPLY TO ALL CONNECTIONS UNLESS SPECIFICALLY NOTED AS SNUG-TIGHT ON THE STRUCTURAL DRAWINGS. WHERE CONNECTIONS ARE NOTED AS SNUG-TIGHT THE CONTRACTOR MAY INSTALL PER CRITERIA FOR SNUG-TIGHT BOLTS. SLIP-CRITICAL CONNECTIONS SHALL USE LOAD INDICATOR WASHERS OR TENSION CONTROL BOLTS. ALL ASTM A307 BOLTS SHALL BE PROVIDED WITH LOCK WASHERS UNDER NUTS OR SELF-LOCKING NUTS. ALL BOLT HOLES SHALL BE STANDARD SIZE, UNLESS NOTED OTHERWISE. ALL BEAM WEBS ARE VERTICAL, UNLESS NOTED OTHERWISE.

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THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE COORDINATION OF THE SELECTION OF OPTIONAL DETAILS SHOWN ON THE DRAWINGS. THE CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO: ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS: WELDING PROCEDURES: REQUIRED ROOT OPENINGS: ROOT FACE DIMENSIONS: GROOVE ANGLES: BACKING BARS: COPES: SURFACE ROUGHNESS VALUES: AND TAPERS OF UNEQUAL PARTS.

SHEAR CONNECTOR STUDS:

ALL SHEAR CONNECTOR STUDS SHALL BE 3/4 INCH IN DIAMETER UNLESS OTHERWISE NOTED. ACCEPTABLE TYPES SHALL BE "TRU-WELD (ICBO *3741) OR NELSON (ICBO *2614). SHEAR CONNECTOR STUDS SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH EQUIPMENT RECOMMENDED BY MANUFACTURER OF STUDS. STEEL STUD MATERIAL, WELDING, AND INSPECTION SHALL BE IN ACCORDANCE WITH AWS D1.1. SHEAR STUDS SHALL BE PLACED AT 2'-0" ON CENTER MAXIMUM ON ALL BEAMS SUPPORTING A STEEL DECK WITH CONCRETE FILL OR A CAST-IN-PLACE CONCRETE SLAB. SEE "SHEAR STUD PLACEMENT" FOR LAYOUT CRITERIA. METAL DECK SHOP DRAWINGS DETAILING THE SHEAR STUD PLACEMENT SHALL BE SUBMITTED TO THE ARCHITECT / ENGINEER FOR REVIEW BEFORE INSTALLATION.

STEEL COMPOSITE DECK:

THE STEEL DECK SHALL BE OF DEPTH SHOWN ON THE STRUCTURAL DRAWINGS. GAGE OF DECK SHALL BE DETERMINED BY THE CONTRACTOR BASED ON THE SPAN CONDITIONS, SHORING REQUIREMENTS, AND THE SUPERIMPOSED LOADS SHOWN ON THE DRAWINGS, LOAD DIAGRAMS, AND NOTES. MINIMUM GAGE IS 20 AND A MAXIMUM DEAD LOAD DEFLECTION OF ½" IN ANY SPAN. WRITTEN VERIFICATION OF CONFORMANCE FOR ALL CONDITIONS IN THE STRUCTURE SHALL BE SUBMITTED FOR ACCEPTANCE PRIOR TO FABRICATION. THE CAPACITIES OF THE DECK SHALL BE BASED ON CURRENT ICBO REPORTS.
SHOP DRAWINGS SHALL BE SUBMITTED SHOWING DECK GAGE, LAYOUT, FASTENING, STUD LAYOUT, AND CLOSURES. IF ANY SHORING IS TO BE USED, IT SHALL BE APPROVED BY THE DESIGN BUILDER AND SHALL BE SHOWN ON THE SHOP DRAWINGS. UNITS SHALL SPAN OVER FOUR SUPPORTS, CONTINUOUS OVER THREE OR MORE SPANS, EXCEPT WHERE FRAMING DOES NOT PERMIT. THE AISI SPECIFICATIONS SHALL GOVERN THE DESIGN OF ALL DECK UNITS. STEEL DECK AND ALL OF ITS FLASHINGS SHALL CONFORM TO ASTM A 446, GRADE A OR GRADE C. THE STEEL SHALL HAVE RECEIVED, BEFORE BEING FORMED, A METAL PROTECTIVE COATING OF ZINC CONFORMING TO ASTM A 525-G60. TYP AND ASTM A 525-G90 AT THE PARKING LEVEL ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3.

CONCRETE BONDING-TYPE UNITS SHALL BE FORMED WITH DEFORMATIONS TO PROVIDE AN INTERLOCK BETWEEN THE CONCRETE AND STEEL. UNLESS SHOWN OTHERWISE, UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE ENDS OF THE UNITS AND AT INTERMEDIATE SUPPORTS AT 12 INCHES ON CENTER WITH 3/4-INCH-DIAMETER PUDDLE WELDS: WHERE TWO UNITS ABUT, EACH UNIT SHALL BE SO FASTENED TO THE STEEL SUPPORTS. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY 1-1/2-INCH TOP SEAM WELDS AT 2'-0" ON CENTER OR BUTTON PUNCHED AT 2'-0" ON CENTER. DECK UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES BY 3/4-INCH-DIAMETER PUDDLE WELDS AT 1'-0" ON CENTER. 3/4-INCH DIAMETER SHEAR STUDS WELDED THROUGH DECK MAY BE USED IN PLACE OF 3/4-INCH DIAMETER PUDDLE WELDS. PROVIDE FLASHING AND CLOSURE PLATES AT WALL ENDS OF ALL UNITS, AROUND COLUMNS, AND AT ALL PERIMETER LOCATIONS REQUIRING CLOSURE. THE DECK INSTALLATION, WHEN COMPLETE, SHALL BE READY TO RECEIVE CONCRETE.

STEEL DECK TYPES SHALL BE VERCO TYPE W, ASC PACIFIC TYPE W, H.H. ROBERTSON TYPE QL-99 OR APPROVED EQUAL.

STEEL ROOF DECK:

THE STEEL DECK SHALL BE OF DEPTH AND GAGE SHOWN ON THE STRUCTURAL DRAWINGS. THE AISI SPECIFICATIONS SHALL GOVERN THE DESIGN OF ALL DECK UNITS. STEEL DECK AND ALL OF ITS FLASHINGS SHALL CONFORM TO ASTM A 446, GRADE A. THE STEEL SHALL HAVE RECEIVED, BEFORE BEING FORMED, A METAL PROTECTIVE COATING OF ZINC CONFORMING TO ASTM A 525-G60. ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.3. MINIMUM GAGE 20: OTHER GAGES ARE INDICATED ON THE DRAWINGS. UNITS SHALL SPAN OVER FOUR SUPPORTS (CONTINUOUS OVER THREE OR MORE SPANS), EXCEPT WHERE THE FRAMING DOES NOT PERMIT. DECK CONTRACTOR SHALL PROVIDE ANGLE SUPPORTS PER DETAIL 20/S1.7
NONCOMPOSITE UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE ENDS OF THE UNITS AND AT INTERMEDIATE SUPPORTS AT 12 INCHES ON CENTER WITH 3/4-INCH DIAMETER PUDDLE WELDS. WHERE TWO UNITS ABUT, EACH UNIT SHALL BE SO FASTENED TO THE STEEL FRAMING. THE SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BETWEEN SUPPORTS BY 1-1/2-INCH TOP SEAM WELDING AT 2'-0" ON CENTER, UNLESS NOTED OTHERWISE. DECK UNITS SHALL BE FASTENED TO THE STEEL SUPPORTS AT THE SIDE BOUNDARIES BY 3/4-INCH DIAMETER PUDDLE WELDS AT 1'-0" ON CENTER.
WHERE STEEL MEMBERS ARE PARALLEL TO THE DECK FLUTES AND AT THE SAME ELEVATION OF THE BOTTOM OF THE DECK, ADJUST DECK LAYOUT AND WELD DECK TO STEEL WITH SAME WELDING AS REQUIRED FOR SIDE BOUNDARIES.

STEEL DECK TYPES SHALL BE VERCO TYPE B, ASC PACIFIC TYPE B VERCO TYPE N24, ASC PACIFIC TYPE N24 OR APPROVED EQUAL, SEE DRAWINGS FOR LOCATIONS.

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STEEL STAIR:

REFER TO PROJECT SPECIFICATIONS, ARCHITECETURAL DRAWINGS AND STRUCTURAL DRAWINGS TO DETERMINE EXTENT OF STAIRS AND WHAT SHALL BE PROVIDED BY THE STAIR MANUFACTURER. STAIRS ARE TO FRAME INTO FLOOR BEAMS AND BUILDING COLUMNS ONLY. DO NOT CONNECT STAIRS TO ELEVATOR GUIDE RAIL SUPPORTS OR DIAGONAL BRACING. VERIFY ALL OTHER CONDITIONS WITH ARCHITECT PRIOR TO PROCEEDING. IF STAIR FRAMING RESULTS IN ECCENTRIC LOADING OF THE STRUCTURAL MEMBERS, STAIR MANUFACTURER SHALL PROVIDE BRACING OF THE STRUCTURAL MEMBERS. CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION.

3

COMPOSITE FLOOR SYSTEM:

FLOOR SLABS SHALL BE CONSTRUCTED TO THE ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE SPECIFICATIONS FOR FLOOR TOLERANCES. CONTRACTOR SHALL INCLUDE THE QUANTITIES OF THE ADDED CONCRETE DUE TO THE METAL DECK DEFLECTION. DESIGN CAMBER SHOWN FOR THE STEEL BEAMS HAS BEEN CALCULATED BASED ON THE DEFLECTION OF THE BEAM DUE TO THE WEIGHT OF THE STEEL AND CONCRETE SLAB.

MASONRY:

CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE BUILDING CODE. ALL HOLLOW LOAD BEARING CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C 90, GRADE M-1. ALL CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLID WITH CONCRETE GROUT. GROUT MIX SHALL CONTAIN PORTLAND CEMENT ONLY PLUS AGGREGATE PLUS A NONSHRINK ADDITIVE. MAXIMUM SIZE OF AGGREGATE SHALL BE 3/8 INCH. SLUMP SHALL BE 8 TO 11 INCHES. WATER-REDUCING ADMIXTURES MAY BE USED. MINIMUM COMPRESSIVE STRENGTH BASED ON 28-DAY TESTS SHALL BE 2,000 PSI. SUBMIT GROUT MIXES TO ARCHITECT FOR REVIEW BEFORE COMMENCING MASONRY CONSTRUCTION. ALL UNITS SHALL BE LAID IN RUNNING BOND USING TYPE M MORTAR WITH HEAD JOINTS. MASONRY DESIGN STRESSES ARE WITHOUT "SPECIAL INSPECTION" WITH THE MINIMUM DESIGN STRENGTH f'm * 1,500 PSI.

REQUIRED MORTAR PROPORTIONS BY VOLUME

TYPE	PORTLAND CEMENT	HYDRATED LIME	AGGREGATE MEASURED IN A DAMP, LOOSE CONDITION
S	1	¼	NOT LESS THAN 2-1/4 AND NOT MORE THAN 3 TIMES THE SUM OF THE VOLUMES OF THE CEMENT

ANCHOR BOLTS:

ANCHOR BOLTS SHALL BE ASTM A307 GRADE A STANDARD HEX HEAD FURNISHED WHITH HEAVY HEX NUTS AND LOCK WASHERS. SIZES EXCEEDING A307 SHALL BE ASTM A36 ROD THREADED UNC-2A FURNISHED WITH DOUBLE HEAVY HEX NUTS, JAMMED, AT ENDS EMBEDDED IN CONCRETE. WHERE SPECIFIED ON THE DRAWINGS, ANCHOR BOLTS SHALL BE ASTM A325 FURNISHED WHITH MATCHING HEAVY HEX NUTS AND LOCK WASHERS. SIZES EXCEEDING A325 SHALL BE ASTM A449 ROD THREADED UNC-2A FURNISHED WHITH DOUBLE HEAVY HEX NUTS, JAMMED, AT ENDS EMBEDDED IN CONCRETE. ANCHOR BOLTS SHALL HAVE SUFFICIENT LENGTH TO PROVIDE THE MINIMUM EMBEDMENT SHOWN ON THE DRAWINGS MEASURED FROM THE FACE OF CONCRETE TO THE NEAR FACE OF THE HEAD OR NUT. ANCHOR BOLTS SHALL BE INSTALLED TO A SNUG TIGHT CONDITION. NO HEATING OR BENDING OF ANCHOR BOLTS IS PERMITTED. NO ENLARGEMENT OF ANCHOR BOLTS HOLES BY BORING IS PERMITTED. PROVIDE DOUBLE NUTS FOR BASE PLATE CONNECTION, TYP.

FIREPROOFING STRUCTURAL STEEL:

REFER TO ARCHITECTURAL PLANS AND SPECIFICATIONS FOR MINIMUM HOURLY VALUES OF STEEL FIRE PROTECTION FOR DETERMINING THE THICKNESS OF SPRAY APPLIED FIREPROOFING. THE STRUCTURAL FRAME CONSISTS OF COLUMNS AND THE GIRDERS, BEAMS AND TRUSSES HAVING DIRECT CONNECTION TO THE COLUMNS AND ALL BRACING MEMBERS. FLOOR OR ROOF MEMBERS WHICH HAVE NO CONNECTION TO COLUMNS SHALL BE CONSIDERED SECONDARY MEMBERS.

STRUCTURAL DESIGN DATA:

LIVE LOADS:

LOBBY	100 PSF
EXHIBITION HALL	350 PSF
PARKING	50 PSF
MEETING ROOMS	150 PSF
CONCOURSES/STAIRS/EXITS	100 PSF
BALLROOM	150 PSF
KITCHEN	125 PSF
STORAGE	150 PSF
MECH./ELECT. ROOMS	125 PSF UNLESS INDIVIDUAL EQUIP. GOVERNS
ROOF	20 PSF
OFFICES	50 PSF * 20 PSF PARTITION LOAD

WIND LOADS: WIND PRESSURE SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. BASIC WIND SPEED * 80 MPH, EXPOSURE C, IMPORTANCE FACTOR OF 1.0.

SEISMIC LOADS: SEISMIC LOADING SHALL BE IN ACCORDANCE WITH THE BUILDING CODE (ZONE 2A).

V * 0.048 W, Z * 0.15, I * 1.0, S * 1.5, C * 2.55, Rw * 8.0

MOMENTS, SHEARS, AND ROTATIONAL FORCES ARE DELIVERED TO THE FOUNDATION BY THE STEEL BRACED FRAMES IN PROPORTION TO THEIR ABILITY TO RESIST LATERAL DEFORMATION.

PILING:

ALL PILES SHALL BE 16-1/2 INCH OCTAGONAL OR 12 INCH SQUARE (OPTIONAL 55 TON P

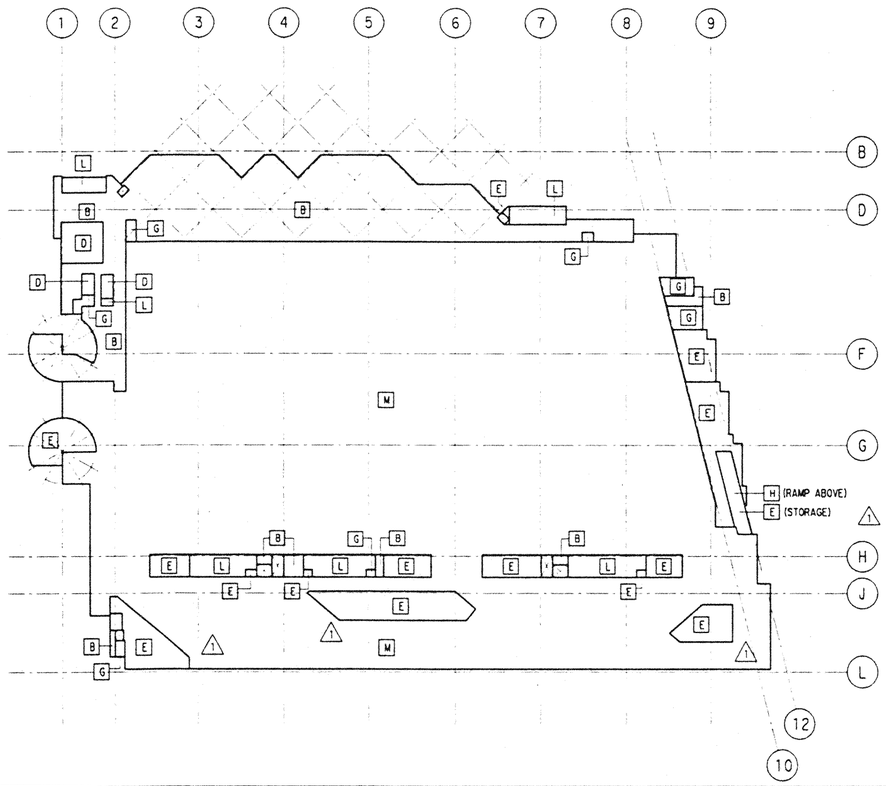
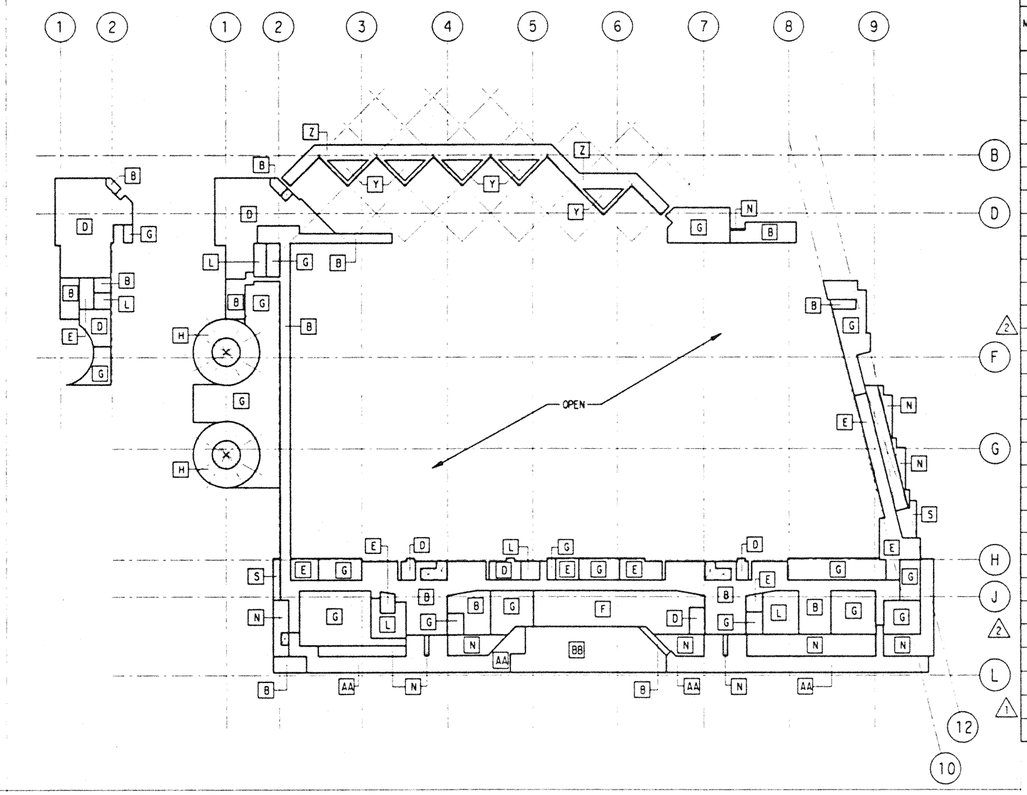
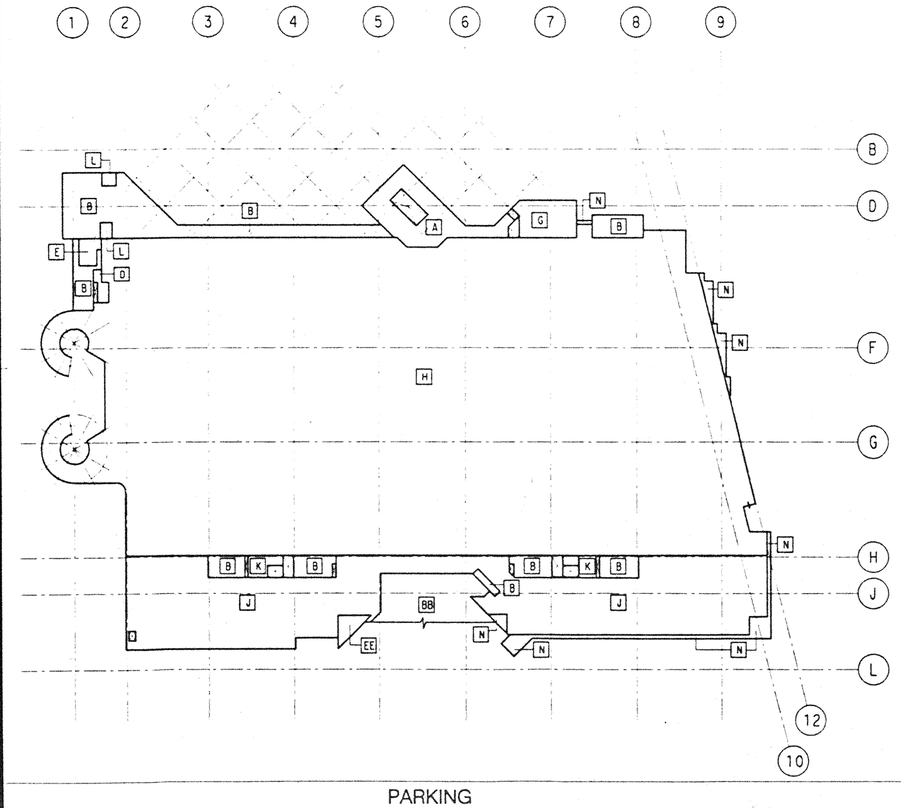


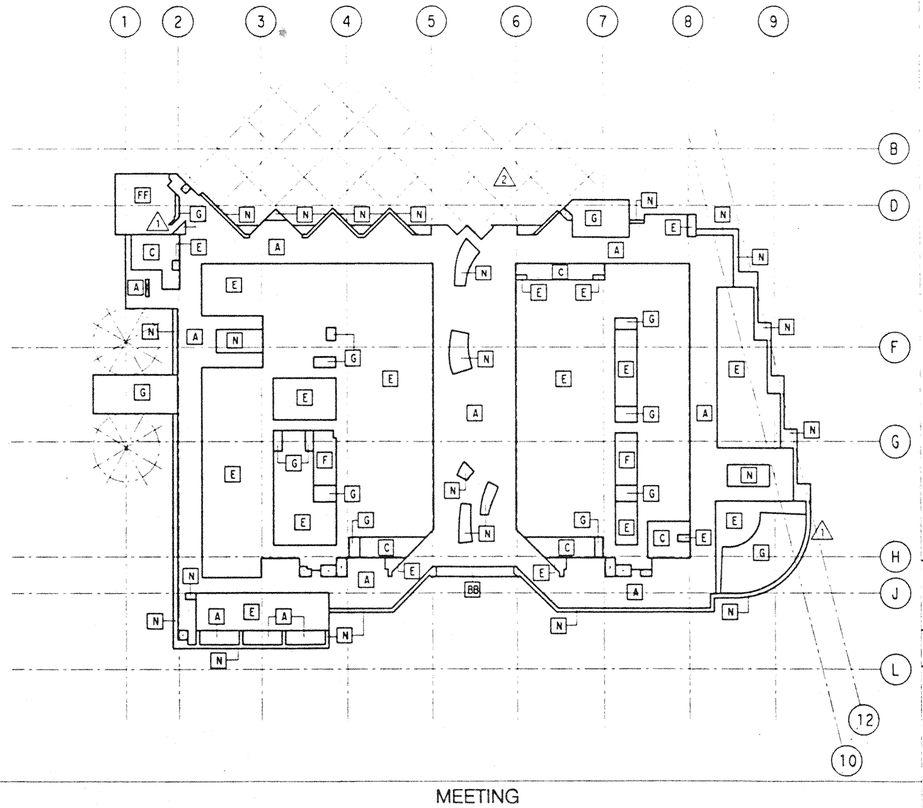
EXHIBIT HALL



INTERMEDIATE



PARKING



MEETING

DESIGN UNIFORM SUPERIMPOSED LOADS

MARK	SUPER-IMPOSED DEAD LOAD (PSF)	LIVE LOAD (PSF)	REMARKS
A	17	100	LOBBY/EXIT/CORRIDOR
B	13	100	LOBBY/EXIT/CORRIDOR
C	17	50	RESTROOMS
D	33	50	OFFICE
E	13	150	MTG RMS/BALLROOM/STORAGE
F	17	125	KITCHEN/KITCHEN CORRIDOR/ LIGHT STORAGE
G	13	125	MECHANICAL/ELECTRICAL/ SHOPS
H	13	50	PARKING
J	50.5	50	PARKING(W/ TOPPING SLAB)
K	59.5	150	STORAGE(W/ TOPPING SLAB)
L	13	50	RESTROOMS
M	0	350 OR H520-44	EXHIBITION HALL
N	253	20	PLANTER PLUS ACTUAL TREE WEIGHTS
P	63	100	TERRACE
Q	36	100	PREFUNCTION
R	63	50	TERRACE RESTROOMS
S	11	20	ROOF
T	50.5	150	BALLROOM(W/ TOPPING SLAB)
V	20	20	TRELLIS/SKYLIGHT
W	50.5	125	KITCHEN/KITCHEN CORRIDOR/ LIGHT STRG.(W/TOPPING SLAB)
X	34	20	BALLROOM ROOF
Y	15	20	CANOPY SKYLIGHT
Z	0	20	CANOPY
AA	50.5	100	LOBBY/EXIT/CORRIDOR (W/ TOPPING SLAB)
BB	99(STAIRS) 268(PLANTER)	100(STAIRS) 20(PLANTER)	GRAND STAIRWAY
CC	75	21.4	MAIN TRELLIS
DD	23	20	ROOF
EE	113	20	POOL
FF	97	100	NORTHWEST THEATRE
GG	121	100	SOUTHEAST THEATRE

NATURAL FREQUENCY OF FLOOR STRUCTURE

AREA	MINIMUN NATURAL FREQUENCY (hz)
EXHIBIT HALL	70.0 hz
MEETING ROOMS	4.0 hz
BALLROOM	9.2 hz

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
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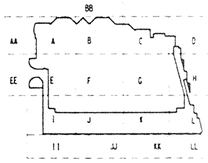
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 18 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
3	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
2	8-31-95		
1	8-4-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

LOAD MAPS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S0.3

Fri Dec 1 13:20:37 1995
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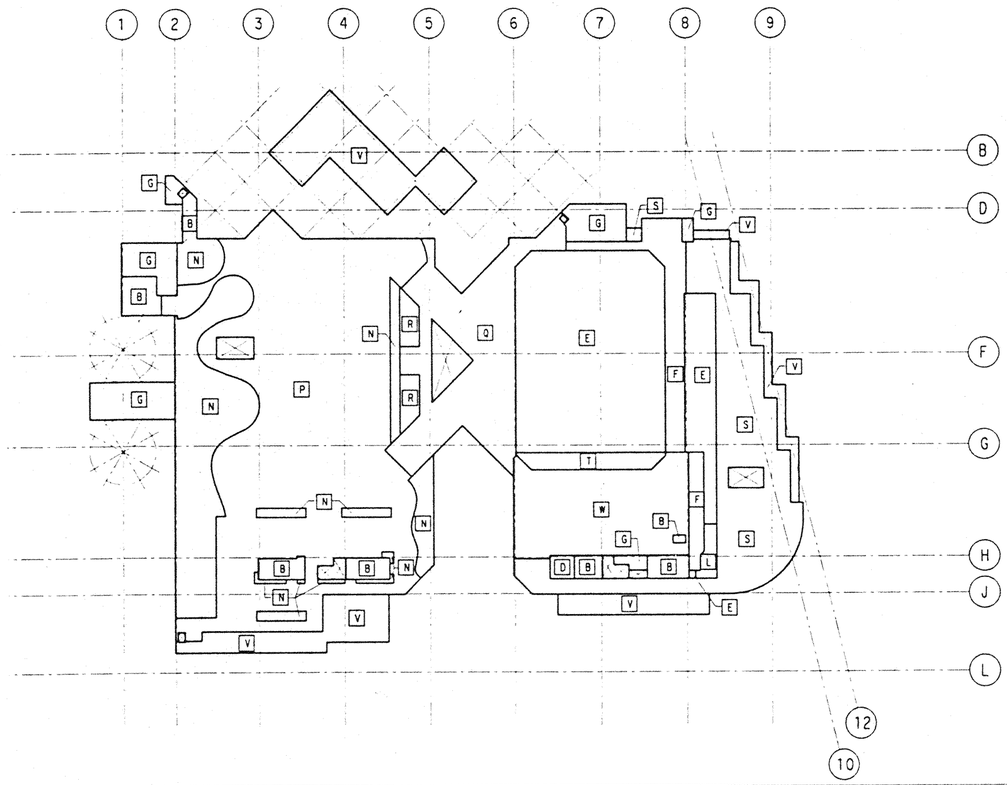
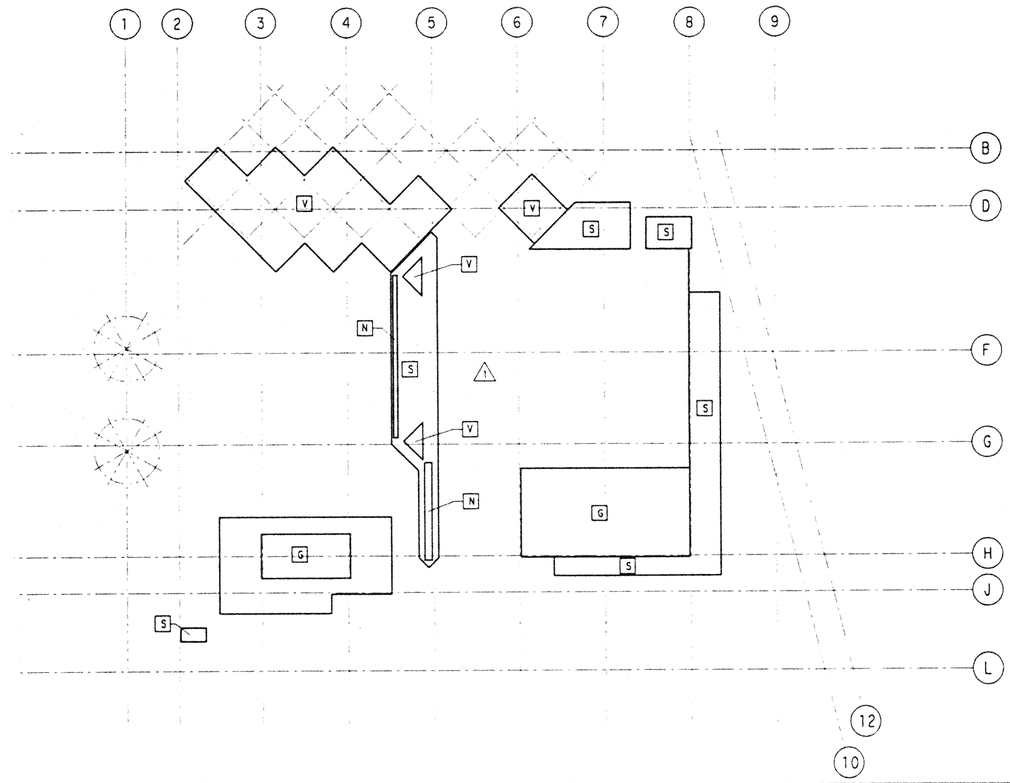
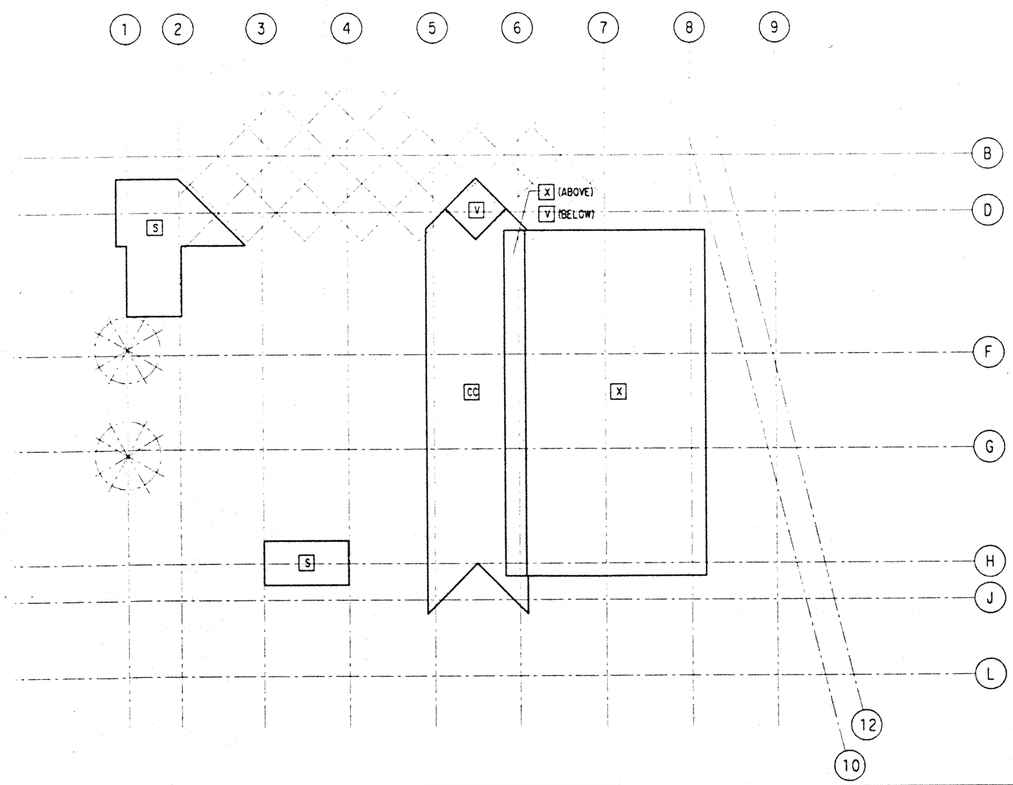


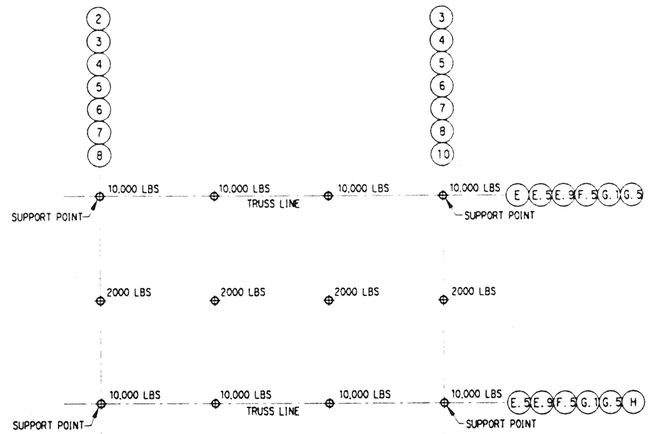
BALLROOM

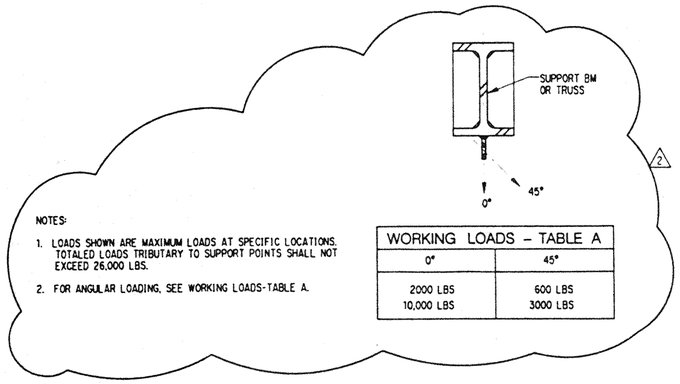


MECHANICAL PENTHOUSE



ROOF





NOTES:

1. LOADS SHOWN ARE MAXIMUM LOADS AT SPECIFIC LOCATIONS. TOTALED LOADS TRIBUTARY TO SUPPORT POINTS SHALL NOT EXCEED 26,000 LBS.
2. FOR ANGULAR LOADING, SEE WORKING LOADS-TABLE A.

WORKING LOADS – TABLE A	
0°	45°
2000 LBS 10,000 LBS	600 LBS 3000 LBS

EXHIBITION HALL RIGGING LOADS

Hawai'i Convention Center

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Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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HAWAII, U.S.A.

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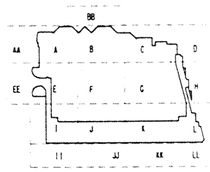
Supervision designed in Title 16 Chapter 18 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
2	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

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Job No. 93036.02.9

Sheet No.

S0.4

Fri Dec 1 13:22:35 1995
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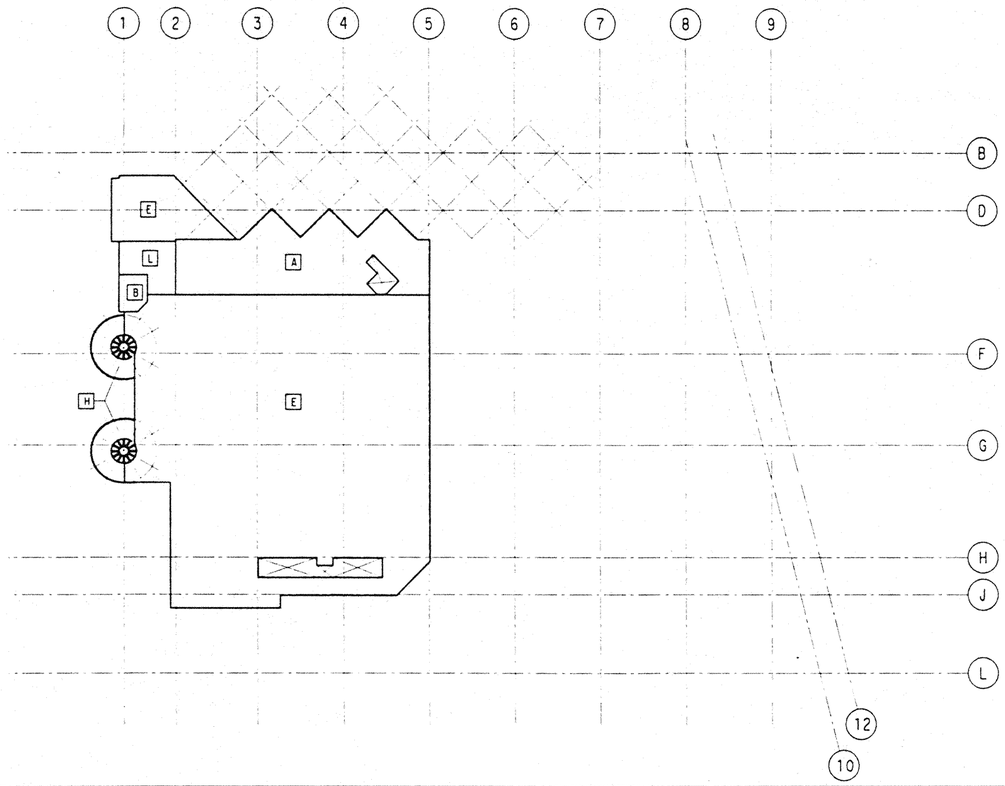
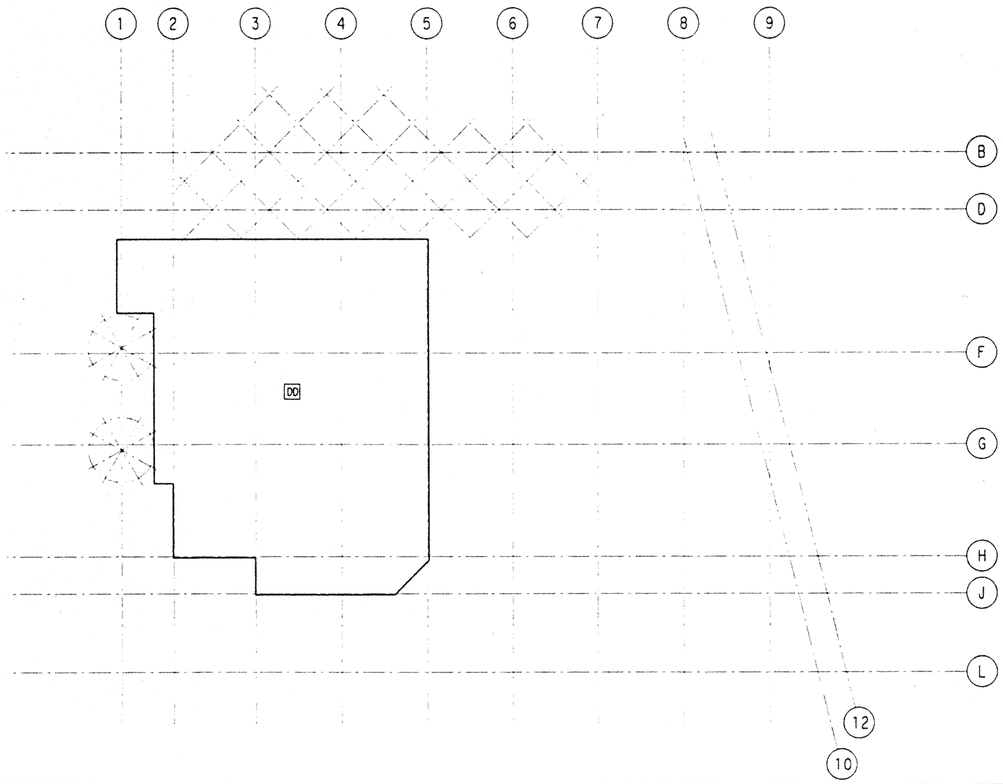
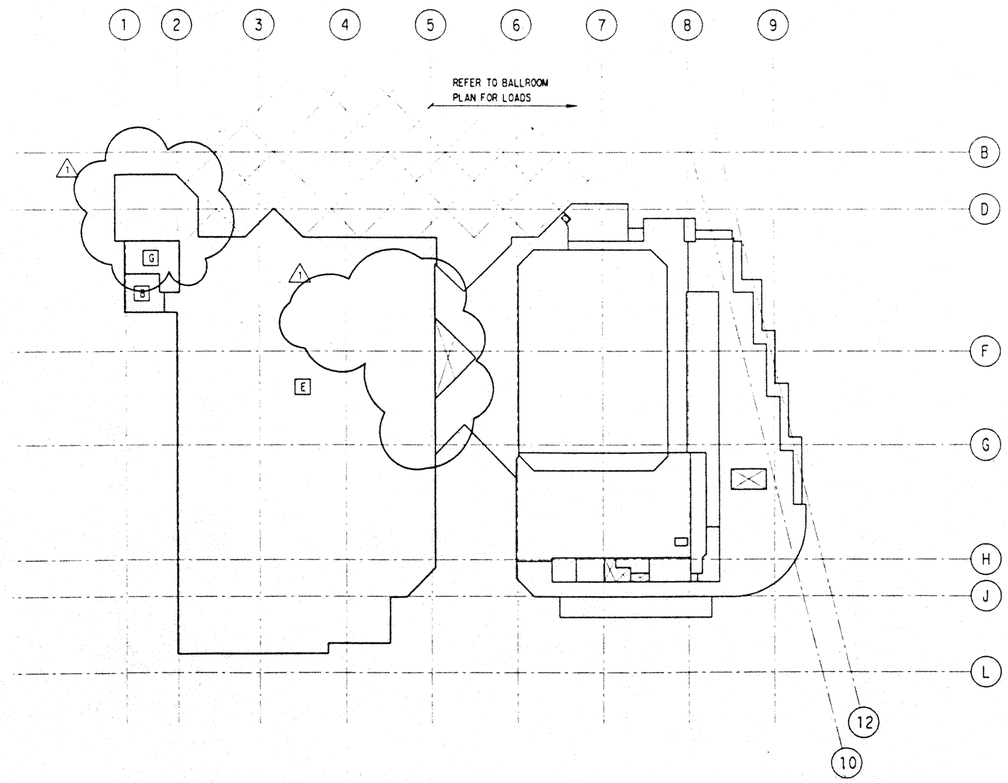


FUTURE EXHIBITION HALL



FUTURE ROOF



FUTURE MEETING

Hawai'i Convention Center

Convention Center Authority
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Honolulu, Hawaii

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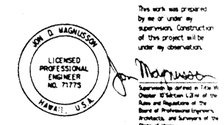

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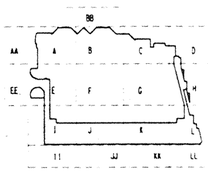
Supervision by persons in Title 16 Chapter 53 Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

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Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

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Fri Dec 1 13:24:30 1995

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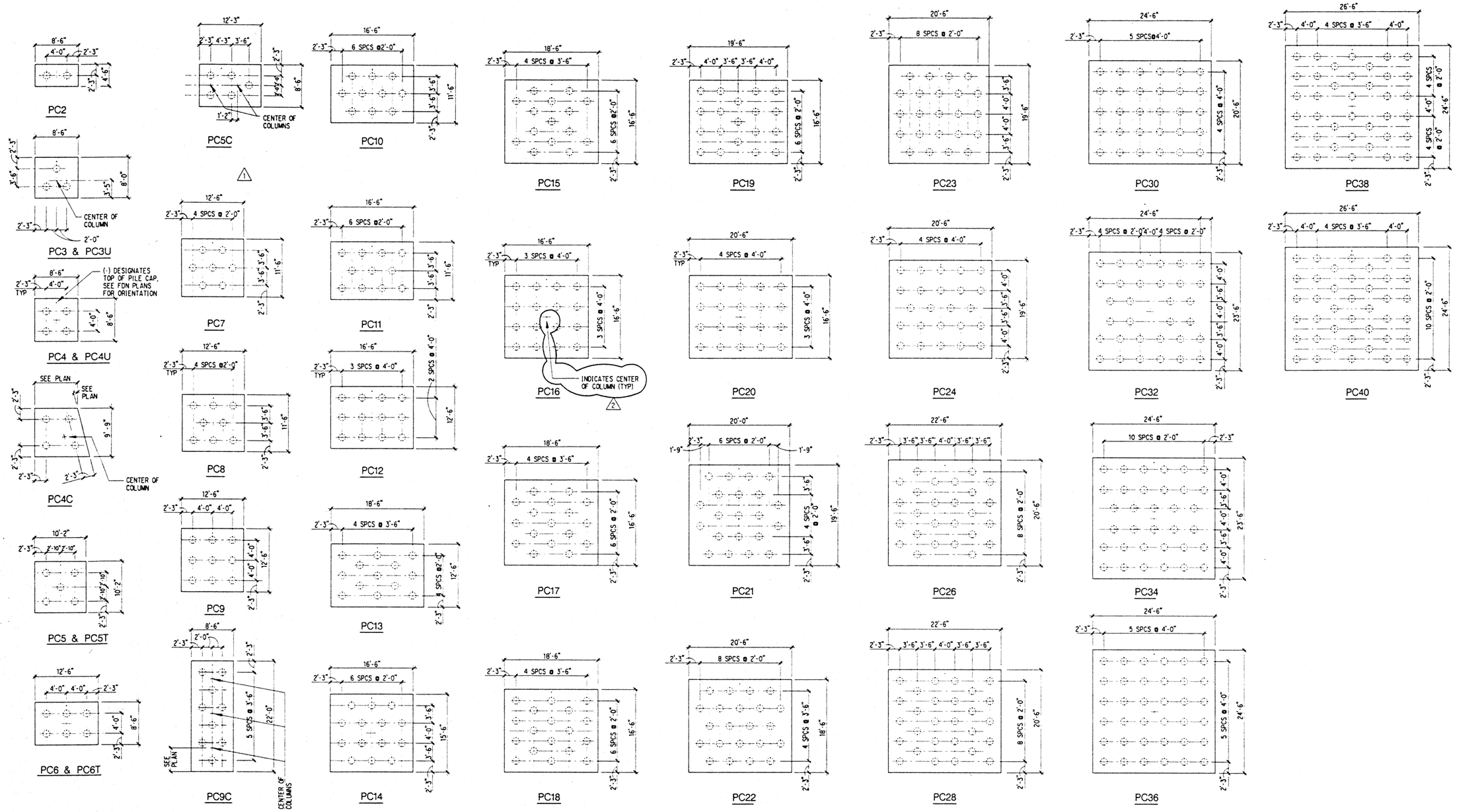

PILE CAP PATTERNS & DIMENSIONS

15

PILE MARK	THICKNESS	BOTTOM REINFORCEMENT LONG DIRECTION	BOTTOM REINFORCEMENT SHORT DIRECTION	TOP REINFORCEMENT LONG DIRECTION	TOP REINFORCEMENT SHORT DIRECTION	PILE CAPACITY	REMARKS
PC1	---	---	---			55T	SEE 18/S1.2 AND/OR INDIVIDUAL DETAILS
PC2	3'-4"	6#9H	10#8H			150T	
PC2R	3'-4"	6#9H	10#8H			150T	LENGTH= 9'-6"
PC3	2'-8"	8#8H	11#8H			150T	3
PC3U	2'-8"	8#8H	11#8H	8#8H	8#8H	50T UPLIFT	SEE 16/S1.2
PC4	3'-5"	10#9H	10#9H			150T	
PC4C	3'-4"	E-W BARS: 9#11H	N-S BARS: 11#11H	#8@12"OC H	#8@12"OC H	150T	SEE 3 & 4/S1.2
PC4U	3'-5"	10#9H	10#9H	10#8H	10#8H	50T UPLIFT	SEE 16/S1.2
PC5	3'-11"	9#11H	9#11H			150T	
PC5T	3'-11"	9#11H	9#11H	N-S BARS: 10#11H	E-W BARS: 12#8H	150T	SEE 5/S1.2 FOR ADDITIONAL REINF
PC5C	2'-8"	11#8H	18#8H	8#8H		50T UPLIFT	
PC6	3'-8"	11#11H	13#9H			150T	1
PC6T	3'-8"	11#11H	13#9H	8#9H	13#11H	150T	SEE 5/S1.2 FOR ADDITIONAL REINF
PC7	4'-3"	10#11H	10#11H			150T	
PC8	4'-2"	13#11H	14#11H			150T	
PC9	5'-4"	13#11H	13#11H			150T	
PC9C	2'-9"	11#8H	21#9H	8#8H		150T	

PILE MARK	THICKNESS	BOTTOM REINFORCEMENT LONG DIRECTION	BOTTOM REINFORCEMENT SHORT DIRECTION	TOP REINFORCEMENT LONG DIRECTION	TOP REINFORCEMENT SHORT DIRECTION	PILE CAPACITY	REMARKS
PC10	4'-7"	10#14	14#11H			150T	
PC11	4'-10"	12#14	18#11H			150T	
PC12	5'-2"	13#14	19#11H			150T	
PC13	5'-9"	18#14	18#11H			150T	
PC14	5'-3"	16#14	16#14			150T	
PC15	5'-3"	17#14	18#14			150T	
PC16	5'-9"	18#14	18#14			150T	
PC17	5'-6"	18#14	17#14			150T	
PC18	5'-7"	20#14	20#14			150T	
PC19	6'-0"	21#14	21#14			150T	
PC20	6'-4"	13#18	22#14			150T	
PC21	5'-11"	22#14	22#14			150T	
PC22	6'-1"	22#14	24#14			150T	
PC23	6'-1"	23#14	24#14			150T	
PC24	6'-5"	25#14	25#14			150T	
PC26	7'-2"	16#18	16#18			150T	
PC28	7'-2"	16#18	18#18			150T	
PC30	7'-7"	17#18	20#18			150T	
PC32	7'-9"	21#18	21#18			150T	

PILE MARK	THICKNESS	BOTTOM REINFORCEMENT LONG DIRECTION	BOTTOM REINFORCEMENT SHORT DIRECTION	TOP REINFORCEMENT LONG DIRECTION	TOP REINFORCEMENT SHORT DIRECTION	PILE CAPACITY	REMARKS
PC34	7'-8"	20#18	20#18			150T	
PC36	8'-9"	23#18	23#18			150T	
PC38	8'-7"	24#18	25#18			150T	
PC40	8'-6"	23#18	25#18			150T	

NOTES:
1. LONG BARS ON BOTTOM FOR BOTTOM PILE CAP REINFORCEMENT.
2. LONG BARS ON TOP FOR TOP PILE CAP REINFORCEMENT.
3. PROVIDE TOP REINFORCMENT IN ADDITION TO BOTTOM REINFORCEMENT @ UPLIFT CONDITIONS ONLY, INDICATED BY PC#U ON PLANS. SEE 16/S1.2
4. SEE "TYPICAL PILE CAP SECTIONS "B" PILE CAP PATTERNS & DIMENSIONS"
5. PROVIDE STANDARD HOOKS @ EA END OF REINFORCEMENT WHEN "H" IS CALLED OUT IN SCHEDULE.

PILE CAP SCHEDULE

20

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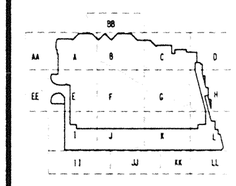
Supervision as defined in Title 16 Chapter 18 Section 1.2 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
3	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

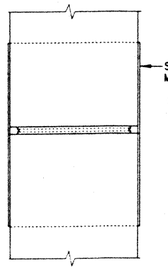
TYPICAL FOUNDATION DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

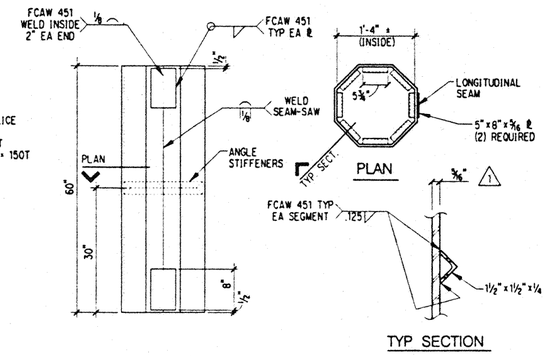
Sheet No.

S1.1

Fri Dec 1 13:26:39 1995 jxhccktypl.dgn DMS pt



TENSION SPLICE

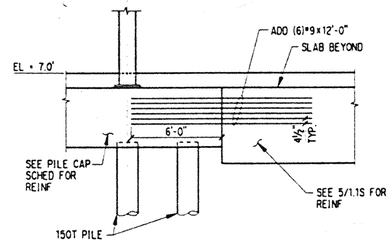


COMPRESSION SPLICE

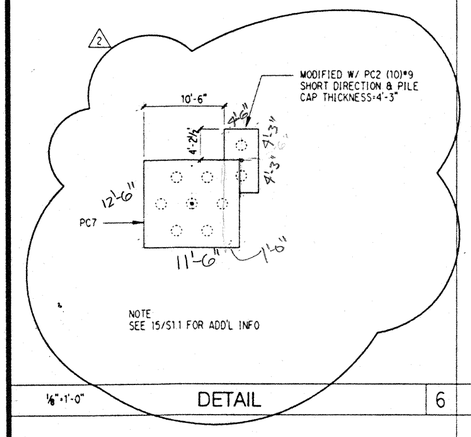
PILE SPLICE DETAILS 2

3

4



¼"=1'-0" SECTION 5



⅛"=1'-0" DETAIL 6

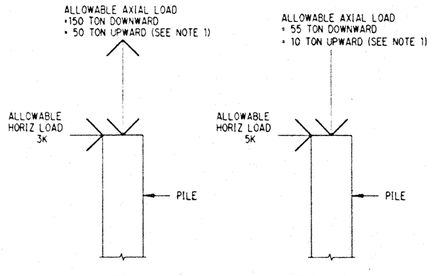
7

8

9

10

11



150 TON PILES 55 TON PILES

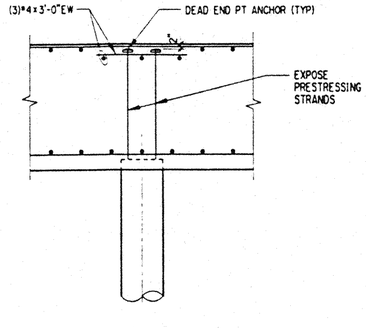
NOTES:
1. PILE CAPS REQUIRING UPLIFT CAPACITY PILES DENOTED WITH A "U" ON THE PLANS EX: PC14U

PILE DESIGN CAPACITY 12

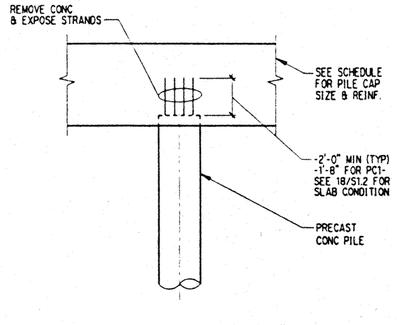
13

14

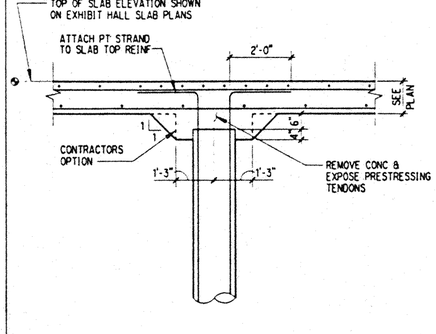
15



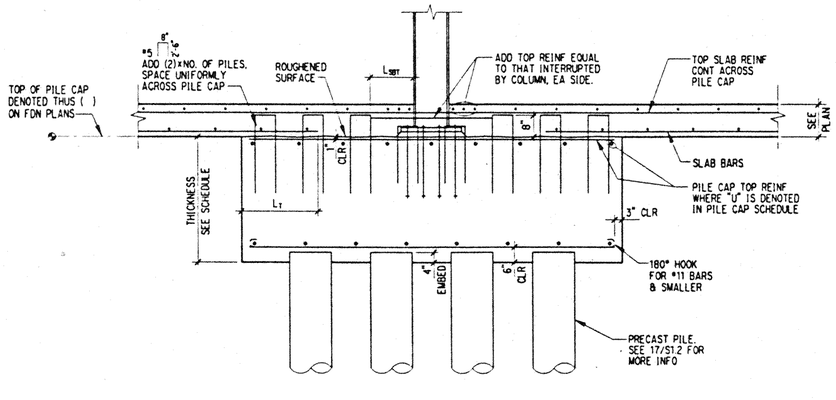
TYPICAL UPLIFT PILE DETAIL 16



TYPICAL PILE DOWEL DETAIL 17



TYP PILE CAP SECT FOR PC1 @ SLAB 18



TYPICAL PILE CAP SECTION FOR PC2-PC32 20

Hawai'i Convention Center

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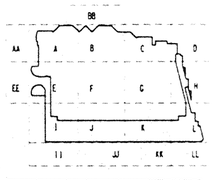
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 8 Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
3	6-19-95		
2	4-28-95		ADDENDUM #1
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

TYPICAL CONCRETE AND FOUNDATION DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S1.2

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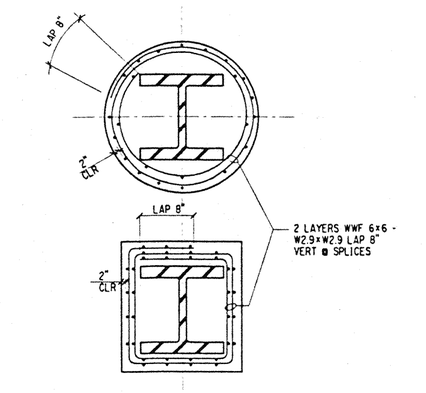
DMS plt



TYPICAL STEEL COLUMN ENCASEMENT | 1

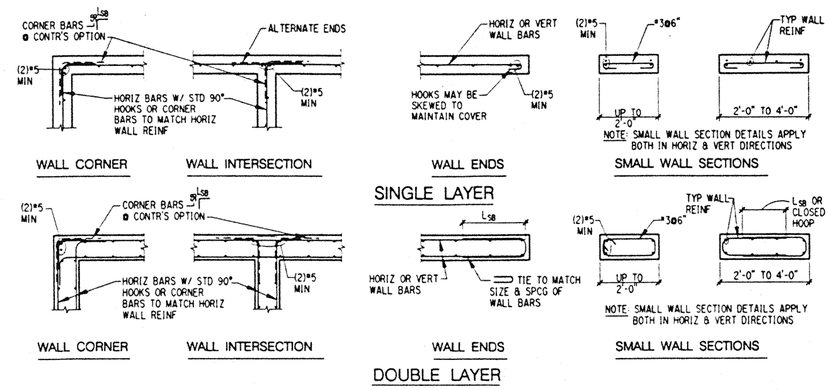
L_D f'c = 4000 psi					
	BEAMS & COLUMNS			ALL OTHERS (SLABS, WALLS, ETC)	
BAR SIZE	CTR TO CTR SPACING (S) S≥3d	3d < S<4d	S≥4d	CONC COVER (C) C≤d	C>d_b
#3	12	12	12	12	12
#4	15	15	15	15	15
#5	24	18	18	24	18
#6	34	24	22	34	24
#7	46	32	25	46	32
#8	60	42	30	60	42
#9	76	53	38	76	53
#10	97	68	48	97	68
#11	119	83	59	119	83
#14	162	113	81	162	113
#18	237	166	119	237	166

CONCRETE REINFORCING BAR DEVELOPMENT & BASIS OF LAP SPLICE LENGTHS | 2

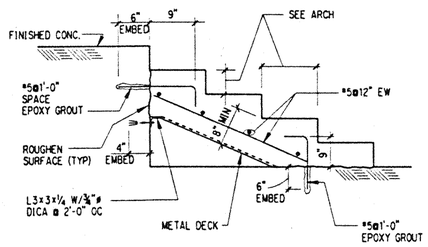
NOTES:

1. NOTATIONS:
   - d_b: NOMINAL BAR DIAMETER
   - L_D: TENSION DEVELOPMENT LENGTH FOR OTHER THAN TOP BARS
   - L_T: DEVELOPMENT LENGTH OF TOP BARS IN TENSION = 1.3 x L_D
   - L_B: DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = 19 x d_b
   - L_C: TIED COLUMN LAP SPLICE IN COMPRESSION = 30 x d_b
   - L_{CS}: SPIRAL COLUMN LAP SPLICE IN COMPRESSION = 22.5 x d_b
   - L_{SB}: TENSION LAP SPLICE LENGTH FOR OTHER THAN TOP BARS = 1.3 x L_D
   - L_{SBT}: TENSION LAP SPLICE LENGTH OF TOP BARS = 1.69 x L_D
   - L_{SBT}: SHALL BE USED FOR ALL TOP BAR CONDITIONS EVEN WHEN SHOWN AS L_{SB} ON DRAWINGS.
2. TOP BARS: HORIZONTAL REINF WITH MORE THAN 12" OF CONCRETE CAST BELOW.
3. REINFORCING fy=60,000 psi
4. FOR f'c=3000 psi L_B =22Xd_b
5. #14 & #18 BARS SHALL NOT BE LAP SPLICED. SEE GENERAL NOTES ALSO.
6. THESE TABLES ARE SIMPLIFIED VERSIONS OF ACI CODE CHAPTER 12 REQUIREMENTS AND MAY NOT MATCH OTHER PUBLISHED TABLES.
7. IN SLABS AND WALLS, FOR #11 OR SMALLER BARS WITH S>6d_b, DEVELOPMENT LENGTH MAY BE MULTIPLIED BY 0.80 EXCEPT IT SHALL NOT BE TAKEN LESS THAN THE LENGTH SHOWN UNDER S≥4d_b.

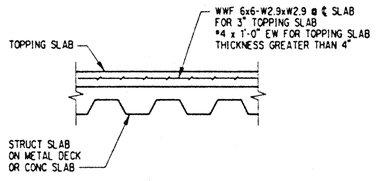
DEVELOPMENT & LAP SPLICE LENGTH NOTES | 3



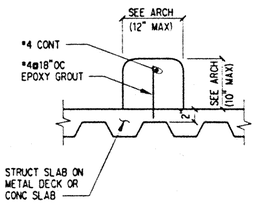
TYPICAL CONCRETE WALL REINFORCING DETAILS | 5



TYPICAL CONCRETE INFILL STAIR | 6

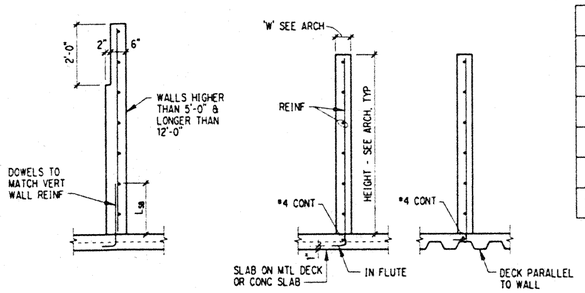


TYPICAL ARCH TOPPING SLAB | 7



TYPICAL MISC. CURB | 8

9 | 10

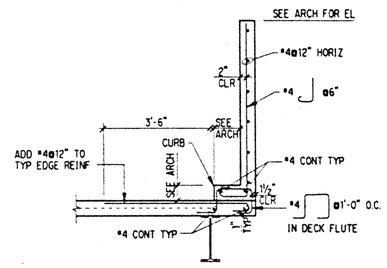


H	WALL THICK W	REINF
0'-3'	6"	#4 @12" ℄ EW
0'-3'	8"	#4 @12" ℄ EW
0'-3'	>8"	#4 @12" EF.EW
3'-4'	6"	#4 @12" ℄ VERT #5 @12" ℄ HORIZ *
4'-5'	6"	#5 @12" ℄ VERT #5 @12" ℄ HORIZ *
>5'	6"	#5 @12" ℄ VERT #5 @ 6" ℄ HORIZ *

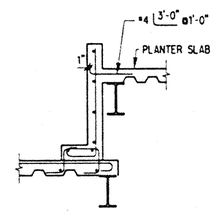
* FOR WALLS LONGER THAN 12'-0" HORIZ REINF = #5 @6"

PLANTER WALLS, CURBS, & SEATS | 12

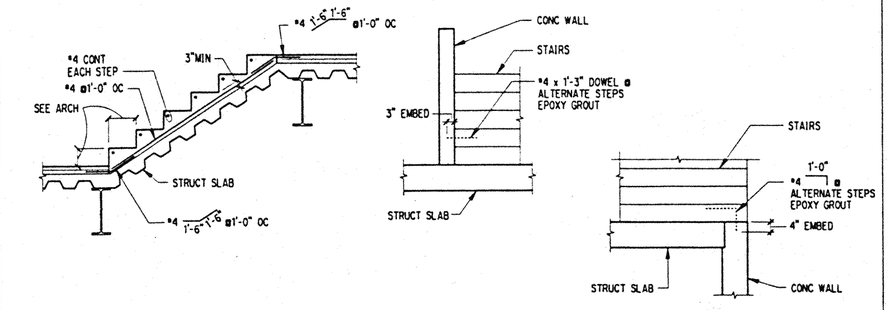
13



VEHICLE BARRIER

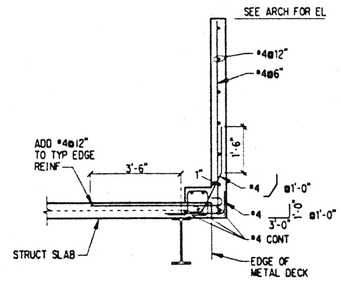


BARRIER @ EAST RAMP & PLANTER

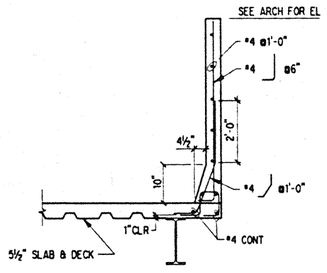


TYPICAL ARCHITECTURAL STAIR ON STRUCTURAL SLAB | 17

18



VEHICLE BARRIER



VEHICLE BARRIER

TYPICAL VEHICLE BARRIER DETAILS | 20

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Consulting Structural and Civil Engineers
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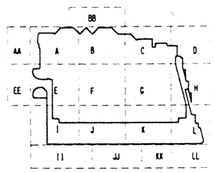
(206) 292-1200 FAX: (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision highlighted in Title 46 Chapter 16 Section 1.3.4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
1	12-15-95		FINAL DOCUMENTS

Sheet Title

TYPICAL ARCHITECTURAL CONCRETE DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 12 / 15 / 95
Job No. 93036.02.9

Sheet No.

S1.3

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STANDARD HOOKS

RECOMMENDED END HOOKS
ALL GRADES
D= FINISHED BEND DIAMETER

BAR SIZE	180° HOOKS D	180° HOOKS A OR G	180° HOOKS J	90° HOOKS A OR G
#3	2¼	5	3	6
#4	3	6	4	8
#5	3¾	7	5	10
#6	4½	8	6	1-0
#7	5¼	10	7	1-2
#8	6	11	8	1-4
#9	9½	1-3	11¾	1-7
#10	10¾	1-5	1-1¼	1-10
#11	12	1-7	1-2¾	2-0
#14	18¼	2-3	1-9¾	2-7
#18	24	3-0	2-4½	3-5

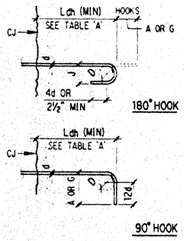


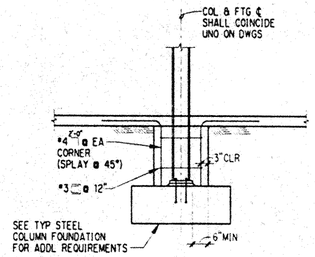
TABLE A
MINIMUM TENSION EMBEDMENT LENGTHS, L_{dh}, FOR STANDARD END HOOKS ON GRADE 60 BARS

BAR SIZE	NORMAL WEIGHT CONCRETE, f'c (psi) 3,000	4,000	5,000	6,000	7,000 & 7,500	8,000
#3	6"	6"	6"	6"	6"	6"
#4	8"	7"	6"	6"	6"	6"
#5	10"	9"	8"	7"	7"	6"
#6	12"	10"	9"	8"	8"	7"
#7	14"	12"	11"	10"	9"	9"
#8	16"	14"	12"	11"	10"	10"
#9	18"	15"	14"	13"	12"	11"
#10	20"	17"	15"	14"	13"	12"
#11	22"	19"	17"	16"	14"	14"
#14	32"	32"	29"	27"	25"	23"
#18	50"	43"	39"	35"	33"	31"

NOTES:
1. SIDE COVER ≥2½ IN
2. END COVER (90° HOOKS) ≥2 IN
3. FOR 180° HOOKS AT RIGHT ANGLES TO EXPOSED SURFACES, SEE TABLE 'B' FOR MINIMUM LENGTHS TO PROVIDE 2 IN MINIMUM COVER TO TAIL.

TABLE B
MINIMUM EMBEDMENT LENGTHS TO PROVIDE 2 IN COVER TO TAIL OF STANDARD 180 END HOOK

#3	#4	#5	#6	#7	#8	#9	#10	#11	#14	#18
6"	7"	7"	8"	9"	10"	12"	14"	15"	20"	25"

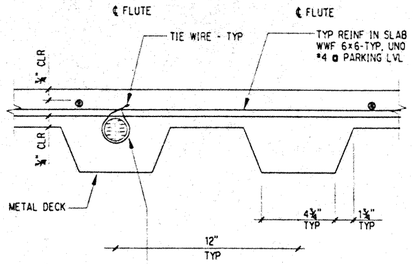


TYP BELOW GRADE STL COL ENCASEMENT DET 2

3

4

5

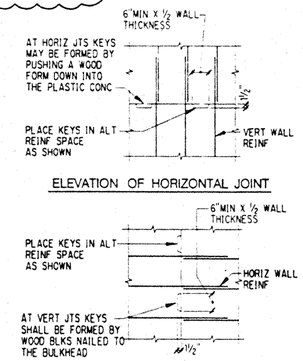


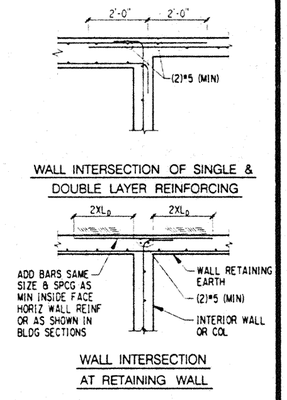
1

NOTES:
1. UNLESS NOTED OR SHOWN OTHERWISE, ALL WALLS ARE TO BE REINFORCED WITH MINIMUM REINFORCEMENT AS SHOWN IN THE FOLLOWING TABLE:

MINIMUM WALL REINFORCEMENT			
WALL THICKNESS	HORIZ. BARS	VERT. BARS	LOCATION
6" & UNDER	#4 @ 13	#4 @ 13	CENTERLINE
OVER 6-8"	#5 @ 15	#5 @ 15	CENTERLINE
OVER 8-10"	#5 @ 12	#5 @ 12	CENTERLINE
OVER 10-12"	#4 @ 12	#4 @ 12	EACH FACE
OVER 12-14"	#5 @ 18	#5 @ 18	EACH FACE
OVER 14-16"	#5 @ 15	#5 @ 15	EACH FACE
OVER 16-18"	#5 @ 14	#5 @ 14	EACH FACE
OVER 18-20"	#5 @ 12	#5 @ 12	EACH FACE
OVER 20-24"	#5 @ 10	#5 @ 10	EACH FACE

2. LAP WALL REINFORCING AT SPLICES L_{SB}.
3. ALL VERTICAL REINFORCING IN CONCRETE WALLS SHALL BE CONTINUOUS FROM STRUCTURAL FLOOR TO STRUCTURAL FLOOR, OR FROM FOOTING TO FIRST STRUCTURAL FLOOR ABOVE, UNLESS NOTED OTHERWISE.
4. START HORIZONTAL AND VERTICAL BARS 1 INCH CLEAR OF EDGE OF OPENINGS. SPACE REINFORCING BARS AT EQUAL SPACES NOT TO EXCEED REQUIRED SPACING.
5. REFER TO TABLES FOR VALUES OF L_D, L_{SB}.
6. SPLICES IN HORIZONTAL REINFORCING SHALL BE STAGGERED. SPLICES IN TWO CURTAINS WHERE USED SHALL NOT OCCUR IN THE SAME LOCATION.





TYP STANDARD HOOKS & EMBEDMENT 6

TYP CONDUIT IN SLAB ON METAL DECK DETAIL 7

TYPICAL CONCRETE WALL REINFORCING DETAILS 10

L_D f'c = 3000 psi

BAR SIZE	BEAMS & COLUMNS CTR TO CTR SPACING (S) S≤3d	3d < S<4d	S≥4d	ALL OTHERS (SLABS, WALLS, ETC) CONC COVER (C) C≤d	C>d
#3	13	13	13	13	13
#4	16	17	17	18	17
#5	27	21	21	27	21
#6	39	27	25	39	27
#7	53	37	29	53	37
#8	70	49	35	70	49
#9	88	62	44	88	62
#10	112	78	56	112	78
#11	137	96	69	137	96
#14	187	131	93	187	131
#18	274	192	137	274	192

L_D f'c = 4000 psi

BAR SIZE	BEAMS & COLUMNS CTR TO CTR SPACING (S) S≤3d	3d < S<4d	S≥4d	ALL OTHERS (SLABS, WALLS, ETC) CONC COVER (C) C≤d	C>d
#3	12	12	12	12	12
#4	15	15	15	15	15
#5	24	18	18	24	18
#6	34	24	22	34	24
#7	46	32	25	46	32
#8	60	42	30	60	42
#9	76	53	38	76	53
#10	97	68	49	97	68
#11	119	83	59	119	83
#14	162	113	81	162	113
#18	237	166	119	237	166

L_D f'c = 5000 psi

BAR SIZE	BEAMS & COLUMNS CTR TO CTR SPACING (S) S≤3d	3d < S<4d	S≥4d	ALL OTHERS (SLABS, WALLS, ETC) CONC COVER (C) C≤d	C>d
#3	12	12	12	12	12
#4	14	13	13	14	13
#5	21	16	16	21	16
#6	30	21	19	30	21
#7	41	29	23	41	29
#8	54	38	27	54	38
#9	68	48	34	68	48
#10	87	61	43	87	61
#11	106	74	53	106	74
#14	145	101	72	145	101
#18	212	149	106	212	149

L_D f'c = 6000 psi

BAR SIZE	BEAMS & COLUMNS CTR TO CTR SPACING (S) S≤3d	3d < S<4d	S≥4d	ALL OTHERS (SLABS, WALLS, ETC) CONC COVER (C) C≤d	C>d
#3	12	12	12	12	12
#4	13	12	12	13	12
#5	20	15	15	20	15
#6	28	19	18	28	19
#7	37	26	21	37	26
#8	49	35	25	49	35
#9	62	44	31	62	44
#10	79	55	40	79	55
#11	97	68	49	97	68
#14	132	92	66	132	92
#18	194	136	97	194	136

L_D f'c = 8000 psi

BAR SIZE	BEAMS & COLUMNS CTR TO CTR SPACING (S) S≤3d	3d < S<4d	S≥4d	ALL OTHERS (SLABS, WALLS, ETC) CONC COVER (C) C≤d	C>d
#3	12	12	12	12	12
#4	12	12	12	12	12
#5	17	13	13	17	13
#6	24	17	15	24	17
#7	32	23	18	32	23
#8	43	30	21	43	30
#9	54	38	27	54	38
#10	68	48	34	68	48
#11	84	59	42	84	59
#14	114	80	57	114	80
#18	168	118	84	168	118

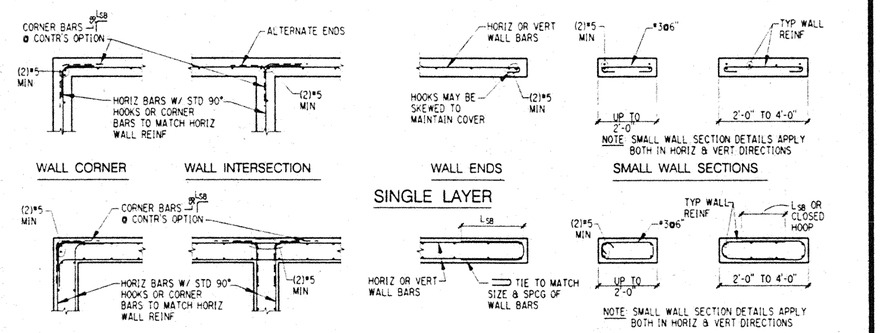
ALL CONCRETE STRENGTHS

BAR SIZE	L_B	L_C	L_{CS}
#3	8	12	12
#4	10	15	12
#5	12	19	14
#6	15	23	17
#7	17	26	20
#8	19	30	23
#9	22	34	26
#10	24	36	29
#11	27	42	32
#14	34	--	--
#18	43	--	--

FIGURES EXPRESSED IN INCHES. APPLICABLE TO STANDARD WEIGHT CONCRETE ONLY. SEE NOTE 4 FOR L_B IN f'c=3,000 psi CONCRETE.

NOTES:
1. NOTATIONS:
   - d_b: NOMINAL BAR DIAMETER
   - L_D: TENSION DEVELOPMENT LENGTH FOR OTHER THAN TOP BARS
   - L_T: DEVELOPMENT LENGTH OF TOP BARS IN TENSION = 1.3 x L_D
   - L_B: DEVELOPMENT LENGTH OF BARS OR DOWELS IN COMPRESSION = 19 x d_b
   - L_C: TIED COLUMN LAP SPLICE IN COMPRESSION = 30 x d_b
   - L_{CS}: SPIRAL COLUMN LAP SPLICE IN COMPRESSION = 22.5 x d_b
   - L_{SB}: TENSION LAP SPLICE LENGTH FOR OTHER THAN TOP BARS = 1.3 x L_D
   - L_{SBT}: TENSION LAP SPLICE LENGTH OF TOP BARS = 1.69 x L_D
   - L_{SBT}: SHALL BE USED FOR ALL TOP BAR CONDITIONS EVEN WHEN SHOWN AS L_{SB} ON DRAWINGS.
2. TOP BARS: HORIZONTAL REINF WITH MORE THAN 12" OF CONCRETE CAST BELOW.
3. REINFORCING fy=60,000 psi
4. FOR f'c=3000 psi L_B =22Xd_b
5. #14 & #18 BARS SHALL NOT BE LAP SPLICED. SEE GENERAL NOTES ALSO.

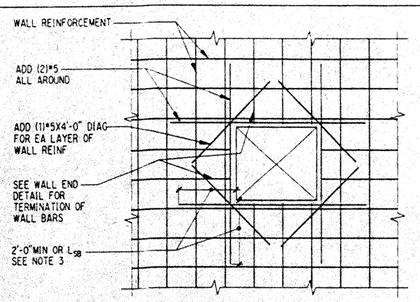
   AND MAY NOT MATCH OTHER PUBLISHED TABLES.
7. IN SLABS AND WALLS, FOR #11 OR SMALLER BARS WITH S>6d_b, DEVELOPMENT LENGTH MAY BE MULTIPLIED BY 0.80 EXCEPT IT SHALL NOT BE TAKEN LESS THAN THE LENGTH SHOWN UNDER S≥4d_b.



CONCRETE REINFORCING BAR DEVELOPMENT & BASIS OF LAP SPLICE LENGTHS 12

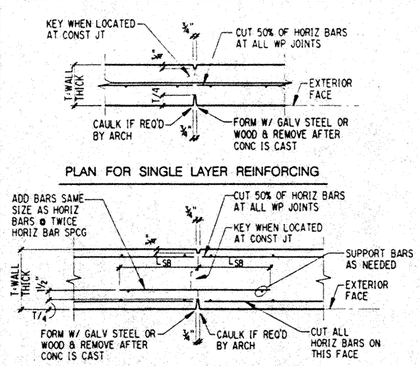
DEVELOPMENT & LAP SPLICE LENGTH NOTES 13

TYPICAL CONCRETE WALL REINFORCING DETAILS 15

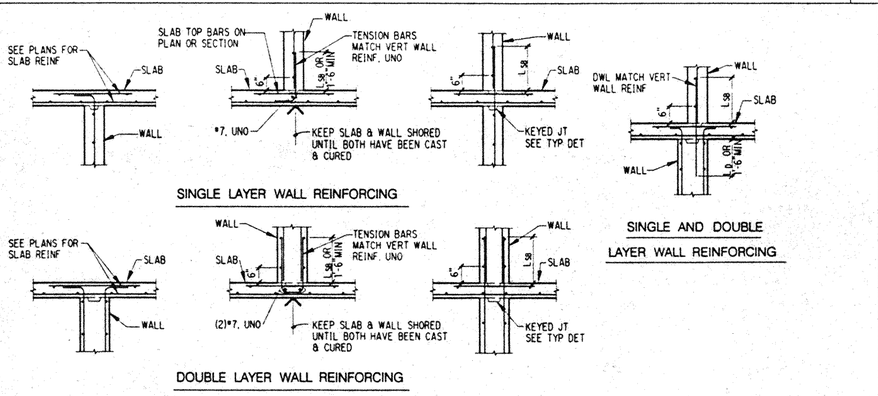


NOTES:
1. OMIT ADDED REINFORCEMENT NOTED ABOVE WHEN SPECIAL REINFORCEMENT INDICATED ON PLANS OR DETAILS EXCEEDS THIS REINFORCEMENT.
2. CONTRACTOR SHALL VERIFY ALL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS WITH THE STRUCTURAL ENGINEER BEFORE PLACEMENT.
3. WHEN EDGE OF CONCRETE CLOSE TO OPENING WILL NOT ALLOW THIS LENGTH, CONSULT STRUCTURAL ENGINEER BEFORE CONSTRUCTION.

TYPICAL CONCRETE WALL OPENING REINFORCEMENT 17



TYP WEAKENED PLANE JT FOR CONC WALL 18



TYPICAL CONCRETE REINFORCING FOR INTERSECTIONS OF SLABS AND WALLS 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC / PCL
A Joint Venture
Honolulu, HI

Consultants

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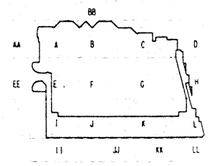
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Contributors of this project will be under my observation.

Supervision is defined in Title 16 Chapter 18 Section 1.3.24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
4	12-15-95		FINAL DOCUMENTS
3	11-7-95		
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

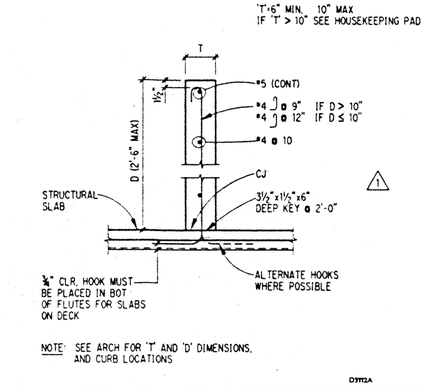
TYPICAL CONCRETE DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S1.4

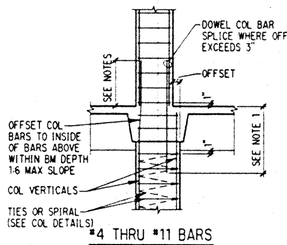
Fri Dec 8 13:09:16 1995
jkhccktyp4.dgn



NOTE: SEE ARCH FOR 'T' AND 'D' DIMENSIONS, AND CURB LOCATIONS

D3712A

TYPICAL CONCRETE CURB 1

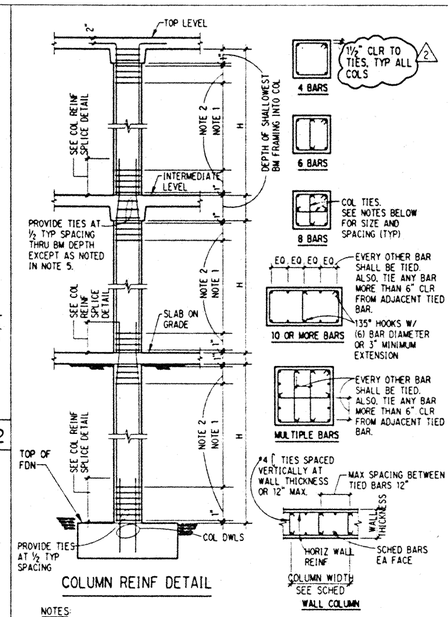


NOTES:

1. SPLICE COLUMN BARS L_{sB} OR WITH WELDED OR MECHANICAL SPLICE DEVELOPING 125% IN TENSION, UNO. WHEN NOTED "C" ON SCHEDULE, SPLICE BARS L_c OR L_{cs} OF SMALLER BAR BUT NOT LESS THAN L_{sB} OF LARGER BAR.
2. AT A CROSS-SECTION THRU A LAP SPLICE: IF THE TOTAL LONGITUDINAL BAR AREA EXCEEDS .08 TIMES THE CONCRETE AREA, THE BARS MUST BE MECHANICALLY SPLICED.
3. CLEAR DISTANCE BETWEEN A CONTACT LAP SPLICE AND ADJACENT BARS SHALL BE NOT LESS THAN 1.5 TIMES THE BAR DIAMETER OR 1½".
4. SPLICE #14 AND #18 BARS BY ONE OF THE FOLLOWING:
   A. FULL PENETRATION BUTT WELDS, ADJACENT WELDS STAGGERED 6" ON CENTER.
   B. "THERMITE WELD" SPLICES STAGGERED 30" ON CENTER VERTICALLY BETWEEN ADJACENT BARS.
   C. MECHANICAL SPLICES MAY BE USED. SUBMIT TO ENGINEER FOR REVIEW.

D333

CONC COLUMN REINF SPLICE DETAIL 2



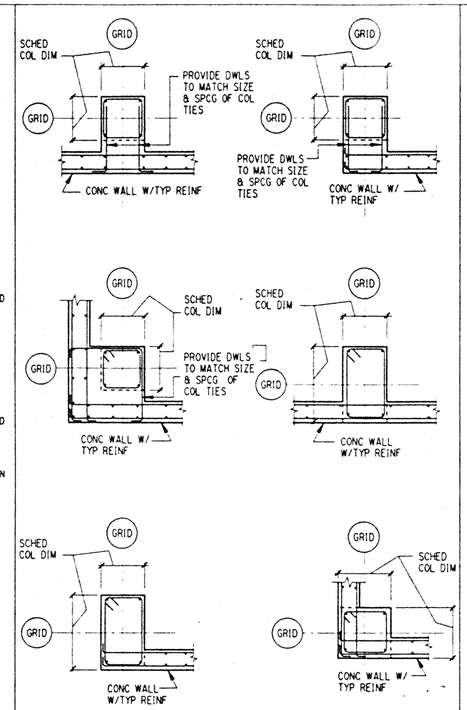
COLUMN REINF DETAIL

NOTES:

1. TYPICAL TIE SPACING SHALL NOT EXCEED SPACING SCHEDULED. 16 LONGITUDINAL BAR DIAMETERS, 48 TIE BAR DIAMETERS, OR LEAST DIMENSION OF COLUMN.
2. TIE SPACING SHALL BE ONE-HALF SPACING REQUIRED BY NOTE #1 FOR H/6, MAXIMUM COLUMN DIMENSION OR 18 INCHES, WHICHEVER IS GREATER.
3. ALL COLUMN TIES SHALL BE #4, UNO.
4. AT CONTRACTOR'S OPTION, LARGER SCHEDULED BARS MAY BE EXTENDED UP ADDITIONAL FLOOR LEVELS WITHOUT SPLICES AT FLOORS.
5. TIES ARE NOT REQUIRED WHERE BEAMS FRAME INTO FOUR SIDES OF COLUMN.

D331

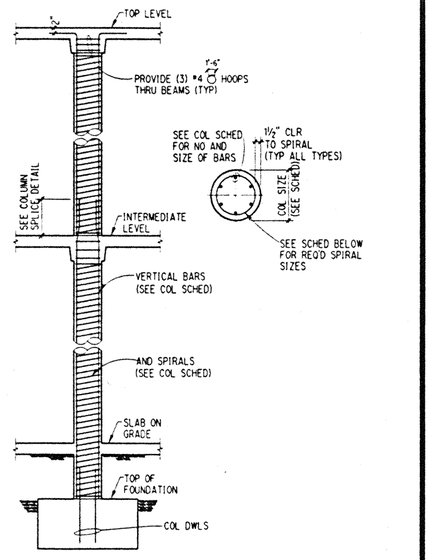
TYP CONC TIED COL & WALL COL DETS (TYPE 'T') 8



NOTES:

1. PROVIDE PILASTER REINF PER TYP CONC COL DETAIL & CONC COL. SCHED
2. PROVIDE TYP TIE SPACING THRUOUT PILASTER HEIGHT. REFER TO TIED COLUMN DETAIL 8/S1.5 FOR ADD'L INFO

TYP CONC PILASTER COLUMN DETAILS (TYPE 'P') 9



COLUMN REINF DETAIL

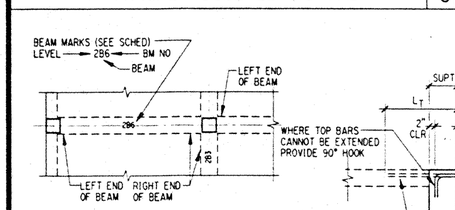
NOTE:

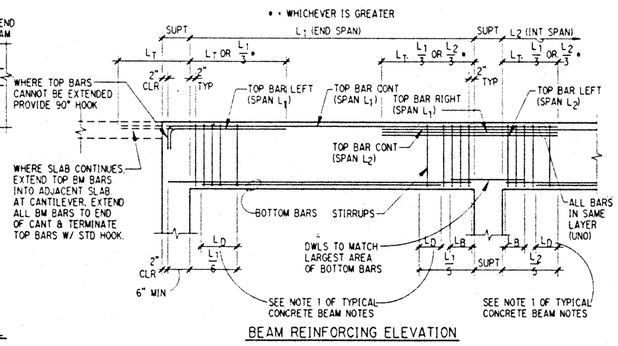
SPIRAL SHALL BE CONTINUOUS FROM TOP OF SLAB OR FOOTING TO SOFFIT ABOVE. ADD ONE-HALF EXTRA TURN AT TOP AND BOTTOM.

GRADE 60 SPIRAL SIZES FOR ROUND COLUMNS		
COL DIA.	F'c = 4,000 PSI	F'c = 7,500 PSI
3'-0"	½" DIA @ 2¼	--
6'-0"	--	--
8'-0"	½" DIA @ 3	½" DIA @ 2¼

D332

TYP CONC SPIRAL COL DETS (TYPE 'SP') 10





D341

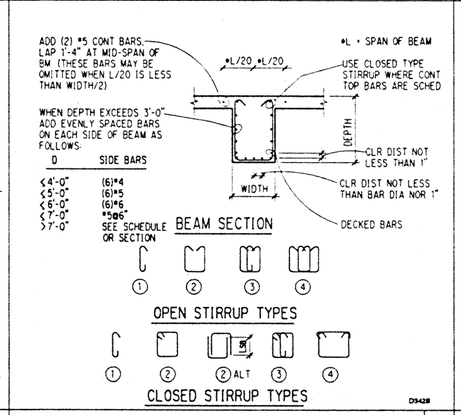
TYPICAL CONCRETE BEAM DETAILS 12

NOTES:

1. AT CONTRACTOR'S OPTION, WHERE REQUIRED TO RELIEVE BAR CONGESTION, NOT MORE THAN 50 PERCENT OF THE AREA OF THE STRAIGHT BOTTOM BARS MAY BE TERMINATED AS SHOWN.
2. BEAM SCHEDULES DO NOT INDICATE REQUIREMENTS FOR ARRANGING BARS. THE CONTRACTOR SHALL DETAIL AND PLACE REINFORCING STEEL IN NO MORE THAN TWO LAYERS ONLY WHERE REQUIRED TO PROVIDE PROPER CLEARANCES BETWEEN BARS IN A LAYER AND WHERE REQUIRED IN ORDER TO PROPERLY CLEAR COLUMN VERTICALS AND SIMILAR REINFORCING.
3. REFER TO TABLE OF SPLICE AND DEVELOPMENT LENGTHS FOR L_T, L_B, AND L_D.
4. EITHER 90- OR 180-DEGREE STANDARD HOOK BARS MAY BE USED FOR LONGITUDINAL BARS.
5. WHERE TOP AND SIDE BARS ARE INDICATED AS CONTINUOUS AND RUN OVER 60 FEET IN LENGTH, SPLICES MAY BE MADE USING THE L_{sB} COLUMN IN THE TABLE OF DEVELOPMENT LENGTHS. TOP BAR SPLICES SHOULD BE IN THE MIDDLE THIRD OF BEAM SPAN. WHERE BOTTOM BARS ARE SHOWN AS CONTINUOUS AND RUN IN EXCESS OF 60 FEET, A LAP SPLICE MAY BE USED EQUAL TO THE DEVELOPMENT LENGTHS SHOWN IN COLUMN L_{sB} OF THE TABLE OF DEVELOPMENT LENGTHS. THE BOTTOM BAR SPLICES SHOULD BE OUTSIDE THE MIDDLE THIRD OF THE BEAM SPAN. THE SIDE BAR SPLICES MAY BE MADE, WHERE CONVENIENT.
6. LOCATE ALL CONSTRUCTION JOINTS WITHIN THE MIDDLE THIRD OF SPANS. JOINTS SHALL BE OFFSET AT A MINIMUM DISTANCE OF TWO TIMES THE WIDTH OF INTERSECTING BEAMS. SUBMIT LOCATION OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW AND ACCEPTANCE BEFORE FORMING.
7. STANDARD HOOKS FOR STIRRUPS SHALL BE 135 DEGREES BEND PLUS 6d EXTENSION, BUT NOT LESS THAN 3 INCHES.

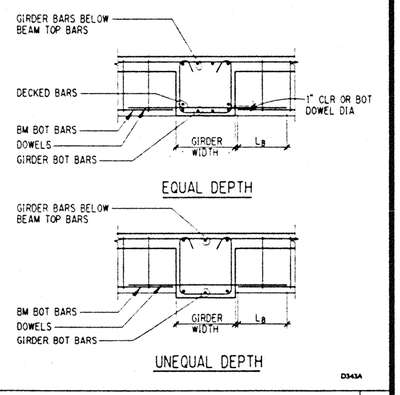
D342A

TYPICAL CONCRETE BEAM NOTES 13



D342B

TYP CONC BM SECTION & STIRRUP DETS 14



D343A

INTERSECT OF CONC BM & GIRDER DETS 15

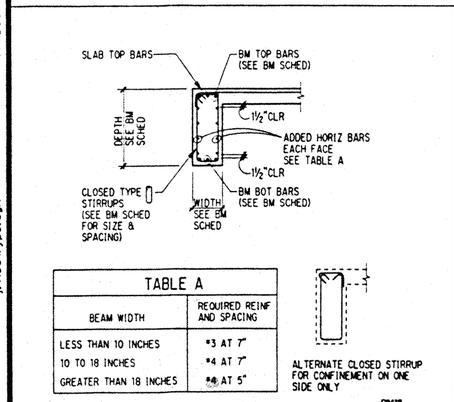
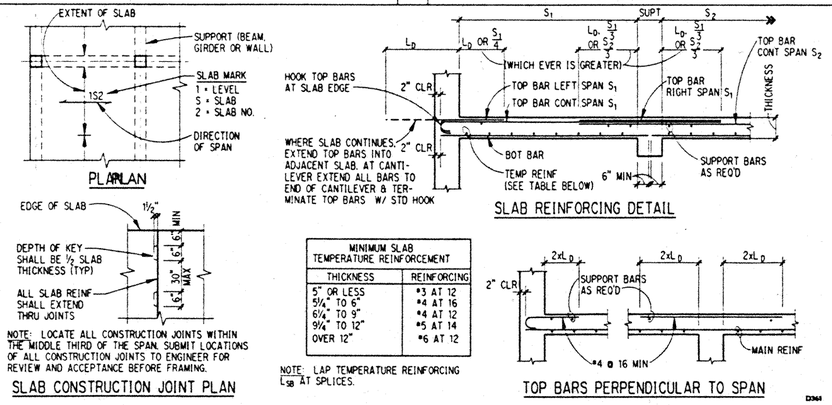


TABLE A	
BEAM WIDTH	REQUIRED REINF AND SPACING
LESS THAN 10 INCHES	*3 AT 7"
10 TO 18 INCHES	*4 AT 7"
GREATER THAN 18 INCHES	*4 AT 5"

D343B

TYP CONC EDGE BM DETAIL 16

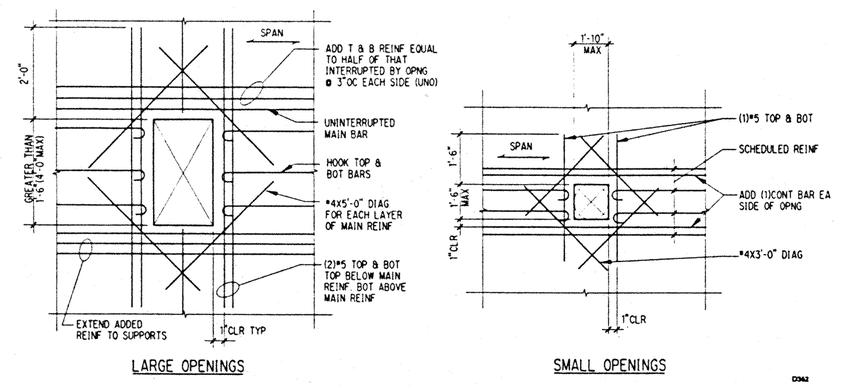


MINIMUM SLAB TEMPERATURE REINFORCEMENT	
THICKNESS	REINFORCING
5" OR LESS	*3 AT 12
5¼" TO 6"	*4 AT 16
6¼" TO 9"	*4 AT 12
9¼" TO 12"	*5 AT 14
OVER 12"	*6 AT 12

NOTE: LAP TEMPERATURE REINFORCING L_{sB} AT SPLICES.

D361

TYPICAL ONE-WAY SLAB DETAILS 18



D362

TYPICAL ONE-WAY SLAB OPENING REINFORCEMENT 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

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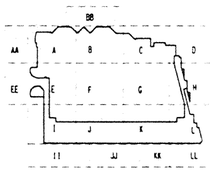
This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 18 Section 1.2.34 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

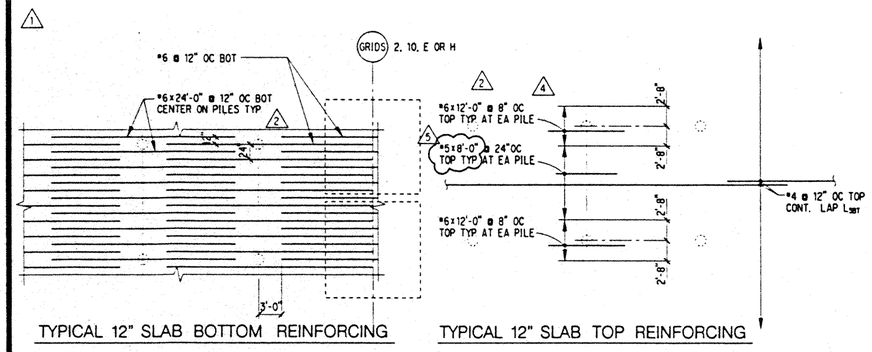
Sheet Title

TYPICAL CONCRETE DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S1.5

Fri Dec 1 13:35:58 1995
jahccttyp5.dgn
DWS plt



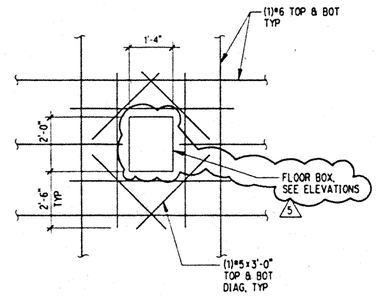
TYPICAL 12" SLAB BOTTOM REINFORCING

TYPICAL 12" SLAB TOP REINFORCING

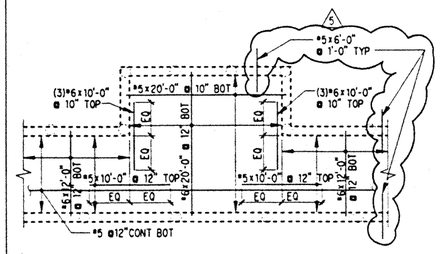
NOTES:

1. BARS SHOWN IN DETAILS ARE SHOWN IN ONLY ONE DIRECTION FOR CLARITY. REINFORCING SHALL BE TYPICAL FOR BOTH DIRECTIONS.
2. SEE PLANS FOR ADDITIONAL REINFORCING NOT SHOWN IN TYPICAL DETAILS.

TYPICAL SLAB REINFORCING FOR EXHIBIT HALL SLAB WITHIN GRIDS 2-10 & E-H | 2

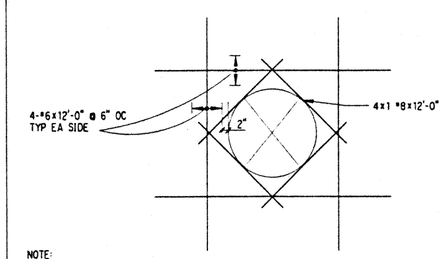


TYPICAL FLOOR BOX DETAIL | 3



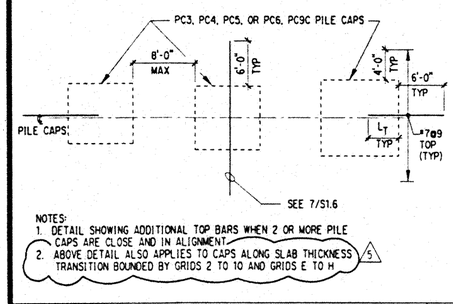
TYPICAL SLAB REINFORCING OVER UTILIDOR

TYPICAL SLAB REINFORCING DETAIL | 4



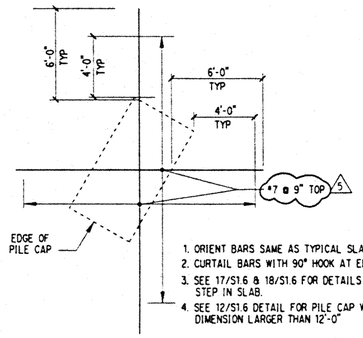
NOTE:
1. TERMINATE STD SLAB REINF 2" CLR OF HOLE.
2. BOTTOM STEEL SHOWN DETAIL REPEATS FOR TOP STEEL

ADD'L REINF AT 6'-0" DIA OPENING | 5



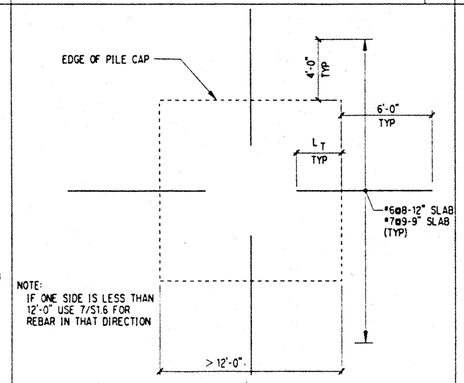
NOTES:
1. DETAIL SHOWING ADDITIONAL TOP BARS WHEN 2 OR MORE PILE CAPS ARE CLOSE AND IN ALIGNMENT.
2. ABOVE DETAIL ALSO APPLIES TO CAPS ALONG SLAB THICKNESS TRANSITION BOUNDED BY GRIDS 2 TO 10 AND GRIDS E TO H

TOP REINF'T AT PILE CAP STRING | 6



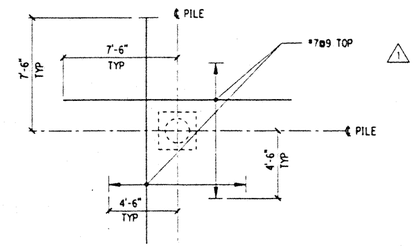
1. ORIENT BARS SAME AS TYPICAL SLAB REINF'T.
2. CURTAIL BARS WITH 90° HOOK AT EDGE OF SLAB
3. SEE 17/S1.6 & 18/S1.6 FOR DETAILS AT STEP IN SLAB.
4. SEE 12/S1.6 DETAIL FOR PILE CAP WITH SIDE DIMENSION LARGER THAN 12'-0"

ADD'L SLAB TOP REINF AT MULTI-PILE CAP | 7



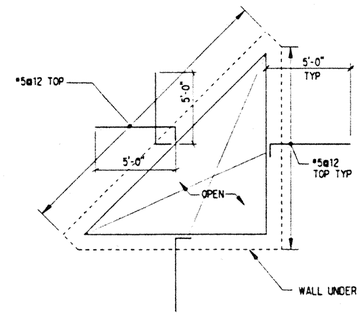
NOTE:
IF ONE SIDE IS LESS THAN 12'-0" USE 7/S1.6 FOR REBAR IN THAT DIRECTION

ADD'L TOP REBAR AT LARGE PILE CAPS | 8



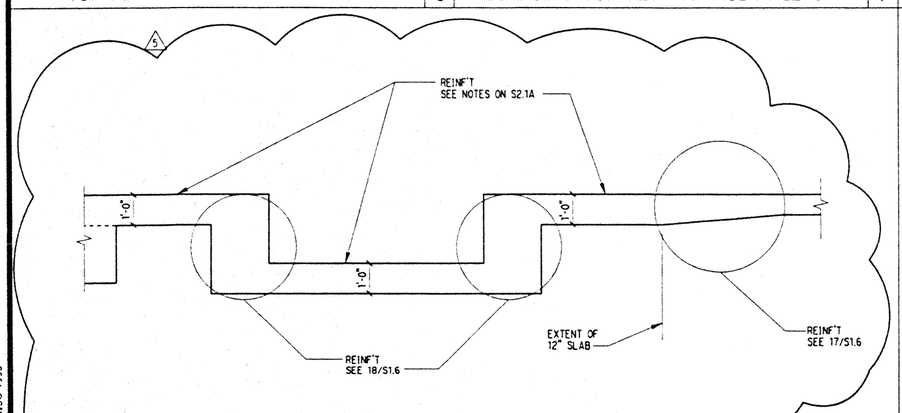
NOTE:
CURTAIL BARS WITH 90° HOOK IF SLAB EDGE ENCOUNTERED

ADD'L SLAB TOP REINF @ SINGLE PILE CAP | 9

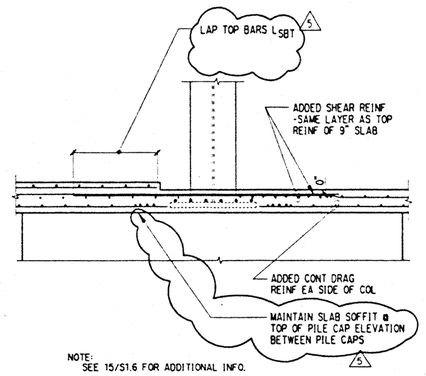


ADDITIONAL BARS @ TYPICAL OPENING OR STEP WITH WALL UNDER

DETAIL | 10

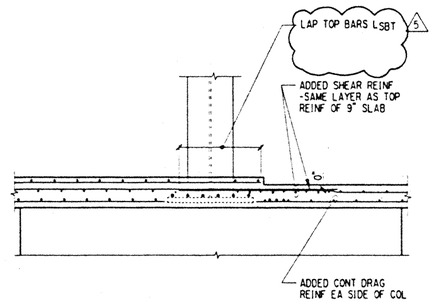


SECTION | 12



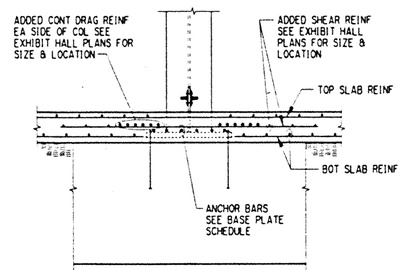
NOTE:
SEE 15/S1.6 FOR ADDITIONAL INFO.

TYPICAL SLAB STEP @ PILE CAP @ GRID E | 13

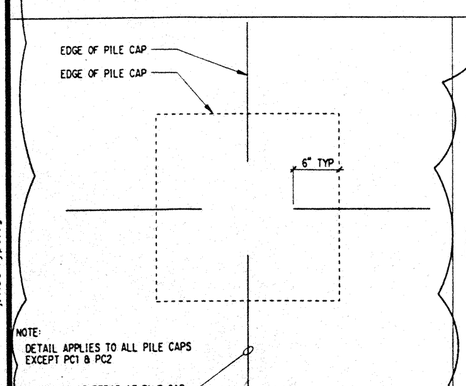


NOTE:
SEE 15/S1.6 FOR ADDITIONAL INFO.

TYP SLAB STEP @ PILE CAP @ GRIDS E & 2 | 14

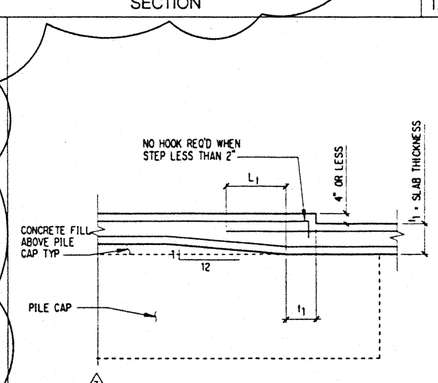


SECTION | 15

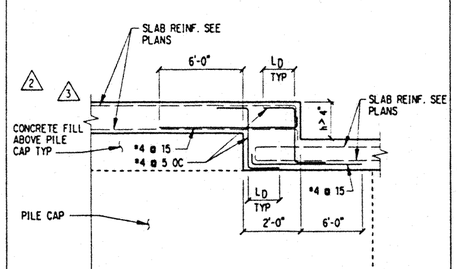


NOTE:
DETAIL APPLIES TO ALL PILE CAPS EXCEPT PC1 & PC2

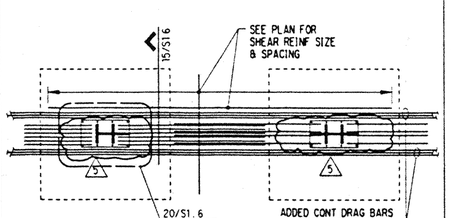
BOTTOM SLAB REBAR AT PILE CAPS | 16



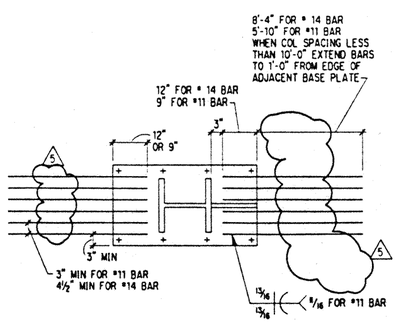
SLAB STEP 4" OR LESS | 17



SLAB STEP GREATER THAN 4" TO 2'-0" | 18



TYP ADDED SLAB REINF @ FRAME COLS | 19



ANCHOR BAR DETAIL | 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

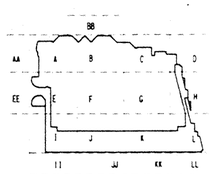
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
5	12-15-95		FINAL DOCUMENT
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-31-95		
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

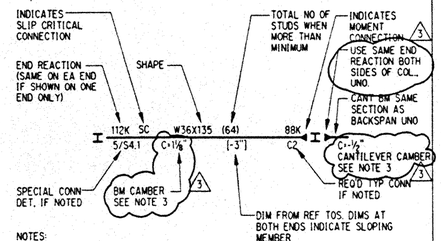
TYPICAL CONCRETE DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S1.6

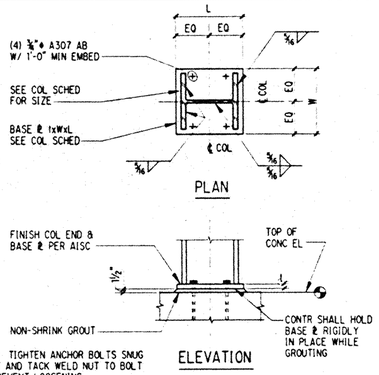
Fri Dec 1 13:38:30 1995
j:\hccttyp6.dgn
DMS plt



NOTES:

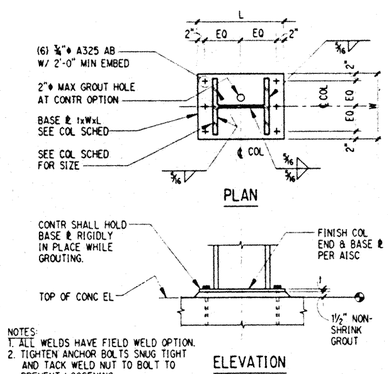
1. NO REACTION AT EITHER END INDICATES MIN CONN FOR BM DEPTH. SEE "GENERAL NOTES FOR STEEL CONNECTIONS."
2. SPACE STUDS PER TYP SHEAR STUD PLACEMENT DETAIL
3. POSITIVE CAMBER UPWARDS, NEGATIVE CAMBER DOWNWARDS. CAMBER SHOWN ON CANTILEVERS IS AT END OF CANTILEVER RELATIVE TO A LINEAR EXTRAPOLATION OF BACKSPAN SUPPORT POINTS.
4. [ETS] AT BEAM END INDICATES BEAM TOS EQUALS SUPPORT BEAM TOS.
5. FOR BU SECTIONS SEE SHEET S1.22

TYPICAL BEAM CALLOUTS | 1



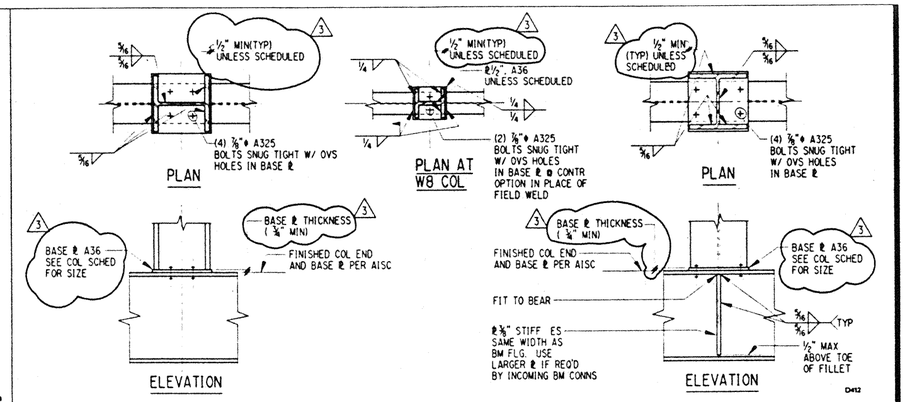
NOTE: TIGHTEN ANCHOR BOLTS SNUG TIGHT AND TACK WELD NUT TO BOLT TO PREVENT LOOSENING

TYPICAL BASE PLATE DETAIL, TYPE 1 | 2

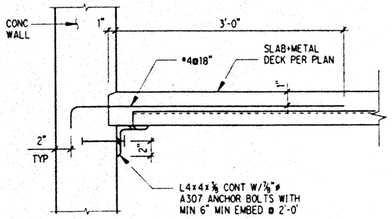


NOTES:
1. ALL WELDS HAVE FIELD WELD OPTION.
2. TIGHTEN ANCHOR BOLTS SNUG TIGHT AND TACK WELD NUT TO BOLT TO PREVENT LOOSENING.

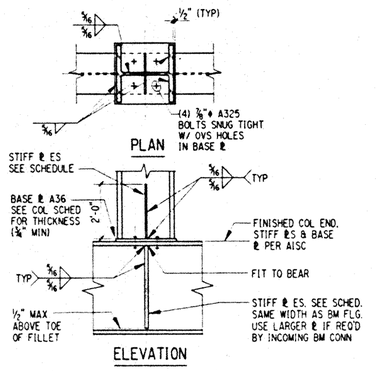
TYPICAL BASE PLATE DETAIL, TYPE 2 | 3

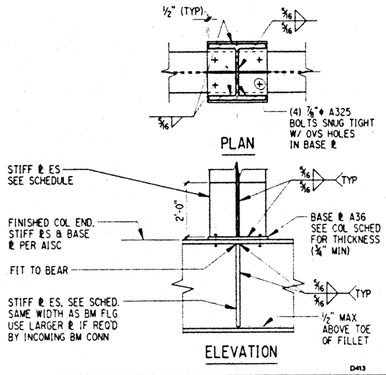


TYPICAL BASE PLATE DETAIL, TYPE 3 | 5

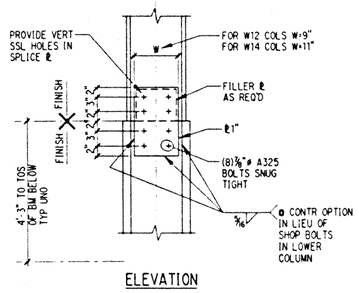


SECTION | 6





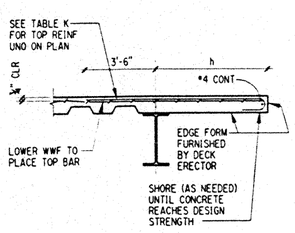
TYPICAL BASE PLATE DETAIL, TYPE 4 | 8



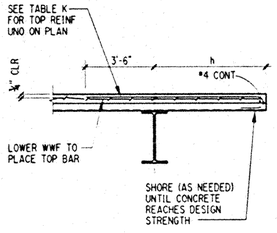
NOTES:
1. CONTRACTOR SHALL PROVIDE ALL ERECTION AND STABILITY AIDS
2. REFER TO AISC FOR SPLICE DETAIL WHERE NOMINAL DEPTH CHANGES

COLUMN SPLICE DETAIL | 9

10



DECK SPANNING PARALLEL TO BEAM

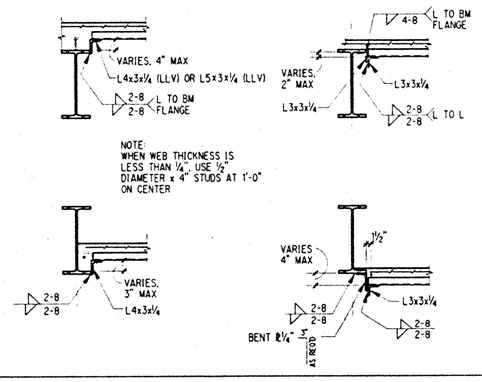


DECK SPANNING PERPENDICULAR TO BEAM

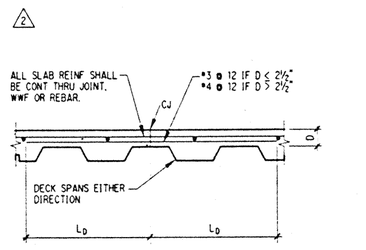
TABLE K	
h	REINF
0" TO 9"	-
OVER 9" TO 1'-4"	#3 @ 12
OVER 1'-4" TO 2'-0"	#4 @ 12
OVER 2'-0" TO 2'-6"	#4 @ 8
OVER 2'-6" TO 3'-0"	#4 @ 6

D472

TYPICAL DECK EDGE CONDITIONS | 12



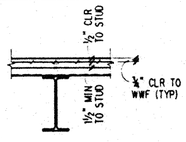
TYPICAL DECK SUPPORT DETAILS WHEN ELEVATION OF DECK VARIES | 14



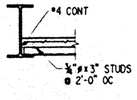
NOTE:
SUBMIT LOCATIONS OF ALL CONSTRUCTION JOINTS TO ENGINEER FOR REVIEW AND ACCEPTANCE BEFORE FORMING.

D476

TYP SLAB ON DECK CJ DETAIL | 15



TYPICAL DECK SUPPORT

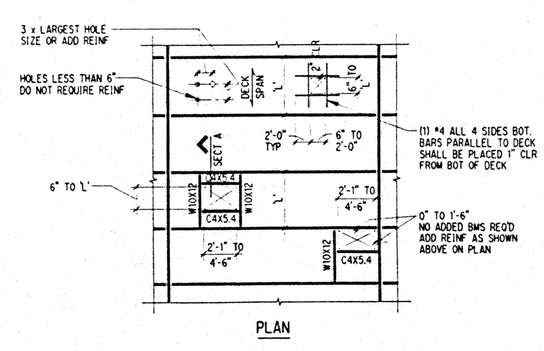


NOTE:
WHEN WEB THICKNESS IS LESS THAN ¼", USE ½" DIAMETER x 3" STUDS AT 1'-0" ON CENTER.

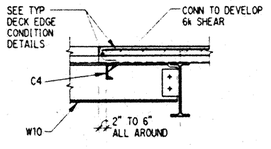
BOTTOM FLANGE DECK SUPPORT

D471

TYPICAL DECK SUPPORT DETAILS | 16



FOR OPENINGS LESS THAN 4'-6"

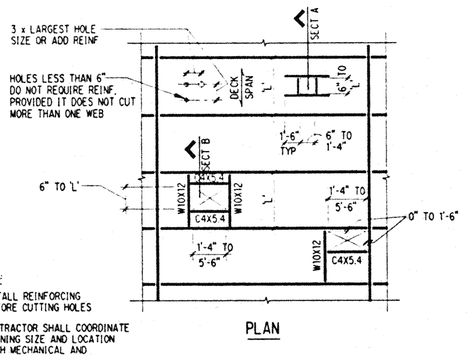


SECTION A

NOTES:
1. BLOCK OUT OPENING BEFORE PLACING CONCRETE.
2. REMOVE BLOCKOUT AND CUT DECK AFTER CONCRETE HAS CURED.
3. CONTRACTOR SHALL COORDINATE OPENING SIZE AND LOCATION WITH MECHANICAL AND ELECTRICAL CONTRACTORS AND ARCHITECTURAL DRAWINGS.
4. W10 AND C4 ARE A36

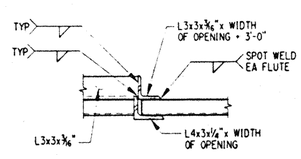
D474

TYPICAL FLOOR DECK OPENING DETAILS | 18

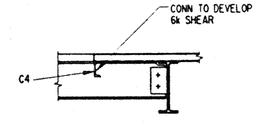


NOTES:
1. INSTALL REINFORCING BEFORE CUTTING HOLES
2. CONTRACTOR SHALL COORDINATE OPENING SIZE AND LOCATION WITH MECHANICAL AND ELECTRICAL CONTRACTORS
3. W10, C4 AND L3x3 ARE A36

FOR OPENINGS LESS THAN 5'-6"



SECTION A



SECTION B

D475

TYPICAL ROOF DECK OPENING DETAILS | 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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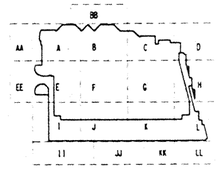
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11773 HAWAII U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

TYPICAL STEEL DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S1.7

Fri Dec 1 13:40:52 1995
jthccktyp7.dgn
pltr

TABLE A - MINIMUM NO. BOLTS		
WIDE-FLANGE BEAM DEPTH	BUILT-UP BEAM DEPTHS (IN)	MIN # OF BOLTS REQUIRED
W8, W10, W12	8 - 13	2
W14, W16, W18	To 19	3
W21, W24	To 25	4
W27, W30	To 31	5
W33, W36, W40	To 39	6
W44	To 45	7

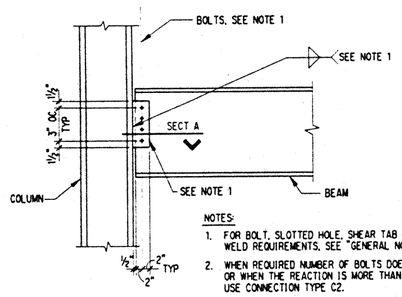
NOTES:
1. SEE PLANS FOR BEAM REACTIONS. WHERE NO DETAIL IS NOTED, USE APPROPRIATE TYPICAL DETAIL.
2. THE MINIMUM NUMBER OF BOLTS IN A BEAM WEB CONECTION SHALL BE AS SHOWN IN TABLE A.
3. WHERE REACTION IS SHOWN, USE TABLE B WITH MINIMUM NUMBER OF BOLTS AS SHOWN IN TABLE A.
4. BEAMS SHALL HAVE STANDARD ROUND HOLES (STD). AND SHEAR TAB PLATES SHALL HAVE HORIZONTAL SHORT SLOTTED HOLES (SSL). UNO.
5. WHEN BEAM WEB IS THINNER THAN REQUIRED WEB THICKNESS SHOWN IN TABLES, ADD WEB DOUBLER PER TYPICAL WEB DOUBLER DETAIL.
6. WHEN CONDITIONS VARY FROM THOSE SHOWN IN THE "TYPICAL DETAILS", DETAIL 3 CONSTRUCTION ACCORDING TO THE AISC MANUAL OF STEEL CONSTRUCTION.
7. CONTRACTOR SHALL COORDINATE THE BOLT SELECTION AND USE BETWEEN FABRICATOR AND ERECTOR.
8. FOR SKEWED CONNECTION SHOWN IN TYPE C1, SECT. A: MULTIPLY MAX. REACTION BY REDUCTION FACTOR TO GET ALLOWABLE REACTION.
9. WHEN REQUIRED NUMBER OF BOLTS DOES NOT FIT WITHIN BM DEPTH, "TYP WEB EXTENSION & DETAIL" MAY BE USED.

TABLE B - MAXIMUM REACTION

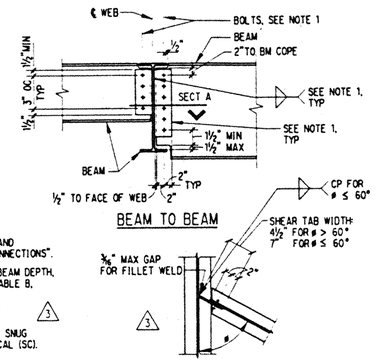
⅜" DIA A325 BOLTS						3 1" DIA A490 BOLTS						REDUCTION FACTORS FOR SKEWED CONNS. 1
# OF BOLTS	MAX REACTION (KIPS)	PLATE THICKNESS (INCH)	WELD SIZE	MIN WEB THICKNESS A36 (INCH)	MIN WEB THICKNESS FY 50 (INCH)	# OF BOLTS	MAX REACTION (KIPS)	PLATE THICKNESS (INCH)	WELD SIZE	MIN WEB THICKNESS A36 (INCH)	MIN WEB THICKNESS FY 50 (INCH)	
2	13	¼	3/16	.20 2	.19	2	20	½	⅜	.26	.24	63%
3	25	¼	3/16	.22	.19	3	41	½	⅜	.38	.34	65%
4	40	3/16	¼	.26	.24	4	65	½	⅜	.47	.42	71%
5	54	3/16	¼	.29	.27	5	81	½	⅜	.52	.47	76%
6	72	⅜	3/16	.32	.29	6	97	½	⅜	.52	.47	80%
7	87	⅜	3/16	.33	.29	7	114	½	⅜	.53	.47	83%
8	96	⅜	3/16	.33	.29	8	130	½	⅜	.53	.47	85%
9	104	7/16	⅜	.33	.29	9	147	½	⅜	.53	.47	85%

1 3 D431

GENERAL NOTES FOR STEEL CONNECTIONS | 2



BEAM TO COLUMN



BEAM TO BEAM

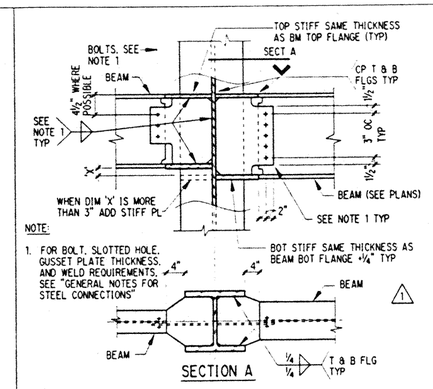
SINGLE PLATE SHEAR CONNECTIONS

NOTES:
1. FOR BOLT, SLOTTED HOLE, SHEAR TAB PLATE THICKNESS, AND WELD REQUIREMENTS, SEE "GENERAL NOTES FOR STEEL CONNECTIONS".
2. WHEN REQUIRED NUMBER OF BOLTS DOES NOT FIT WITHIN BEAM DEPTH, OR WHEN THE REACTION IS MORE THAN THE MAXIMUM IN TABLE B, USE CONNECTION TYPE C2.
3. BOLTS IN CONNECTIONS TO COLUMNS SHALL BE INSTALLED AS SLIP CRITICAL. BOLTS IN CONNECTIONS OF BEAM TO BEAM/GIRDER MAY BE SNUG TIGHT, UNLESS SPECIFICALLY CALLED OUT AS SLIP CRITICAL (SC). 3
4. FOR SPANDREL BEAMS SEE CONNECTION TYPE C26.

SECTION A (SKEWED BEAMS)

D432

TYPICAL STEEL CONNECTION, TYPE C1 | 4

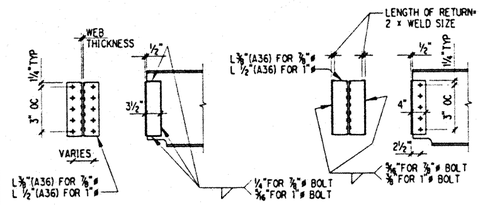


SECTION A

BEAM TO COLUMN WEB MOMENT CONNECTION

D435A

TYPICAL STEEL CONNECTION, TYPE C5 | 5



CONN C2A CONN C2B

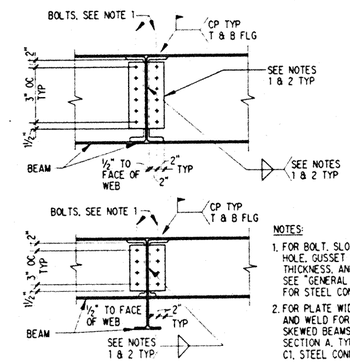
NOTES:
1. FOR REQUIRED NUMBER OF BOLTS, SEE TABLE C.
2. FOR MINIMUM NUMBER OF BOLTS, SEE NOTE 2, "GENERAL NOTES FOR STEEL CONNECTIONS".
3. WHEN BEAM WEB IS THINNER THAN REQUIRED WEB THICKNESS SHOWN IN TABLE C, REDUCE ALLOWABLE REACTION WITH THE RATIO OF BEAM WEB THICKNESS TO MIN WEB THICKNESS. FOR HIGHER REACTIONS ADD WEB DOUBLER PER TYPICAL WEB DOUBLER DETAIL.
4. TOLERANCE ON RETURN WELD SHALL BE +1/4, -0".
5. BOLTS IN CONNECTIONS TO COLUMNS SHALL BE INSTALLED AS SLIP CRITICAL. BOLTS IN BEAM TO BEAM CONNECTIONS MAY BE SNUG TIGHT UNLESS SPECIFICALLY CALLED (SC) ON PLAN.

TABLE C 1

	⅜" DIA A325N BOLTS W/ ⅜"THICK ANGLES (A36)					
	CASE C2A-WELD TO BEAM			CASE C2B-WELD TO SUPPORT		
# OF BOLTS	MAX REACTION (KIPS)	MIN WEB THICK A36 (INCH)	MIN WEB THICK FY 50 (INCH)	MAX REACTION (KIPS)	MIN WEB THICK A36 (INCH)	MIN WEB THICK FY 50 (INCH)
2	45	.39	.28	18	.21	.19
3	72	.42	.30	40	.33	.29
4	98	.44	.32	66	.43	.38
5	124	.46	.33	95	.50	.44
6	150	.48	.35	125	.55	.49
7	176	.50	.36	156	.60	.53
8	202	.51	.37	186	.63	.56
9	228	.53	.38	216	.65	.58
10	254	.54	.39	246	.67	.60
11	280	.55	.39	276	.69	.62
12	306	.55	.40	305	.70	.63

	1" DIA A490N BOLTS W/ ½"THICK ANGLES (A36)					
	CASE C2A-WELD TO BEAM			CASE C2B-WELD TO SUPPORT		
# OF BOLTS	MAX REACTION (KIPS)	MIN WEB THICK A36 (INCH)	MIN WEB THICK FY 50 (INCH)	MAX REACTION (KIPS)	MIN WEB THICK A36 (INCH)	MIN WEB THICK FY 50 (INCH)
2	56	.49	.35	21	.26	.24
3	88	.52	.38	48	.42	.37
4	121	.55	.40	80	.54	.48
5	154	.58	.42	114	.64	.57
6	186	.60	.44	150	.71	.63
7	219	.63	.45	186	.76	.68
8	251	.64	.46	223	.80	.72
9	284	.66	.48	259	.84	.75
10	316	.68	.49	295	.86	.77
11	349	.69	.50	330	.88	.79
12	382	.70	.50	366	.90	.80

TYPICAL STEEL CONNECTION, TYPE C2 | 7

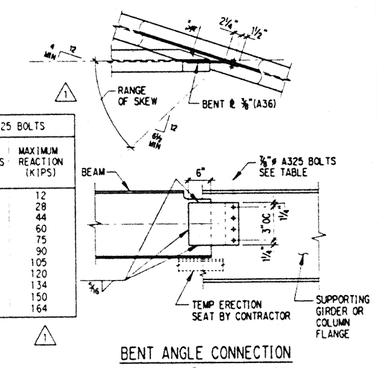


BEAM TO BEAM MOMENT CONNECTION

NOTES:
1. FOR BOLT, SLOTTED HOLE, GUSSET PLATE THICKNESS, AND WELD, SEE "GENERAL NOTES FOR STEEL CONNECTIONS."
2. FOR PLATE WIDTH AND WELD FOR SKEWED BEAMS, SEE SECTION A, TYPE C1, STEEL CONNECTION.

D434A

TYPICAL STEEL CONNECTION, TYPE C3 | 8



⅜"# A325 BOLTS	
# OF BOLTS	MAXIMUM REACTION (KIPS)
2	12
3	28
4	44
5	60
6	75
7	90
8	105
9	120
10	134
11	150
12	164

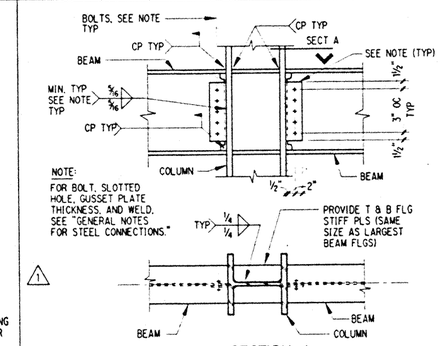
1

NOTE:
1. FOR MINIMUM NUMBER OF BOLTS, SEE NOTE 2, "GENERAL NOTES FOR STEEL CONNECTIONS".

BENT ANGLE CONNECTION

D434A

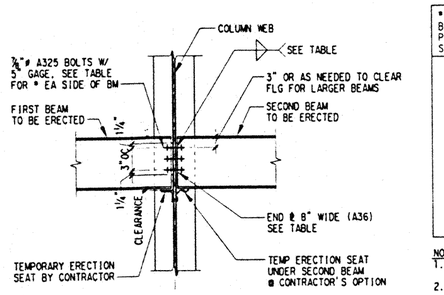
TYPICAL SKEWED BEAM, TYPE C8 | 9



SECTION A

BEAM TO COLUMN FLANGE MOMENT CONNECTION

TYPICAL STEEL CONNECTION, TYPE C4 | 10



BEAM TO COLUMN WEB NON-MOMENT CONNECTION
NON - MOMENT CONNECTION AT FLANGES

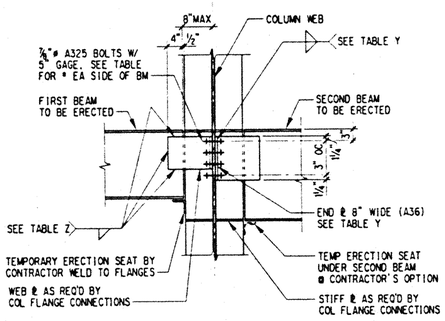
1

TABLE Y

	⅜" DIA A325 BOLTS						
# OF BOLTS PER SIDE	ALLOWABLE REACTION (KIPS)	END PLATE THICK	WELD SIZE	MINIMUM BEAM WEB THICKNESS A36	MINIMUM BEAM WEB THICKNESS A572	MINIMUM COLUMN WEB THICKNESS A36	MINIMUM COLUMN WEB THICKNESS A572
2	18	¼	3/16	.28	.2	.2	.2
2	45	⅜	3/16	.72	.52	.38	.33
3	29	¼	3/16	.27	.2	.2	.2
3	72	⅜	3/16	.72	.52	.4	.36
4	57	¼	3/16	.41	.29	.24	.22
4	98	⅜	3/16	.72	.52	.41	.36
5	88	5/16	¼	.49	.35	.3	.26
5	124	⅜	3/16	.72	.52	.41	.37
6	119	5/16	¼	.55	.39	.33	.3
6	150	⅜	3/16	.72	.52	.41	.37
7	176	⅜	3/16	.69	.5	.42	.37
8	202	⅜	3/16	.69	.5	.42	.37
9	228	⅜	3/16	.69	.5	.42	.38
10	254	⅜	3/16	.69	.5	.42	.38
11	280	⅜	3/16	.69	.5	.42	.38
12	306	⅜	3/16	.69	.5	.42	.38

NOTES:
1. WHEN BEAM WEB IS LESS THAN VALUE SHOWN REDUCE ALLOWABLE REACTION BY THE RATIO WITH MINIMUM THICKNESS.
2. FOR BEAMS ON ONLY ONE SIDE THE MINIMUM COLUMN WEB THICKNESS IS ONE HALF OF THE VALUE SHOWN. 4
3. WHEN COLUMN WEB IS LESS THAN VALUE SHOWN REDUCE ALLOWABLE REACTION BY THE RATIO WITH THE MINIMUM THICKNESS.
4. MINIMUM NUMBER OF BOLTS PER SIDE SHALL CONFORM TO TABLE A-MINIMUM NO. BOLTS IN GENERAL NOTES FOR STEEL CONNECTIONS.
5. END PLATE SHALL FIT WITHIN BEAM DEPTH.
6. ¼ INCH MAXIMUM SHIMS ALLOWED ON EACH SIDE.
7. PROVIDE MINIMUM BLOCKS OF BEAM FLANGES WHEN WIDER THAN COLUMN.

TYPICAL STEEL CONNECTION, TYPE C6 | 12



BEAM TO COLUMN WEB NON-MOMENT CONNECTION
MOMENT CONNECTION AT FLANGES

1

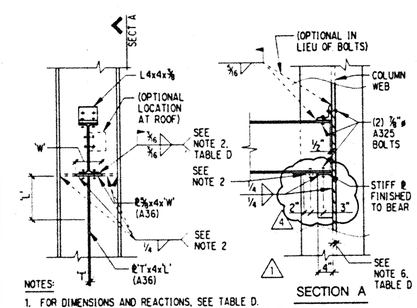
D436

TYPICAL STEEL CONNECTION, TYPE C7 | 14

TABLE Z

# OF BOLTS PER SIDE	MAXIMUM REACTION (KIPS)	WELD WEB PLATE TO BM WEB	WEB PLATE THICKNESS (INCHES)
2	7	¼	½
3	17	¼	½
4	31	¼	½
5	48	¼	½
6	105	5/16	⅜
7	145	5/16	⅜
8	190	5/16	⅜
9	228	5/16	⅜
10	254	5/16	⅜

NOTE: 1. SEE CONNECTION C6 FOR END PLATE SIZE. WELD AND MINIMUM BEAM WEB THICKNESS.



NOTES:
1. FOR DIMENSIONS AND REACTIONS, SEE TABLE D.
2. FOR COLUMNS WEIGHING MORE THAN 120 LB/FT AT SLAB EDGES, EXTEND TOP & CP TO ACCOMMODATE (2) ADDITIONAL ⅜"# BOLTS AS SHOWN IN SECTION A. EXTEND TOP & TO COLUMN FLANGES AND WELD TOP & TO COLUMN FLANGE AS SHOWN.

SECTION A

SEATED BEAM TO COLUMN WEB NON-MOMENT CONNECTION
ALTERNATE TO TYPE C6 CONNECTION

D451

TYPICAL STEEL CONNECTION, TYPE C20 | 15

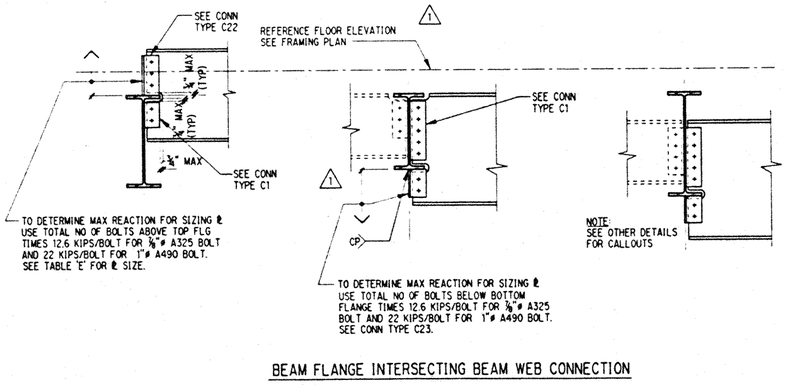
TABLE D MINIMUM BEAM SIZE FOR Fy36 & Fy50 BEAMS

Reaction (kips)	"L" (in)	"T" min. (in)	"W" min. (in)	W36x 36	W36x 50	W33x 36	W33x 50	W30x 36	W30x 50	W27x 36	W27x 50	W24x 36	W24x 50	W21x 36	W21x 50	W18x 36	W18x 50	W16x 36	W16x 50	W14x 36	W14x 50	W12x 36	W12x 50	W10x 36	W10x 50
15	6	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	26	26	22	22	14	14	12	12
23	6	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	31	26	26	26	30	19	17	15
30	7	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	35	35	40	31	38	30	35	35	30	26
38	8	1/2	6	135	135	118	118	90	90	84	84	55	55	44	44	46	40	45	40	48	38	45	35	39	30
46	9	1/2	6	135	135	118	118	90	90	84	84	55	55	50	50	55	46	50	45	53	48	58	45	54	39
55	10	5/8	6	135	135	118	118	90	90	84	84	76	55	68	57	60	55	77	50	68	53	65	58	60	54
64	11	3/4	6	135	135	118	118	90	90	94	84	84	76	73	68	86	60	77	77	74	68	72	65	68	60
73	12	7/8	6	135	135	118	118	99	90	94	84	94	84	83	73	86	86	89	77	99	74	--	72	77	60
82	13	7/8	7	135	135	118	118	108	99	102	94	117	84	111	83	97	86	89	77	99	99	--	79	88	68
92	14	1	7	135	135	130	118	116	99	114	102	117	117	111	83	97	97	--	89	--	99	--	79	88	68
101	15	1-1/8	8	135	135	141	118	124	108	129	114	131	117	122	111	--	97	--	89	--	109	--	--	--	77
111	16	1-1/4	8	150	135	152	130	132	116	--	114	--	117	--	111	--	97	--	--	--	109	--	--	--	77
121	17	1-3/8	9	160	150	152	141	--	124	--	129	--	131	--	122	--	106	--	--	--	120	--	--	--	88
131	18	1-1/2	9	170	150	169	152	--	132	--	129	--	131	--	122	--	--	--	--	--	--	--	--	--	88
140	19	1-1/2	10	182	160	--	152	--	148	--	161	--	146	--	132	--	--	--	--	--	--	--	--	--	100
150	20	1-5/8	10	182	170	--	169	--	148	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	100
160	21	1-3/4	10	199	170	--	169	--	191	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
169	22	1-7/8	11	210	182	--	201	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--
179	23	2	11-1/4	210	182	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--

NOTES:
1. SEE PLANS FOR BEAM REACTIONS.
2. WHEN FLANGE IS LESS THAN 6", USE ½"# ERECTION BOLTS AND WELD FLANGE TO SEAT.
3. W MINIMUM IS SHOWN IN THE TABLE, BUT HORIZONTAL PLATE MUST BE WIDE ENOUGH TO ACCOMMODATE BOLT GAGE AND PROPER EDGE DISTANCES. 4
4. T MINIMUM IS SHOWN IN THE TABLE, BUT T MUST BE AT LEAST EQUAL TO: THE BEAM WEB THICKNESS FOR A 36 BEAMS THE BEAM WEB THICKNESS TIMES 1.4 FOR FY 50 BEAMS
5. THE MINIMUM BEAM SIZES FOR GIVEN REACTIONS ARE SHOWN. THESE ARE BASED ON WEB CRIPPLING OF THE BEAM BEARING ON A 4-INCH SEAT (3-1/2 INCHES OF BEARING). FOR BEAMS WITH REACTIONS THAT FALL OUTSIDE THIS TABLE, PROPERLY DETAILED AISC CONNECTIONS MAY BE USED.
6. FOR SEATED CONNECTIONS ON BOTH SIDES OF A COLUMN WEB, THE MINIMUM COLUMN WEB THICKNESS REQUIRED IS AS FOLLOWS: FY = 36 KSI, COLUMNS -½INCH: FY = 50 KSI, COLUMNS -⅜INCH. WHERE THIS CONDITION IS EXCEEDED, CONNECTION MUST BE RESIZED PER AISC WITH SMALLER WELD STRESSES.
7. FOR BUILT-UP BEAMS, CHECK WEB CRIPPLING OF BEAM BEARING ON A 4-INCH SEAT (3-1/2-INCH BEARING). ADD DOUBLERS OR USE PROPERLY DETAILED AISC CONNECTION

D452

STEEL CONNECTION TYPE C20 TABLE AND NOTES | 18



BEAM FLANGE INTERSECTING BEAM WEB CONNECTION

D453

TYPICAL STEEL CONNECTION, TYPE C21 | 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Consultants

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1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
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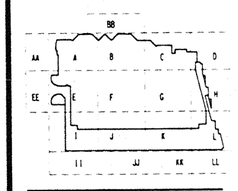
JON D. MAGNUSSON LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is required in Title 16, Chapter 10, Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TYPICAL STEEL DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S1.8

Fri Dec 11 3:43:35 1995
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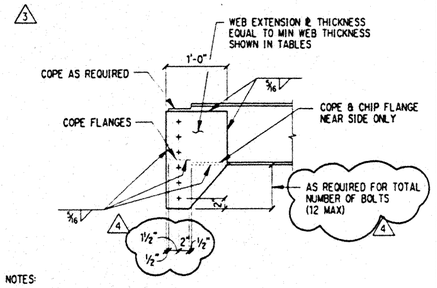
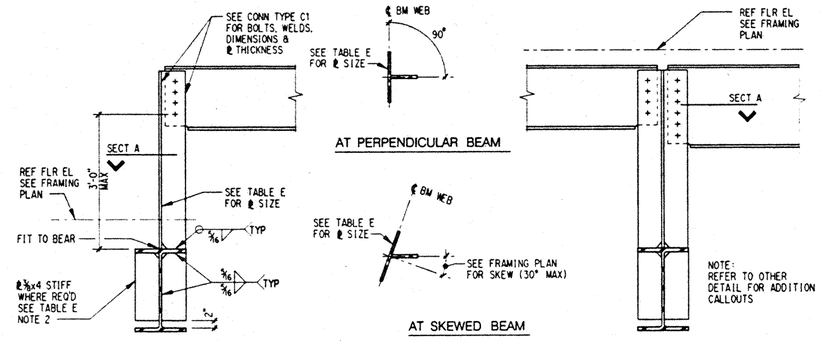


NOTES:

1. DETAIL APPLIES FOR SINGLE PLATE CONNECTIONS.
2. USE THIS DETAIL WHEN REQUIRED NUMBER OF BOLTS DOES NOT FIT WITHIN BEAM DEPTH.

TYPICAL WEB EXTENSION PLATE DETAIL | 1



BEAM TO ELEVATED BEAM CONNECTION

D454

TYPICAL STEEL CONNECTION, TYPE C22 | 3

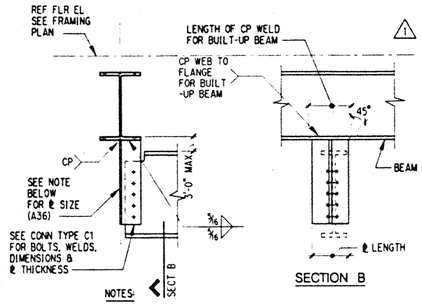
TABLE E			
PLATE SIZE REQ'D (A 36)	MAXIMUM REACTION (KIPS)	MINIMUM WEB THICKNESS FY=36	FY=50
PL 1/2 x 6	54	3/8	1/4
PL 3/4 x 8	106	1/2	3/8
PL 1 x 10	170	5/8	1/2
PL 1-1/4 x 12	230	3/4	9/16

NOTES:

1. WHERE TWO OR MORE BEAMS FRAME INTO PLATE, THE MAXIMUM REACTION IS THE SUM OF THE REACTIONS OF THE BEAMS.
2. WHEN BEAM WEB THICKNESS IS LESS THAN SHOWN IN TABLE, ADD STIFFENERS.
3. AT SIM CONDITION, PROVIDE CONNECTION SIMILAR TO TYPE C1 INTO ℄ SPECIFIED IN TABLE E.

D454A

STEEL CONN TABLES FOR TYPE C21 AND C22 | 4

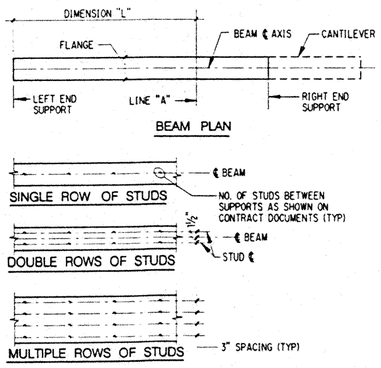


NOTES:

1. PLATE TO BE SAME THICKNESS AS BEAM WEB, BUT NOT LESS THAN ⅜". DETERMINE PLATE LENGTH BY USING A MAXIMUM TENSION IN THE ℄ OF 12 KSI, I.E. ℄ LENGTH = TOTAL REACTION / (℄ THICKNESS X 12 KSI). LENGTH TO BE NOT LESS THAN 6".
2. CONTRACTOR SHALL PERFORM ULTRASONIC TESTING AND INSPECTION OF BEAM BOTTOM FLANGE PER SPECIFICATION FOR WELDED CONNECTIONS.

D455

TYPICAL STEEL CONNECTION, TYPE C23 | 5



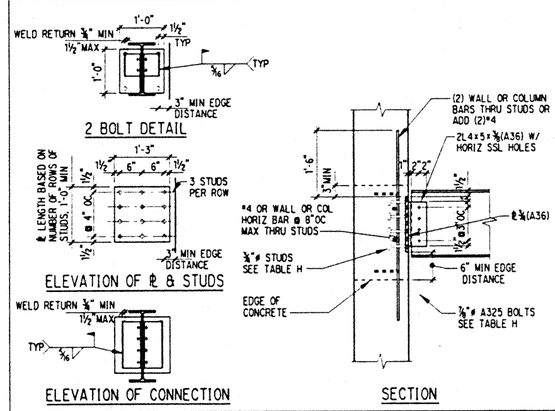
NOTES:

1. DIMENSION "L" EQUALS HALF THE BEAM SPAN U.O.N. ON THE CONTRACT DOCUMENTS. ("L" IS MEASURED FROM THE RIGHT END ON BEAMS MARKED "OPP.")
2. PLACE STUDS BETWEEN SUPPORTS WITH AN EQUAL NUMBER OF STUDS ON EITHER SIDE OF LINE A. ARRANGE SYMMETRICALLY ABOUT THE BEAM CENTERLINE AXIS.
3. PLACE THE STUDS IN THE FOLLOWING SEQUENCE UNTIL THE NUMBER OF STUDS DEFINED ON THE DRAWINGS ARE USED:
   a. DETERMINE HOW MANY ROWS ARE REQUIRED BASED ON A STUD SPACING OF 1'-0" FOR DECK FLUTES PERPENDICULAR TO THE BEAM OR 6" FOR DECK FLUTES PARALLEL TO THE BEAM.
   b. IF 1 ROW OF STUDS IS REQ'D, PLACE STUDS 2'-0" ON CENTER FROM BEAM ENDS TOWARD LINE A. (CENTER STUD IN FLUTE WHERE APPLICABLE).
   c. PLACE REMAINDER OF THE STUDS AT 2'-0" ON CENTER, STARTING 1'-0" FROM BEAM ENDS, TOWARD LINE A: 1'-0" SPACING OF STUDS WILL RESULT.
   d. WHERE DECK FLUTES ARE PARALLEL TO BEAM AND STUDS ARE STILL REMAINING AFTER STEP c. ABOVE, PLACE THE REMAINDER OF THE STUDS IN SUCH A WAY AS TO END UP WITH A MINIMUM OF 6" STUD SPACING STARTING FROM THE ENDS OF THE BEAM.
   e. IF MORE THAN ONE ROW IS REQUIRED, PLACE ALL FULL ROWS AT THE MINIMUM (6" OR 12") SPACING. PLACE THE LAST ROW, OR ROWS, IN ACCORDANCE WITH STEPS b, c, AND d, NOTING THE REQUIREMENT TO ARRANGE STUDS SYMMETRICALLY ABOUT BEAM CENTERLINE AXIS.
4. STUDS ON CANTILEVERING SEGMENTS SHALL BE PLACED ON BEAM CENTERLINE AXIS AT 2'-0" ON CENTER AND ARE NOT INCLUDED IN THE NUMBER OF STUDS GIVEN ON THE DRAWINGS.
5. WHEN WWF IS USED AS SLAB REINFORCING, ADD THE FOLLOWING REINFORCING PERPENDICULAR TO THE BEAMS:

   1 OR 2 STUDS/FT - ADD NONE
   3 STUDS/FT - ADD #4 x 5'-0" @ 12"
   4 STUDS/FT - ADD #4 x 5'-0" @ 10"

D461

SHEAR STUD PLACEMENT | 7



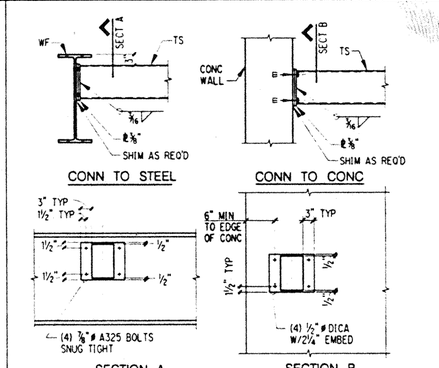
D456A

TABLE H				
REACTION (KIPS)	#BOLTS	#STUDS	LENGTH OF STUD (INCH)	MIN WALL THICKNESS (INCH)
18	2	4	5	8
40	3	6	5	8
66	4	12	6	10
95	5	15	6	10
125	6	18	6	12
153	7	24	8	12
180	8	27	8	14
204	9	30	8	14
230	10	33	8	16

NOTES:

1. THIS CONNECTION IS TO BE USED WHERE STEEL BEAMS ARE SUPPORTED ON CONCRETE FRAMING.
2. SEE CONN C2 FOR WEB DOUBLER REQ'MTS IF BEAM IS COPED.
3. WHERE WALLS ARE THINNER THAN "MIN WALL THICKNESS" SEE DETAILS FOR ADDED REINFORCING.

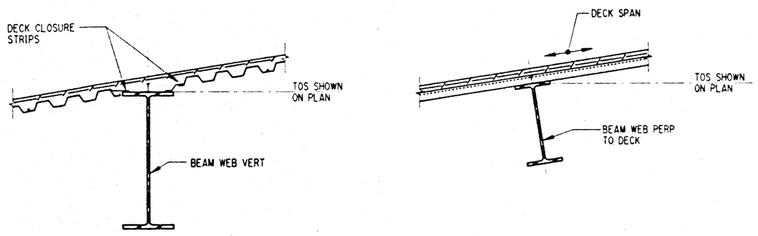
TYPICAL STEEL CONNECTION, TYPE C24 | 9



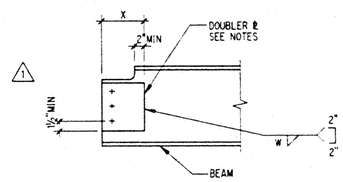
ELEVATOR SPREADER BEAM CONNECTION

D457A

TYPICAL STEEL CONNECTION TYPE C25 | 10



TYPICAL DETAILS AT SLOPING SLABS | 12



NOTES:

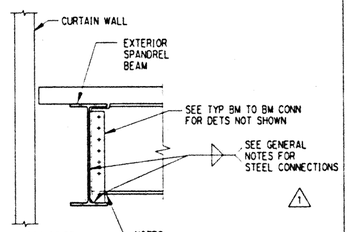
1. USE FOLLOWING FORMULA TO DETERMINE DOUBLER PLATE THICKNESS: MIN. WEB THICKNESS PER CONN TABLES B OR C - BEAM WEB THICKNESS + COEFFICIENT

FOR X	COEFFICIENT
≤ 5	⅛"
≤ 7	¼"
≤ 10½	⅜"

MINIMUM PLATE THICKNESS SHALL BE 1/4". MIN. WIDTH 5"

2. PLATE SHALL BE SAME GRADE STEEL AS BEAM.
3. WELD SIZE 'W' SHALL BE 1/16 LESS THAN PLATE THICKNESS.

TYPICAL WEB DOUBLER DETAIL | 13

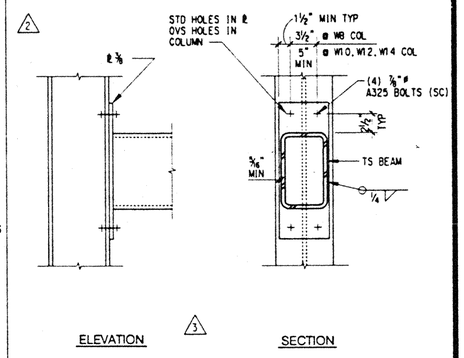


NOTES:

1. SHEAR TAB SHALL EXTEND TO THE BOTTOM FLANGE OF ALL EXTERIOR SPANDREL BEAMS.

D457B

TYP SPANDREL BM CONN TYPE C26 | 14



TS BEAM TO COLUMN FLANGE

TYPICAL STEEL CONNECTION TYPE C29 | 15

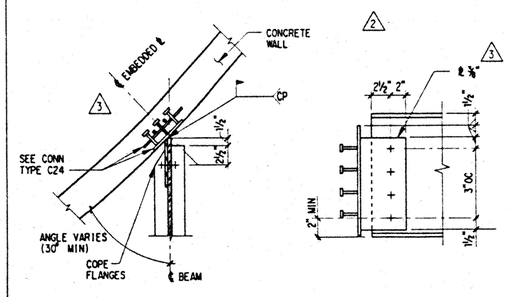
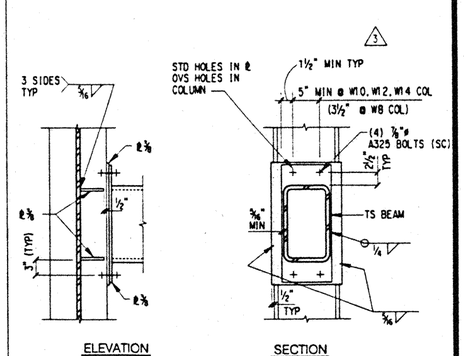


TABLE				
REACTION (KIPS)	# OF BOLTS	# OF STUDS	LENGTH OF STUD (INCH)	MIN WALL THICKNESS (INCH)
8	2	4	5	8
17	3	6	5	8
30	4	12	5	8
42	5	15	5	8

NOTES:

1. THIS CONNECTION IS TO BE USED WHERE STEEL BEAMS ARE SKEWED TO CONCRETE FRAMING SUPPORTING THE BEAM, AND CONNECTION TYPE C24 WILL NOT WORK.
2. SEE CONNECTION TYPE C24 FOR EMBEDDED PLATE AND ADDED REINFORCEMENT IN CONCRETE AT EMBEDDED PLATE.
3. SEE TABLE B, "GENERAL NOTES FOR STEEL CONNECTIONS" FOR MINIMUM BEAM WEB THICKNESS.

TYPICAL SKEWED STEEL CONNECTION, TYPE C28 | 19



TS BEAM TO COLUMN WEB

TYPICAL STEEL CONNECTION TYPE C30 | 20

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Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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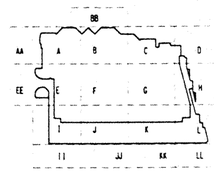
LICENSED PROFESSIONAL ENGINEER NO. 11773 HAWAII, U.S.A.

This work was prepared by me or under my supervision, Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Section 1.3.14 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

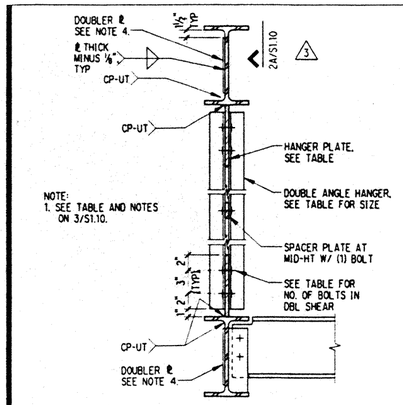
Sheet Title

TYPICAL STEEL DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

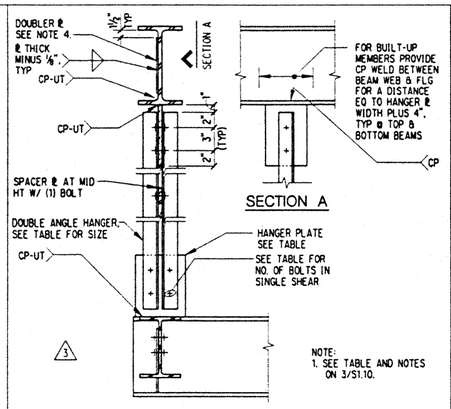
Sheet No.

S1.9



NOTE:
1. SEE TABLE AND NOTES ON 3/S1.10.

TYP STEEL CONNECTION, TYPE C31 | 1



NOTE:
1. SEE TABLE AND NOTES ON 3/S1.10.

TYP STEEL CONNECTION, TYPE C32 | 2

MAXIMUM REACTION (kips)	NUMBER OF BOLTS IN SINGLE SHEAR	NUMBER OF BOLTS IN DOUBLE SHEAR	DOUBLE ANGLE SIZE	MIN PLATE THICKNESS (INCHES)	MIN PLATE WIDTH (INCHES)
36	4	2	(2) - L 4 x 4 x 1/4	5/16	10
54	6	3	(2) - L 4 x 4 x 5/16	5/16	12
72	8	4	(2) - L 4 x 4 x 3/8	5/16	14
90	10	5	(2) - L 4 x 4 x 3/8	1/2	14
108	12	6	(2) - L 4 x 4 x 1/2	1/2	14
126	14	7	(2) - L 4 x 4 x 5/8	5/8	14
144	16	8	(2) - L 4 x 4 x 5/8	5/8	14
162	18	9	(2) - L 4 x 4 x 3/4	3/4	14

3

NOTES:

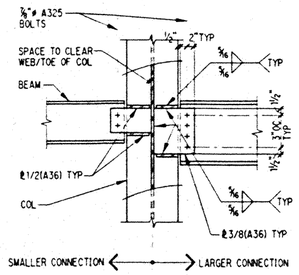
1. HANGER LOAD IS EQUAL TO THE SUM OF BEAM END REACTIONS OF ALL THE BEAMS FRAMING INTO THE SUPPORTED JOINT, UNO. 4
2. HANGER ANGLES SHALL BE A572 GRADE 50 STEEL.
3. HANGER BOLTS SHALL BE 7/8 INCH DIA, A325. ANGLES SHALL HAVE STANDARD HOLES. HANGER CONNECTION PLATES SHALL HAVE OVERSIZED HOLES. 3 USE STANDARD AISC ANGLE GAGE.
4. WEB DOUBLER PLATES ARE REQUIRED WHERE THE BEAM WEB IS THINNER THAN THE HANGER PLATE. THE THICKNESS OF THE BEAM WEB AND DOUBLER PLATE SHALL BE EQUAL TO OR GREATER THAN THE THICKNESS OF THE HANGER PLATE. THE WEB DOUBLER PLATE SHALL BE THE SAME WIDTH AS THE HANGER PLATE. 3

STEEL CONN. TYPE C31 & C32 TABLE | 3

BEAM WEB PENETRATION SCHEDULE

MARK	DIMENSIONS ℄ #1 T1 x B1 x L1	W1	℄ #2 T2 x B2 x L2	W2	℄ #3 T3 x B3 x L3	W3	DOUBLER ℄ #4 L4 x T4	W4	REMARKS
A	--		--		--		--		
B	--		--		3/8"x6"x1'-0"	1/4"	--		
C	3/8"x4"x1'-3"	1/4"	--		--		--		
D					3/8"x7"x1'-4"	3/16"	1		
E					3/8"x1'-0"x2'-3"	3/16"	2		
F					3/8"x10"x2'-0"	3/16"	5		
G	1/2"x3"x1'-2"	3/16"	1/2"x3"x1'-2"	3/16"					
H	3/8"x6"x1'-2"	3/16"	3/8"x6"x1'-2"	3/16"					
J			1/2"x3"x0'-8"	1/4"					
K									
L	3/8"x3"x0'-8"	3/16"	(2)3/8"x3"x0'-9"	3/16"	3/8"x5"x1'-0"	3/16"			
M					1/2"x5"x1'-0"	1/4"			

2



NOTES:
1. FOR BOLT REQUIREMENTS, SEE TABLE J.
2. THIS CONNECTION MUST BE USED WHERE SPECIFIED.
3. BEAM FLANGES SHALL NOT BE COPED.

BEAM TO COL WEB NON-MOMENT CONNECTION
(NON-MOMENT CONNECTION AT FLANGES)

D436

TYP STEEL CONNECTION, TYPE C34 | 6

TABLE J

NUMBER OF BOLTS IN SMALLER CONNECTION	SERVICE LOAD OF SMALLER CONNECTION (KIPS)	SERVICE LOAD CAPACITY OF LARGER CONNECTION (K I P S) — NUMBER OF BOLTS IN LARGER CONNECTION: 2	3	4	5	6	7	8	9	10	11	12
0*		4	8	15	23	32	43	54	67	80	93	106
2	25	25	26	27	35	45	55	66	79	91	104	116
3	37		37	40	47	54	64	74	85	97	110	121
4	50			50	58	63	72	81	92	102	114	126
5	63				63	71	78	87	98	108	119	130
6	75					75	83	93	103	113	124	135
7	88						88	97	108	118	127	138
8	100							100	111	121	131	142
9	113								113	124	135	144
10	126									126	137	145
11	138										138	150
12	151											151

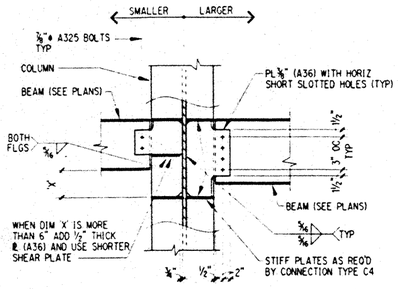
NOTES:

1. * ONE-SIDED CONNECTION.
2. THE CAPACITY OF THE LARGER CONNECTION IS DEPENDENT ON THE NUMBER OF BOLTS IN THE SMALLER CONNECTION. TO FIND THE NUMBER OF BOLTS IN THE LARGER CONNECTION, USE THE LOAD VALUES IN THE SAME ROW AS THE PREVIOUSLY-DETERMINED SMALLER CONNECTION.

D437

STEEL CONNECTION TYPE C33 AND C34 TABLE | 8

BEAM WEB PENETRATION SCHEDULE | 10

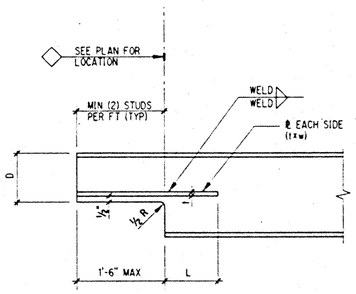


NOTES:
1. FOR BOLT REQUIREMENTS, SEE TABLE J.
2. BEAM FLANGES SHALL NOT BE COPED.

(BEAM TO COLUMN WEB NON-MOMENT CONNECTION)
(MOMENT CONNECTION AT FLANGES)

D435B

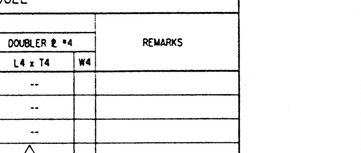
TYP STEEL CONNECTION, TYPE C33 | 11



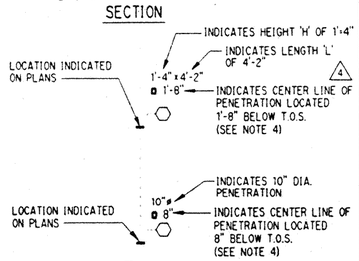
BEAM HAUNCH SCHEDULE

MARK	D	℄ (t x w)	L	WELD	REMARKS
A					
B					
C					
D					

TYPICAL HAUNCHED BEAM TYPE I | 14



SECTION



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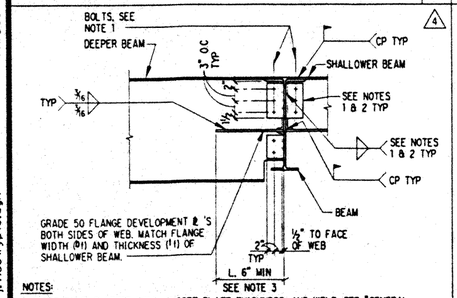
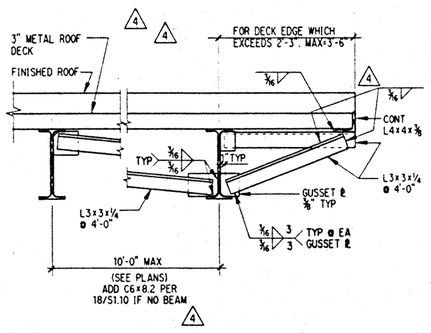


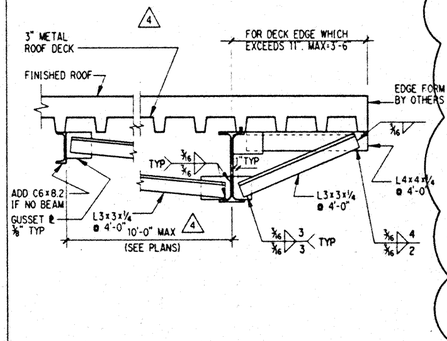
NOTES:
1. FOR BOLT, SLOTTED HOLE, GUSSET PLATE THICKNESS, AND WELD, SEE "GENERAL NOTES FOR STEEL CONNECTIONS".
2. FOR PLATE WIDTH AND WELD FOR SKEWED BEAMS, SEE SECTION A, TYPE C1 STEEL CONNECTION.
3. FLANGE DEVELOPMENT PLATE LENGTH, L=(3xbf x tf) + 3".

BEAM TO BEAM MOMENT CONNECTION
BEAMS OF DIFFERENT DEPTH

TYP STEEL CONNECTION, TYPE C3A | 16



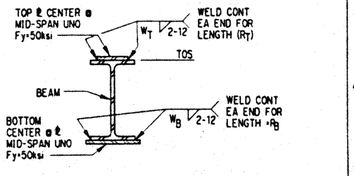
ROOF DECK EDGE @ DECK SPAN PERPENDICULAR TO EDGE BM | 17



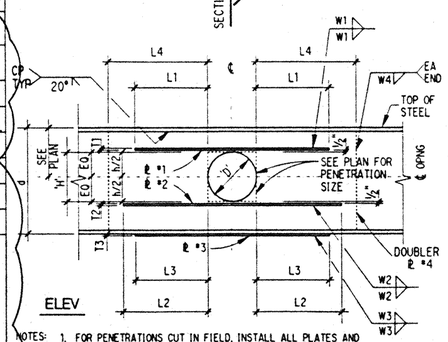
ROOF DECK EDGE @ DECK SPAN PARALLEL TO EDGE BM | 18

BEAM COVER PLATE SCHEDULE

MARK	BOTTOM PLATE	W_B	R_B	TOP PLATE	W_T	R_T	REMARKS
CP1	℄ 1/2x4 1/2x2'7"-0"	3/16"	1'-0"	℄ 1/2x4 1/2x2'7"-0"	3/16"	1'-0"	
CP2	℄ 3/8x10 x7'-8"	3/16"	1'-8"				CENTER AT TRANSFERED COL
CP3	℄ 1/4x10 x17'-4"	1/4"	1'-8"				



TYPICAL BEAM COVER PLATE | 19



ELEV

NOTES:
1. FOR PENETRATIONS CUT IN FIELD, INSTALL ALL PLATES AND COMPLETE ALL WELDING PRIOR TO CUTTING PENETRATION.
2. ALL PLATE Fy=50Ksi UNO
3. EXTEND DOUBLER PLATE TO END OF BEAM IF CENTER LINE OF OPENING IS WITHIN 3'-0" FROM END OF BEAM.
4. PENETRATIONS SHOWN ON PLANS WITHOUT DIMENSION FROM TOP OF STEEL SHALL BE LOCATED @ d/2.

D493A

TYPICAL BEAM PENETRATION DETAIL | 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

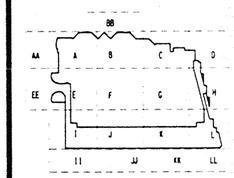
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11773 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Section 1.2.24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TYPICAL STEEL DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

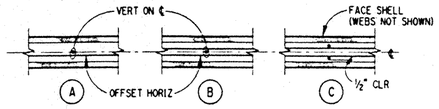
S1.10

Fri Dec 1 13:48:28 1995 jxhccktyp10.dgn 1995.plt

CMU REINFORCING SCHEDULE								
NOMINAL WALL THICKNESS	BOND BEAM REINF		VERICAL REINFORCING			BAR LOCATION TYPE	MAXIMUM WALL HEIGHT (FT)	REMARKS
	NUMBER & SIZE	LAP LENGTH	NUMBER & SIZE	SPACING	LAP LENGTH			
10"	1 #5	30"	1 #5	16"	30"	A	21'	EXTERIOR

NOTES:

1. BOND BEAM REINFORCEMENT ABOVE AND BELOW OPENINGS SHALL BE CONTINUOUS (I.E., UNSPLICED) AND EXTENDED 2 FEET MINIMUM BEYOND OPENING.
2. WHERE BOND BEAM DOES NOT ALIGN WITH TOP OR BOTTOM OF OPENINGS, ADD A BOND BEAM COURSE EXTENDING 2 FEET BEYOND THE OPENING.
3. LOCATE SCHEDULED BARS FROM TABLE EACH SIDE OF EACH OPENING, EACH CORNER AND WALL ENDS.
4. BOND BEAM BARS MAY BE USED TO ARRIVE AT TOTAL STEEL AREA REQUIRED BY REINFORCING SCHEDULE.
5. GROUT ALL CELLS THAT CONTAIN REINFORCEMENT OR EMBEDDED ITEMS.



BAR LOCATION TYPES

TYPICAL CMU LOAD BEARING WALLS REINFORCING SCHEDULE & NOTES | 2

BLOCK LINTEL SCHEDULE				
THICK	CLEAR SPAN	DEPTH (IN)	HOR. REINF.	STIRRUPS
6"	8'-16'	24"	2-#4 TOP 2-#4 BOT	#3 J @16
8"	8'-16'	24"	2-#4 TOP 2-#4 BOT	#3 J @16
8"	16'-22'	40"	2-#6 TOP 2-#6 BOT	#3 J @16

3 2 1

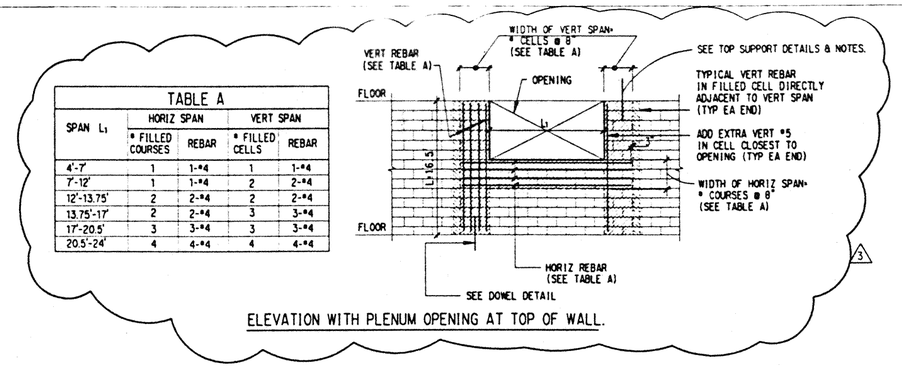
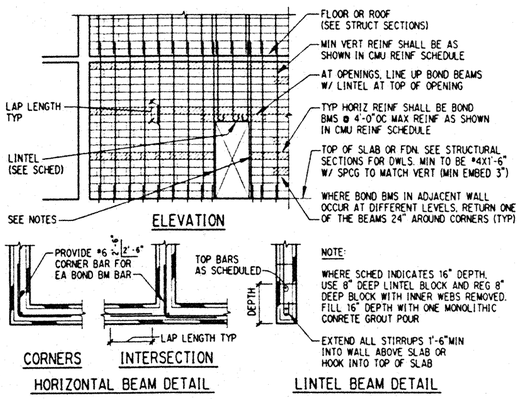
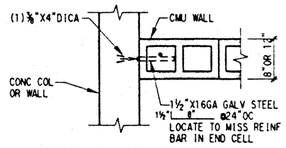


TABLE A				
SPAN L_1	HORIZ SPAN		VERT SPAN	
	# FILLED COURSES	REBAR	# FILLED CELLS	REBAR
4'-7'	1	1-#4	1	1-#4
7'-12'	1	1-#4	2	2-#4
12'-13.75'	2	2-#4	2	2-#4
13.75'-17'	2	2-#4	3	3-#4
17'-20.5'	3	3-#4	3	3-#4
20.5'-24'	4	4-#4	4	4-#4

3

ELEVATION WITH PLENUM OPENING AT TOP OF WALL.

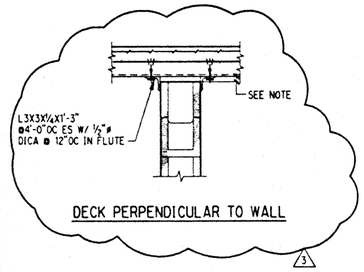


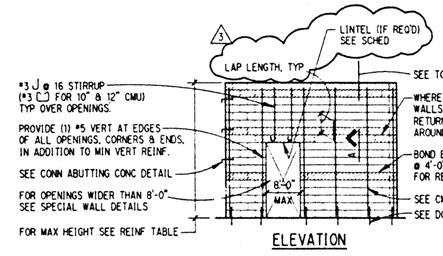


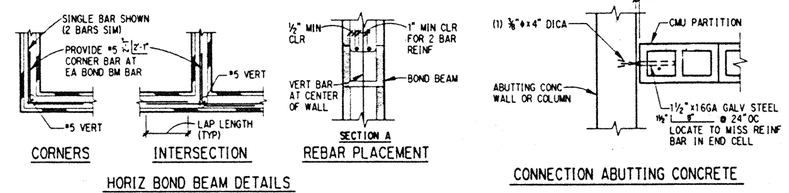
CONN TO ABUTTING CONCRETE

BLOCK LINTEL SCHEDULE				
THICK	CLEAR SPAN	DEPTH (IN)	HOR. REINF.	STIRRUPS
8"	4'-0" OR LESS	8	(2) #4 BOT -	#3 J @16
	4'-0" TO 6'-4"	16	(2) #5 BOT (1) #4 TOP	#3 J @8
	6'-4" TO 8'-0"	16	(2) #6 BOT (1) #4 TOP	#3 J @8
10" OR 12"	4'-0" OR LESS	8	(2) #5 BOT	#3 ▢ @16
	4'-0" TO 6'-4"	16	(2) #6 BOT (1) #4 TOP	#3 ▢ @8
	6'-4" TO 8'-0"	16	(2) #7 BOT (1) #5 TOP	#3 ▢ @8

TYPICAL CMU LOAD BEARING WALL DETAILS | 7







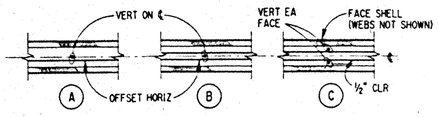
HORIZ BOND BEAM DETAILS

CONNECTION ABUTTING CONCRETE

CMU REINFORCING SCHEDULE								
NOMINAL WALL THICKNESS	BOND BEAM REINF		VERICAL REINFORCING			BAR LOCATION TYPE	MAXIMUM WALL HEIGHT (FT)	REMARKS
	NUMBER & SIZE	LAP LENGTH	NUMBER & SIZE	SPACING	LAP LENGTH			
6"	1-#5	30"	1-#5	48"	24"	A	15'	INT
6"	1-#5	30"	1-#5	48"	30"	A	9'	EXT
6"	1-#5	30"	1-#4	8"	24"	A	12.5'	EXT
8"	2-#5	39"	1-#5	48"	30"	A	20'	INT
8"	1-#5	30"	1-#5	24"	30"	A	15'	EXT
8"	1-#5	30"	1-#4	8"	24"	A	16.5'	EXT/INT

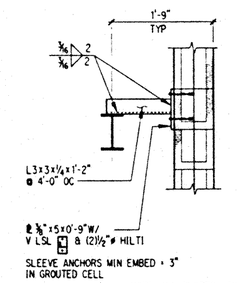
NOTES:

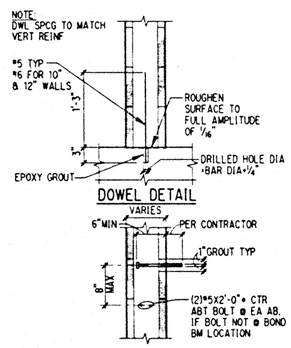
1. BOND BEAM REINFORCEMENT ABOVE AND BELOW OPENINGS SHALL BE CONTINUOUS (I.E., UNSPLICED) AND EXTENDED 2 FEET MINIMUM BEYOND OPENING.
2. WHERE BOND BEAM DOES NOT ALIGN WITH TOP OR BOTTOM OF OPENINGS, ADD A BOND BEAM COURSE EXTENDING 2 FEET BEYOND THE OPENING. 3 WHERE OPENING IS WIDER THAN 8'-0" SEE BLOCK LINTEL SCHEDULE, 20/S1.12.
3. BOND BEAM BARS MAY BE USED TO ARRIVE AT TOTAL STEEL AREA REQUIRED BY REINFORCING SCHEDULE.
4. GROUT ALL CELLS THAT CONTAIN REINFORCEMENT OR EMBEDED ITEMS.
5. PACK TOP COURSE CELLS SOLID WITH GROUT, TYPICAL.
6. PROVIDE 3/3 1" INCH MINIMUM, 1½ INCH MAXIMUM CLEAR SPACE BETWEEN TOP WALL AND BOTTOM OF PLATE, DECK OR FIREPROOFING ON BEAMS.
7. FOR DICA'S IN BOTTOM OF SLAB, EMBED SO THAT THEY EXTEND A MINIMUM OF 1 INCH ABOVE THE UPPER PART OF THE DECK FLUTES.
8. PROVIDE SPECIAL INSPECTION FOR DICA AT ANCHORS FOR WALLS 16'-0" AND HIGHER.
9. LOCATE BRACE FOR BEAM BOTTOM FLANGE AT EACH CMU TOP SUPPORT AT 4'-0" ON CENTER, MAXIMUM.
10. 3 MAXIMUM WALL HEIGHT IS DEFINED AT DISTANCE FROM TOP OF WALL TO BOT OF WALL OR FROM TOP/BOT OF WALL TO INTERMEDIATE SUPPORT.

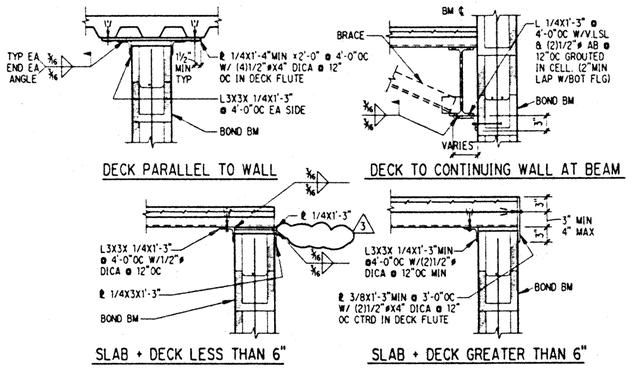


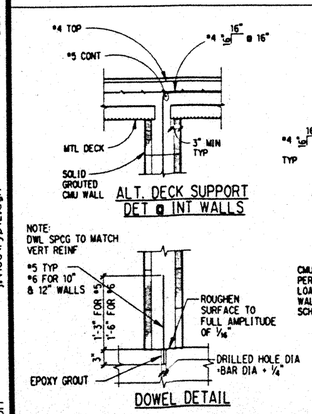
BAR LOCATION TYPES

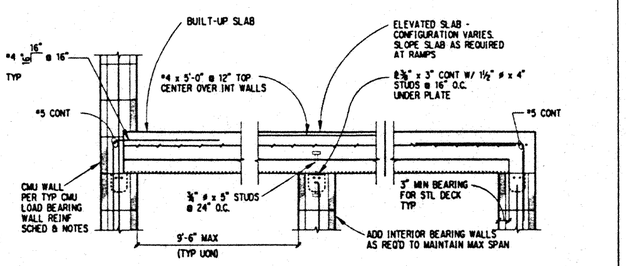
TYPICAL CMU NON-BEARING WALLS REINFORCING SCHEDULE & NOTES | 12



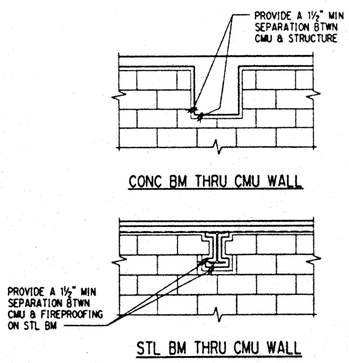


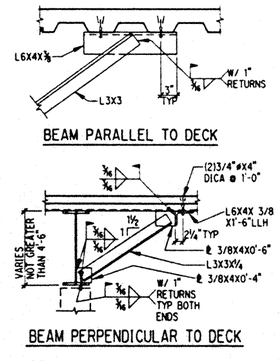


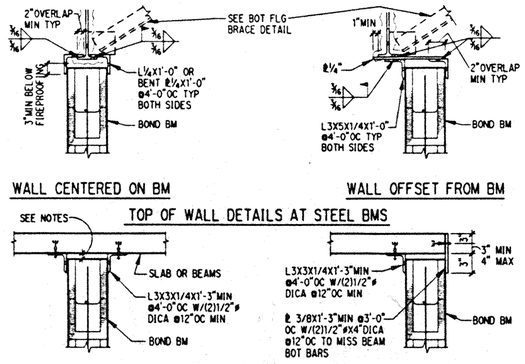




TYPICAL BUILT-UP SLAB DETAIL | 17







TYPICAL CMU NON-BEARING WALL DETAILS | 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

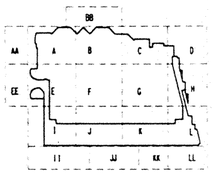
Consultants

Skilling Ward Magnusson Barkshire Inc.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-[illegible] FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 7175 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Expiration date of license in Chapter 16 Section 1.24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
3	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-31-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TYPICAL CMU DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S1.12

Fri Dec 1 19:23:00 1995
jxhccityp12.dgn
DMS plt

CONCRETE COLUMN SCHEDULE

LEVEL	COLUMN MARK	1-F.3 1-F.7	1.5-G.4	1-E.9 1-G.1	3-G 5-F 8-G 3-F 6-G 8-F 4-G 6-F 4-F 7-G 5-G 7-F	2.5-B 4.5-B 6.1-C 3.1-B 5.1-B 3.8-B 5.5-C	2.1-C 4.8-C 3.5-C 5.8-D 4.1-C 6.5-D
FUTURE LOADS MEETING			Pu=226k	Pu=4649k Mu=12998k-ft Vu=350k	SEE NOTE 6 △3	△1	
PARKING	SIZE				6'-0"ø		
	TYPE				SP		
	REINF				(30) #14		
	SPLICE						
	REMARKS				f'c=7.5ksi		
MEZZANINE	SIZE	3'-0"x3'-3"	SEE 4/5.1S	8'-0"ø	6'-0"ø		
	TYPE	P	P	SP	SP		
	REINF	(12) #10	(8) #8	(33) #14	(30) #14		
	SPLICE						
	REMARKS				f'c=7.5ksi		
EXHIBIT	SIZE	3'-0"x3'-3"	SEE 4/5.1S	8'-0"ø	6'-0"ø	3'-4"ø	3'-4"ø
	TYPE	P	P	SP	SP	SP	SP
	REINF	(12) #10	(8) #8	(33) #14	(30) #14	(21) #9	(21) #11
	SPLICE					TENSION	TENSION △1
	REMARKS				f'c=7.5ksi	NOTE 5	NOTE 5
FDN	SIZE	3'-0"x3'-3"	SEE 4/5.1S	8'-0"ø	6'-0"ø	3'-4"ø	3'-4"ø
	TYPE	P	P	SP	SP	SP	SP
	REINF	(12) #10	(8) #8	(33) #14	(30) #14	(21) #9	(21) #11
	SPLICE					TENSION	TENSION
	DOWELS	(12) #7	(8) #6	(33) #10	(30) #10	NOTE 5	NOTE 5
	REMARKS				f'c=7.5ksi		
DOWELS							

NOTES:

1. ALL COLUMNS SHALL BE CAST WITH f'C=4.0 KSI CONCRETE, UNLESS NOTED OTHERWISE.
2. SEE TYPICAL DETAILS FOR DETAILS OF TIED (T) COLUMNS, SPIRAL (SP) COLUMNS, OR PILASTER (P) COLUMNS.
3. ALL BARS SHALL BE SPLICED PER TYPICAL SPLICE DETAIL.
4. SEE 1/4.1S FOR DETAILS OF CONC COL'S @ REGISTRATION LOBBY TREES. △1
5. COVER TO MAIN BARS 3½" AWAY FROM TOP REVEAL. SEE 4/2.2S FOR REVEAL AND NOTCH DETAIL. △2
6. FUTURE WORKING LOADS AS FOLLOWS:
   3-F, P=3204k; 3-G, P=3379k; 4-F, P=3200k; 4-G, P=3322k; 5-F, P=2315k; 5-G, P=1695k. △3

Hawai'i Convention Center

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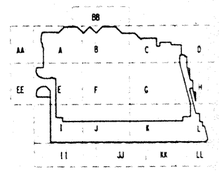
SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 292-1021

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7777S
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Rule 16 Chapter 16 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
5	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
2	8-31-95		
1	8-4-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

CONCRETE COLUMN SCHEDULE

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S1.20

J:\hcc\sched_1.dgn Sat Dec 2 09:30:08 1995 plt

STEEL COLUMN SCHEDULE

5

MARK	2.1-E.25 2.1-E.5 2.1-E.8 2.1-F.1 2.1-F.35 2.1-F.65 2.1-F.95 2.1-G.25 2.1-G.45 2.1-G.7	1-G.8	9.2-K	8.65-K 5-J.7 5.5-J.7 6-J.7	1.2-D.4 1.2-C.6 8.1-D.8 9.7-G.8 11-G.55 12-E.55 12.7-E.45	11-E.85 11-F.35 11-F.55 11-F.9	11-E.95 11-G.15 11-G.4	11-E.6 12.7-E.85 12.7-F.4 12.7-F.95 9.7-H.3	9.5-H.3	9-K 4-K 5.1-C.7	7-K 7.5-K 6.5-K	8-K 4.5-K		9.1-G.8 9.5-G.8		5.5-J 9-J.1	2.25-D.4 8.6-D.8	1.6-C.6 1.6-D.4	5.5-H	8.4-D.8 8.6-E.1 12.3-E.3 12-F.3 12-F.5 12-F.85 12-G.1 12-G.35	12-E.8 12-E.9	12-E.5 12-G.5	2.5-H	8.5-H 9-H 5-J.2	3.75-K	3.5-K	2.5-K	8-J 8.5-J	7-J 7.5-J		2-J.1	3-K	6.5-J	4-J	
LEVEL																																		
FUTURE LOADS																													183k			206k		
ROOF																																		
PENTHOUSE																																		
BALLROOM/ TERRACE																																		
MEETING																									W14x43	W14x82	W14x90	W14x61	W14x82		W14x90	W14x90	W14x61	W14x109	
PARKING																																		
INTERMEDIATE																																		
EXHIBIT	W8x24	W14x43	W14x74	W14x90	W14x43	W14x48	W14x53	W14x61	W14x82	W14x90	W14x99	W14x109		W14x68		W14x145	W14x43	W14x61	W14x211	W14x61	W14x68	W14x90	W14x159	W14x176	W14x68	W14x145	W14x159	W14x159	W14x176		W14x145	W14x211	W14x176	W14x211	
FOUNDATION																																		
BASE ℄ SIZE	¾x7½x0'-9"	¾x9x1'-3"	1⅞x11½x1'-3½"	2x16x1'-3½"	¾x9x1'-3"	¾x9½x1'-3"	¾x9½x1'-3"	¾x11x1'-3"	1½x11½x1'-3½"	2x16x1'-3½"	2x16x1'-4½"	2x16x1'-5½"		1⅛x11½x1'-3½"		2¼x16½x1'-6½"	¾x9x1'-3"	1½x11x1'-3"	2⅝x20 x1'-11"	1⅛x11 x1'-3"	1⅛x11½x1'-3½"	1½x16 x1'-3½"	2¼x18x1'-8"	2¼x19½x1'-9"	1½x11½x1'-3½"	2¼x18x1'-7½"	2¼x19½x1'-9"	2¼x18½x1'-8"	2¼x18½x1'-8½"		2¼x17x1'-6½"	2¼x20x1'-10½"	2¼x18x1'-8"	2¼x20x1'-11"	
BASE ℄ TYPE	1	1	1	1	1	1	1	1	1	1	1	1		1		1	1	1	1	1	1	1	1	1	1	1	1	1	1		1	1	1	1	

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFER TO FLOOR PLANS ON S2.* FOR COLUMN LOCATIONS AND ORIENTATIONS.
2. UNLESS OTHERWISE NOTED, COLUMNS SHALL BE FY = 50 KSI.
3. UNLESS OTHERWISE NOTED, COLUMN BASEPLATES SHALL BE FY = 36 KSI.
4. FOR COLUMN BASEPLATE AND SPLICE DETAILS SEE S1.7.
5. REFER TO BRACED FRAME AND TRUSS ELEVATIONS ON S4.* AND S5.* FOR ADDITIONAL COLUMN CALLOUTS.
6. FUTURE LOADS SHOWN ARE ANTICIPATED LOADS FROM THE FUTURE EXPANSION SHOWN ON S9.1 THROUGH S9.3. FUTURE LOADS ARE ASSUMED TO BE APPLIED AT THE BALLROOM / TERRACE LEVEL, UNO. COLUMN TIERS ABOVE THE BALLROOM / TERRACE LEVEL HAVE NOT BEEN DESIGNED FOR THE FUTURE LOADS.
7. ALL COL SPLICES SHALL BE TYPE 1 UNO ON SCHEDULE

STEEL COLUMN SCHEDULE

15

MARK	3.5-J	4.5-J	2.5-J	3-J	0.7-C.6 0.7-D.4 1.8-C.6	0.7-B.8 2.25-C.5	2-D.4	1-E.25		1.9-B.8	0.7-E	1.35-E.65	1.4-B.8	1.35-E.25	8.1-D.3	6.6-D.2 7.9-D.3	7.5-D.3	7.3-C.8	6.7-C.8	7-C.8	2-K	2.2-E	2.5-K.4	6-J	0.1-F.75	1-E.65	2.5-D.4	4.7-E	7-G.75	3-H	2.7-E	5.15-E	6-H.9 6.2-1.8		
LEVEL																																			
FUTURE LOADS	315k	267k	261k	358k	99k	107k	173k	120k		146k	23k	255k	287k	164k												99k	153k @ MEETING LEVEL	317k			143k				
ROOF																																			
PENTHOUSE																																			
BALLROOM/ TERRACE					W14x43	W14x43	W14x61	W14x43		W14x43	W14x43	W14x43	W14x43	W14x43							W14x43					W14x43			W14x90	W14x43	W14x43				
MEETING	W14x132	W14x99	W14x120	W14x145	W14x61	W14x61	W14x82	W14x74		W14x68	W14x43	W14x90	W14x99	W14x90	W14x43	W14x43	W14x61	W14x48	W14x61	W14x61	W14x61				W14x68	W14x90		W14x90			W14x82			W14x43	
PARKING																											W14x90								
INTERMEDIATE																																			
EXHIBIT	W14x233	W14x211	W14x257	W14x283	W14x74	W14x61	W14x82	W14x90		W14x99	W14x43	W14x99	W14x120	W14x132	W14x53	W14x61	W14x90	W14x90	W14x90	W14x99	W14x99	W14x43	W14x61	W14x159	W14x74	W14x90									
FOUNDATION																																			
BASE ℄ SIZE	2¾x21½x2'-0½"	2⅝x20½x1'-11"	2¾x22x2'-1"	3x23x2'-2"	1⅞x11½x1'-4½"	1⅞x11x1'-3"	1⅞x12x1'-5"	2x16x1'-3½"		2x16x1'-5"	¾x9x1'-3"	2x16x1'-5½"	2¼x17x1'-7"	2⅝x17x1'-7½"	¾x9½x1'-3"	⅞x11x1'-3"	1¼x16 x1'-3½"	1⅜x16x1'-3½"	1½x16x1'-3½"	2x16x1'-3½"	2x16x1'-5"	¾x9x1'-3"	¾x11x1'-3"	2½x17½x1'-7"	1x11½x1'-3½"	1⅞x16x1'-3½"	1⅛x16 x1'-3½"	3x16x2'-0½"	1½x16x1'-4½"	1x9x1'-10½"	1½x11½x2'-9"	3x16x2'-0½"		
BASE ℄ TYPE	1	1	1	1	1	1	1	1		1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	3	3	3	3	4 (w/ ⅜" TH STIFFENER)	3	

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON 5/S1.21.

Fri Dec 1 13:53:07 1995

jitnoc\sched2.dgn

pit

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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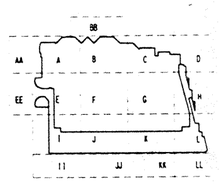
JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Licensees as defined in Title 16 Chapter 18 Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-31-95		
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

STEEL COLUMN SCHEDULE

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S1.21

STEEL COLUMN SCHEDULE

5

MARK	2-K.4	9.5-J.1	8.5-K 9.5-K	8.2-J 5.4-D.75 5.7-E 4.5-J.4 4.9-J.4 5.9-J.3 6.05-J.15 6.1-H.8	9.3-G.9	9-I.1	4.8-J 8.45-G.1 8.45-G.5 8.55-E 8.9-G.5 10-E.9 10-F.5 10-G.1 10-G.5 11.8-E.5 11.8-E.9 11.8-F.5 11.8-G.1 11.8-G.5	5.25-E 5-H.95	5.5-F.5	5.5-E.5 5.5-E.9	4.3-E	8.3-E.5 8.3-H	8.3-G.1 8.3-G.5	8.3-E.9 8.3-F.5	6.3-E	3.7-H 3.7-I	6.4-I 7-I 7.5-I 8-I 8.3-I	7-G.3	7-G.5	3-G.85 3-I 3.5-G.85 3.5-I 4-H 4-I 4-G.85	6.5-H	1-F.3	1-F.75	6.25-J.8	3-K.4 3.5-K.4 4-K.4	3.65-E	2.4-E	2-E.65 2.4-E.2 2.7-E.2 3-E.2 3.3-E.2 3.65-E.2 4-E.2 4.3-E.2 4.6-E.5 4.6-E.7 4.6-E.9 4.7-E.2 4.85-E.4 4.85-E.5	4.85-E.7 4.85-E.9 4.6-F.2 4.6-F.5 4.6-F.7 4.85-F.2 4.85-F.5 4.85-F.7 4.85-F.9 4.85-G.3 4.85-G.6 4.4-H 4.85-H 4.4-I 6.8-I	2-K.3 2.05-K	3.5-H	2.05-K.3	9.7-J.1	9.7-K	5.5-E
LEVEL	4.8-K 5-J.5 5.5-J.35 6-J.35																																						
FUTURE LOADS											193k												153k APPLIED @ TOP OF COL	153k APPLIED @ TOP OF COL			174k	145k										
ROOF																																							
PENTHOUSE																					W14x43																		
BALLROOM/ TERRACE												W14x61	W14x74	W14x82	W14x68	W14x43	W14x43	W14x61	W14x90	W14x43							W14x43	W14x43	W14x43	W14x43	W14x61	W14x53				
MEETING					W14x61	W14x68	W14x43	W14x61	W14x82	W14x90	W14x90															W14x82	W14x82					W14x53				
PARKING				W14x43																			W12x170	W14x90														W14x43
INTERMEDIATE	W14x43	W14x53	W14x61																						W14x53	W14x48								W14x43	W14x43	
EXHIBIT																																							
FOUNDATION																																							
BASE SIZE											2¼x16x2'-6"			1¼x11½x2'-0"				1x11x1'-5"	1x16x1'-3½"				2½x20x1'-11½"	2x16x1'-3½"			1⅜x11½x2'-0½"	1⅜x11½x2'-6"				1⅜x11x2'-6½"		⅜x11x1'-5"	⅜x9x1'-3"	2x9x2'-0"
BASE TYPE	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	1	1	3	3	3	3	3	3	3	3	3	1	SEE 1/2.1S	3

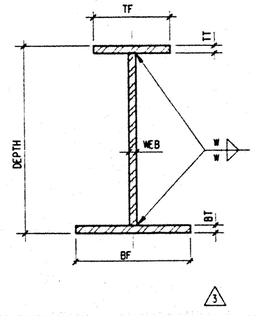
NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON 5/S1.21.

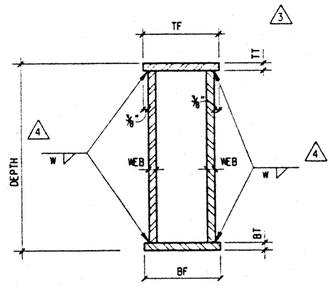
BUILT-UP BEAM SCHEDULE

12

BEAM MARK	DIMENSIONS DEPTH	WEB	TOP FLANGE TF	TT	BOTTOM FLANGE BF	BT	WELD W	BEAM MARK	DIMENSIONS DEPTH	WEB	TOP FLANGE TF	TT	BOTTOM FLANGE BF	BT	WELD W
BU60x119	60"	1/2"	7"	3/8"	7"	3/8"	3/16"	BU60x414	60"	1"	20"	1-1/2"	20"	1-3/4"	5/8"
BU60x150	60"	11/16"	5"	5/16"	5"	5/16"	1/4"	BU60x438	60"	3/4"	10"	1-1/4"	20"	3-3/4"	5/8"
BU60x162	60"	11/16"	7"	1/2"	7"	1/2"	1/4"	BU60x463	60"	1"	20"	2"	20"	2"	5/8"
BU60x172	60"	11/16"	10"	1/2"	10"	1/2"	1/4"	BU60x471	60"	1-1/4"	20"	1-1/2"	20"	1-7/8"	3/4"
BU60x188	60"	11/16"	10"	5/8"	10"	7/8"	5/16"	BU60x478	60"	3/4"	10"	2"	20"	4"	5/8"
BU60x200	60"	3/4"	10"	3/4"	10"	3/4"	5/16"	BU60x510	60"	1-1/4"	20"	2"	20"	2"	3/4"
BU60x212	60"	3/4"	10"	3/4"	11"	1"	3/8"	BU28x354	28"	1"	20"	2"	20"	2"	3/4"
BU60x220	60"	3/4"	12"	7/8"	12"	7/8"	5/16"	BU30x108	30"	5/8"	8"	7/8"	8"	7/8"	5/16"
BU60x226	60"	3/4"	10"	3/4"	20"	3/4"	7/16"	BU30x162	30"	1"	8"	1-1/4"	8"	1-1/4"	1/2"
BU60x242	60"	3/4"	10"	3/4"	16"	1-1/4"	7/16"	BU44x248	44"	3/4"	14"	1-1/2"	14"	1-1/2"	7/16"
BU60x243	60"	5/8"	12"	1-1/2"	12"	1-1/2"	3/8"								
BU60x252	60"	1"	8"	1"	8"	1"	3/8"								
BU60x258	60"	1"	10"	3/4"	10"	1"	3/8"								
BU60x264	60"	5/8"	14"	1-1/2"	14"	1-1/2"	3/8"								
BU60x266	60"	5/8"	17"	1-1/4"	17"	1-1/4"	3/8"								
BU60x273	60"	3/4"	10"	1"	18"	1-1/2"	1/2"								
BU60x275	60"	5/8"	18"	1-1/4"	18"	1-1/4"	3/8"								
BU60x288	60"	1"	10"	1"	15"	1-1/8"	1/2"								
BU60x291	60"	3/4"	17"	1-1/4"	17"	1-1/4"	7/16"								
BU60x298	60"	3/4"	17"	1"	17"	1-5/8"	1/2"								
BU60x300	60"	3/4"	18"	1-1/4"	18"	1-1/4"	7/16"								
BU60x304	60"	3/4"	17"	1-3/8"	17"	1-3/8"	7/16"								
BU60x307	60"	3/4"	18"	1"	18"	1-5/8"	1/2"								
BU60x314	60"	3/4"	18"	1-3/8"	18"	1-3/8"	7/16"								
BU60x319	60"	3/4"	17"	1-1/2"	17"	1-1/2"	7/16"	BUTS 27x132	27"	9/16"	10"	1¼"	10"	1¼"	5/16"
BU60x329	60"	3/4"	18"	1-1/4"	18"	1-3/4"	1/2"								
BU60x351	60"	3/4"	18"	1-1/2"	18"	1-7/8"	1/2"								
BU60x362	60"	1"	15"	1-1/2"	18"	1-1/2"	5/8"								
BU60x396	60"	1-1/4"	10"	1-1/2"	20"	1-1/2"	3/4"								



TYP. BU SECTION 18



TYP. BUTS SECTION 19

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

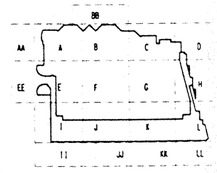
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 7177S — JOHN D. MAGNUSSON — HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising engineer defined in Title 16 Chapter 18 Section 1.3.2-e of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

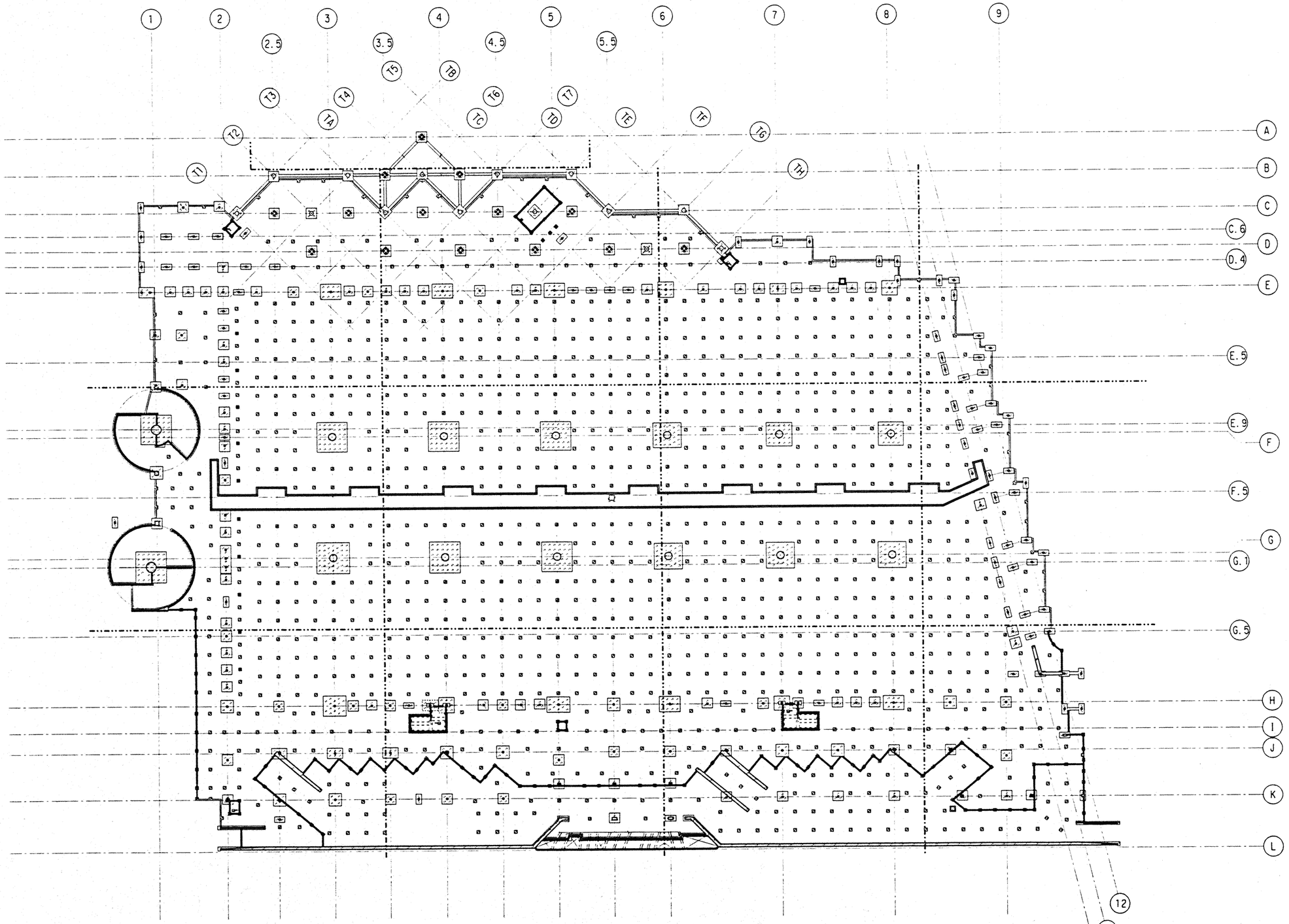
Sheet Title

STEEL COLUMN & BUILT-UP BEAM SCHEDULES

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S1.22

Fri Dec 1 13:55:16 1995 jxhcc\sched.3.dgn



FOUNDATION PLAN

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

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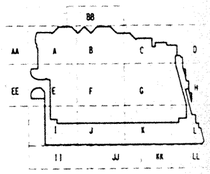
DAN S. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision defined in Title 16 Chapter 97 Section 1-2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
	4-28-95		ADDENDUM #1
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.0.0

Fri Dec 1 16:51:01 1995
jjvncvfoundat1.dgn
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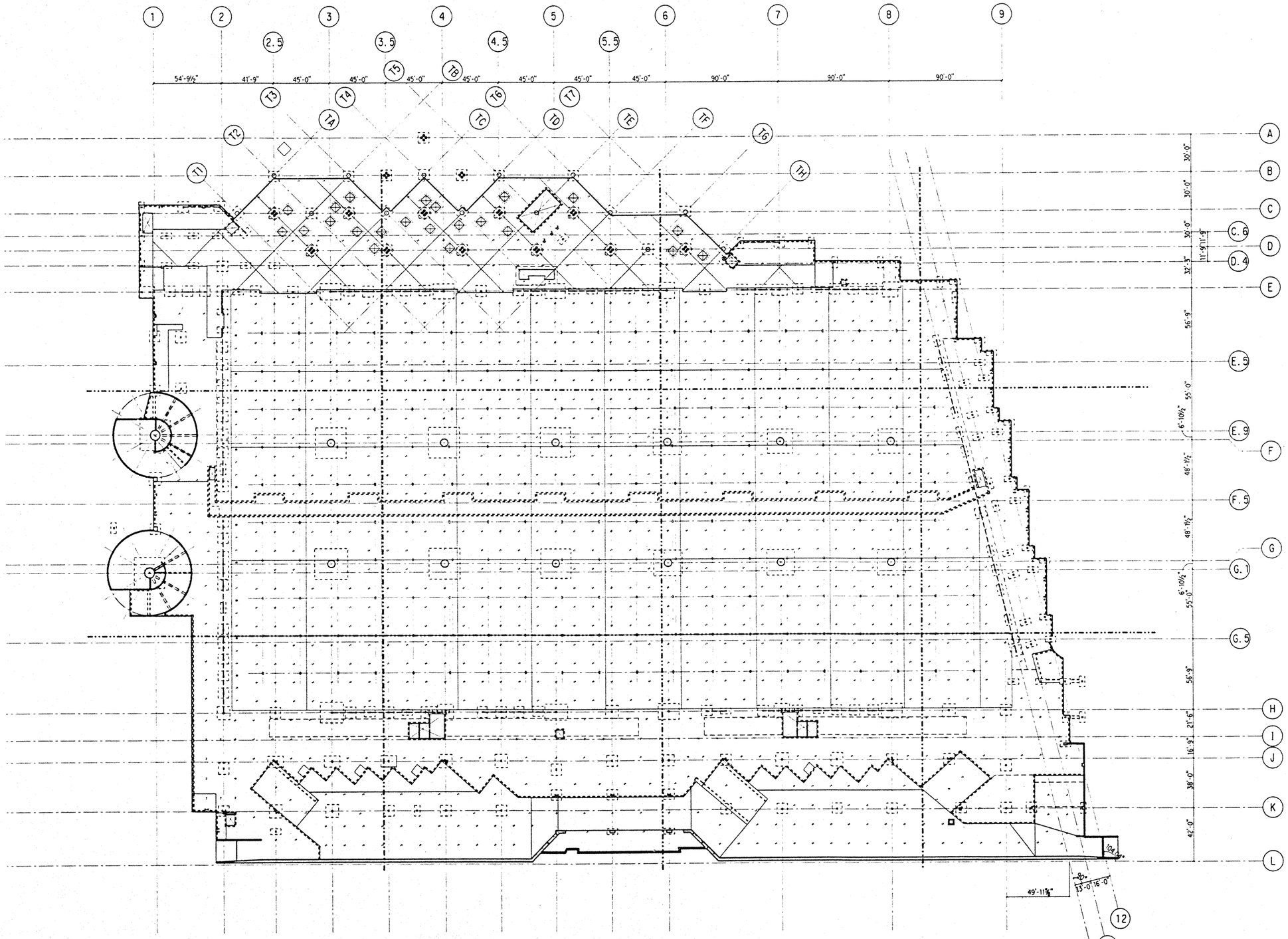

EXHIBIT HALL PLAN

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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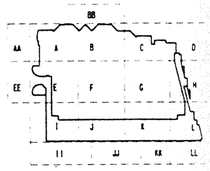
JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Professionally stamped in Title of Chapter 16 Section 1-3-4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL PLAN

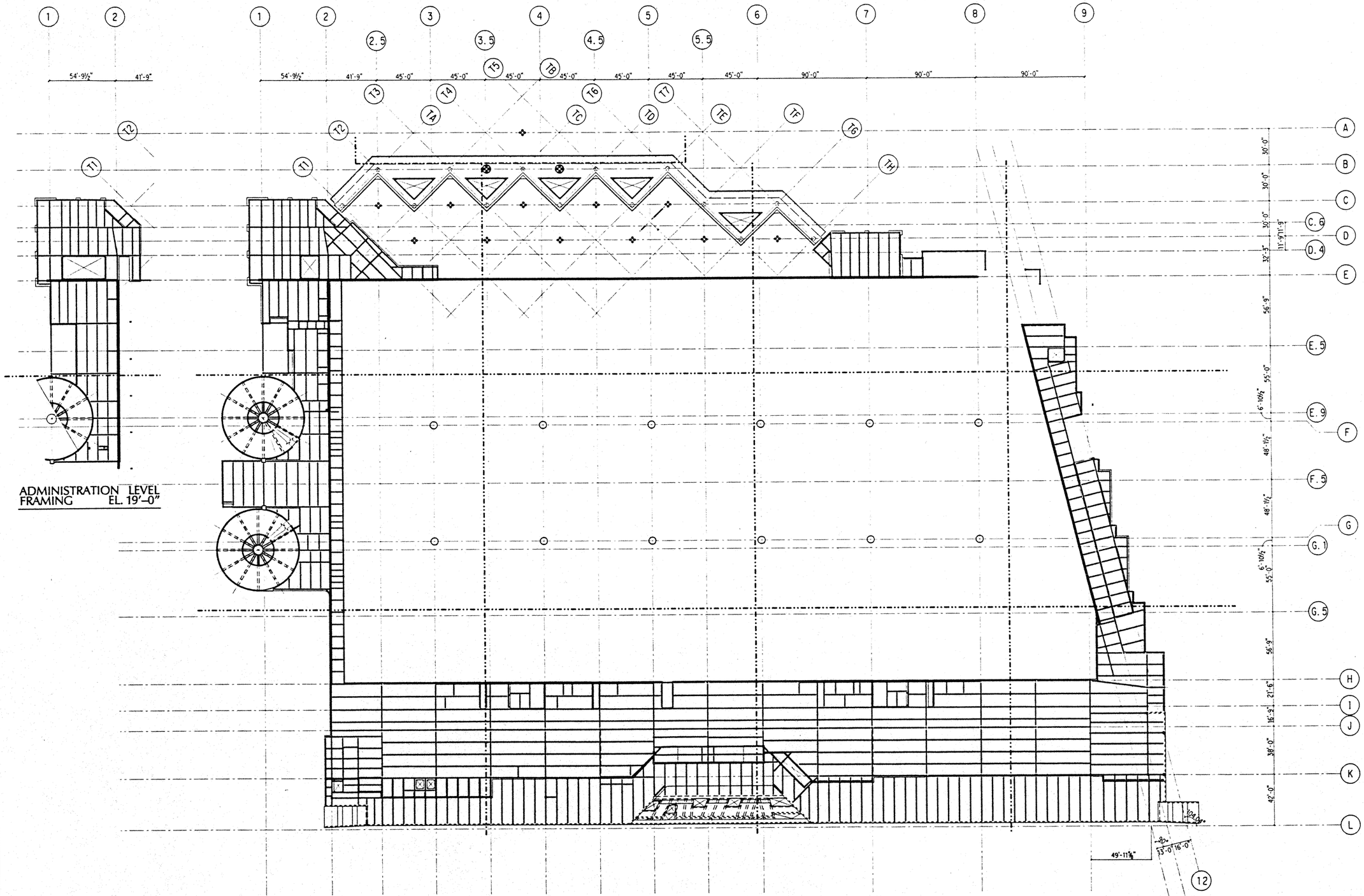
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.0.1

Sat Dec 2 11:09:06 1995

j:\hcc\exhibit.dgn

ws pit



Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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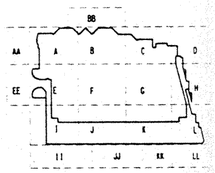
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11773
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Professionally defined in Title 16 Chapter 18 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.2

Fri Dec 1 16:01:05 1995
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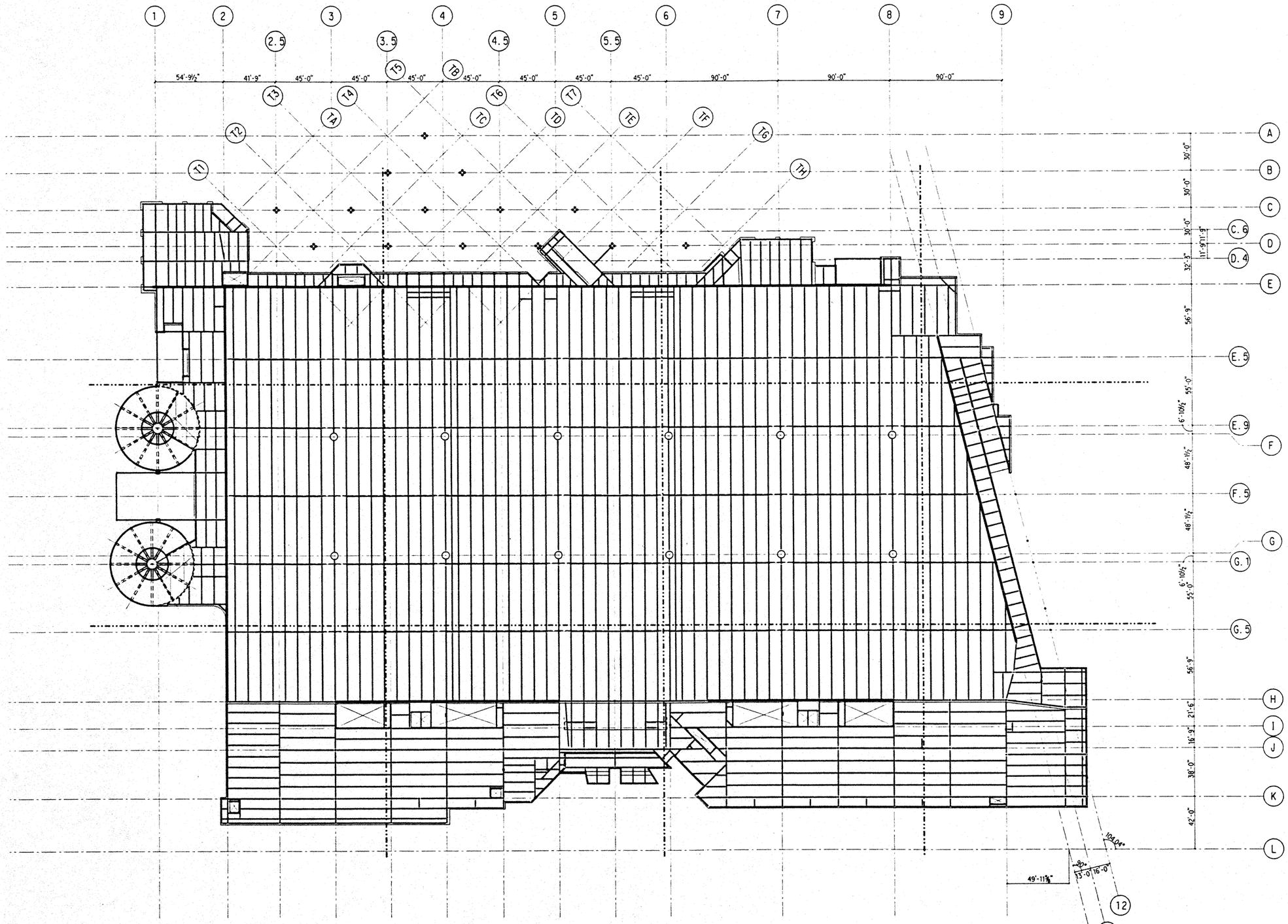

PARKING FRAMING PLAN

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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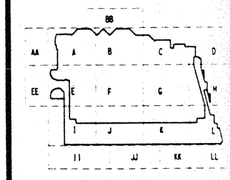
(206) 292-1200 FAX: (206) 292-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision satisfied as Title 16 Chapter 18 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

PARKING FRAMING PLAN

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.3

Fri Dec 1 08:34:23 1995
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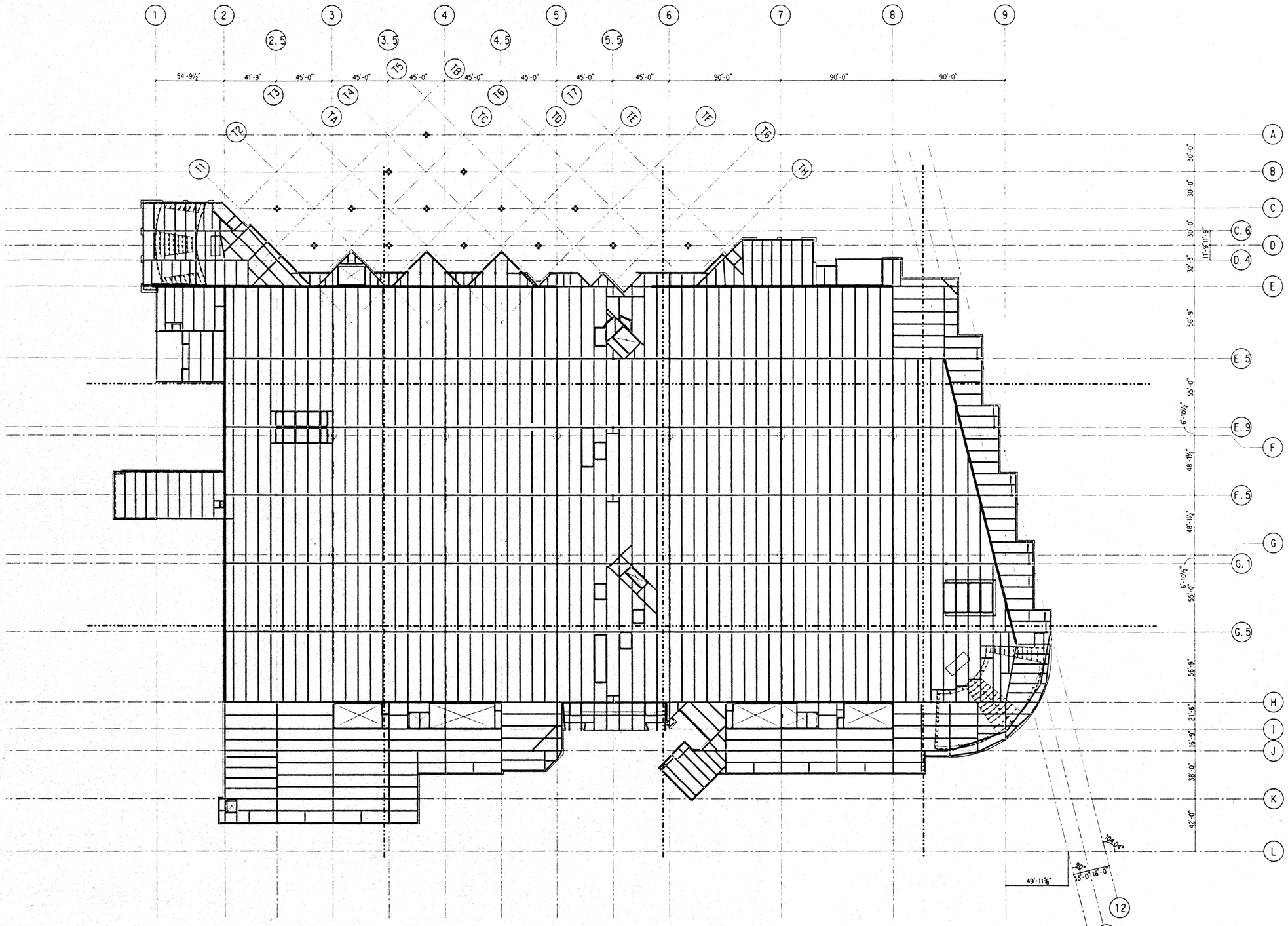

MEETING FRAMING PLAN

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

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SKILLING WARD MAGNUSSON BARKSHIRE INC.
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1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
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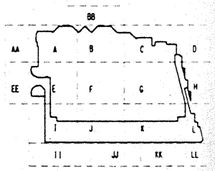
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11773
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision specified in Title 16 Chapter 8 Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

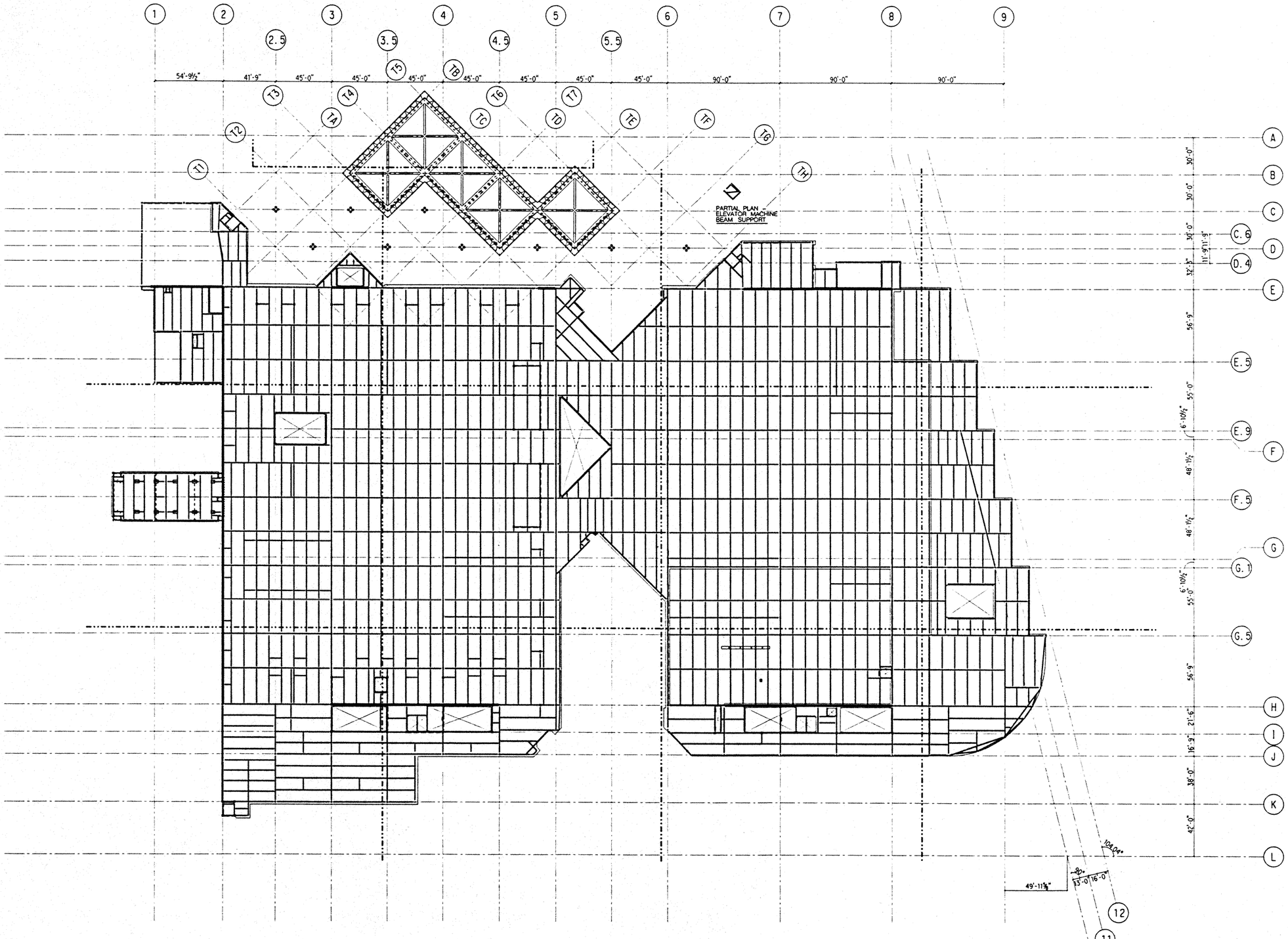
Sheet Title

MEETING FRAMING PLAN

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.0.4

DWS: pit Thu Nov 30 18:00:48 1995 jhccmeeting.dgn



BALLROOM FRAMING PLAN

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

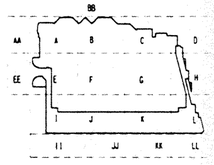
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer in Title in Charge of Building Line of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

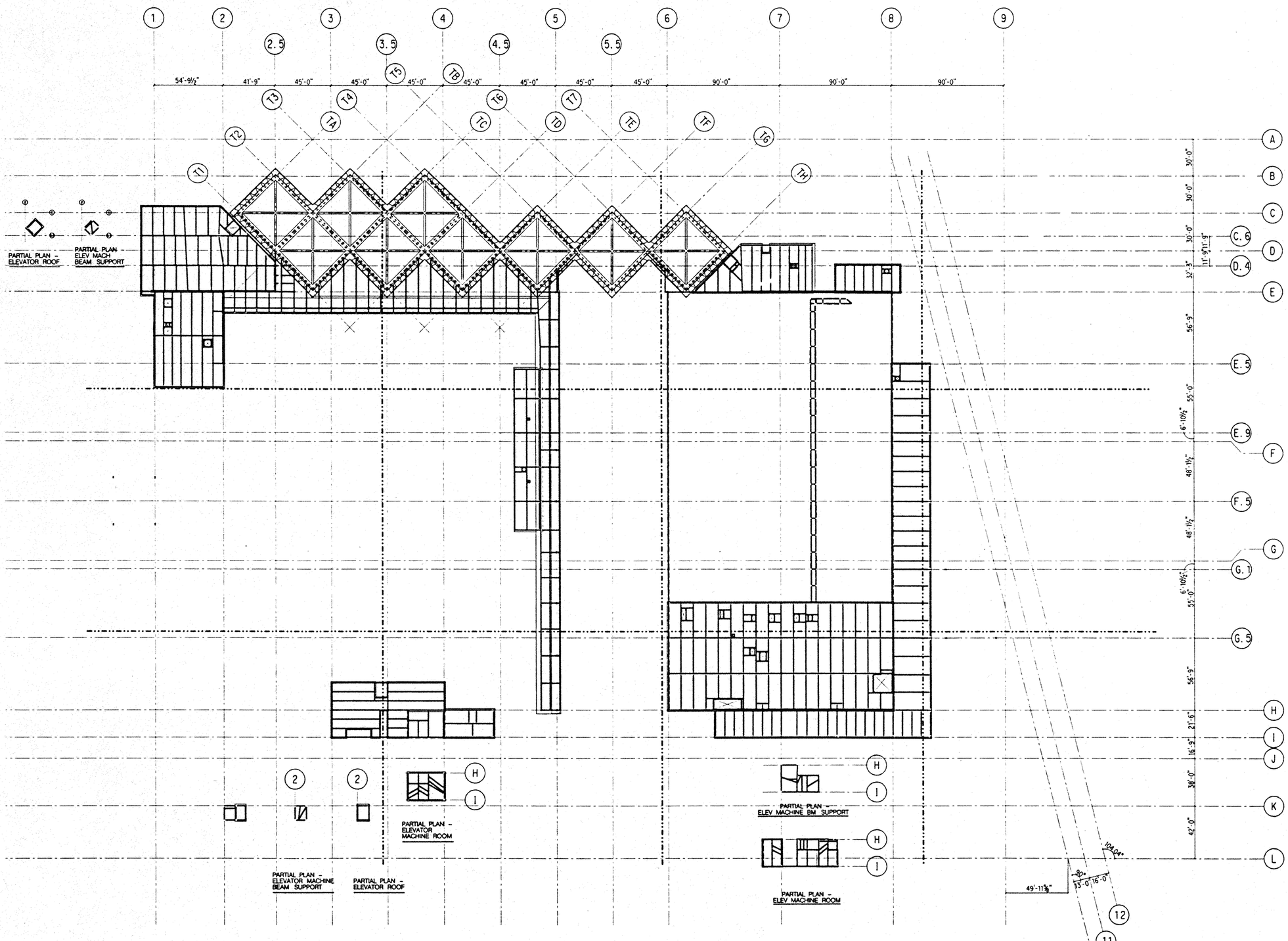
BALLROOM FRAMING PLAN

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.5

Thu Nov 30 17:22:46 1995 j:\hcc\ballroom.dgn pit EWS



MECHANICAL PENTHOUSE FRAMING

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
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Honolulu, HI

Consultants

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ARN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7177S
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 18 Section L-3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.

Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

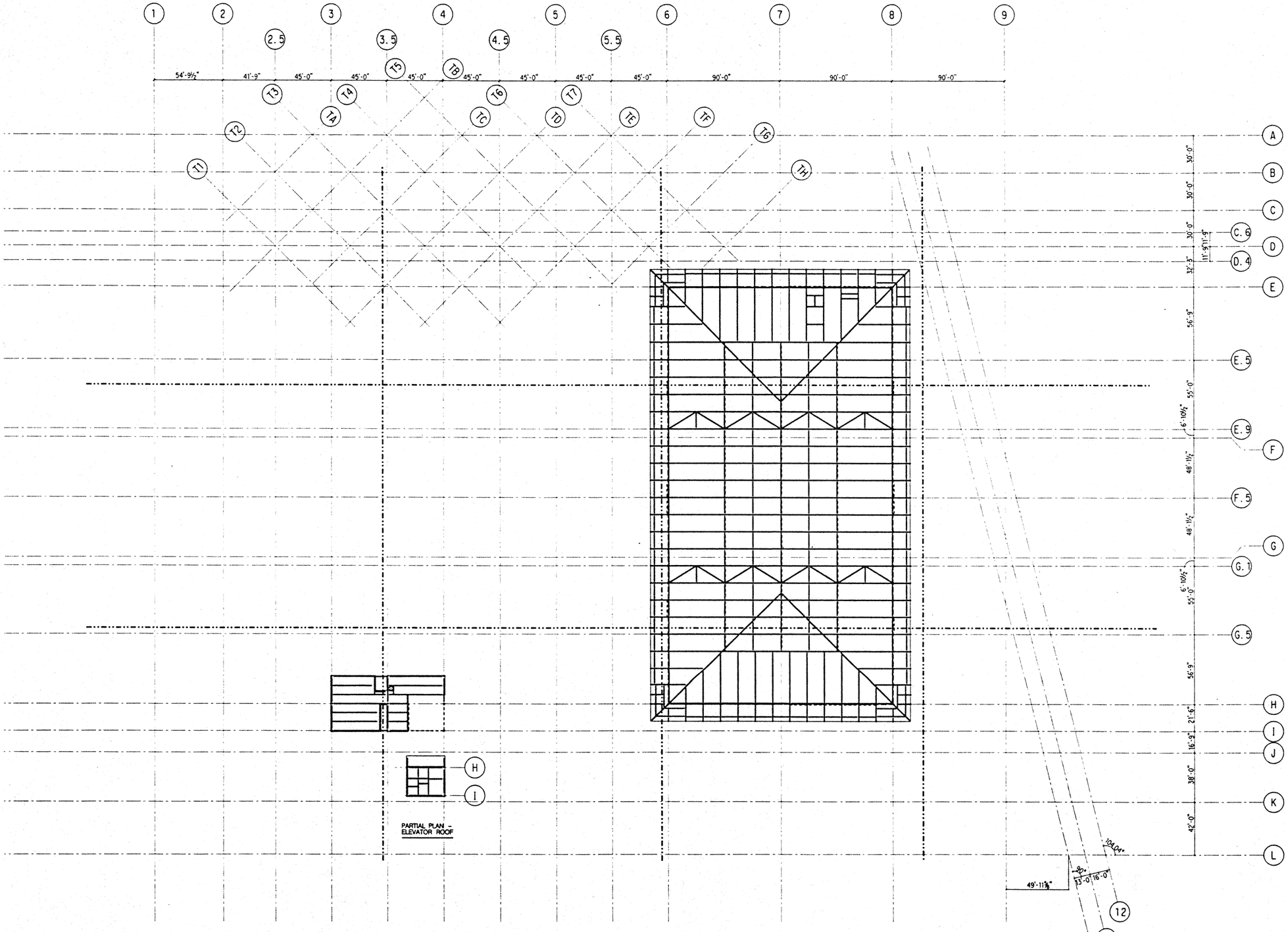
MECH PENTHOUSE FRAMING

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.6

Thu Nov 30 16:24:21 1995 j:\hcc\mechanic.dgn pit



ROOF FRAMING PLAN

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 623-1201

LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

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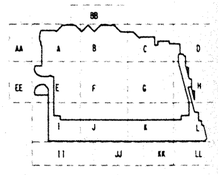
Supervision as defined in Title 16 Chapter 18 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

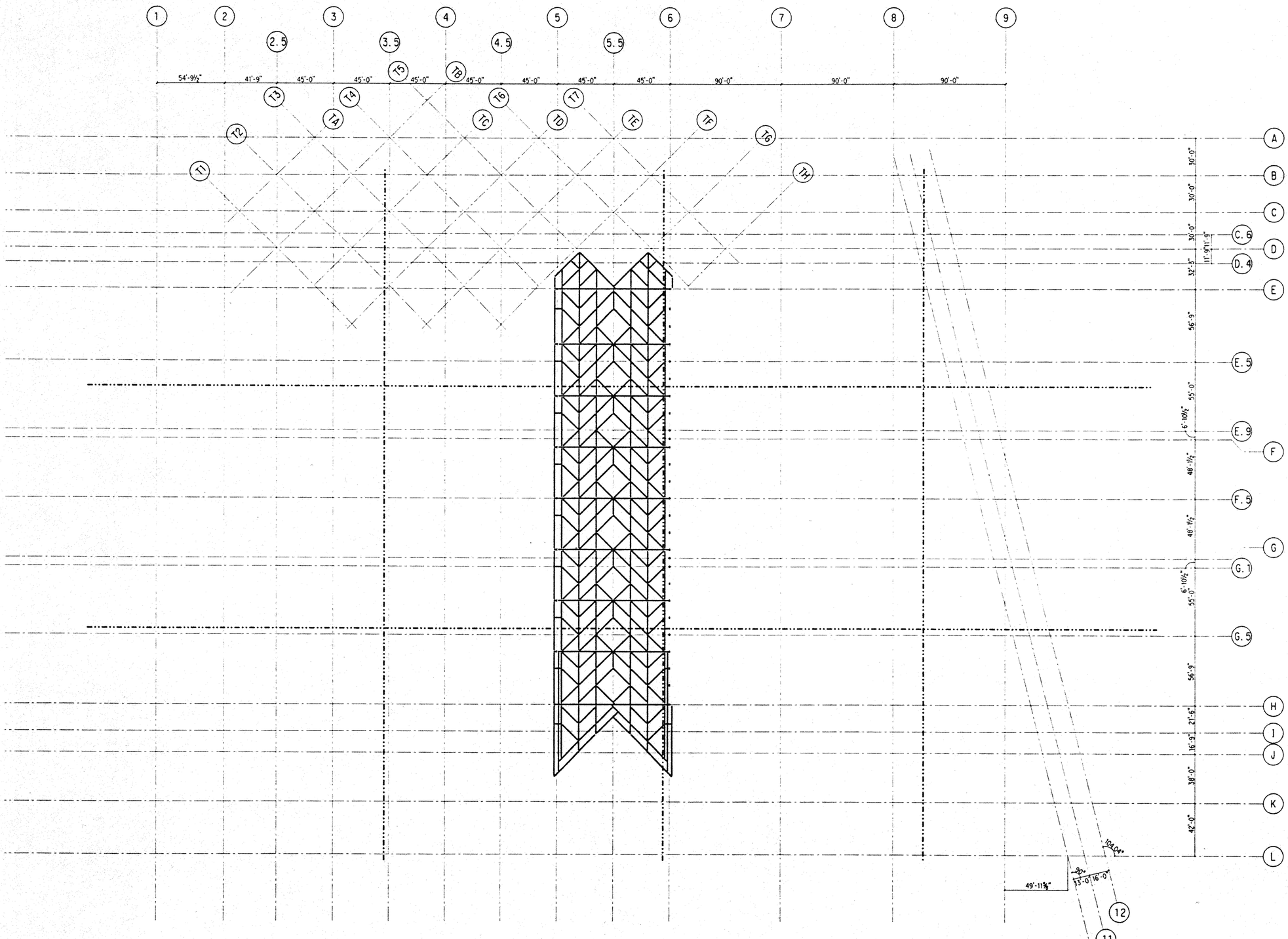
ROOF FRAMING PLAN

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.7

Thu Nov 30 15:03:00 1995 j:hccroof.dgn pit DMS



ROOF TRELLIS FRAMING PLAN

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

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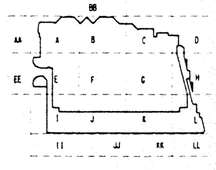
JAN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervisorship defined in Title 16 Chapter 3 Section 1.3 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

ROOF TRELLIS FRAMING PLAN

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.7T

Thu Nov 30 15:05:10 1995
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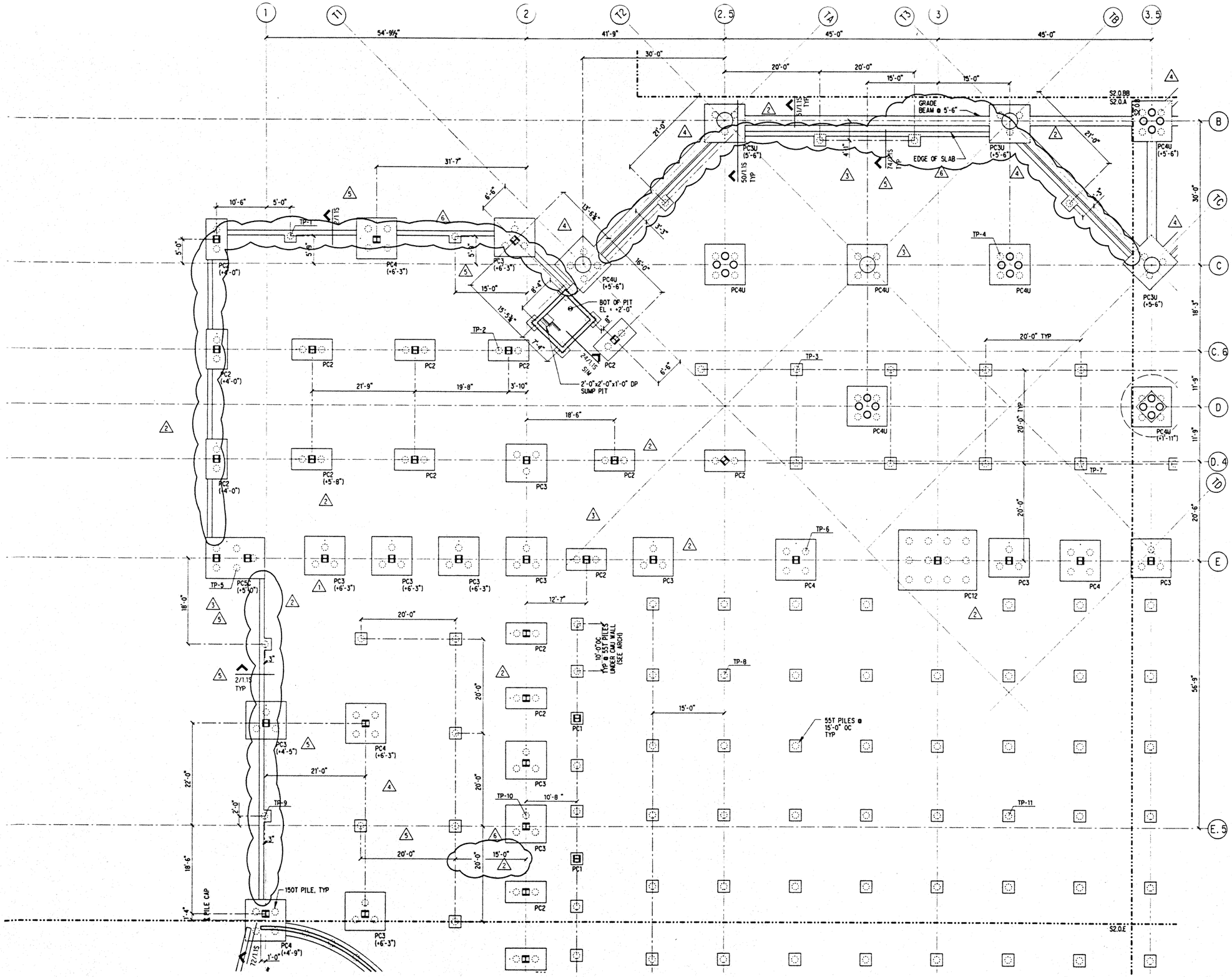
DWS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.• SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.• TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU PLANS
   - S1.20 CONCRETE SCHEDULES
   - S1.21 S1.22 STEEL SCHEDULES
   - S3.• TYPICAL STEEL COLUMN TREE ELEVATIONS AND DETAILS
   - S2.• PLANS
   - S4.• BRACED FRAME ELEVATIONS
   - S5.• TRUSS ELEVATIONS
   - S6.• BRACED FRAME & TRUSS DETAILS
   - DETAIL BOOK STRUCTURAL DETAILS
2. PILES FOR PC2 THRU PC40 ARE 150 TON CAPACITY PILES. ALL SINGLE PILES ARE 55 TON CAPACITY PILES.
3. PILECAPS AND PILES REQUIRING UPLIFT CAPACITY ARE DENOTED WITH A "U" ON THE PLANS, EX: PC14U. PROVIDE TENSION SPLICES FOR PILES AT THESE PILECAPS.
4. FOR TOP OF PC1 ELEV SEE 18/S1.2.
5. TOP OF PILECAP AT ELEVATION +6.0 FEET, TYP UNLESS NOTED OTHERWISE.
6. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, BOLLARDS, EDGE ANGLES, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
7. TP-" " DENOTES TEST PILE NUMBER, SEE SPECIFICATIONS.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
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Loschky Marquardt & Nesholm
Seattle, Washington

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Consultants
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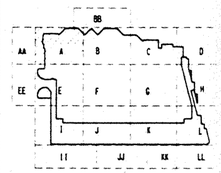
SKILLING WARD MAGNUSSON BARKSHIRE INC. LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer certified to Title 16, Chapter 53 Section 1.24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	9-31-95		
3	8-14-95		
2	6-19-95		
1	4-28-95		ADDENDUM #1
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

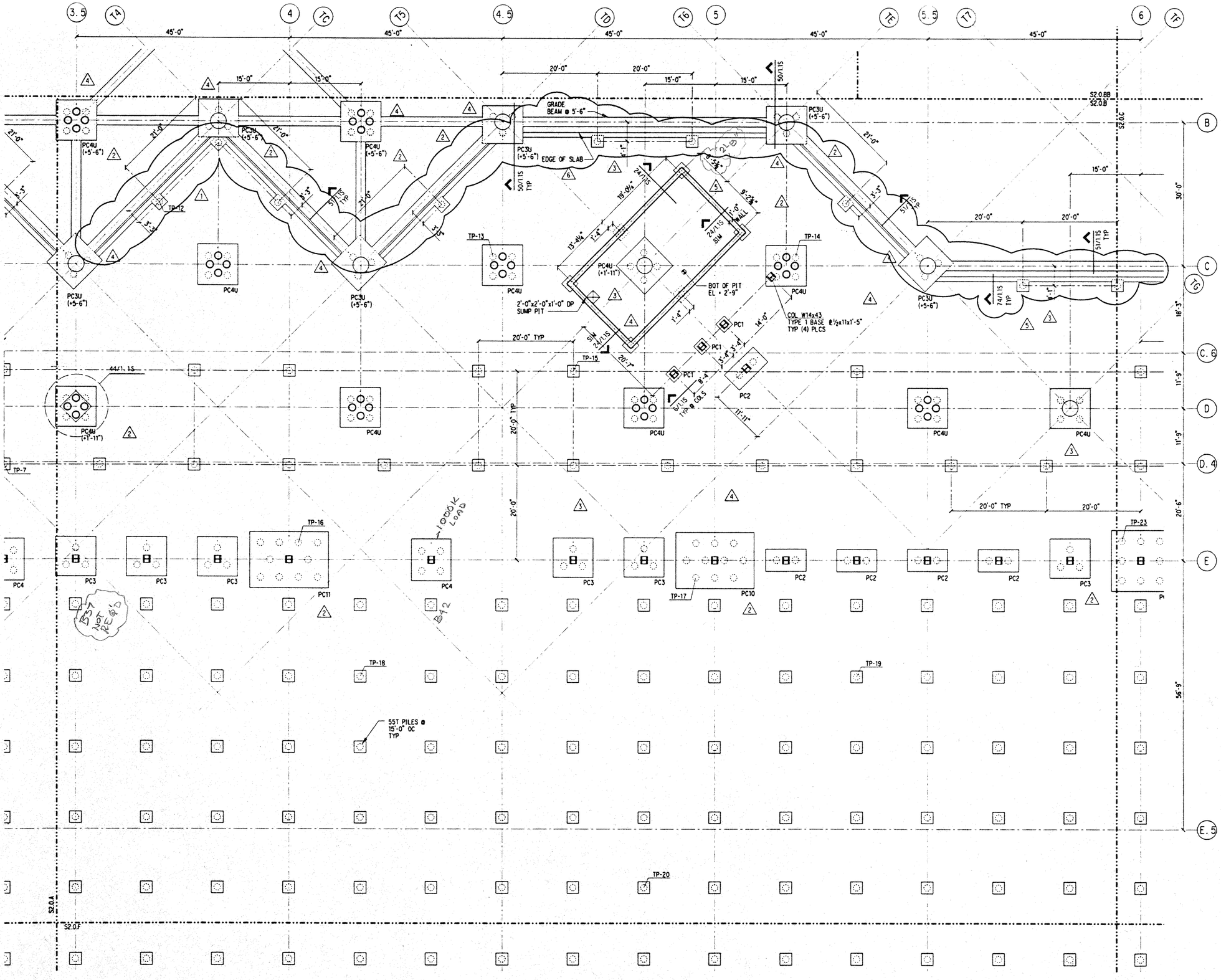
FOUNDATION PLAN SECTOR A

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.A

FriDec 1 16:53:36 1995
J:\hcc\fdn_a.dgn
DMS pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

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LICENSED PROFESSIONAL ENGINEER NO. 11778 HAWAII, U.S.A.

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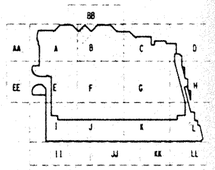
Supervision as defined in Title 16 Chapter 18 Subpart 1.2.14 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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3	8-4-95		
2	6-19-95		
1	4-28-95		ADDENDUM #1
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4/ 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.B

Fri Dec 1 16:56:03 1995

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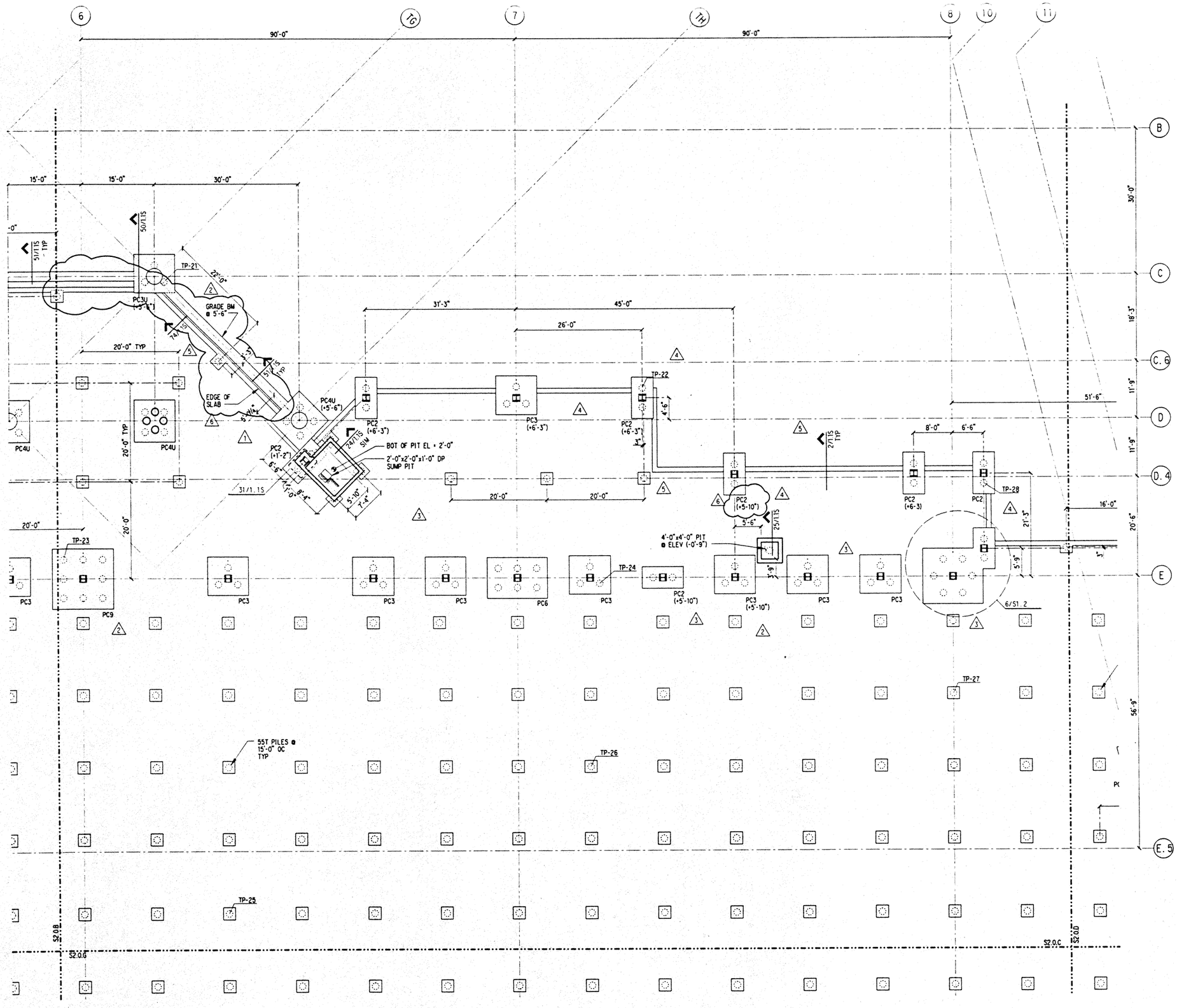
DMS pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

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JON D. MAGNUSSON
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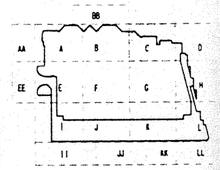
NO. 11773
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision Engineer as Title in Chapter 16 Subpart 1.3.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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3	8-4-95		
2	6-19-95		
1	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.C

Fri Dec 1 16:58:30 1995

jwhccvfdn_c.dgn

pit

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

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JOHN D. MAGNUSSON
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NO. 11775
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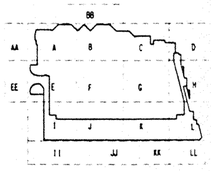
Supervision as defined in Title 16 Chapter 16 Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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4	8-31-95		
3	8-4-95		
2	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION
PLAN
SECTOR D

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.0.D

Sat Dec 2 09:32:19 1995
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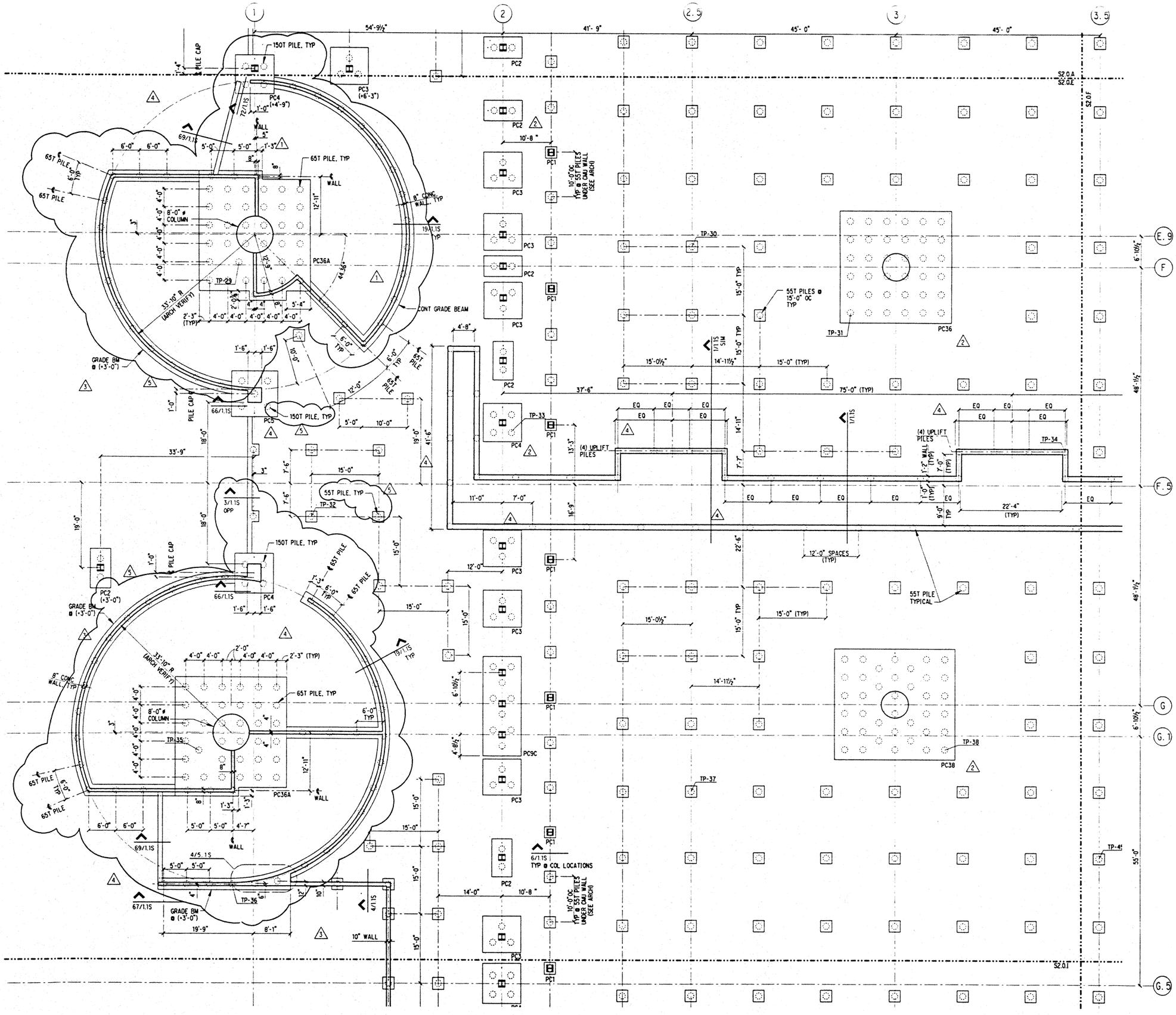
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

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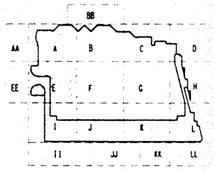
Supervision as defined in Title 16 Chapter 8 Section 1.0.4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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2	6-19-95		
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION
PLAN
SECTOR E

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.E

Fri Dec 1 17:03:22 1995
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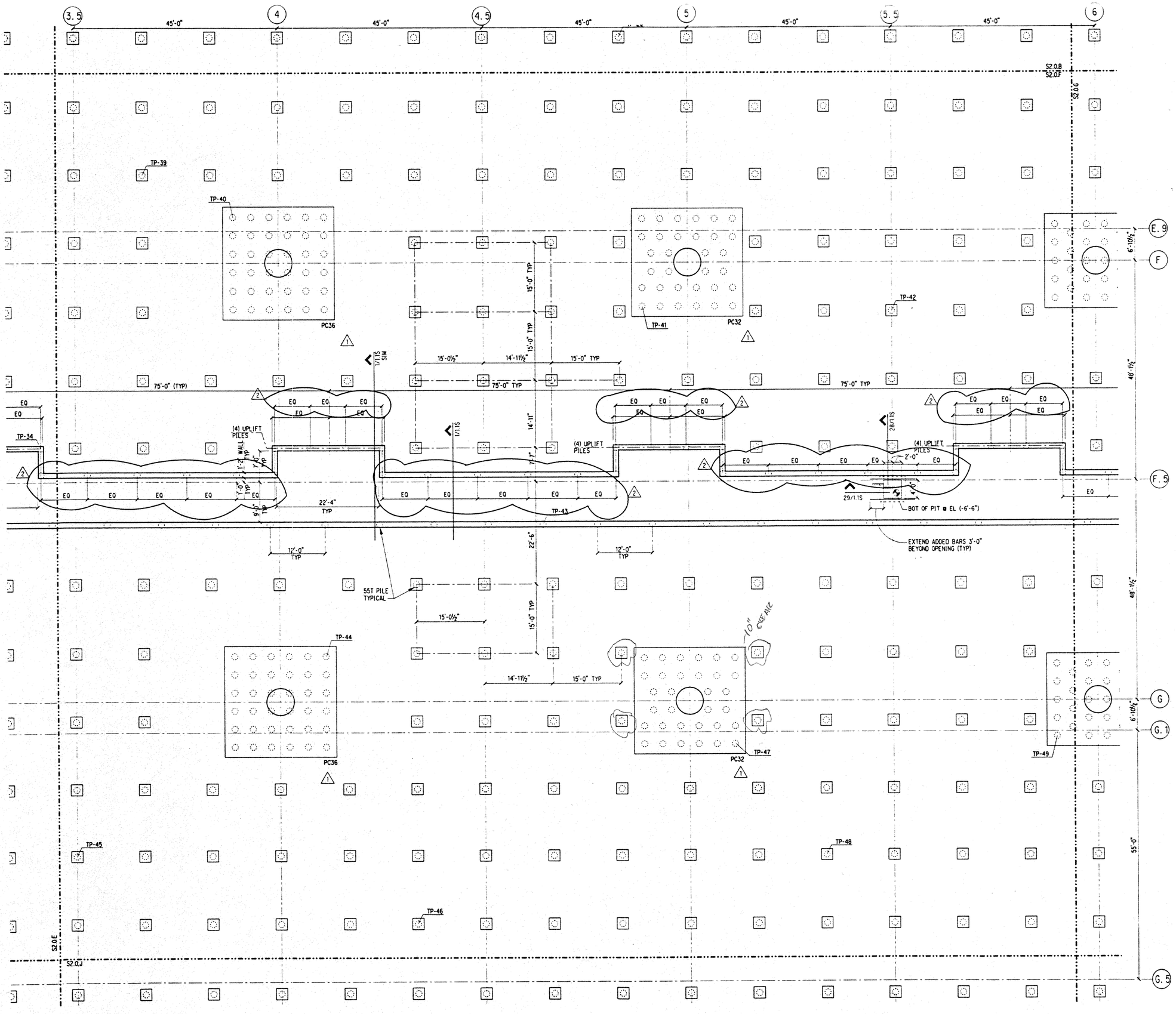

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

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Honolulu, Hawaii

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Honolulu, Hawaii

in association with

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NO. 11775
HAWAII, U.S.A.

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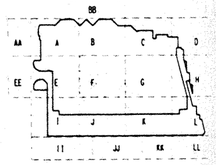
Supervision described in Title 16 Chapter 18 Subpart 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.F

Fri Dec 1 17:05:49 1995

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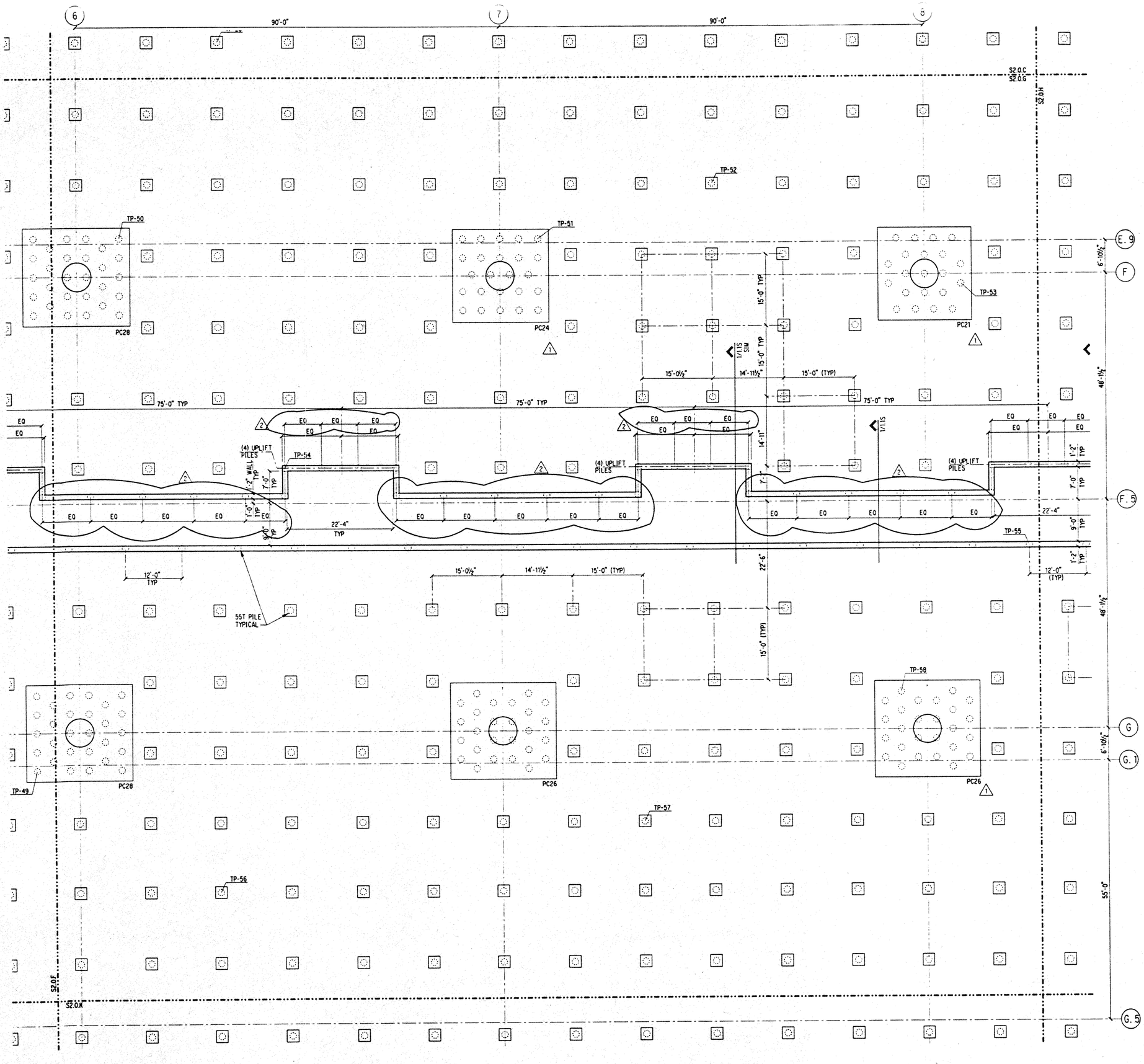

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

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Honolulu, Hawaii

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Honolulu, Hawaii
in association with
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Seattle, Washington

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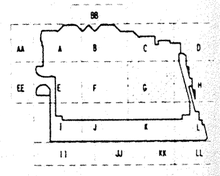
Supervision as defined in Title 16 Chapter 18 Section 18.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN SECTOR G

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.G

Fri Dec 11 17:08:16 1995

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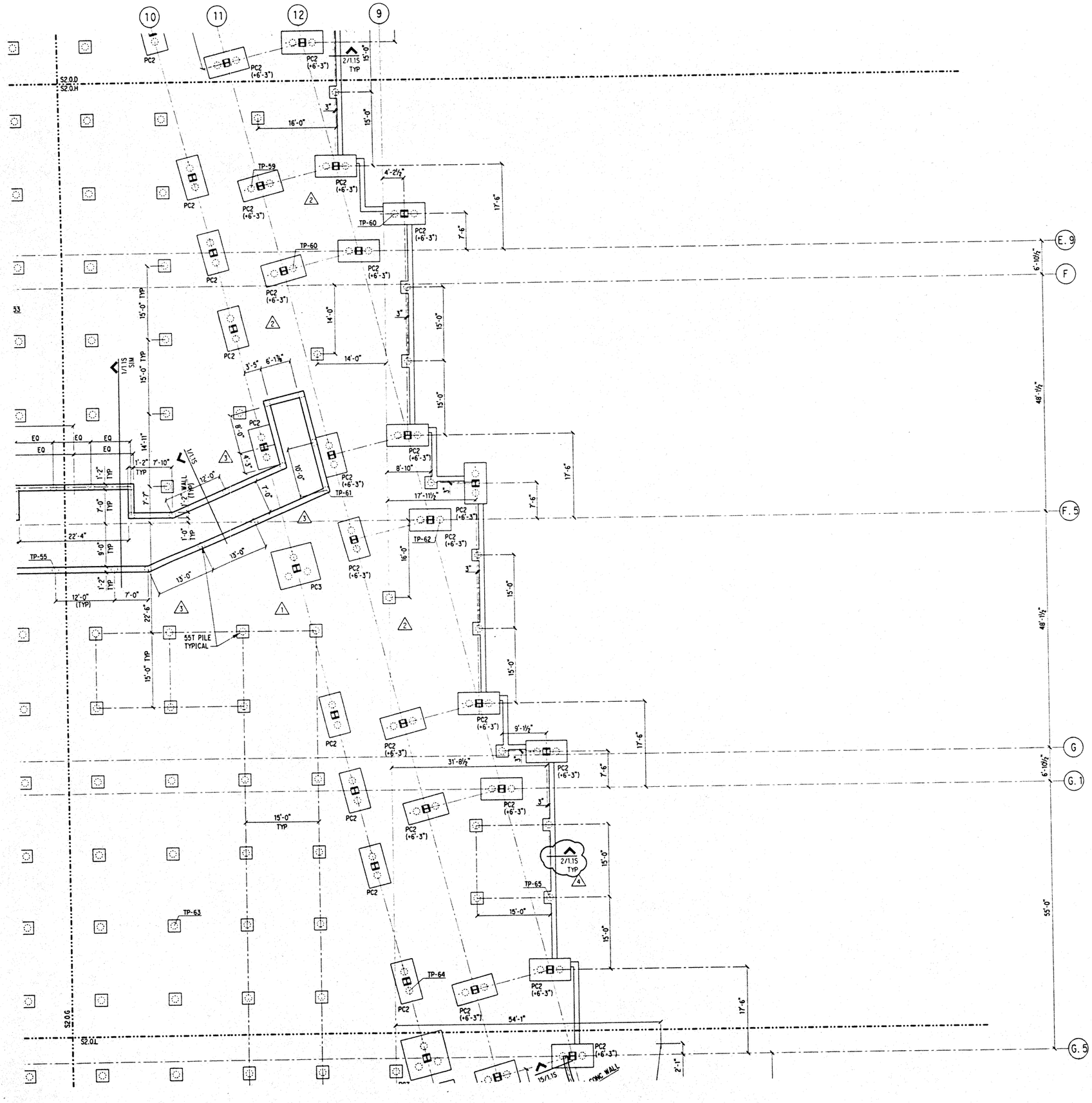

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Honolulu, Hawaii
in association with
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Seattle, Washington

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HAWAII U.S.A.

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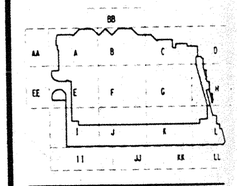
Supervised as defined in Title 16 Chapter 18 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION
PLAN
SECTOR H

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.H

Fri Dec 1 17:10:41 1995

jwccvfdn_h.dgn

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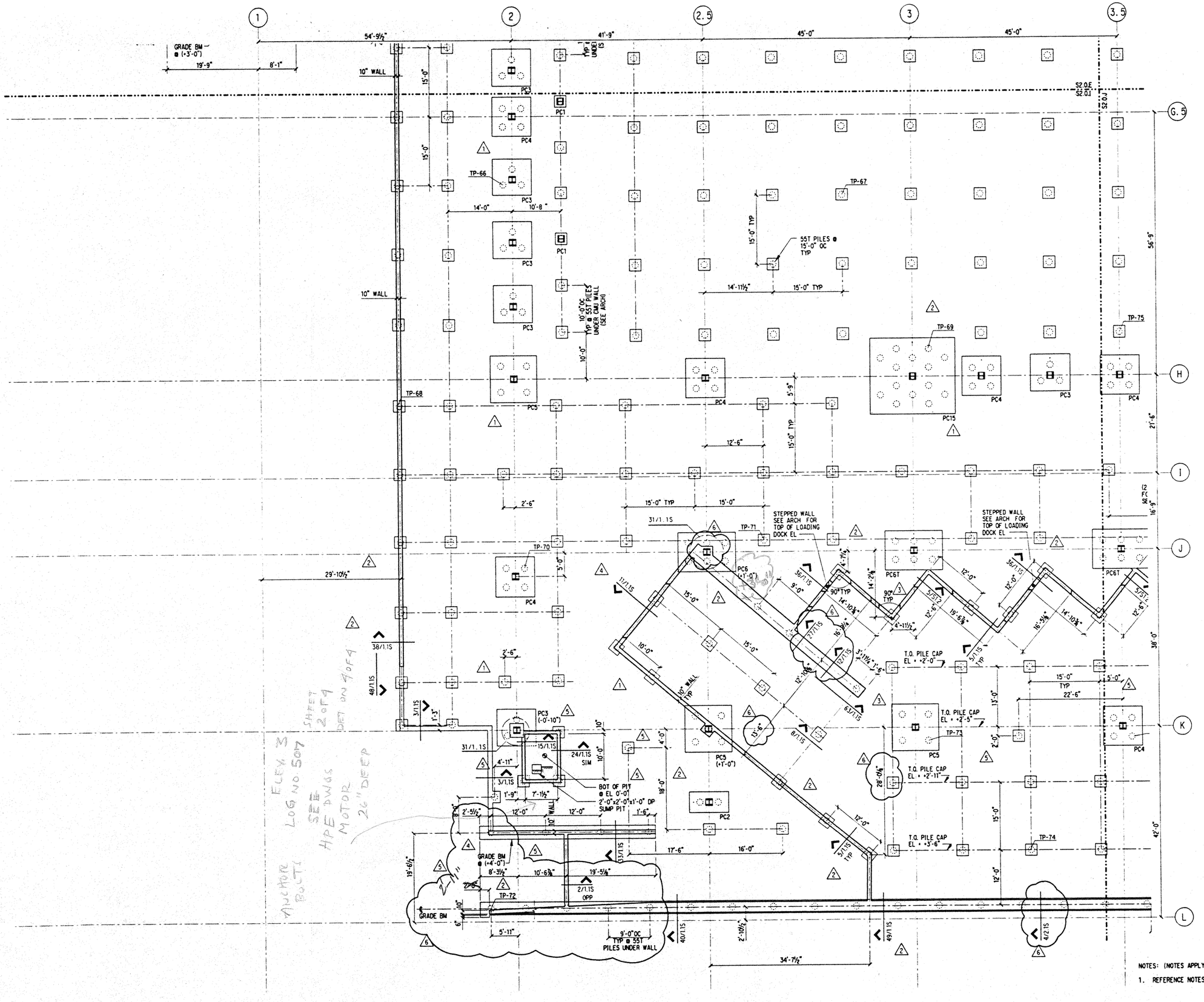

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

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Honolulu, Hawaii

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in association with

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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
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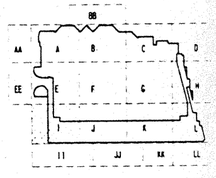
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 53 Subpart 1.2.b of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

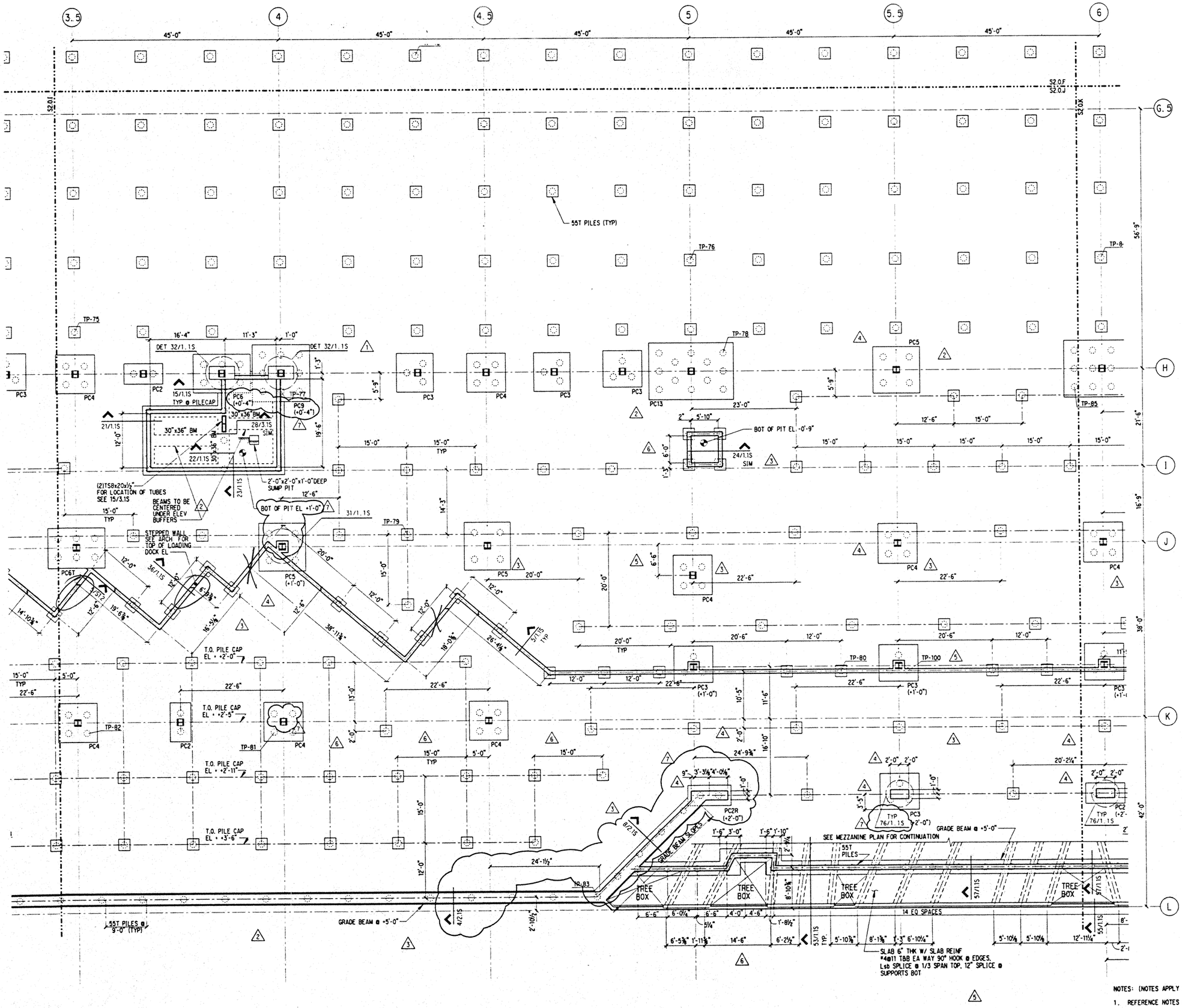
FOUNDATION PLAN SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.I

Fri Dec 1 17:12:47 1995 j:\hccv\fdn_i.dgn plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

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Honolulu, Hawaii

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in association with
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Seattle, Washington

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JON D. MAGNUSSON
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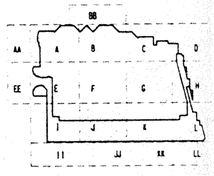
Supervision disclaimed in Title 16 Chapter 18 Subpart 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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3	8-4-95		
2	6-19-95		
1	4-28-95		ADDENDUM #1
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION
PLAN
SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.0.J

Fri Dec 1 17:15:14 1995

jkhccvdn_j.dgn

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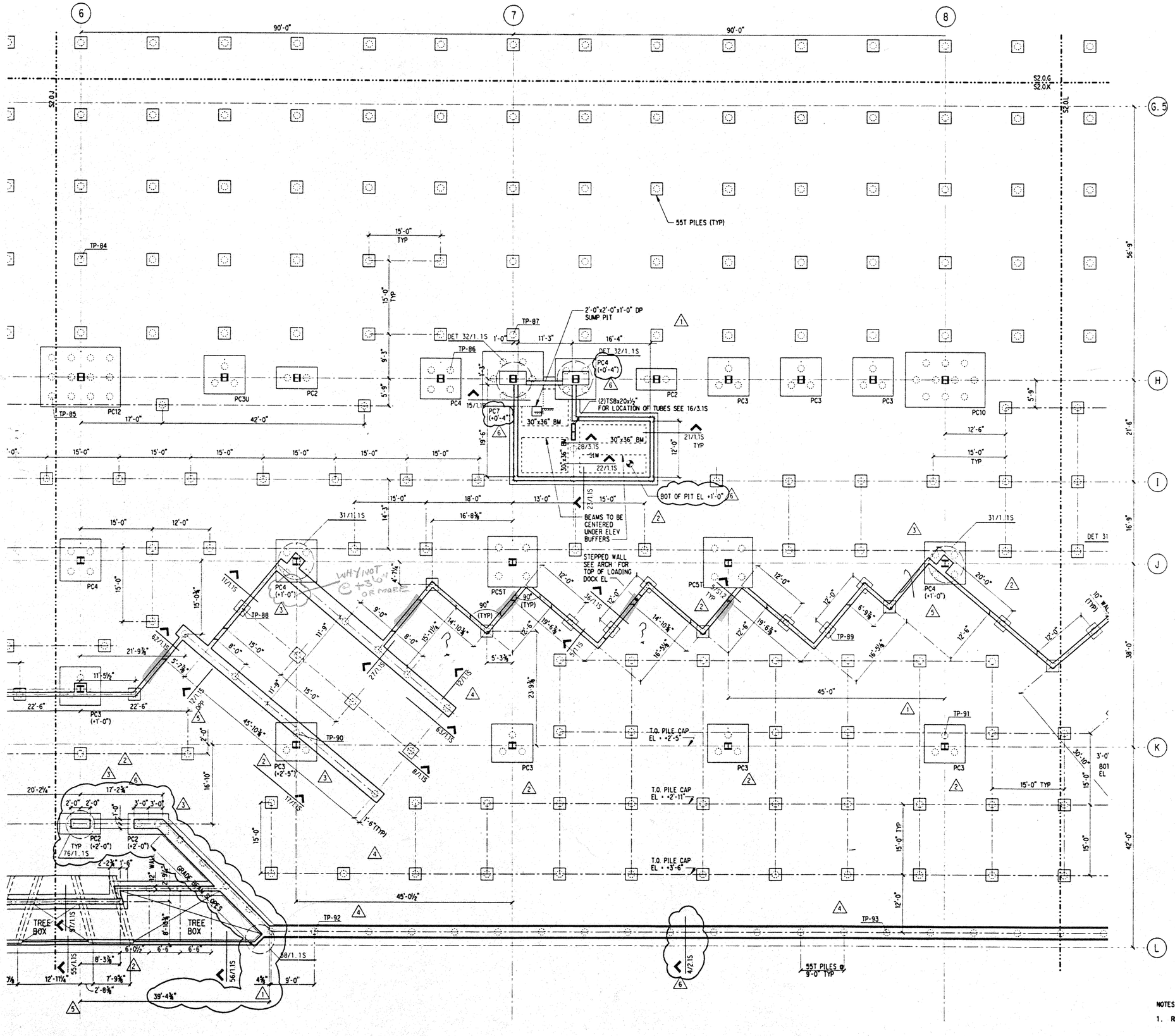

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

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Convention Center Authority
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Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
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SIM D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
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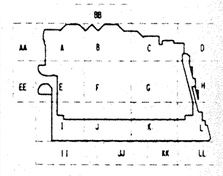
Supervisory Engineer in Title 16 Chapter 18 Section 1-2 of the Rules and Regulations of the Board of Registered Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-11-95		
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION PLAN SECTOR K

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

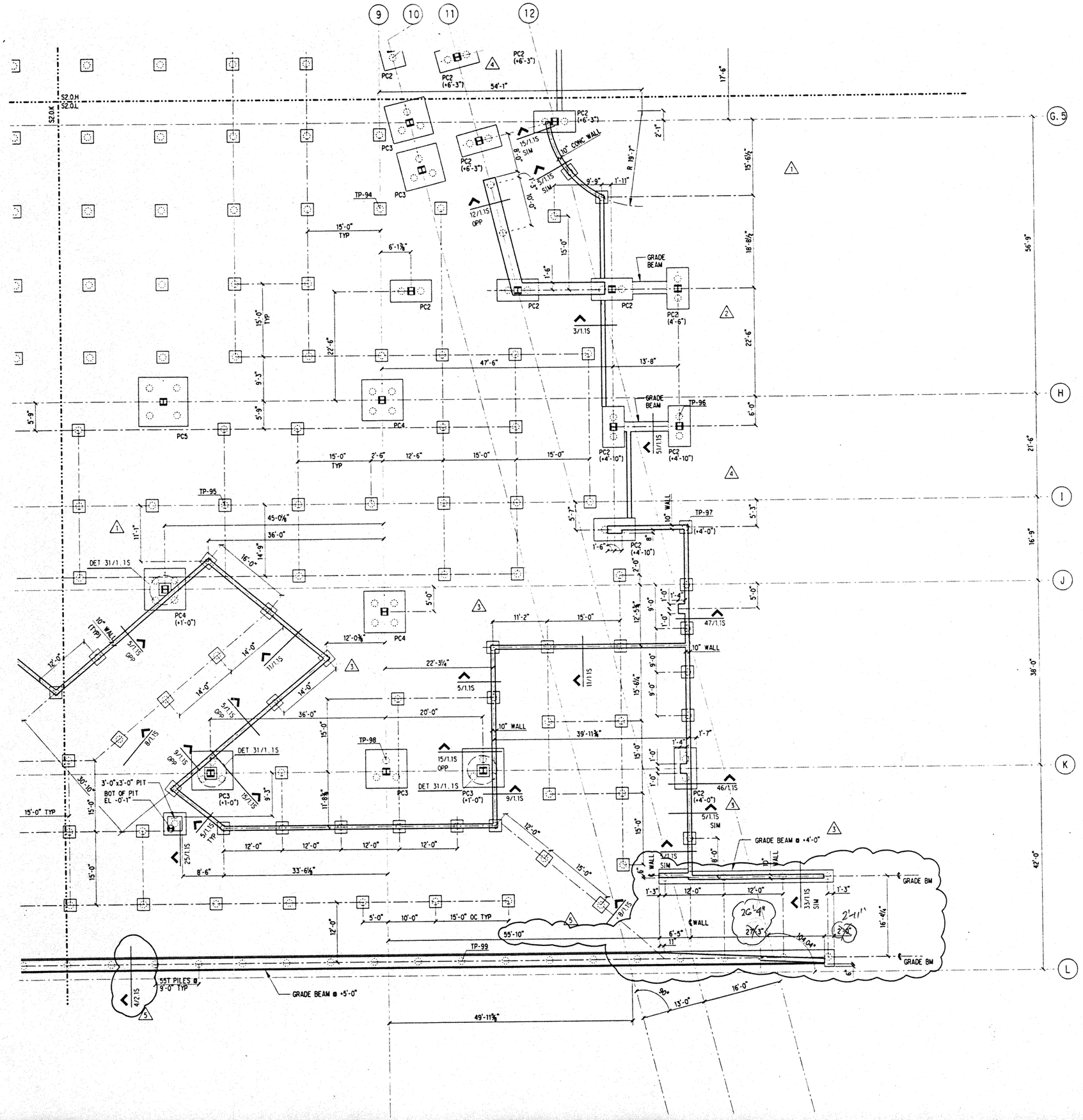
Sheet No.

S2.0.K

Sat Dec 2 09:34:18 1995

jxhccfdn.k.dgn

DMS pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

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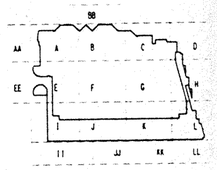
Licensed professional in Title 16 Chapter 10 Section 1-3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
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2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION
PLAN
SECTOR L

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.0.L

Sat Dec 2 09:36:35 1995

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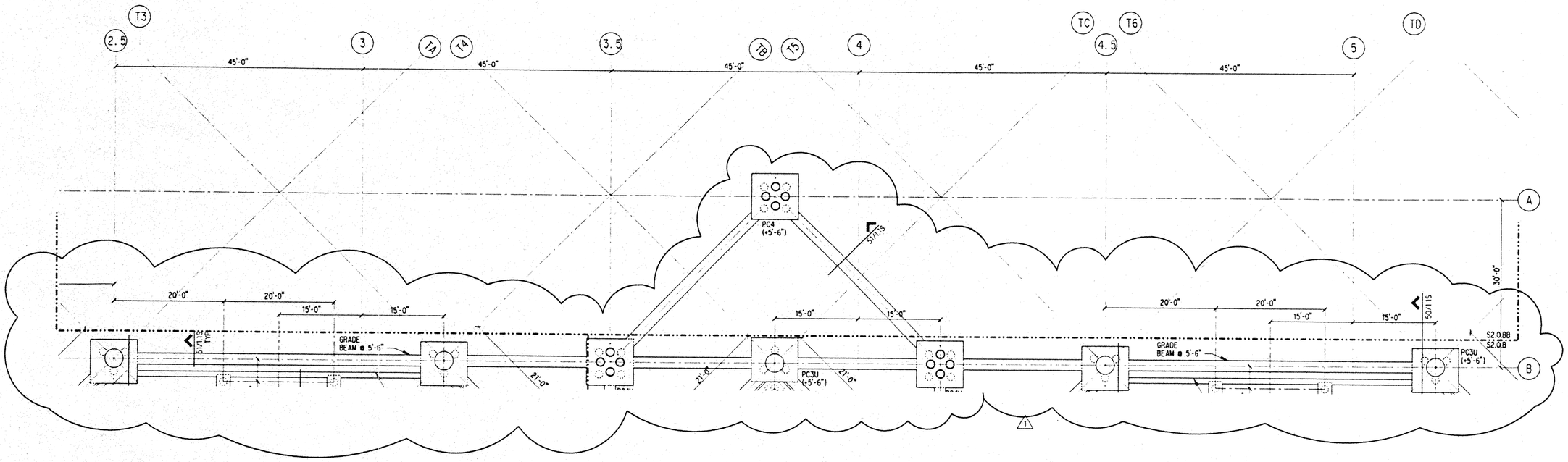

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.0.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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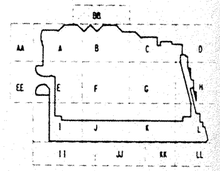
Supervisory defined as Title 16 Chapter 18 Section L.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
1	8-4-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

FOUNDATION
PLAN
SECTOR BB

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

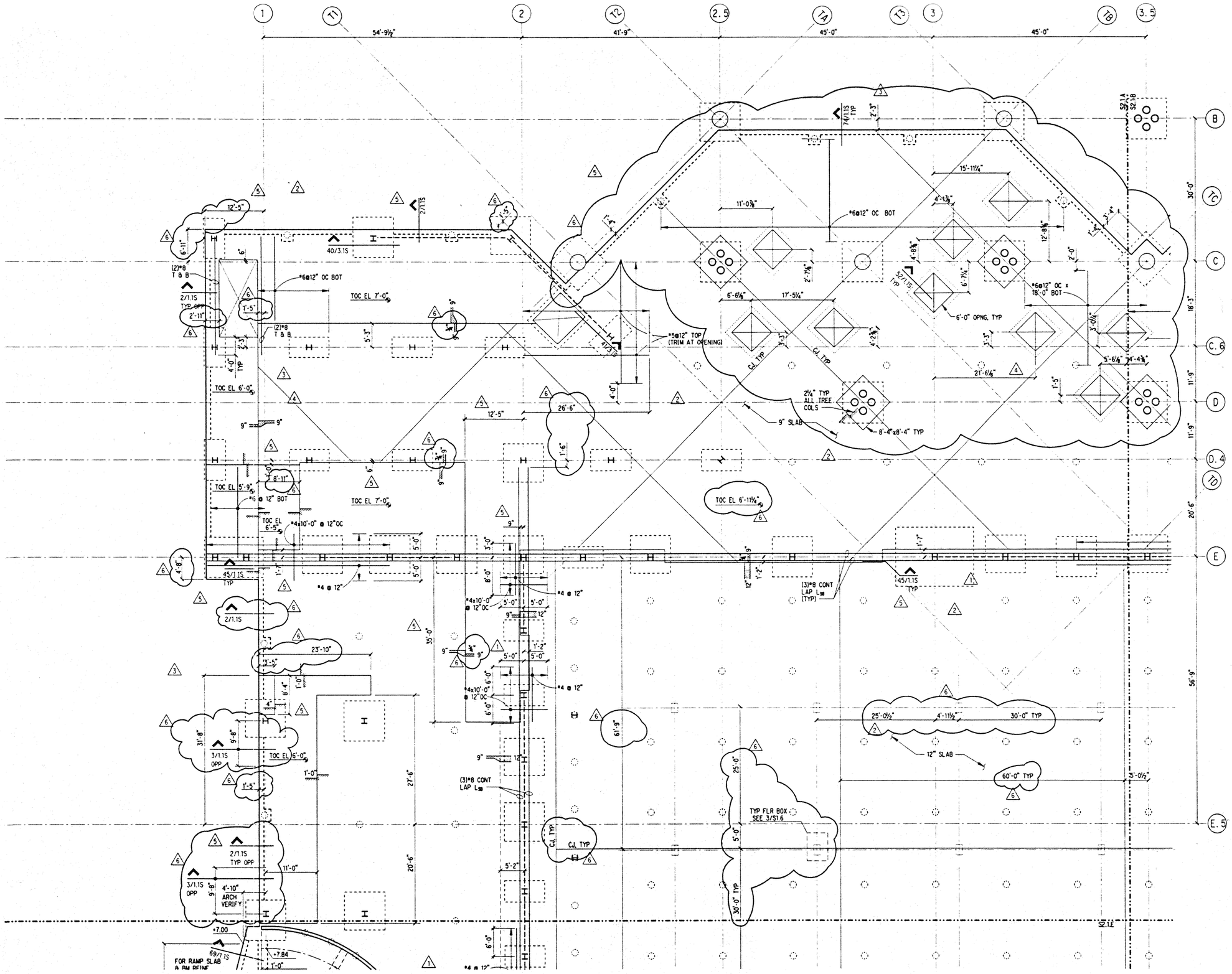
Sheet No.

S2.0.BB

Sat Dec 2 09:38:50 1995

jxhccfdn_bb.dgn

DMS pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.* — SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.* — TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
   - S1.20 — CONCRETE SCHEDULES
   - S1.21 S1.22 — STEEL SCHEDULES
   - S2.* — PLANS
   - S3.* — TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
   - S4.* — BRACED FRAME ELEVATIONS
   - S5.* — TRUSS ELEVATIONS
   - S6.* — BRACED FRAME & TRUSS DETAILS
   - DETAIL BOOK — STRUCTURAL DETAILS
2. REFERENCE FLOOR ELEVATION = 7'-0"
3. TOP OF CONCRETE SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
4. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, BOLLARDS, EDGE ANGLES, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
5. REBAR SEQUENCE
   1. EAST/WEST BOTTOM BARS
   2. NORTH/SOUTH BOTTOM BARS
   3. EAST/WEST TOP BARS
   4. NORTH/SOUTH TOP BARS
6. ALL REINFORCING SHOWN ON PLAN IS IN ADDITION TO THAT SPECIFIED IN NOTES 7 AND 8.
7. FOR TYPICAL SLAB REINFORCING WITHIN GRIDS (2)-(10) & (E)-(H) SEE 2/S1.6
8. SLAB REINFORCING OUTSIDE GRIDS (2)-(10) & (E)-(H) SHALL BE AS FOLLOWS.
   a) *6@12 EA WAY BOT *
      *4@12 EA WAY TOP *
      *BARS CONTINUOUS - LAP LSB (TYP)
   b) SEE DETAIL 5/S1.6 FOR ADDITIONAL REINFORCEMENT AT LARGE SLAB OPENINGS IN LOBBY.
   c) SEE DETAILS 6/S1.6-9/S1.6 FOR ADDITIONAL TOP BARS AT PILE CAPS
   d) SEE DETAIL 11/S1.6 FOR BOTTOM STEEL LAYOUT AT PILE CAP
   e) SEE DETAILS 10/S1.6 FOR ADDITIONAL REINFORCEMENT AT WALL SUPPORTED SLAB EDGES.

Hawai'i Convention Center

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This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervised in person in Title or Chapter 16 Subtitle 13 Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.

Revisions

No.	Date	By	Description
6	12-15-95		FINAL DOCUMENT
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-14-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

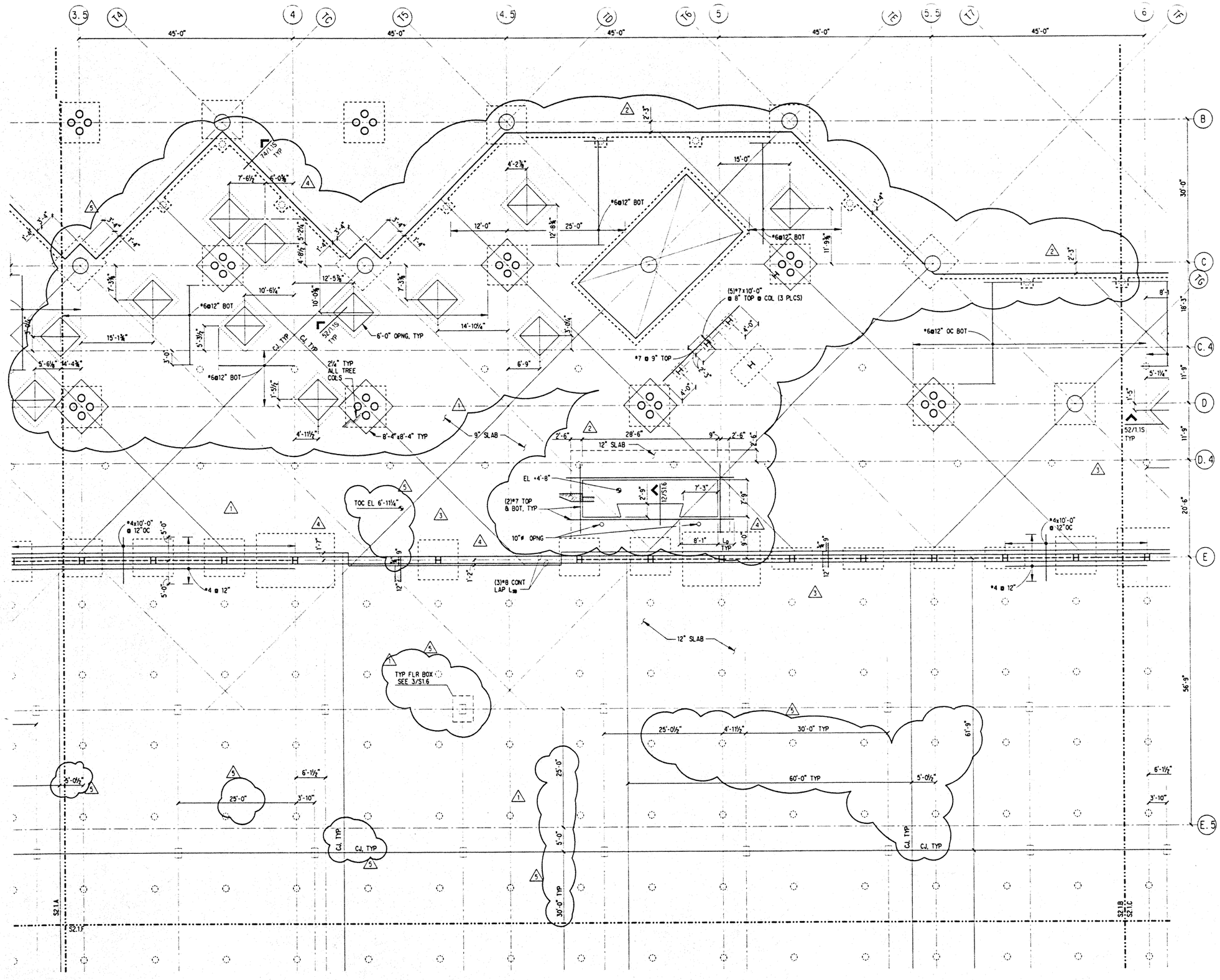
Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR A

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.1.A

Mon Dec 4 12:11:49 1995
jthcc\exhib_a.dgn
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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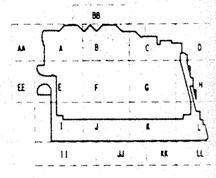
Supervised as defined in Title 16 Chapter 18 Section 1.2.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.B

Mon Dec 4 12:13:57 1995
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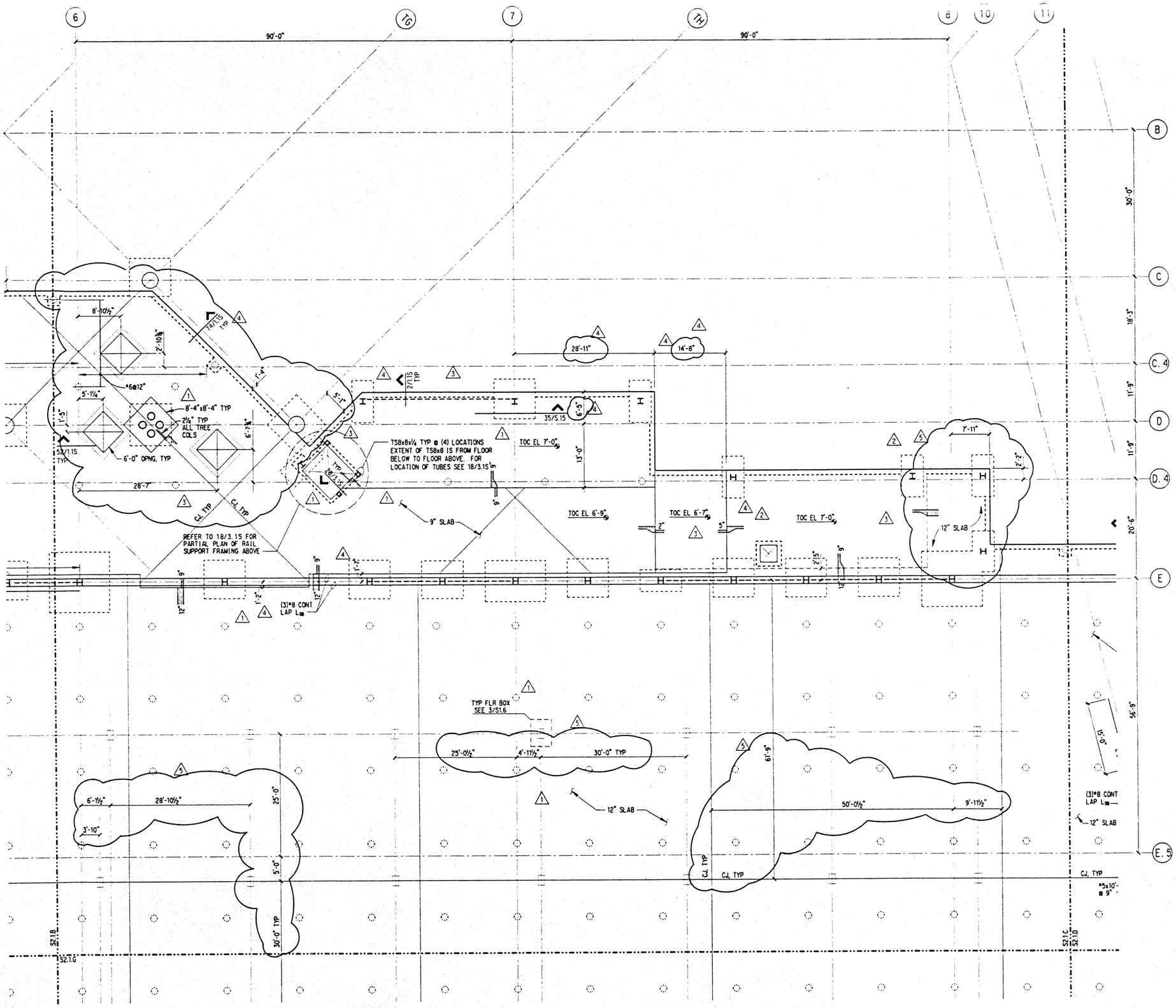

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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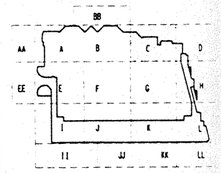
Supervision as defined in Title 16 Chapter 16 Subpart 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.C

Mon Dec 4 12:16:04 1995
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NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Honolulu, Hawaii

in association with

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Seattle, Washington

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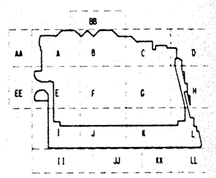
Supervision as defined in Title 16 Chapter 15 Section 1-3-4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR D

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.D

Sat Dec 2 11:18:22 1995

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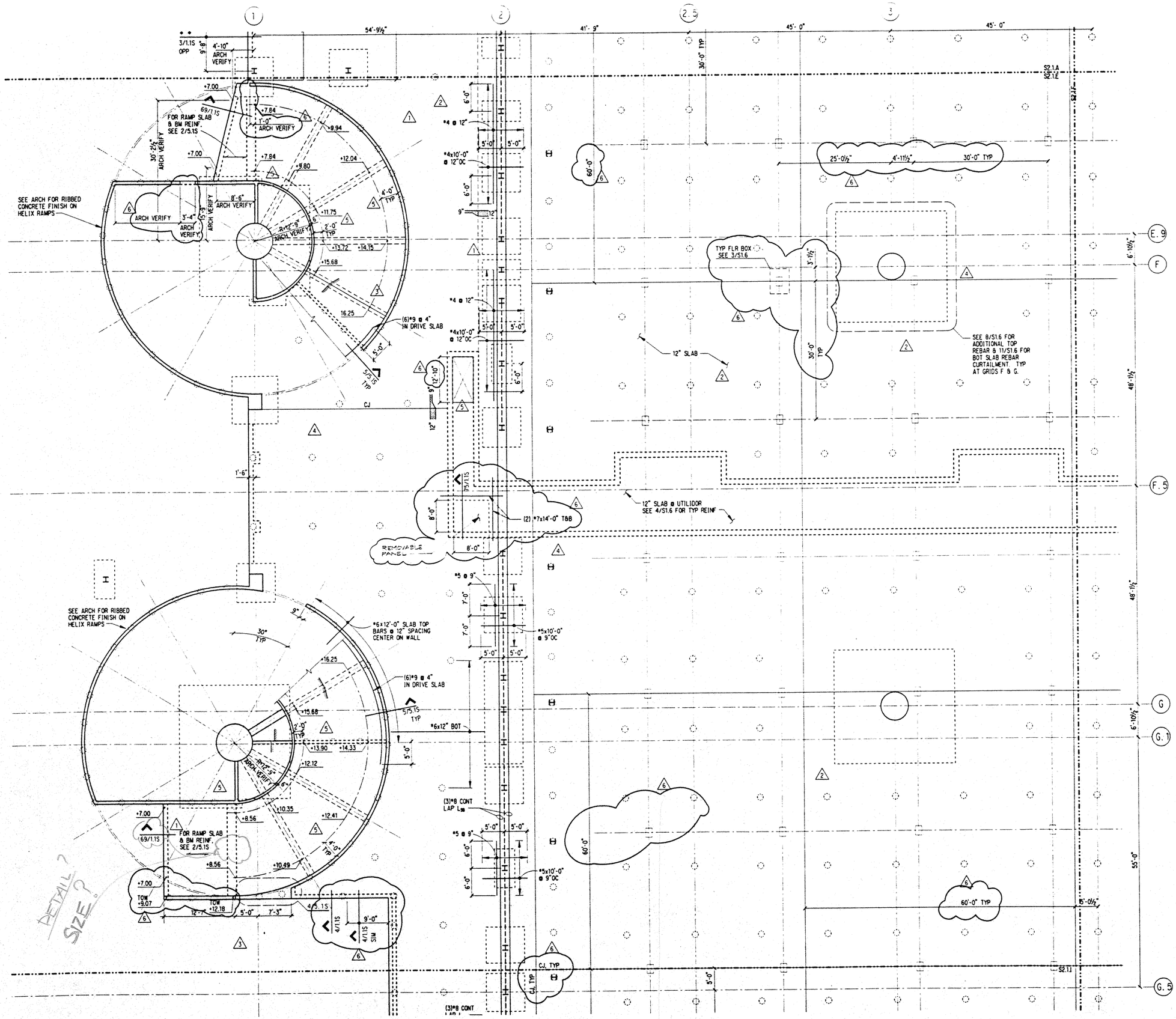

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Honolulu, Hawaii
in association with
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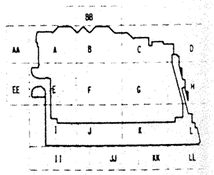
Supervision is defined in Title 16 Chapter 18 Subpart 3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR E

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.E

Sat Dec 2 11:20:39 1995

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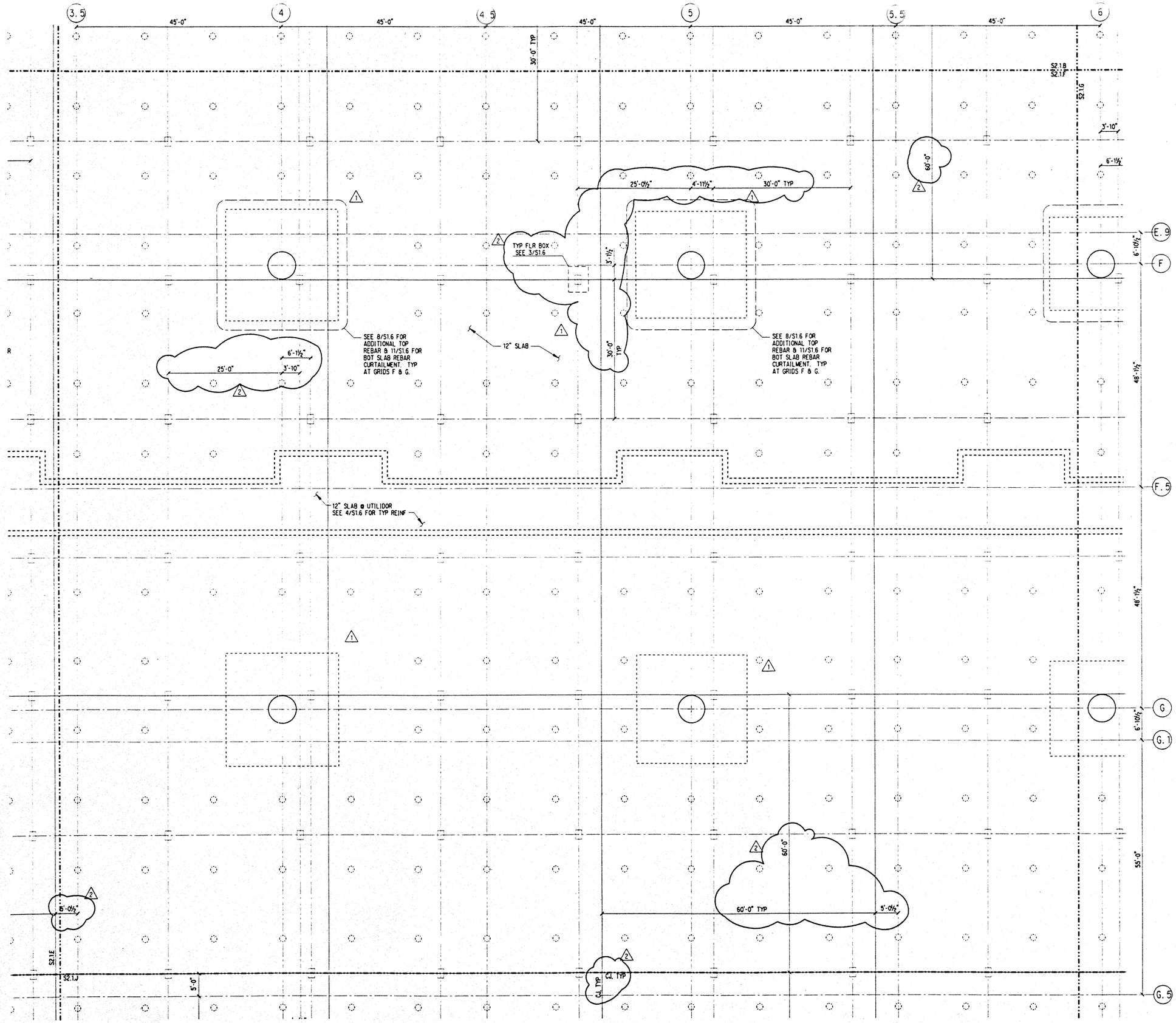

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

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Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

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NO. 7177S
HAWAII, U.S.A.

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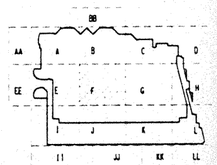
Supervision defined in Title 16 Chapter 8 Section 1-2.04 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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No.	Date	By	Description
2	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.F

Sat Dec 2 11:22:44 1995

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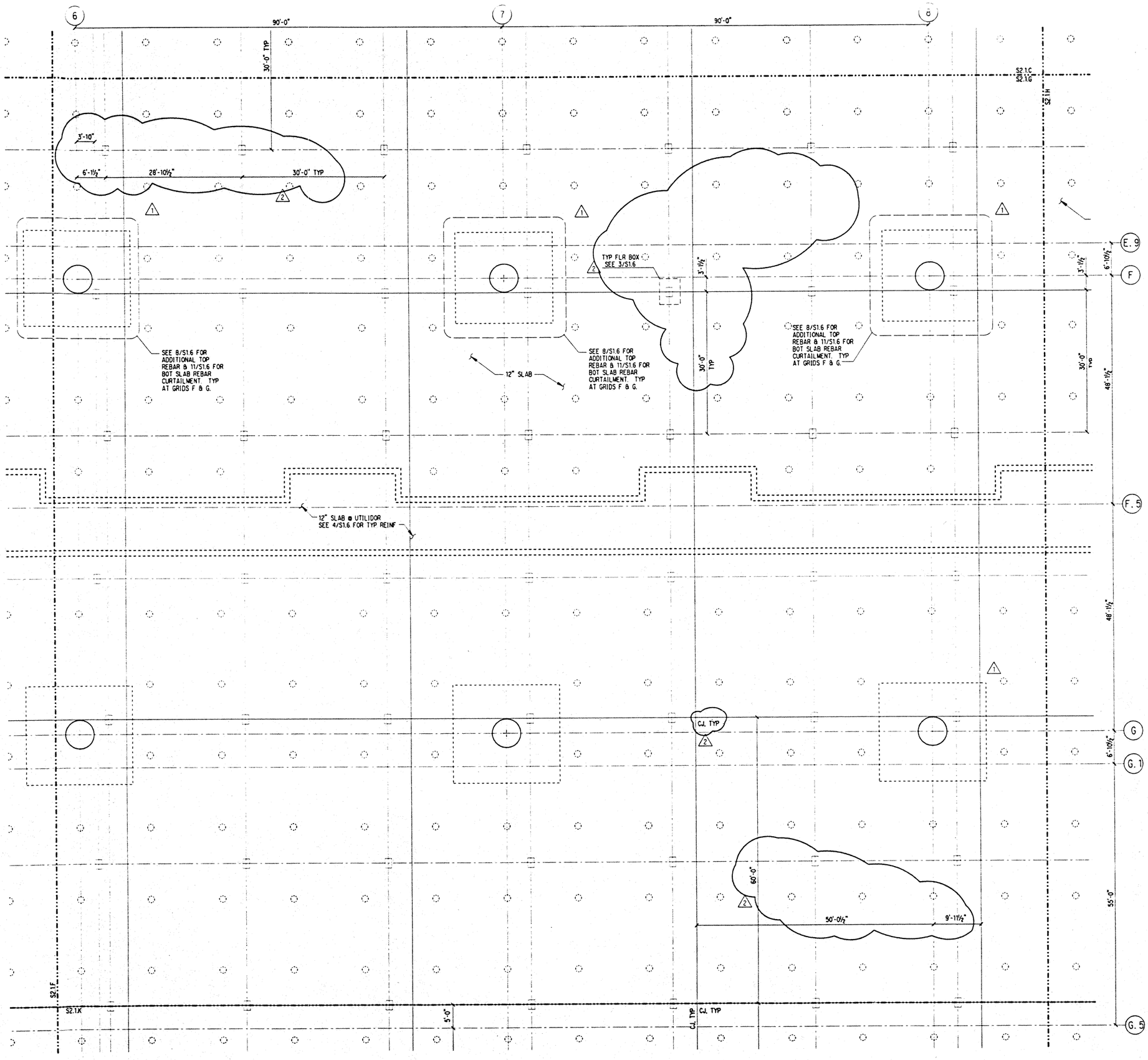

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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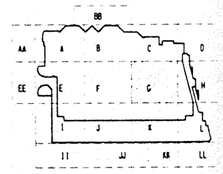
This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title III Chapter 16 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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No.	Date	By	Description
2	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR G

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.G

Sat Dec 2 11:43:56 1995

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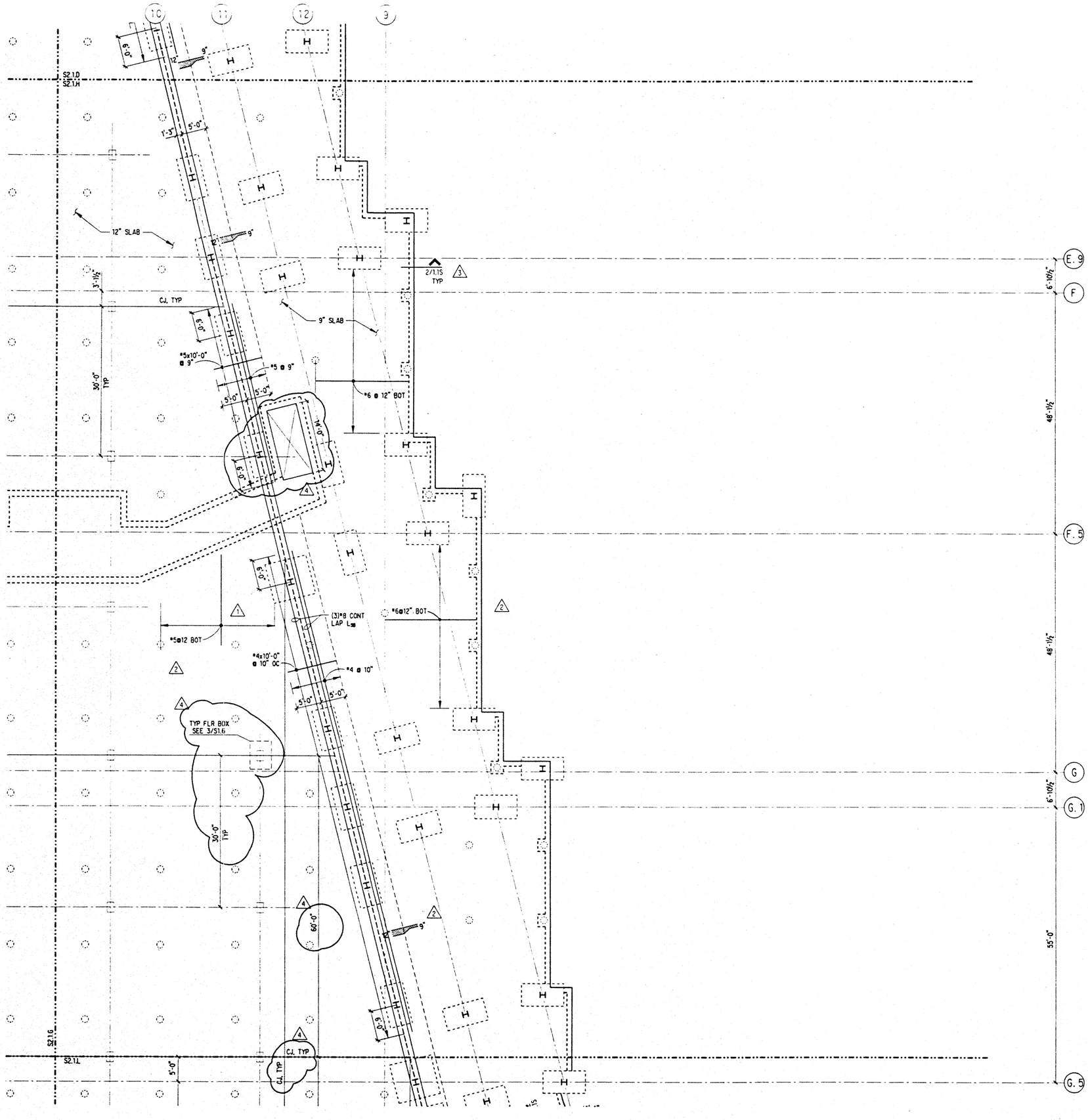
vs pt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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HAWAII, U.S.A.

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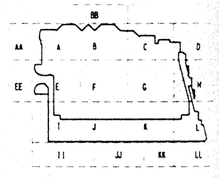
Supervision as defined in Title 16 Chapter 18 Section 1.2 in of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
4	12-15-95		FINAL DOCUMENT
3	11-7-95		
	9-15-95		BUILDING PERMIT SET
2	8-31-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

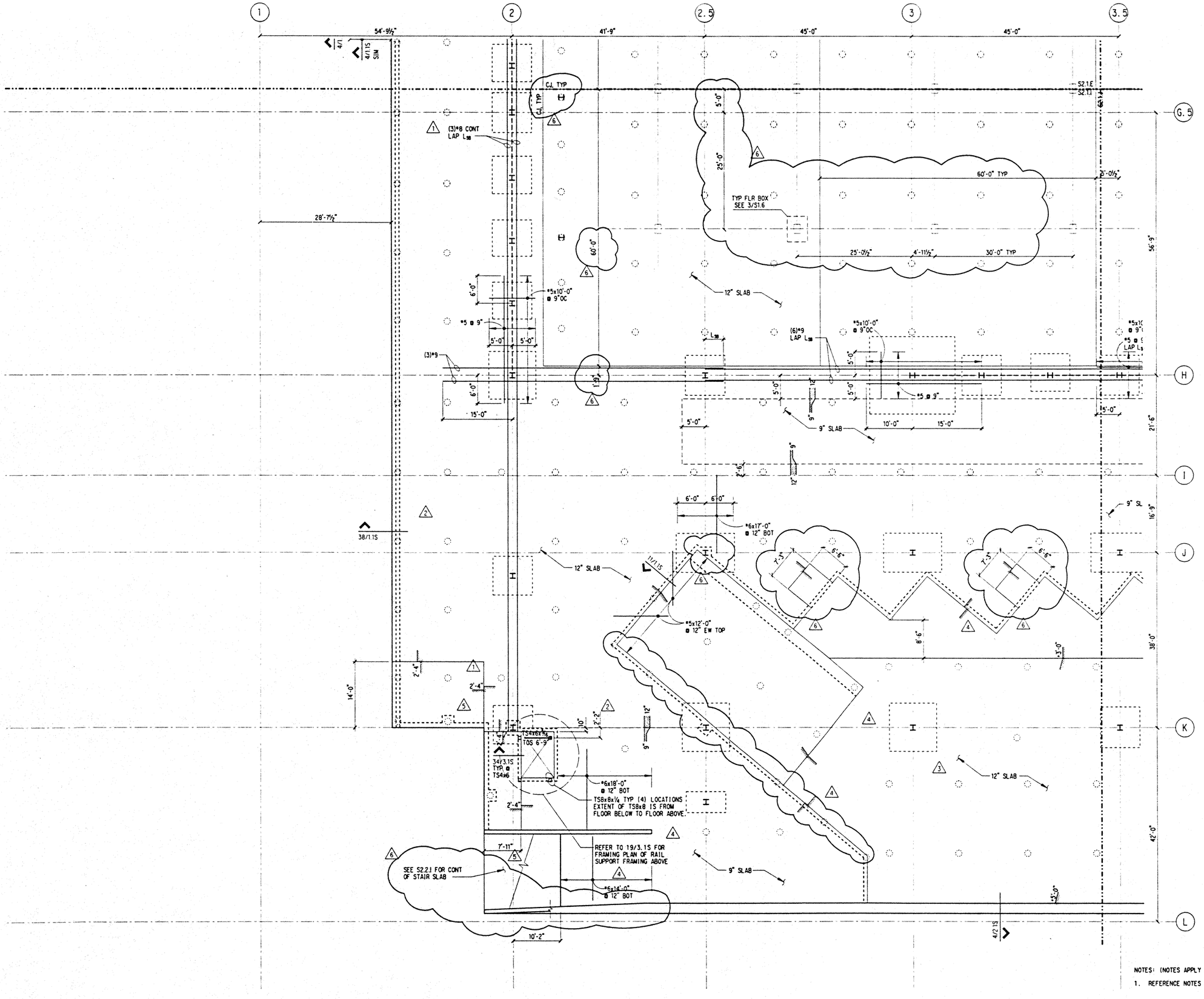
EXHIBIT HALL
SLAB PLAN
SECTOR H

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.H

Sat Dec 2 11:27:28 1995 jthccexhib.h.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 7777S
JOHN D. MAGNUSSON
HAWAII, U.S.A.

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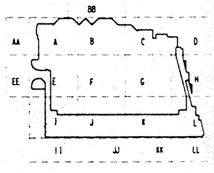
Supervised, administered in Title 16 Chapter 18 Regulation 1.2(a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-11-95		
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

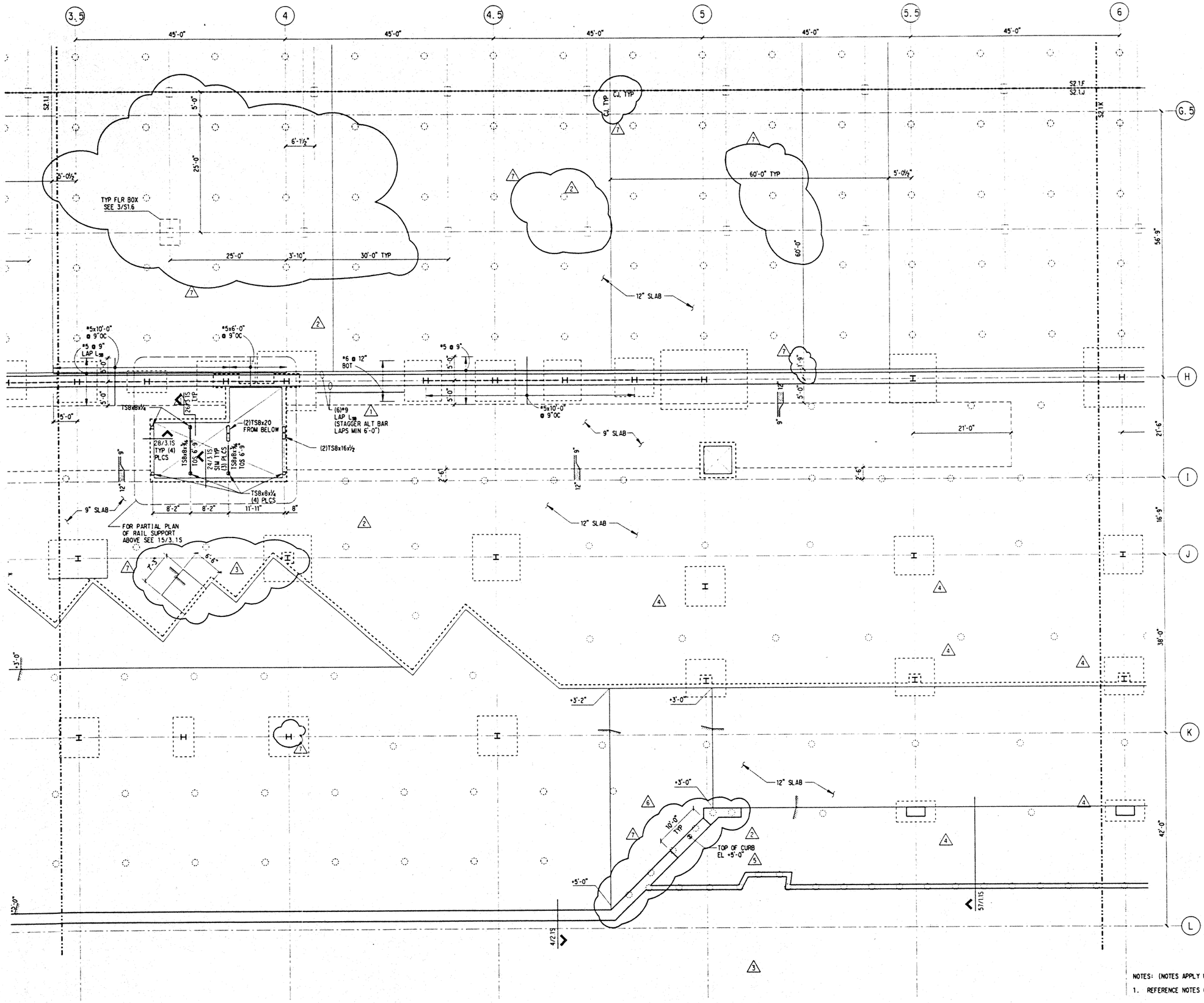
EXHIBIT HALL
SLAB PLAN
SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.I

Sat Dec 2 11:29:32 1995
jyhccexhlb.i.dgn
DMS pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

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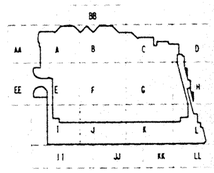
Supervision is performed in Title 16 Chapter 8 Section 1-29 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

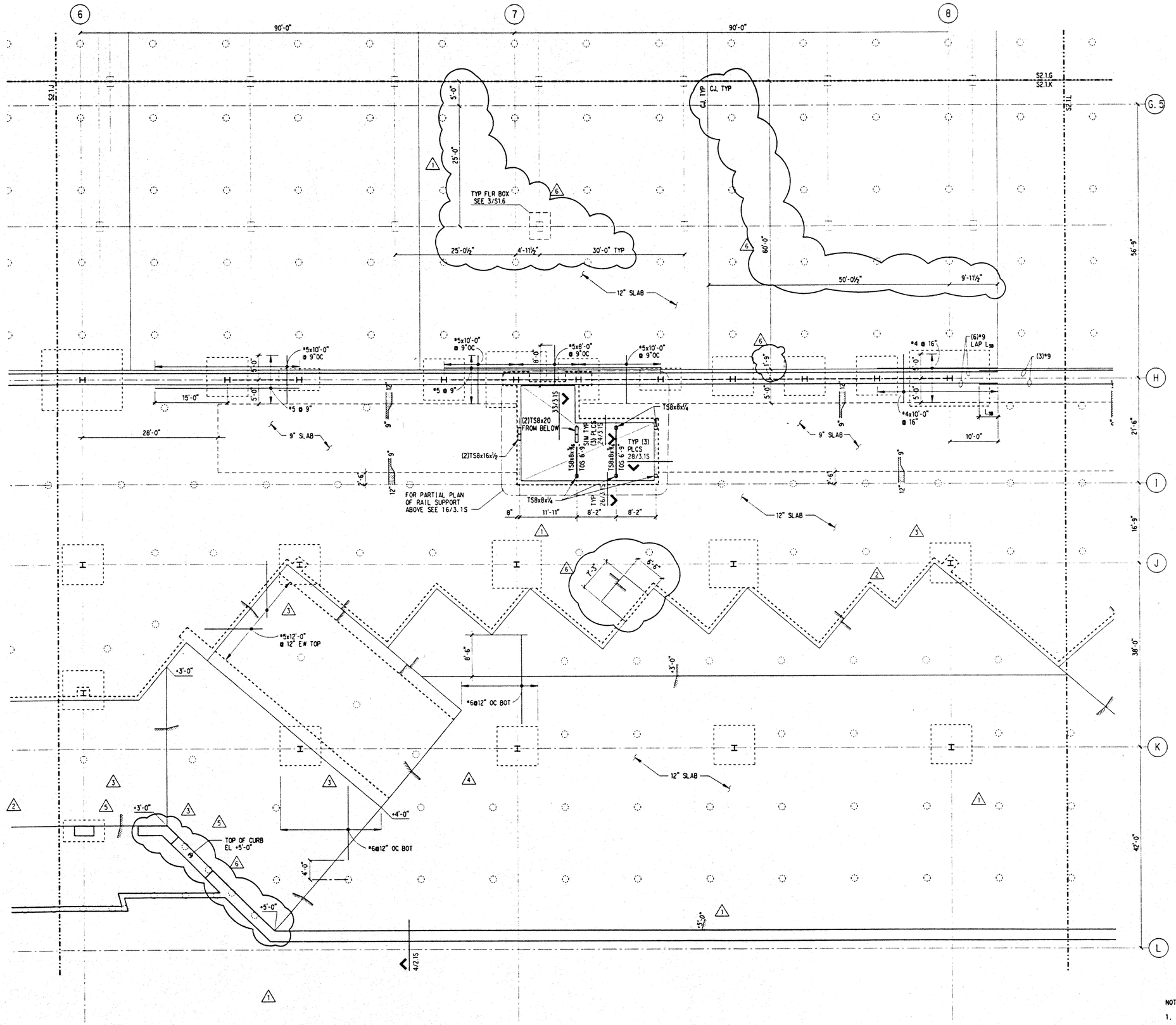
Sheet No.

S2.1.J

Sat Dec 2 11:31:49 1995

j:\hcc\exhib_1.dgn

pl1



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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in association with
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JOHN D. MAGNUSSON
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NO. 7777S
HAWAII, U.S.A.

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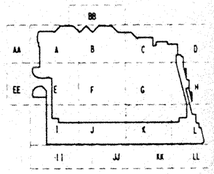
Supervision as defined in Title 16 Chapter 16/Section 1.2 Rule of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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3	8-11-95		
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL
SLAB PLAN
SECTOR K

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.1.K

Sat Dec 2 11:34:05 1995
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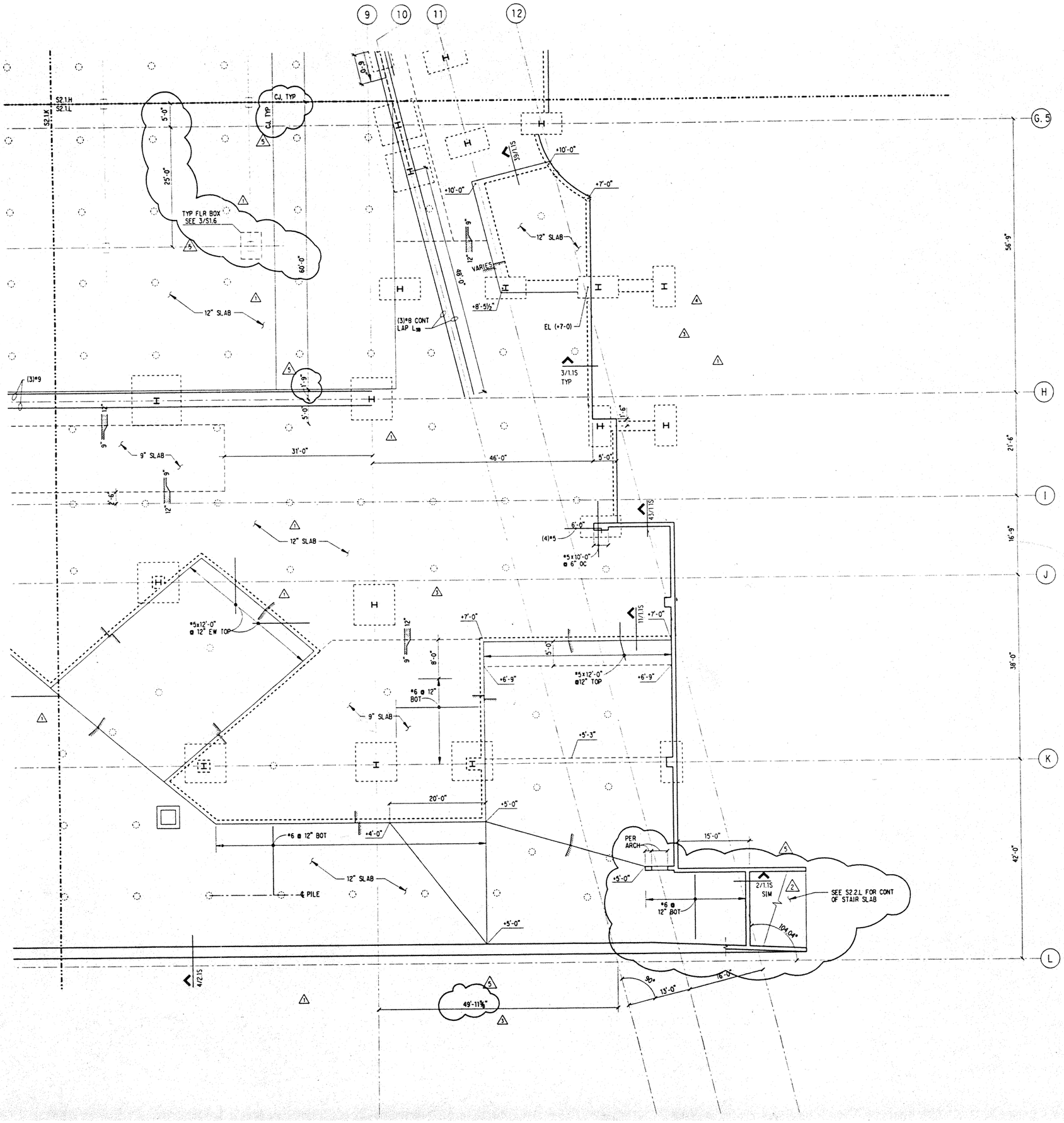

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.1.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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(206) 292-1200 FAX: (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII, U.S.A.

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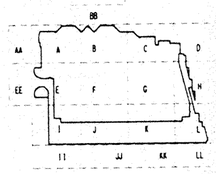
Surveyed in accordance with Title 16 Chapter 97 Section 1-29 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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3	8-31-95		
2	8-4-95		
1	6-19-95		
	4-17-95		ISSUED FOR FOUNDATION PERMIT

Sheet Title

EXHIBIT HALL SLAB PLAN SECTOR L

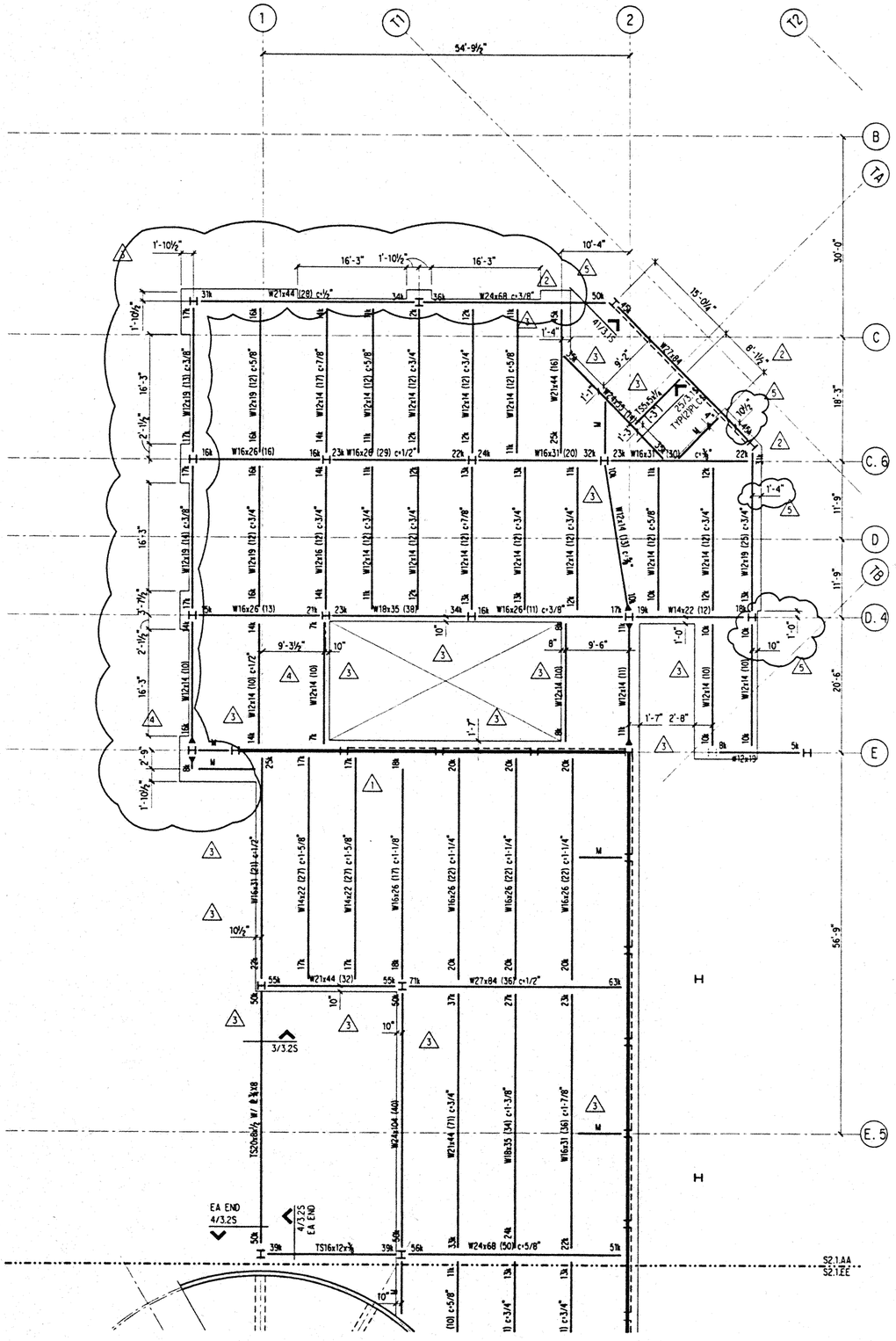
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.L

Sat Dec 2 11:36:22 1995

jkhcc\exhlb1.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A
2. REFERENCE FLOOR ELEVATION = 19'-0"

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

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Consultants

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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
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HAWAII, U.S.A.

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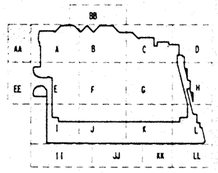
Supervision as defined in Title 16 Chapter 18 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

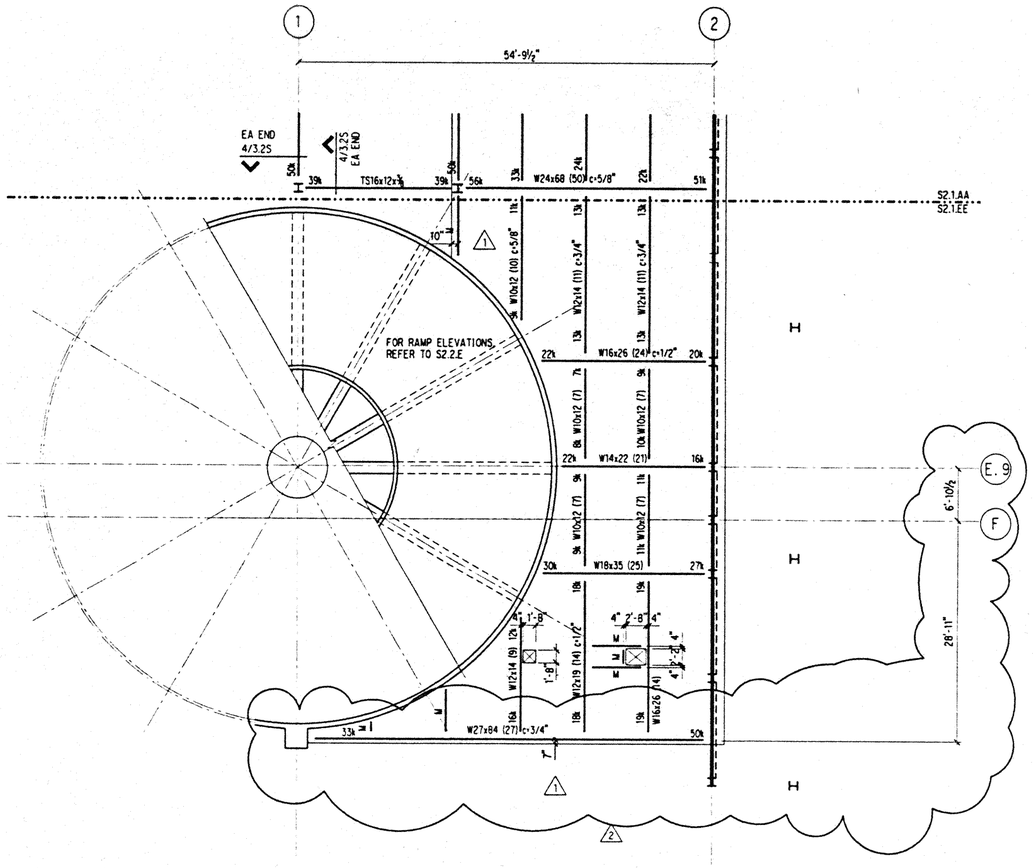
ADMINISTRATION
FRAMING PLAN
SECTOR AA

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.AA

DWS plt j:\hccmezz.aa.dgn Sat Dec 2 09:01:32 1995



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A
2. REFERENCE FLOOR ELEVATION = 19'-0"

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Honolulu, Hawaii

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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

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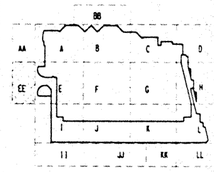
Supervision is defined in Title 16 Chapter 18 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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No.	Date	By	Description
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2	11-7-95		
	9-15-95		BUILDING PERMIT SET
1	8-4-95		

Sheet Title

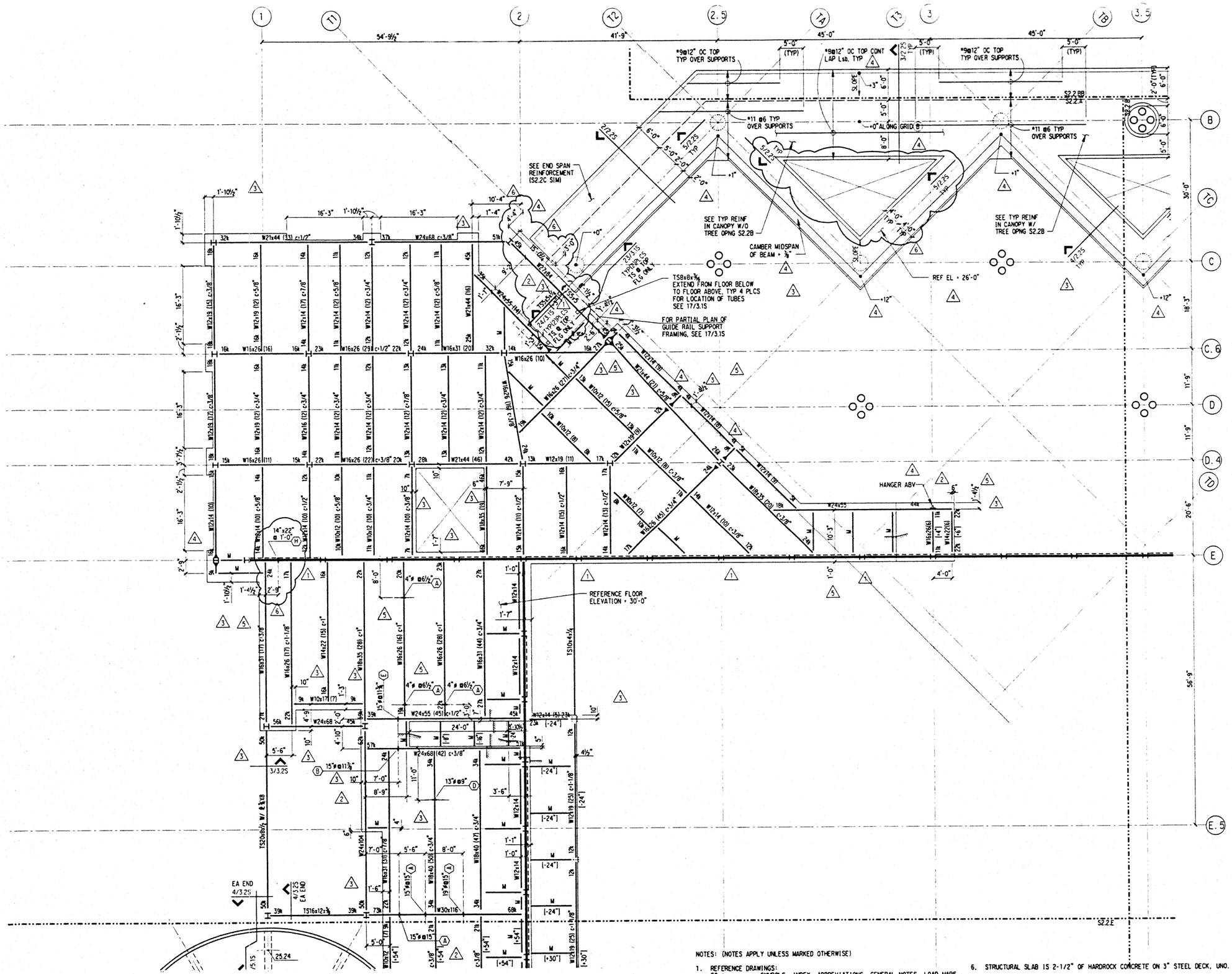
ADMINISTRATION
FRAMING PLAN
SECTOR EE

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.1.EE

DMS plt j:\hcc\mezz_ee.dgn Sat Dec 2 09:03:40 1995



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.• SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.• TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
   - S1.20 CONCRETE SCHEDULES
   - S1.21 S1.22 STEEL SCHEDULES
   - S2.• PLANS
   - S3.• TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
   - S4.• BRACED FRAME ELEVATIONS
   - S5.• TRUSS ELEVATIONS
   - S6.• BRACED FRAME & TRUSS DETAILS
   - DETAIL BOOK STRUCTURAL DETAILS
2. REFERENCE FLOOR ELEVATION = 25'-6"
3. TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
4. REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION, UNO.
5. WHERE NOTED THUS [+/- 8"], TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
6. STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
7. REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
8. "W" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
9. INDICATES BEAM PENETRATION, SEE S1.10
10. INDICATES HAUNCHED BEAM, SEE S1.10
11. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.
12. ORDER OF BAR PLACEMENT AT CANOPY BETWEEN GRIDS 2 & 6.5: (FOR FURTHER INFO SEE 5/2.2S)

BOTTOM BARS	TOP BARS
1. N-S	5. NW-SE ⟨ DIAGONAL BARS-APPLIES
2. E-W & TRANSVERSE DIAGONAL	6. NE-SW TO MAIN REINF ONLY
3. NW-SE ⟨ DIAGONAL BARS-APPLIES	7. E-W & TRANSVERSE DIAGONAL

Hawai'i Convention Center

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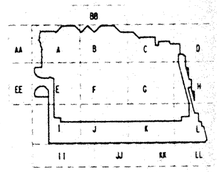
JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7177S
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Signature as defined in Title 4 Chapter 16 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
6	12-15-95		FINAL DOCUMENT
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-14-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

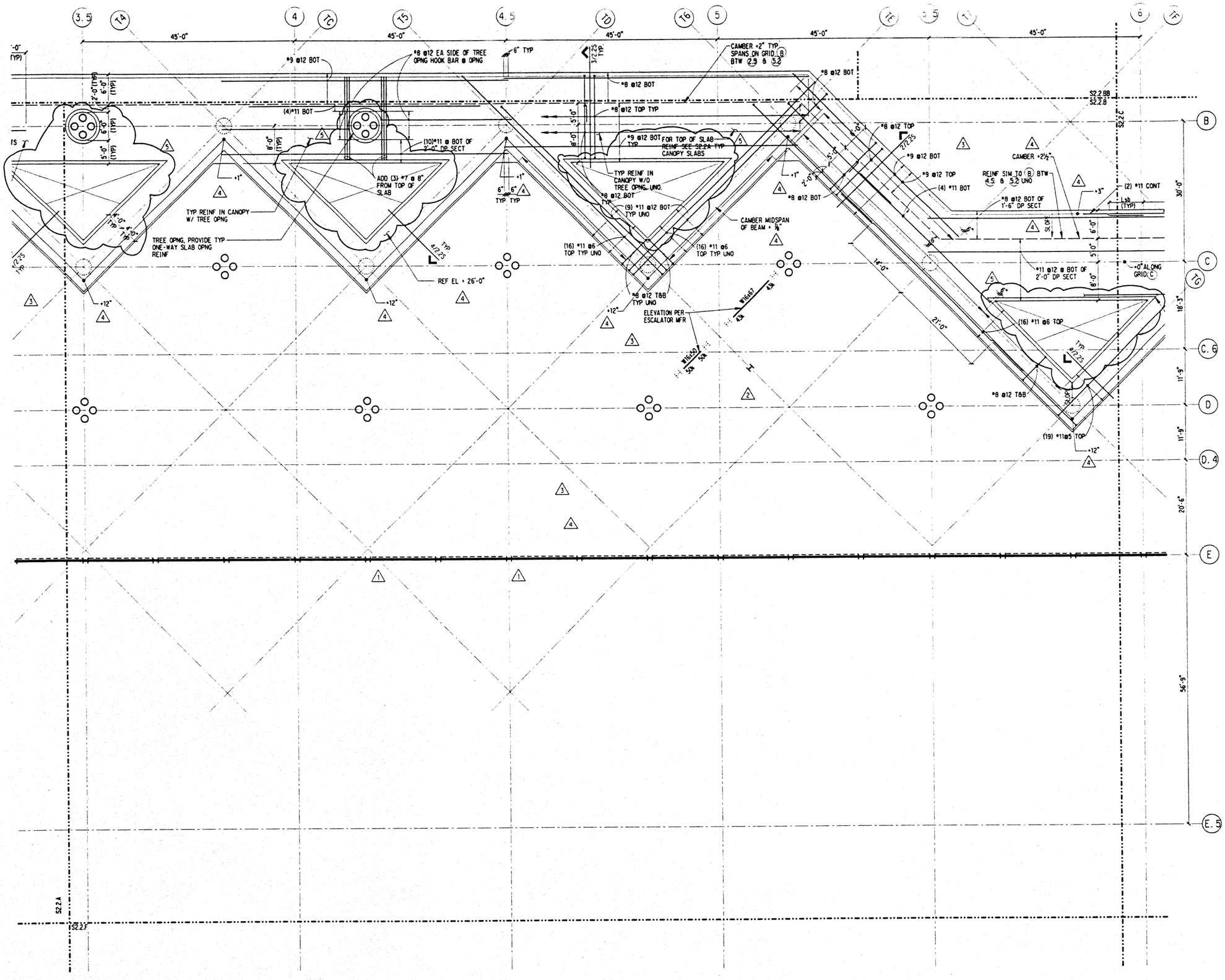
INTERMEDIATE LEVEL FRAMING PLAN SECTOR A

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.2.A

Sat Dec 2 09:05:59 1995 jkhccmezz.o.dgn SWK plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
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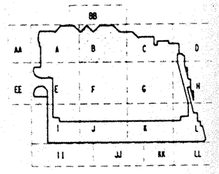
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 7777S
JON D. MAGNUSSON
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision referred to Title III Chapter 16 Section 1-23-4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.2.B

Sat Dec 2 09:08:00 1995
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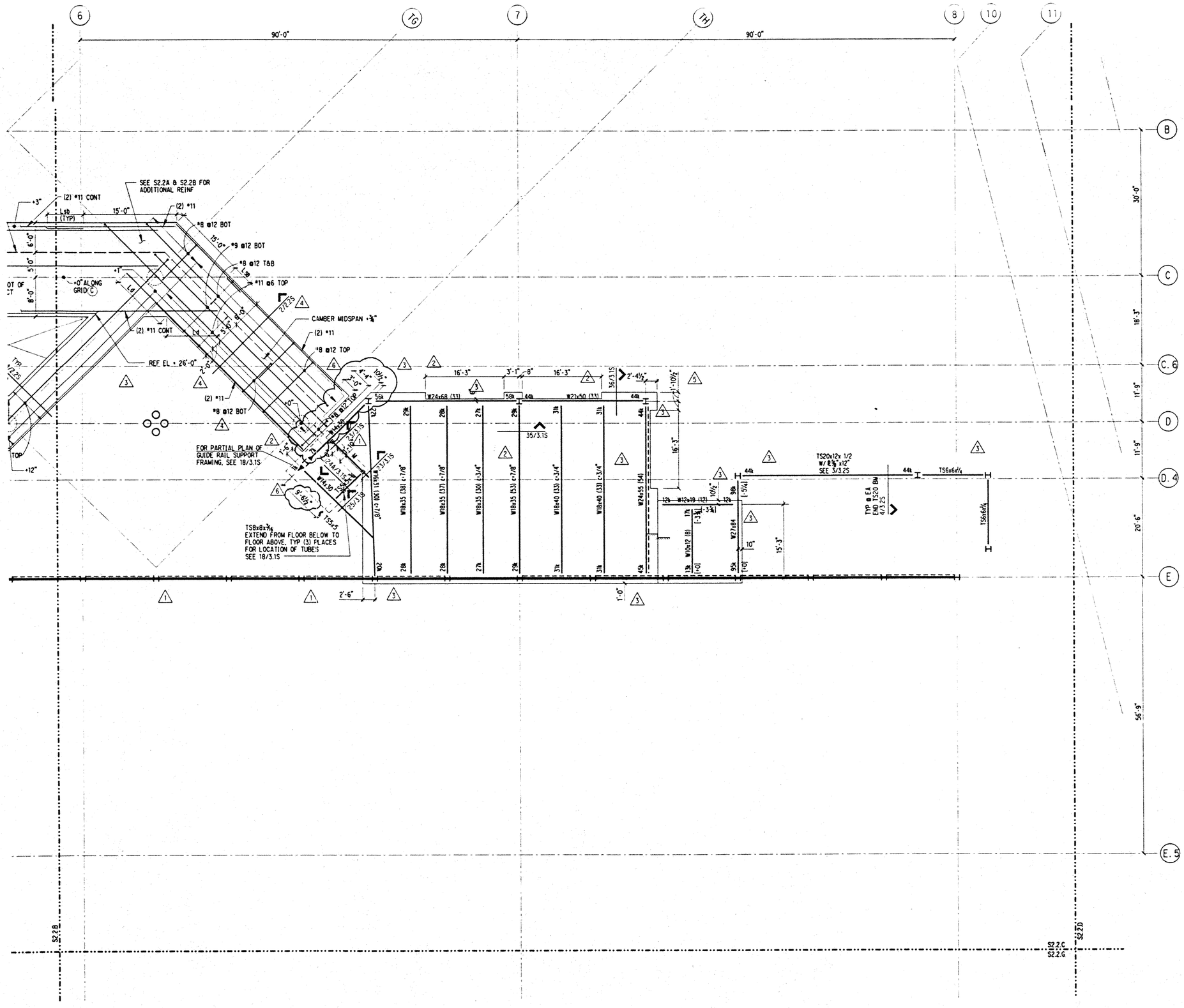
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A
2. REFERENCE FLOOR ELEVATION = 26'-6"

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Honolulu, Hawaii
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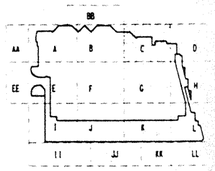
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11773 JON D. MAGNUSSON HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervisor(s) defined in Title 16 Chapter 18 Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.2.C

Sat Dec 2 09:10:17 1995
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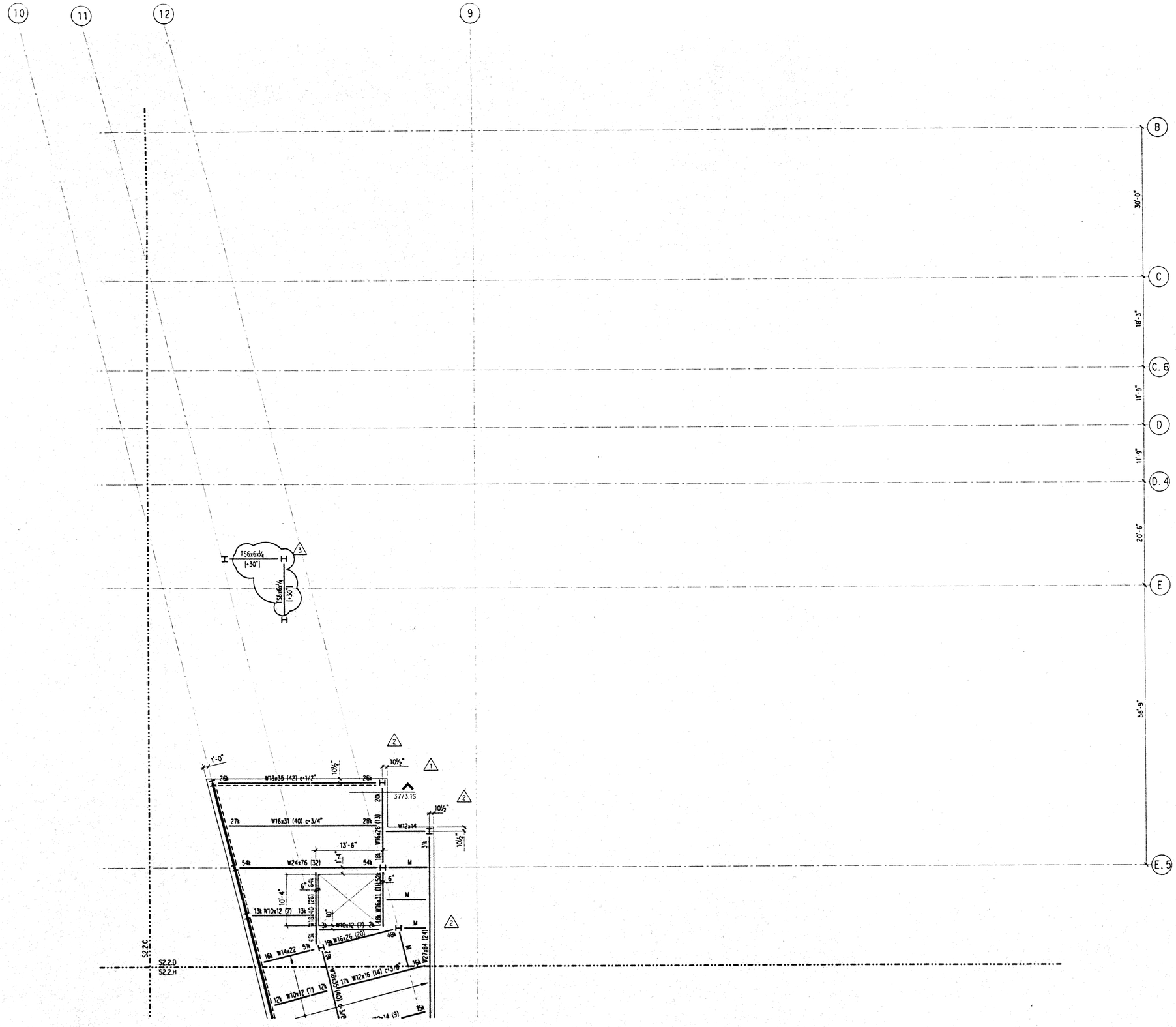
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

2

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC / PCL
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Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
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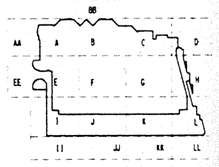
(206) 292-1200 FAX (206) 292-1201

ARI D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Section 1.2 (e) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
3	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR D

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

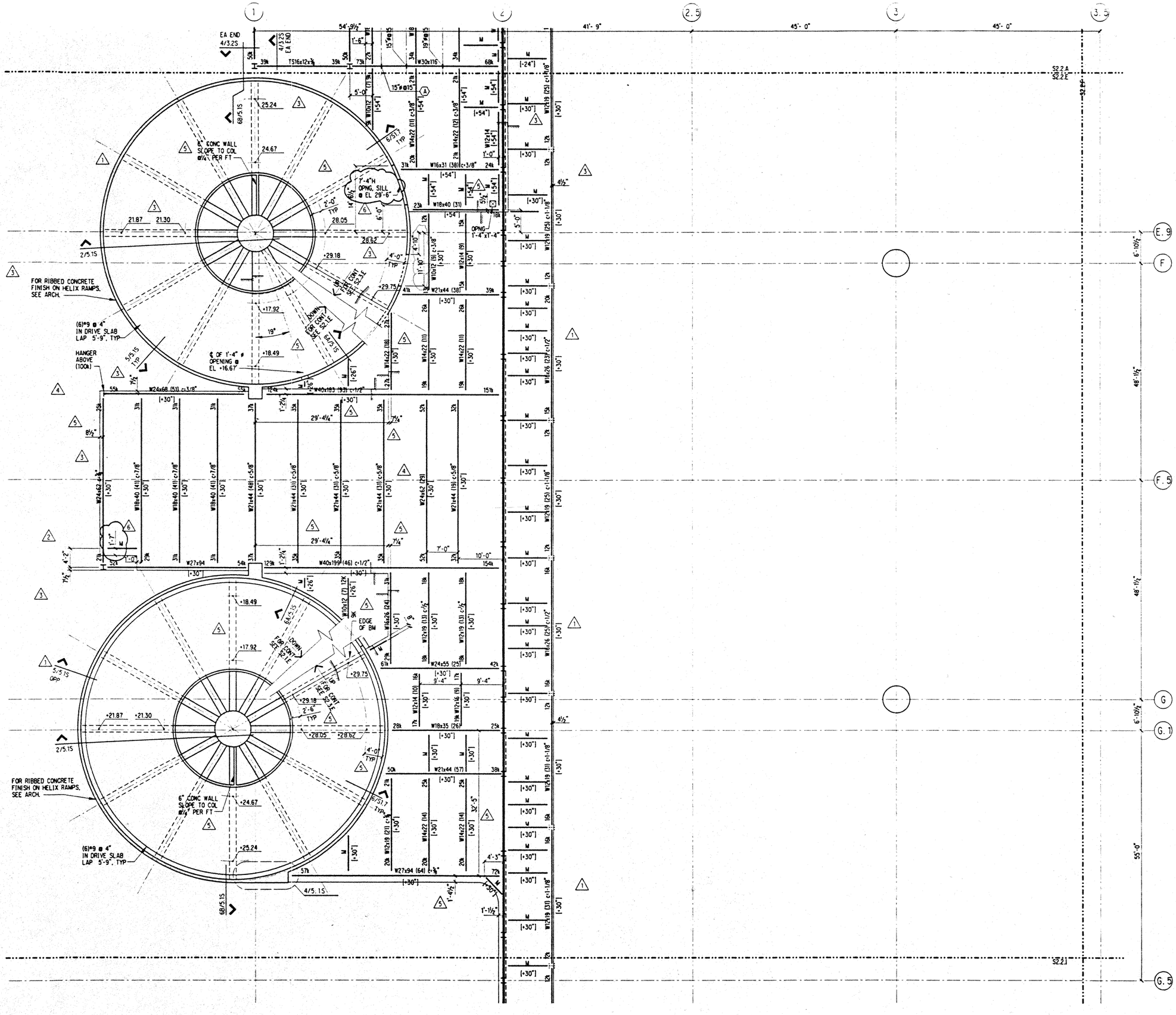
Sheet No.

S2.2.D

Sat Dec 2 09:12:23 1995

j:\hcc\mezz_d.dgn

pl1



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER NO. 11773
HAWAII U.S.A.

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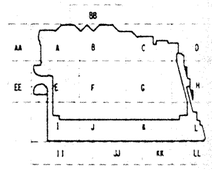
Supervision by licensed in lieu of Chapter 16 Section 1.29 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
6	12-15-95		FINAL DOCUMENT
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR E

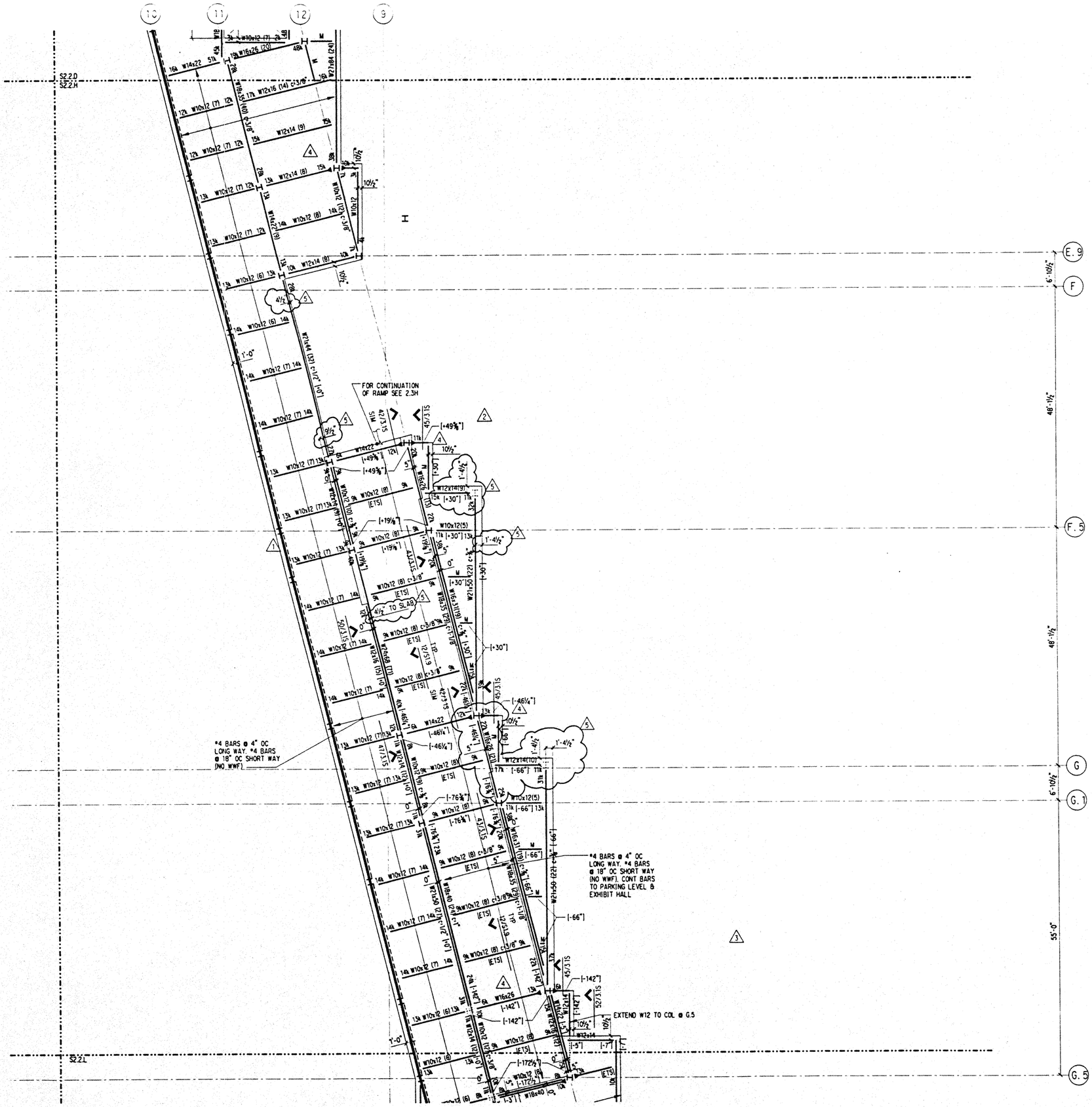
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.2.E

Sat Dec 2 09:14:32 1995

jhccmezz.e.dgn

DMS.plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
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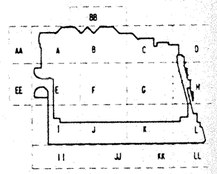
LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer — Title of Owner in Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

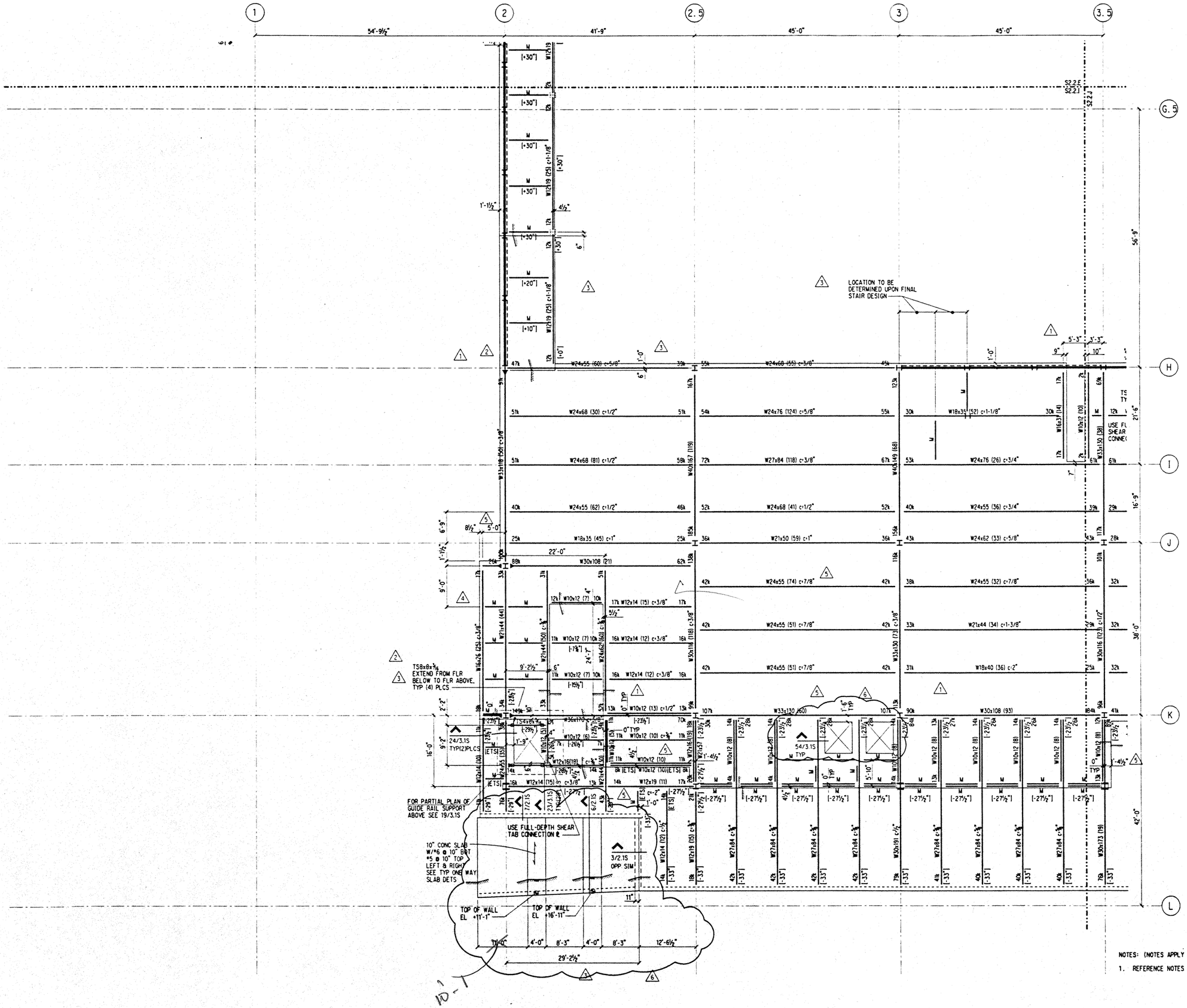
INTERMEDIATE LEVEL FRAMING PLAN SECTOR H

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.2.H

Sat Dec 2 09:16:42 1995 jhncmezz.h.dgn DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

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(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII, U.S.A.

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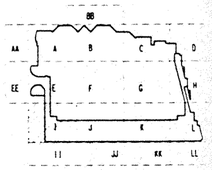
Supervision is required in Title of Chapter 16 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

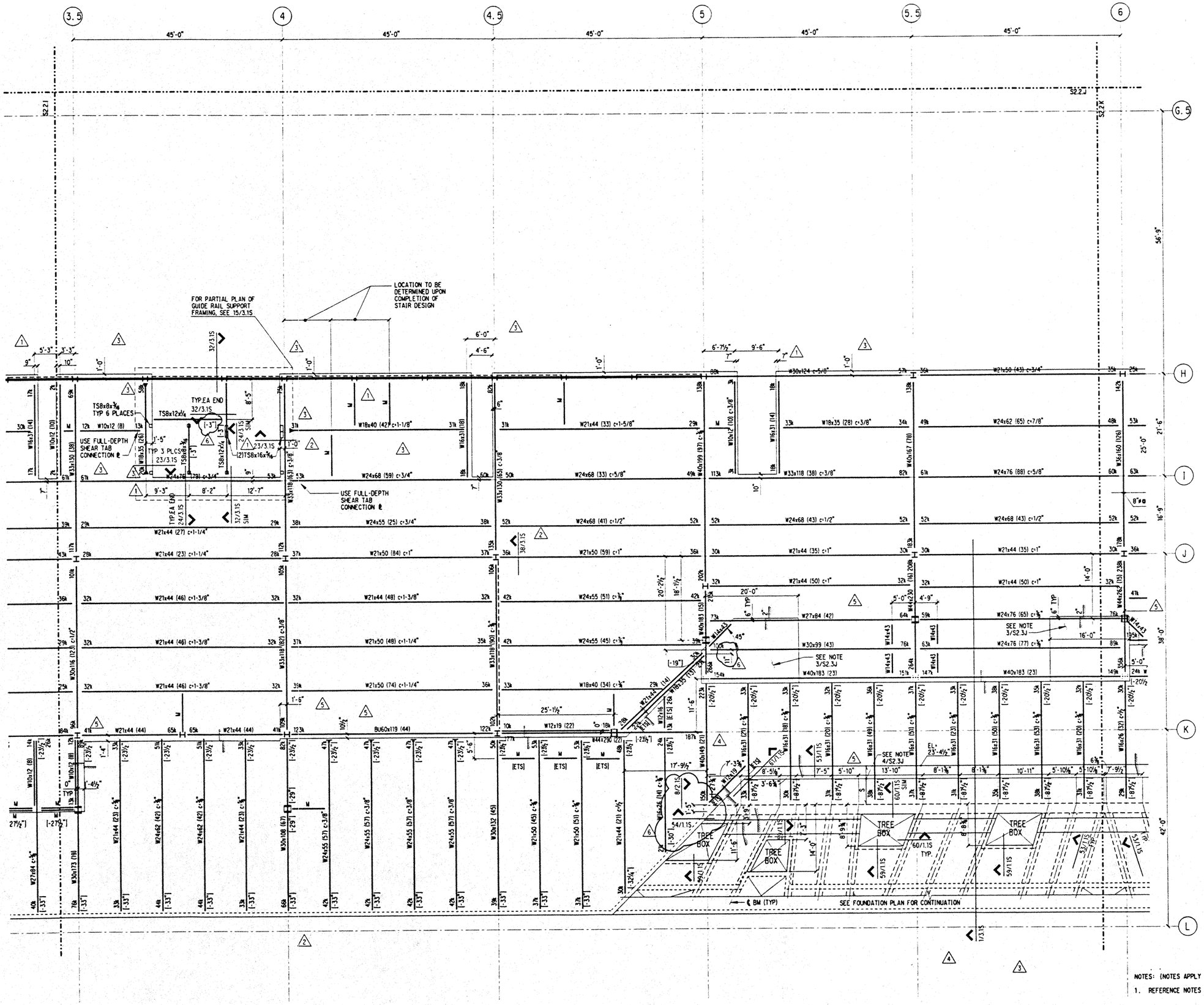
Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.2.I

Sat Dec 2 09:18:42 1995 jtncc\mezz_1.dgn DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
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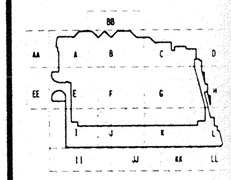
LICENSED PROFESSIONAL ENGINEER NO. 11773 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer pursuant to Title 16 Chapter 18 Subpart 3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

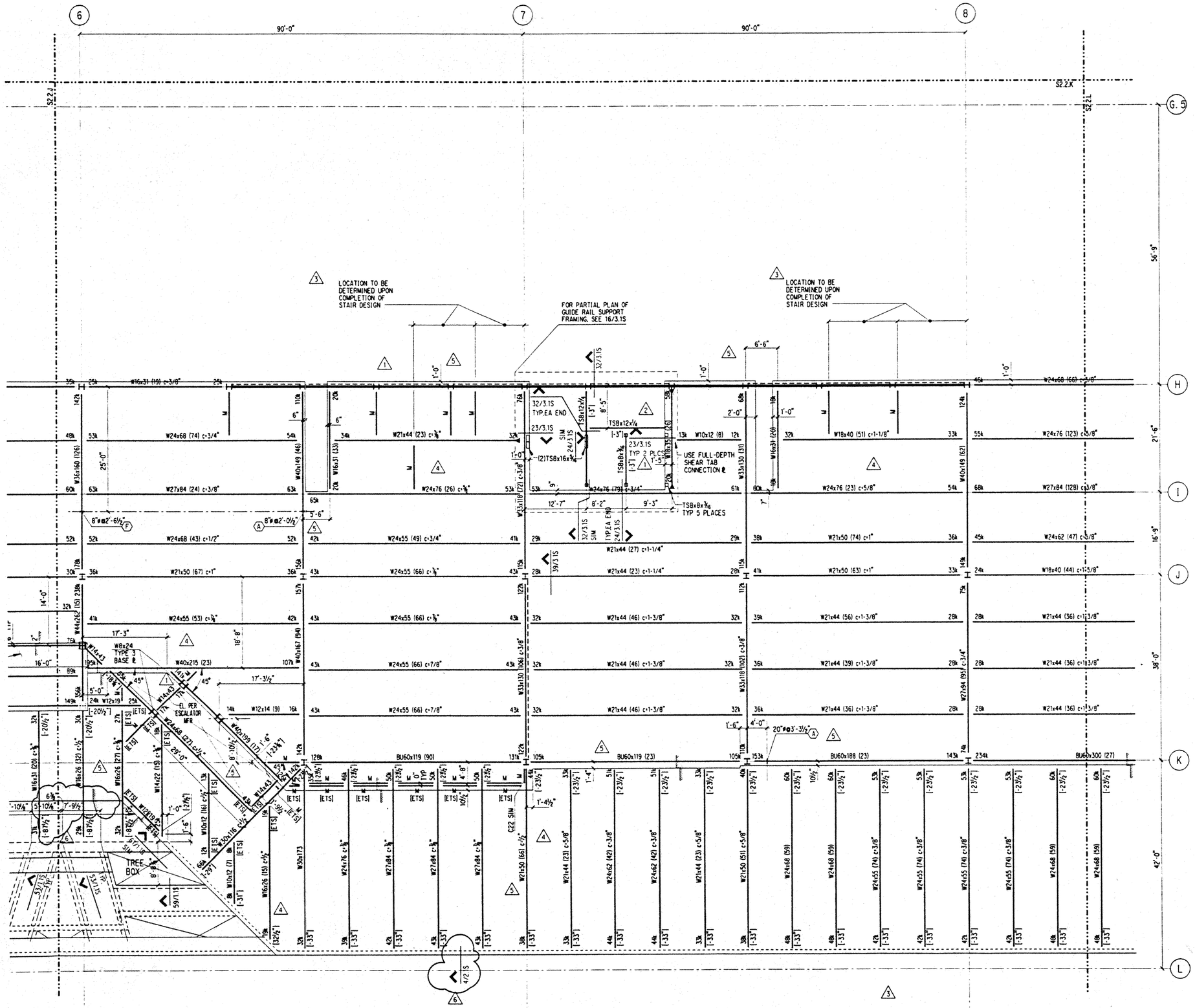
INTERMEDIATE LEVEL FRAMING PLAN SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.2.J

Sat Dec 2 09:20:53 1995 jtnccwmezz.jdgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
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Seattle, Washington

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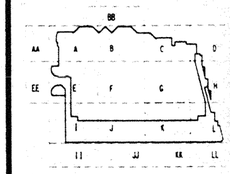
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Jon D. Magnusson

Supervising Engineer as defined in Title 16, Chapter 8, Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR K

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

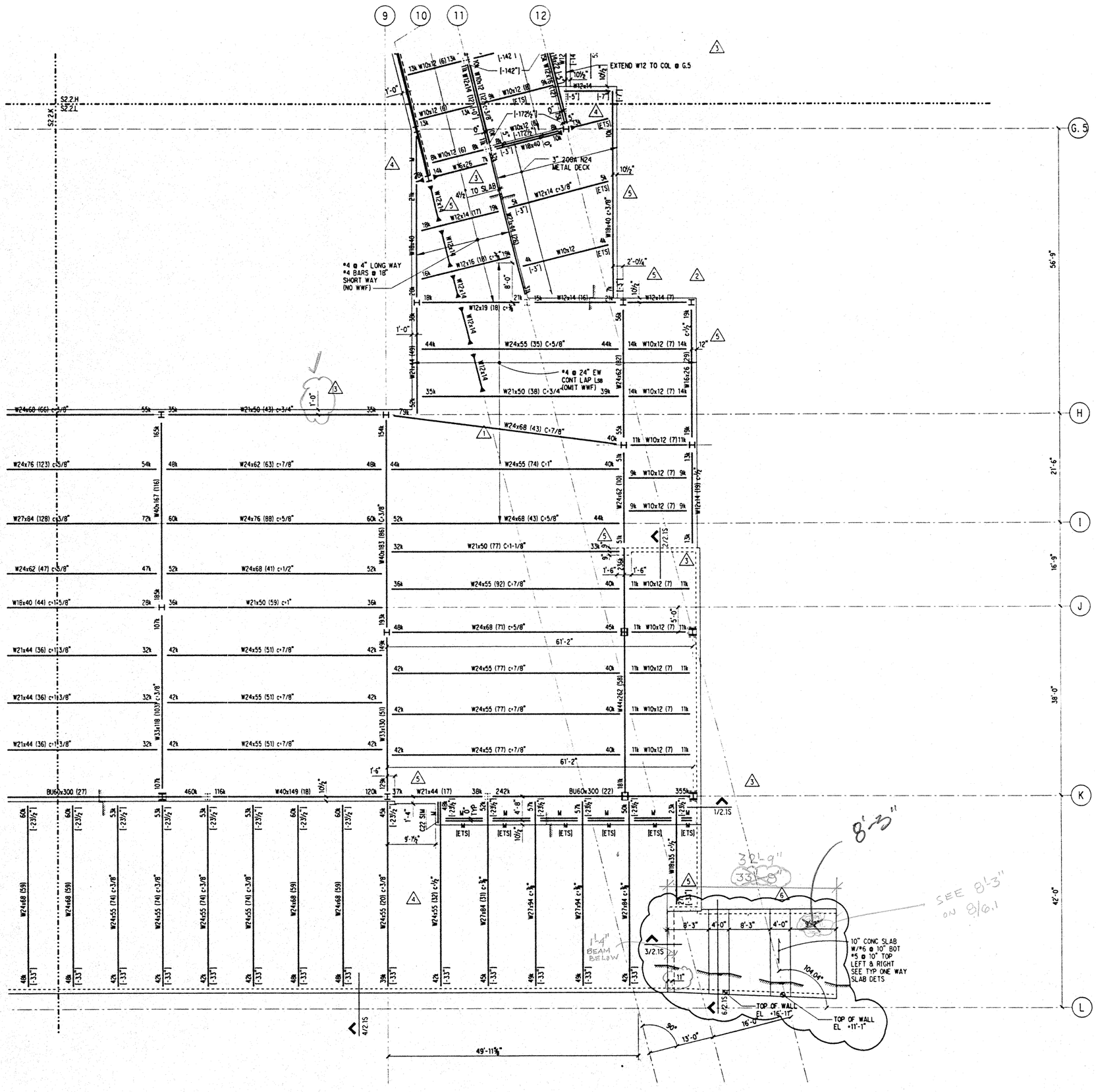
Sheet No.

S2.2.K

Sat Dec 2 09:23:05 1995

J:\hccmezz.k.dgn

pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII U.S.A.

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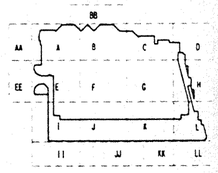
Supervision as defined in Title 16 Chapter 18 Subtitle 1 Part of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	7-28-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

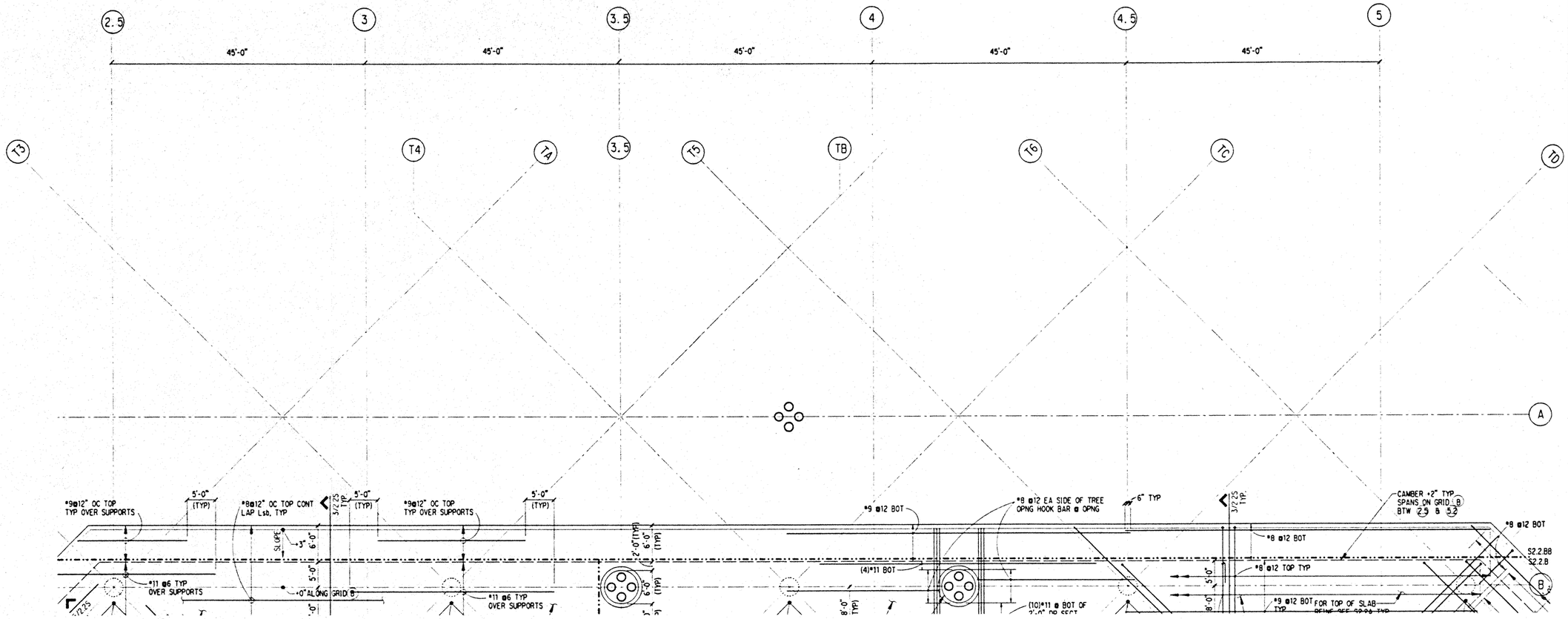
Sheet Title

INTERMEDIATE LEVEL FRAMING PLAN SECTOR L

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.2.L

Sat Dec 2 09:25:17 1995
jrhccvmezz.1.dgn
LMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.2.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
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Honolulu, HI

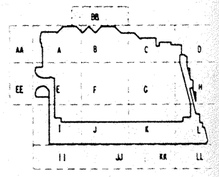
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Licensed to practice in Title 16 Chapter 18 Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

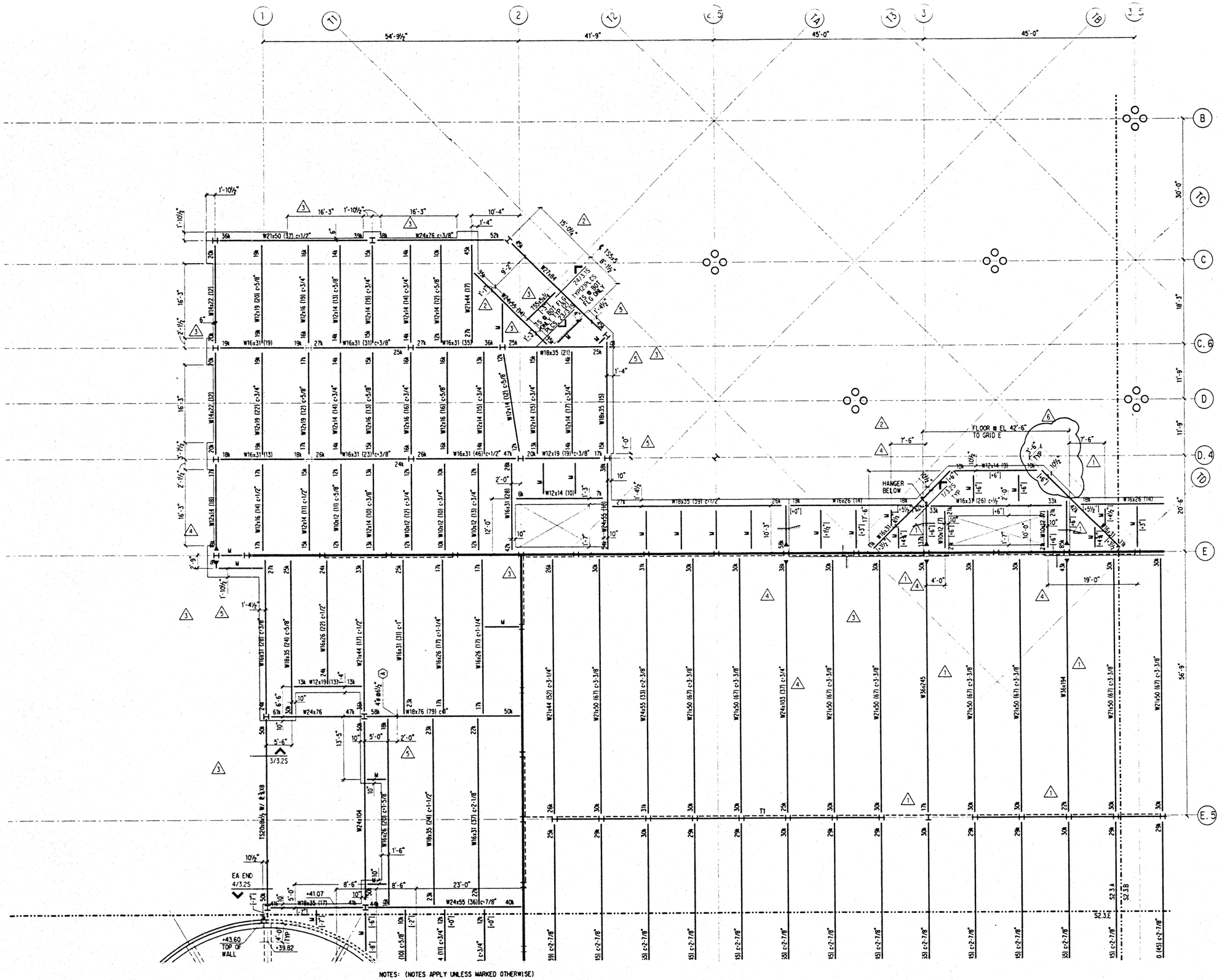
INTERMEDIATE
LEVEL
FRAMING PLAN
SECTOR BB

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.2BB

Sat Dec 2 09:27:27 1995
j:\hcc\mezz.bb.dgn
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.• SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.• TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
   - S1.20 CONCRETE SCHEDULES
   - S1.21 S1.22 STEEL SCHEDULES
   - S2.• PLANS
   - S3.• TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
   - S4.• BRACED FRAME ELEVATIONS
   - S5.• TRUSS ELEVATIONS
   - S6.• BRACED FRAME & TRUSS DETAILS
   - DETAIL BOOK STRUCTURAL DETAILS
2. REFERENCE FLOOR ELEVATION = 42'-0"
3. TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
4. REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE
5. WHERE NOTED THUS [+/- 8"], TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
6. STRUCTURAL SLAB IS 2-½" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO. STEEL DECK SHALL HAVE A ZINC COATING CONFORMING TO ASTM A525-G90.
7. REINFORCE SLAB WITH #4 @ 18"OC EACH WAY, UNO. 1
8. "M" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
9. INDICATES BEAM PENETRATION, SEE S1.10
10. INDICATES HAUNCHED BEAM, SEE S1.10
11. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION
12. SEE DETAIL 1/2.2S FOR THICKENED CONCRETE ALONG TRUSS/FRAME LOCATIONS, OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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(206) 292-1200 FAX (206) 292-1201

JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

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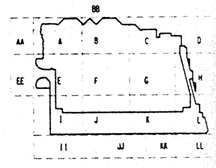
Supervising Engineer defined in Title 16 Chapter 18 Subpart 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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No.	Date	By	Description
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4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

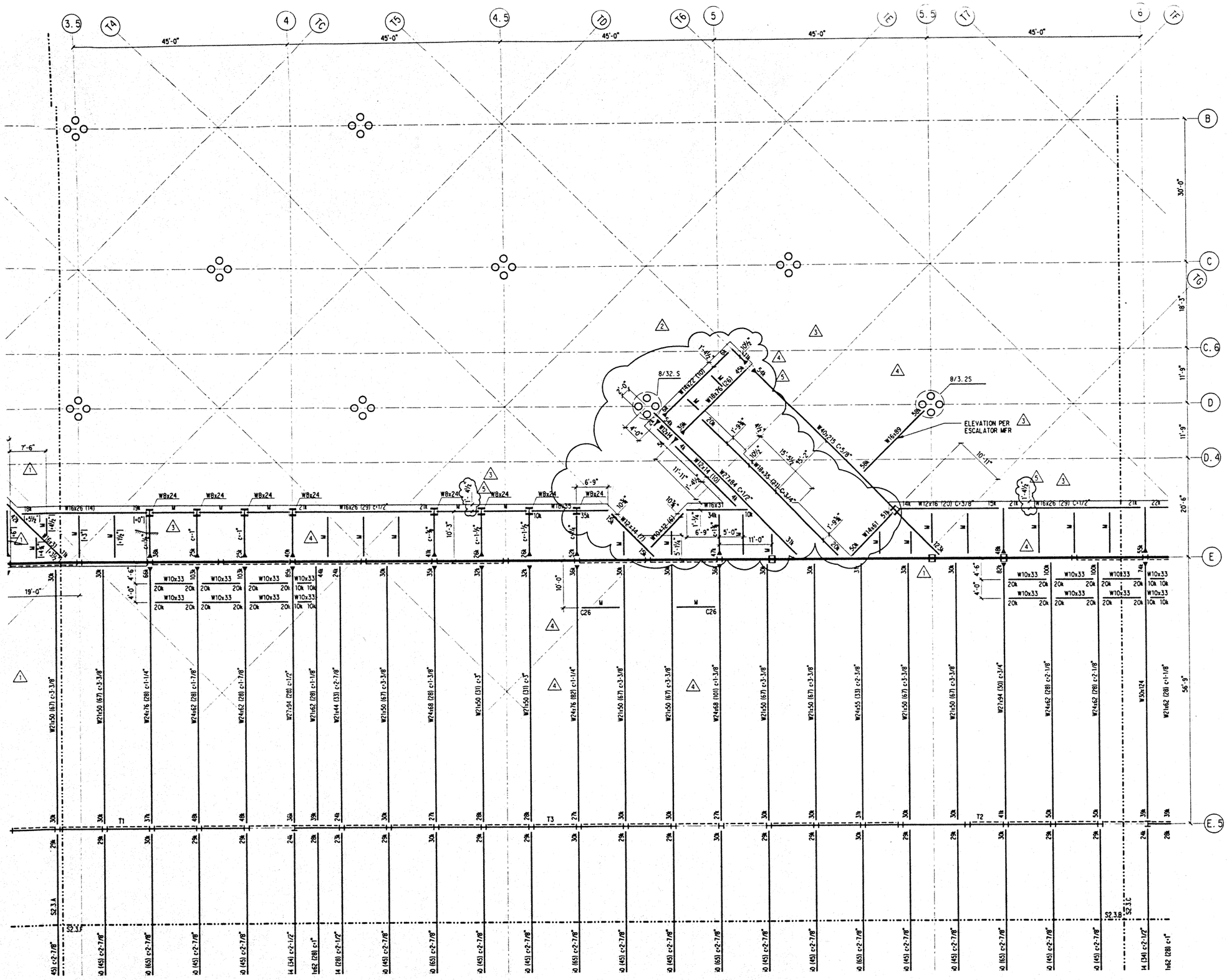
Sheet Title

PARKING
FRAMING PLAN
SECTOR A

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.3.A

Fri Dec 1 08:36:33 1995 jhncpark.a.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

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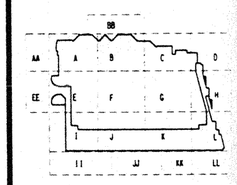
Supervising Engineer as defined in Title III Chapter 16 Subpart 1 Part of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

PARKING
FRAMING PLAN
SECTOR B

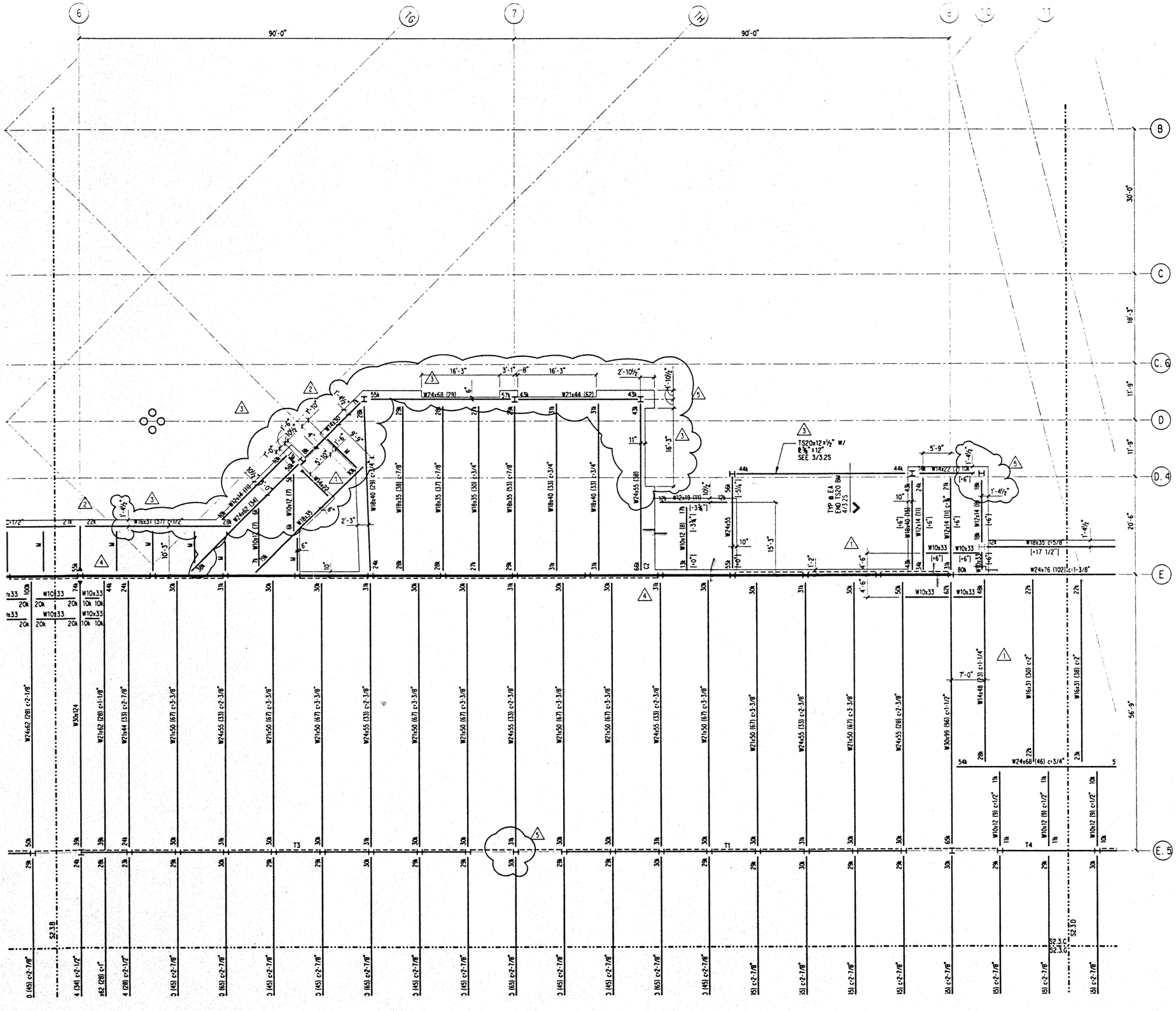
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.3.B

FriDec 10 8:38:44 1995

jkhccpark.b.dgn

DWS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A

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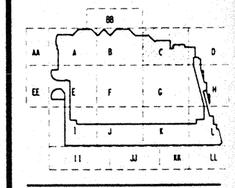
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 7777S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer Approved as Filed in Chapter 16 Subtitle 1, D.A.R. of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

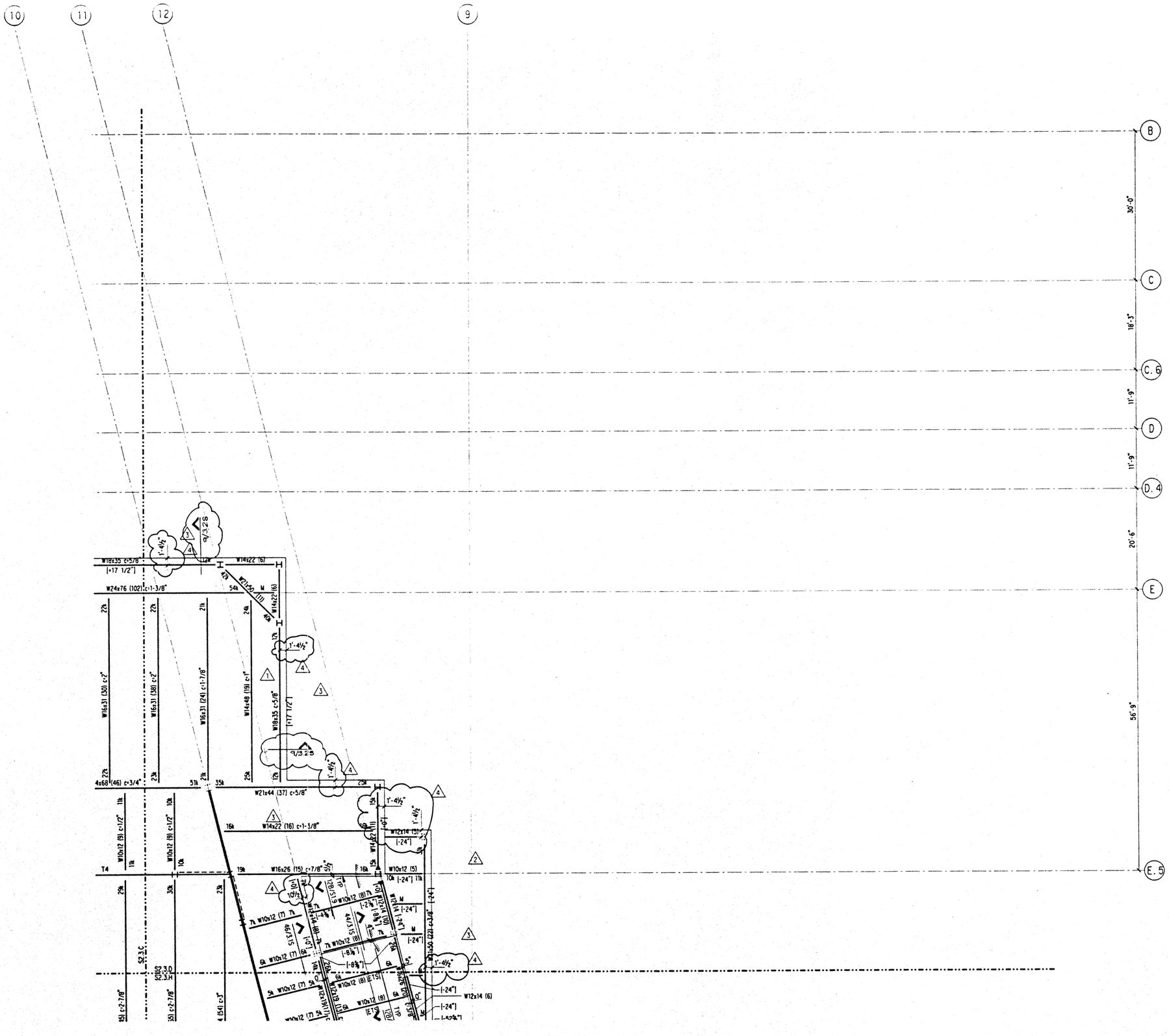
Sheet Title

PARKING
FRAMING PLAN
SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.3.C

DMS pit Fri Dec 1 08:40:55 1995 hhccpark.c.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
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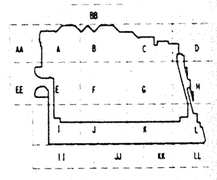
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11173
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervised by licensed in Title 16 Chapter 18 Section 1.3 in of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

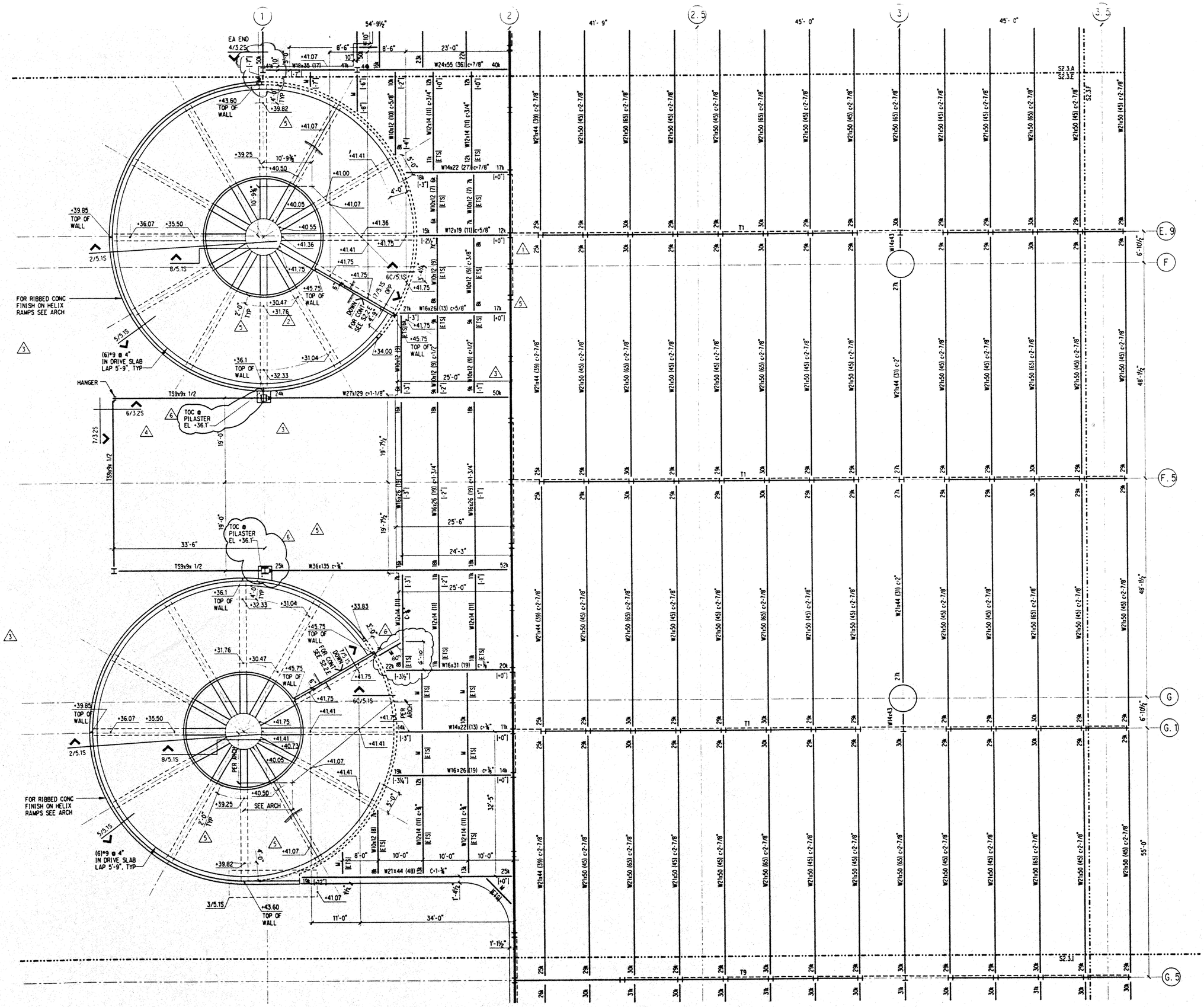
PARKING
FRAMING PLAN
SECTOR D

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.3.D

FriDec 1 08:43:04 1995 jatecbark.dgn plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
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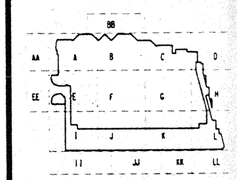
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Section 1.2.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
4	12-15-95		FINAL DOCUMENT
3	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

PARKING FRAMING PLAN SECTOR E

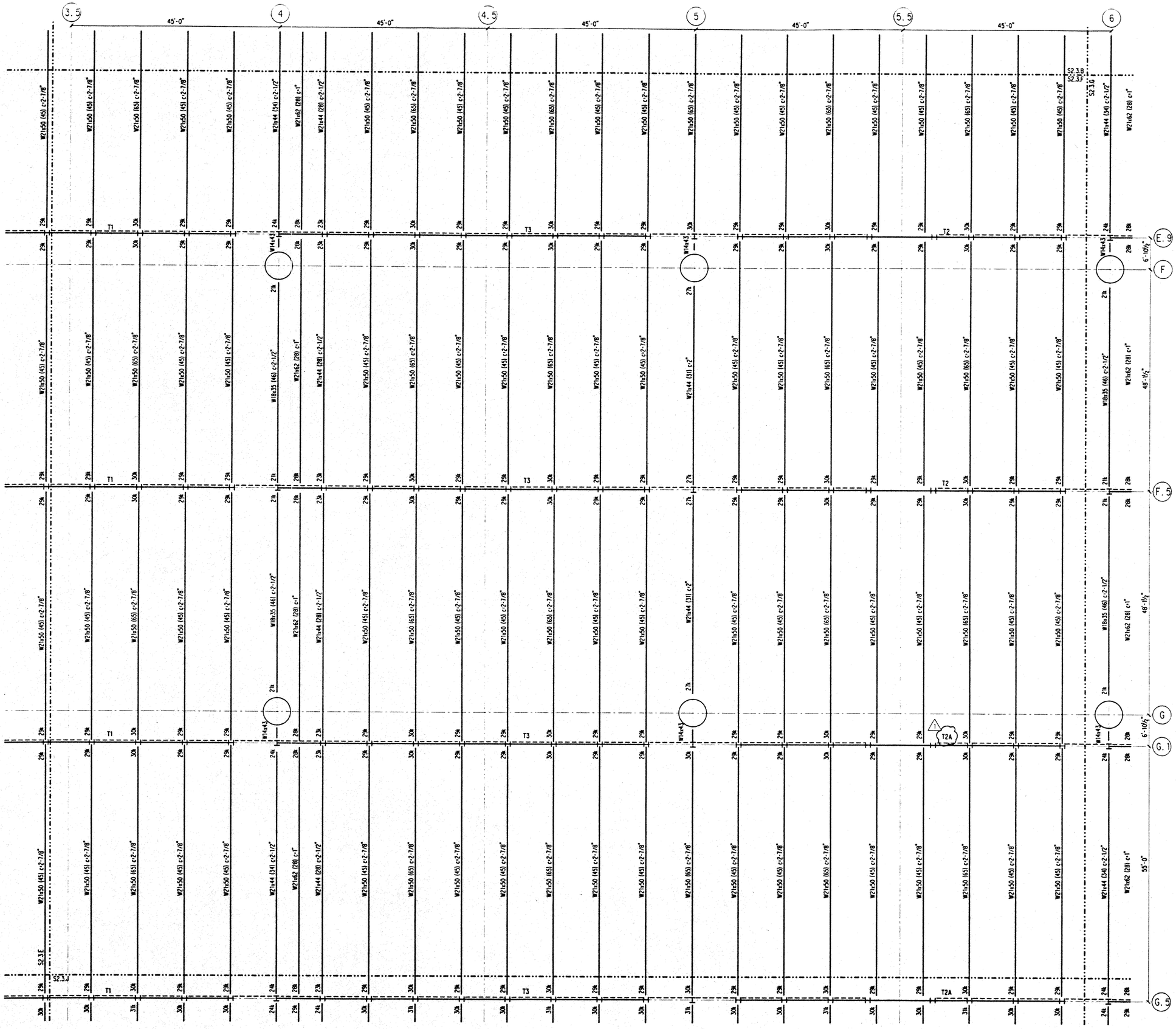
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.3.E

Fri Dec 1 08:45:15 1995

j:\hcc\park_e.dgn

pl1



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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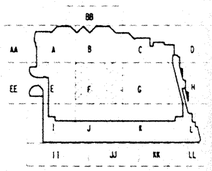
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 18 Subpart 1.2.4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	8-4-95		

Sheet Title

PARKING
FRAMING PLAN
SECTOR F

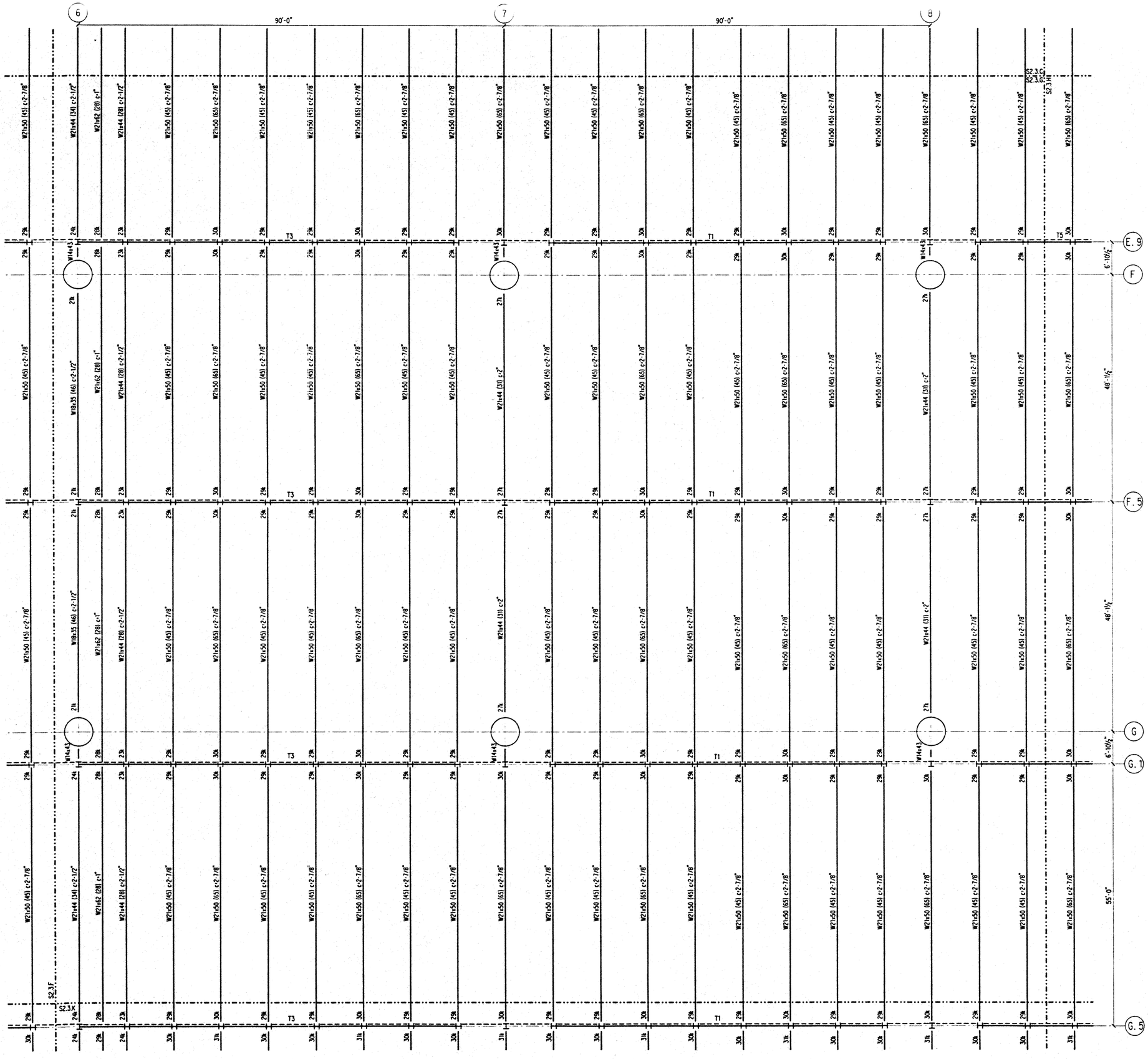
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.3.F

Fri Dec 1 08:47:26 1995

j:\hccpark_f.dgn

plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

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Seattle, Washington

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JOHN D. MAGNUSSON
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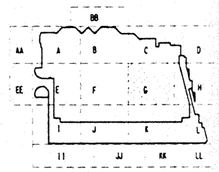
NO. 7177S
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervisor Registered in Title 46 Chapter 16 Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

PARKING
FRAMING PLAN
SECTOR G

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.3.G

Fri Dec 1 08:49:37 1995

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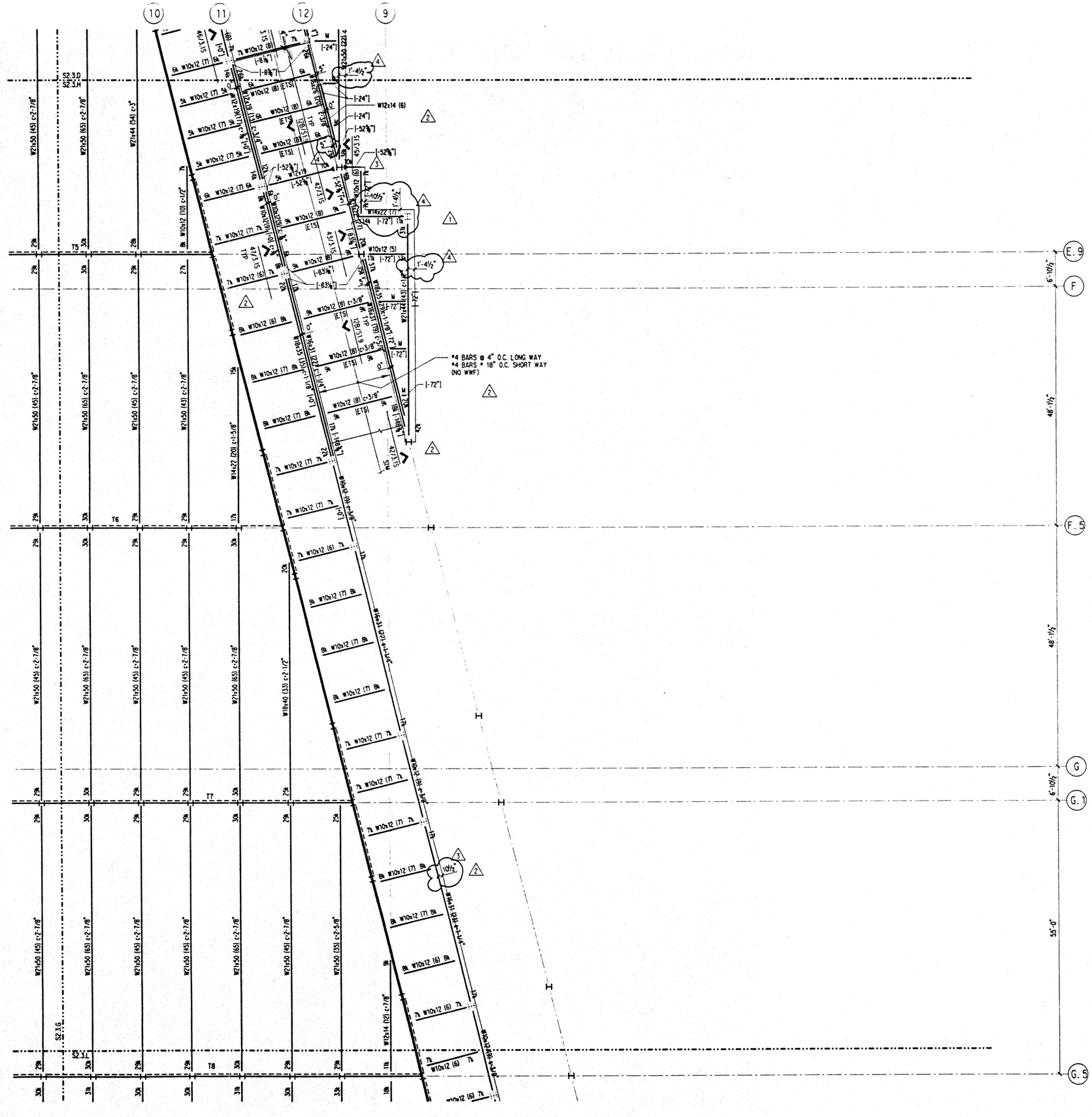
/WS pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

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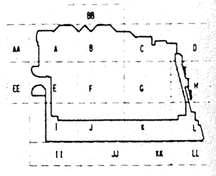
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 8 Subtitle 1 Part of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-21-95		
2	7-28-95		
1	6-19-95		

Sheet Title

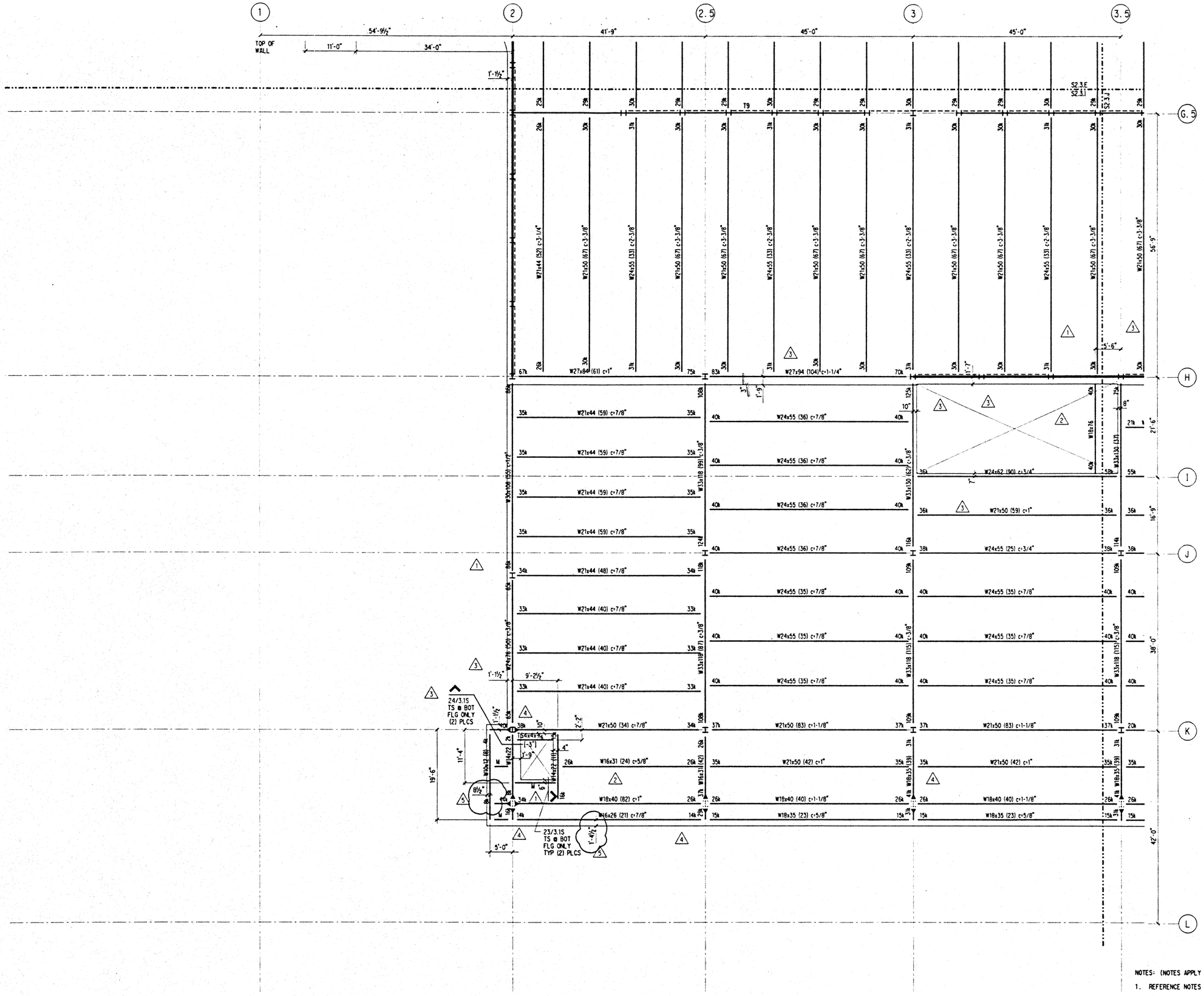
PARKING
FRAMING PLAN
SECTOR H

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.3.H

Fri Dec 1 08:51:48 1995
j:\hccpark.h.dgn
MS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A
2. SLOPE STRUCTURAL STEEL DOWN AT 2.45 PERCENT SOUTH OF GRID H, UNO.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

Consultants

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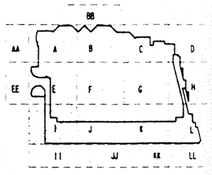
JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7173
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Submission is defined in Title 16 Chapter 18 Section 1-3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

PARKING
FRAMING PLAN
SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.3.I

Fri Dec 1 08:53:49 1995
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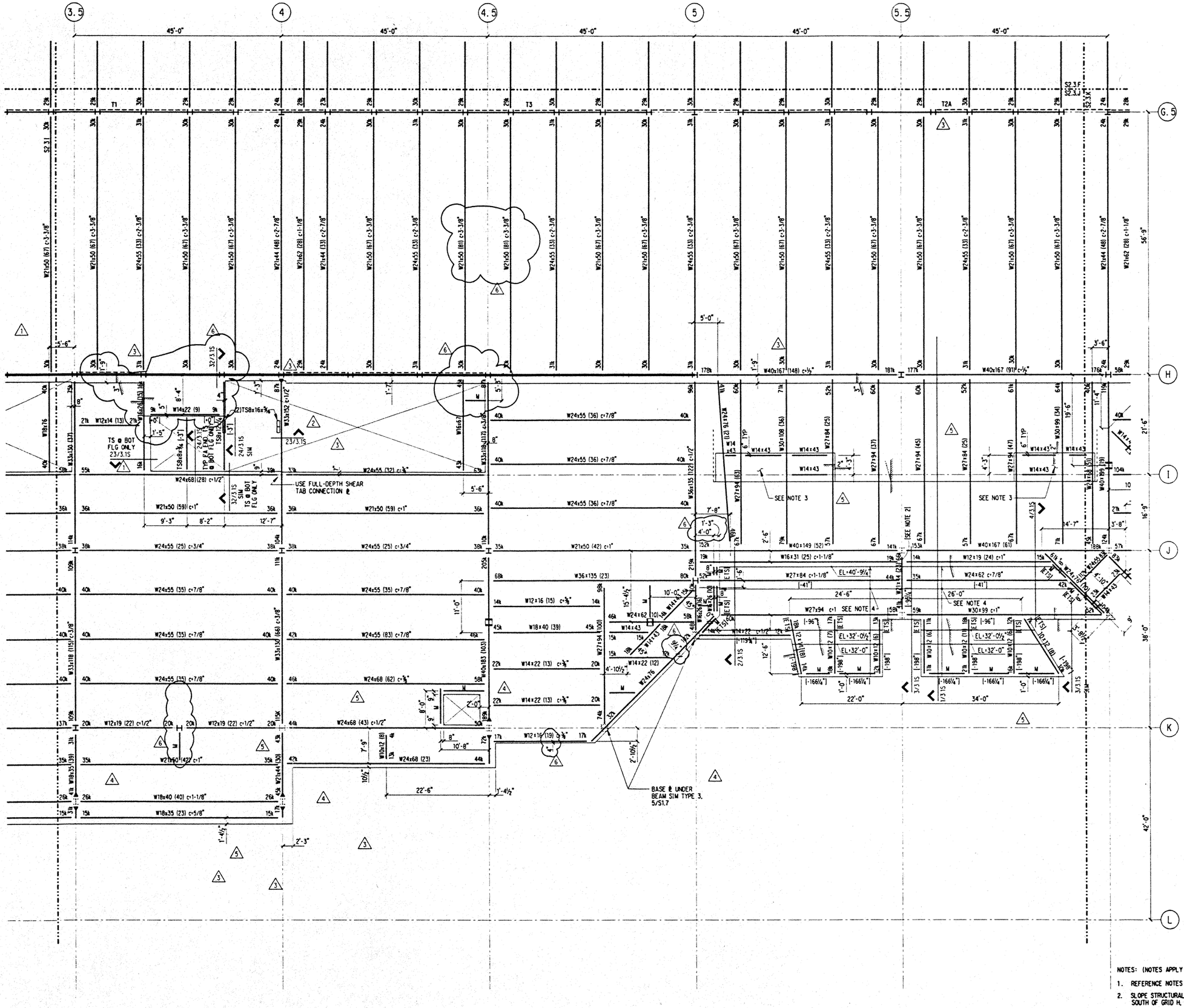
pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A
2. SLOPE STRUCTURAL STEEL DOWN AT 2.45 PERCENT SOUTH OF GRID H, UNO.
3. STRUCTURAL SLAB IS 4½" OF HARDROCK CONCRETE ON 3" 18 GA STEEL DECK. REINFORCE PER "SECTION" ON 7/3.3S.
4. STRUCTURAL SLAB IS 2½" OF HARDROCK CONCRETE ON 3" 16GA STEEL DECK.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
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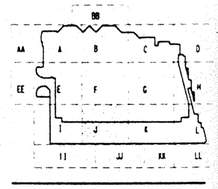
JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Section 1.3 # of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

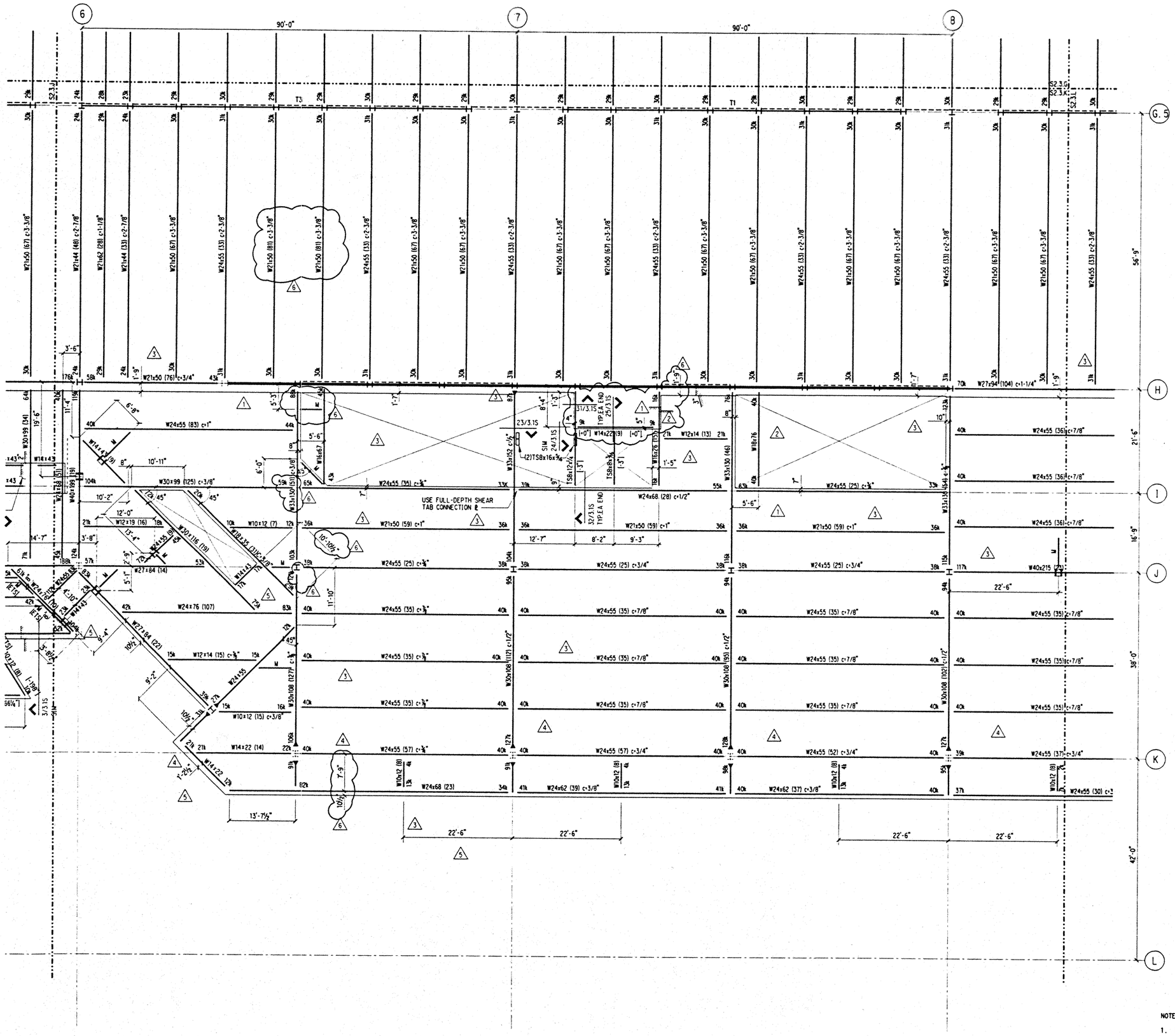
PARKING
FRAMING PLAN
SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.3.J

Fri Dec 1 15:08:38 1995
j:\hcc\park_j.dgn


6
7
8
90'-0"
90'-0"
G.5
H
I
J
K
L
56'-9"
21'-6"
16'-9"
38'-0"
42'-0"
USE FULL-DEPTH SHEAR TAB CONNECTION &
W24x55 (83) c+1"
W30x99 (125) c+3/8"
W24x76 (107)
W24x55 (35) c+7/8"
W24x55 (57) c+3/4"
W24x62 (39) c+3/8"
W24x68 (23)
W24x55 (52) c+3/4"
W24x62 (37) c+3/8"
W24x55 (37) c+3/4"
W24x55 (30)
W24x55 (36) c+7/8"
W24x55 (25) c+3/4"
W21x50 (59) c+1"
W24x68 (28) c+1/2"
W27x94 (104) c+1-1/4"
W40x215 (27)
W30x108 (127)
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W30x108 (95) c+1/2"
W30x108 (102) c+1/2"
W14x22 (14)
W14x22 (9)
W12x14 (15) c+3/8"
W10x12 (15) c+3/8"
W10x12 (8)
W12x14 (13)
W18x76
W33x130 (46)
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W21x50 (76)
W21x50 (81)
W24x55 (33)
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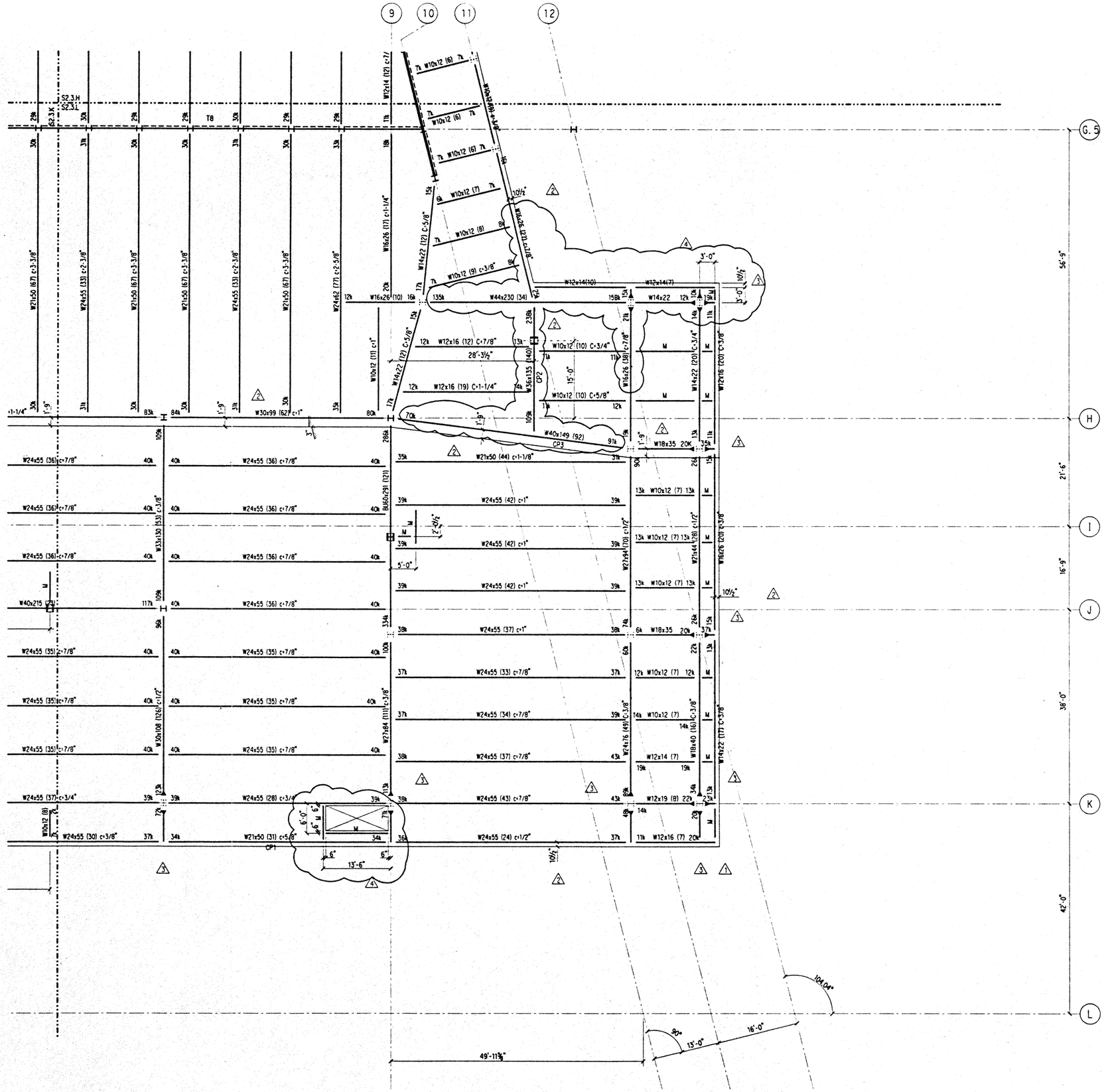
W21x50 (67) c+3/8"
W24x55 (33) c+2-3/8"
W21x50 (67) c+3-3/8"
W21x50 (67) c+3-



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.3.A
2. SLOPE STRUCTURAL STEEL DOWN AT 2.45 PERCENT SOUTH OF GRID H, UNO.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
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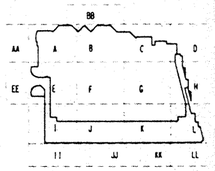
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Section 1.3 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-31-95		
2	8-4-95		
1	6-19-95		

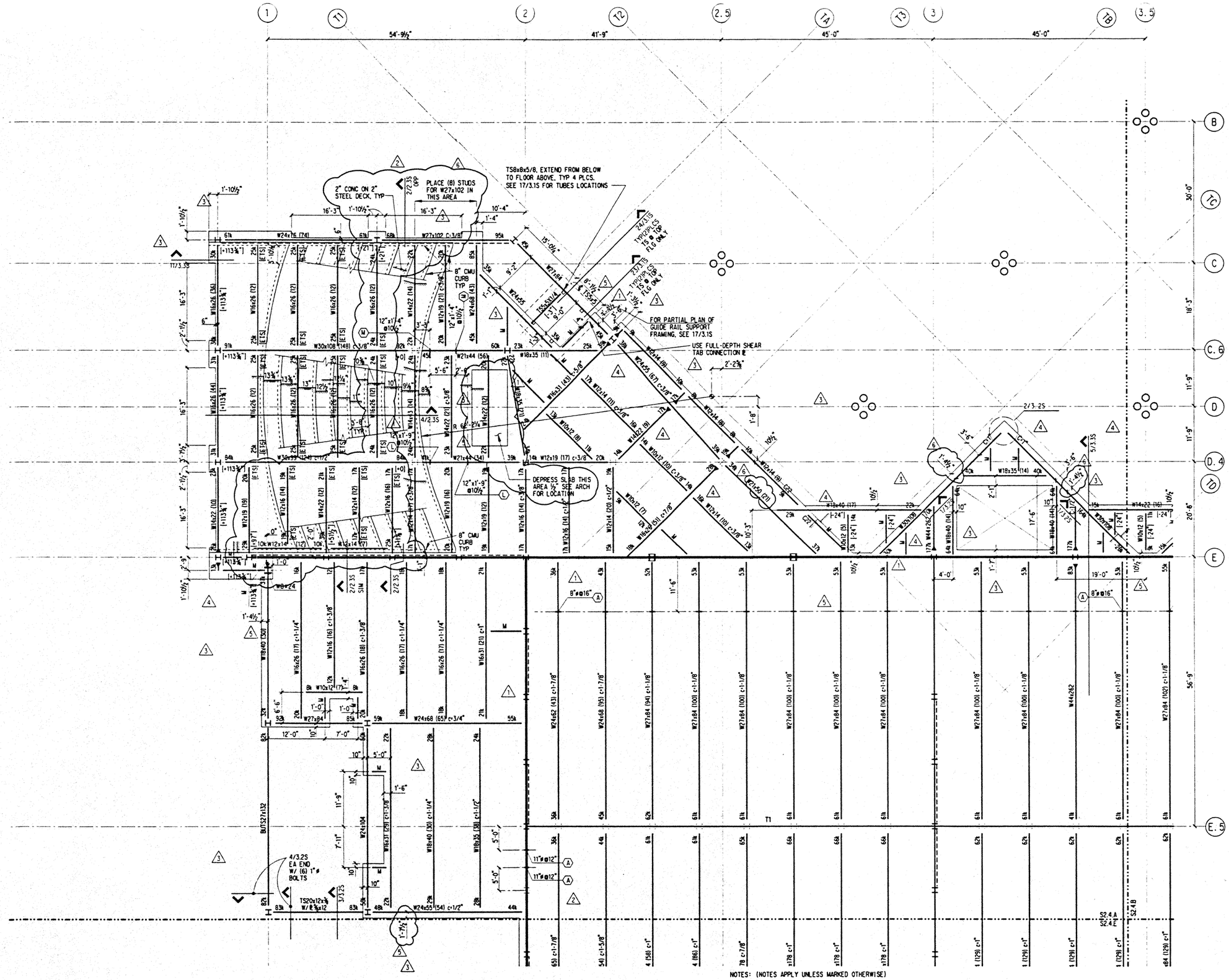
Sheet Title

PARKING
FRAMING PLAN
SECTOR L

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.3.L

Fri Dec 1 09:00:23 1995 jyncpark.1.dgn plt DMS



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.• SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.• TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
   - S1.20 CONCRETE SCHEDULES
   - S1.21 S1.22 STEEL SCHEDULES
   - S2.• PLANS
   - S3.• TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
   - S4.• BRACED FRAME ELEVATIONS
   - S5.• TRUSS ELEVATIONS
   - S6.• BRACED FRAME & TRUSS DETAILS
   - DETAIL BOOK STRUCTURAL DETAILS
2. REFERENCE FLOOR ELEVATION = 54'-6"
3. TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
4. REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE
5. WHERE NOTED THUS [+/- 8"], TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
6. STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
7. REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
8. "M" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
9. INDICATES BEAM PENETRATION, SEE S1.10
10. INDICATES HAUNCHED BEAM, SEE S1.10
11. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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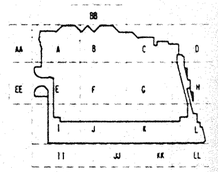
(206) 292-1200 FAX (206) 292-1201

KIM D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 18 Subtitle 1.2 Part of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-14-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

MEETING
FRAMING PLAN
SECTOR A

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.A

Thu Nov 30 18:03:24 1995
j:\hccmeet.a.dgn
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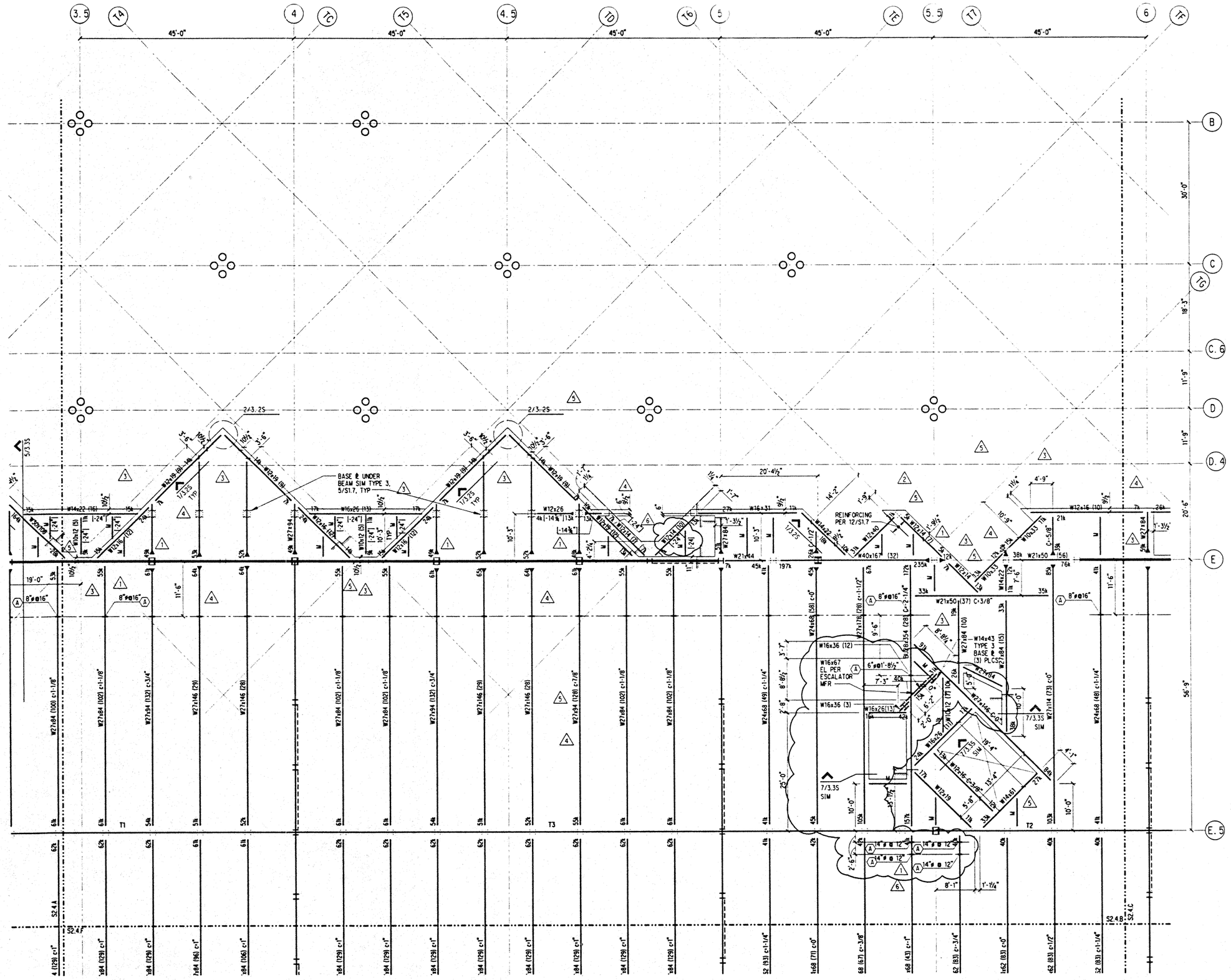

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
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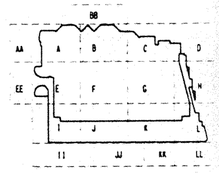
JOHN S. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in 16.1 in Chapter 16 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

MEETING
FRAMING PLAN
SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

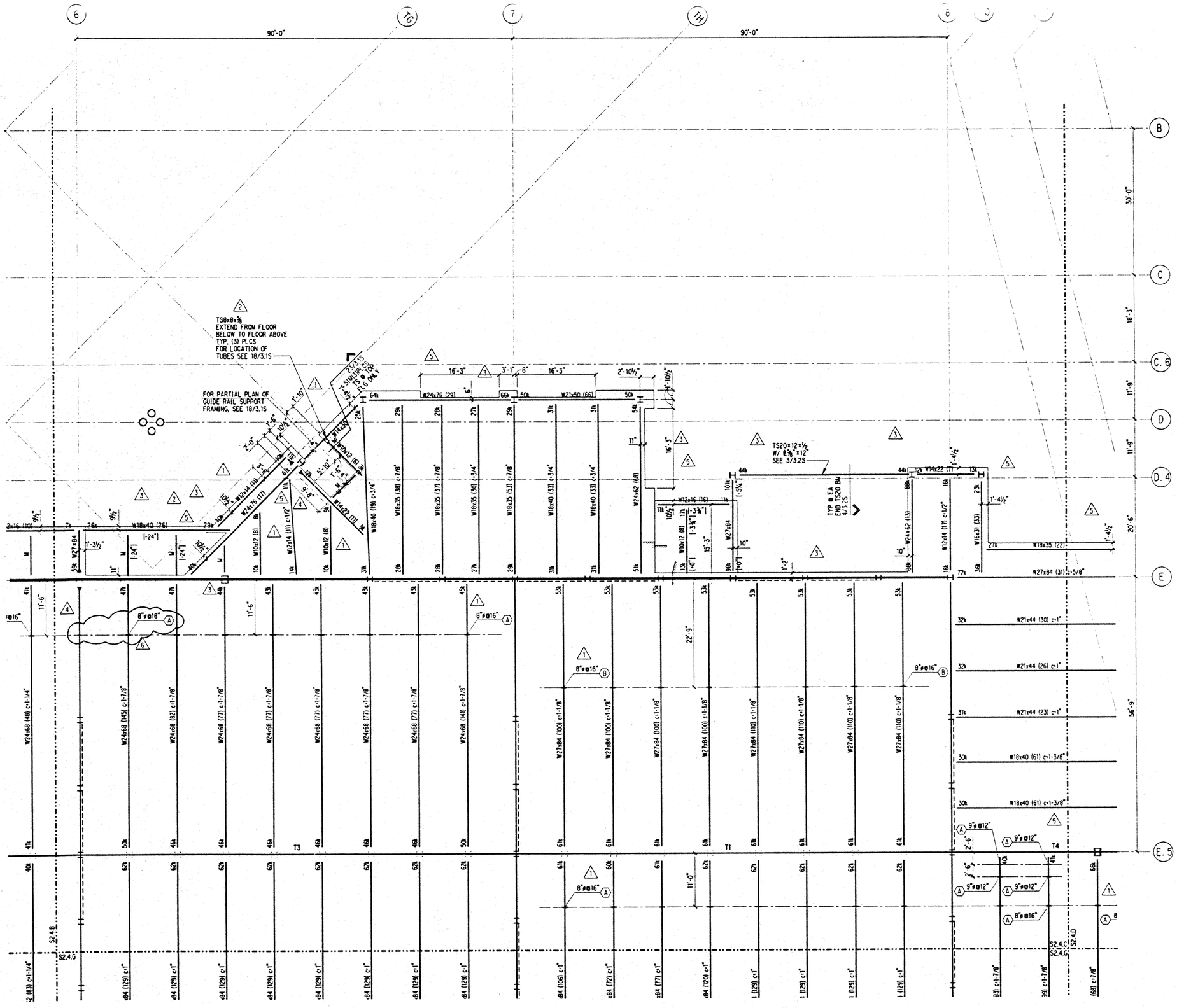
Sheet No.

S2.4.B

Thu Nov 30 18:05:59 1995

j:\hcc\meet_b.dgn

ws pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

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SKILLING WARD MAGNUSSON BARKSHIRE INC.
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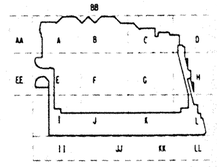
(206) 292-1200 FAX (206) 292-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 16 Subtitle 1 Part VI of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

MEETING FRAMING PLAN SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.C

Thu Nov 30 18:08:31 1995
jhccmeet.c.dgn
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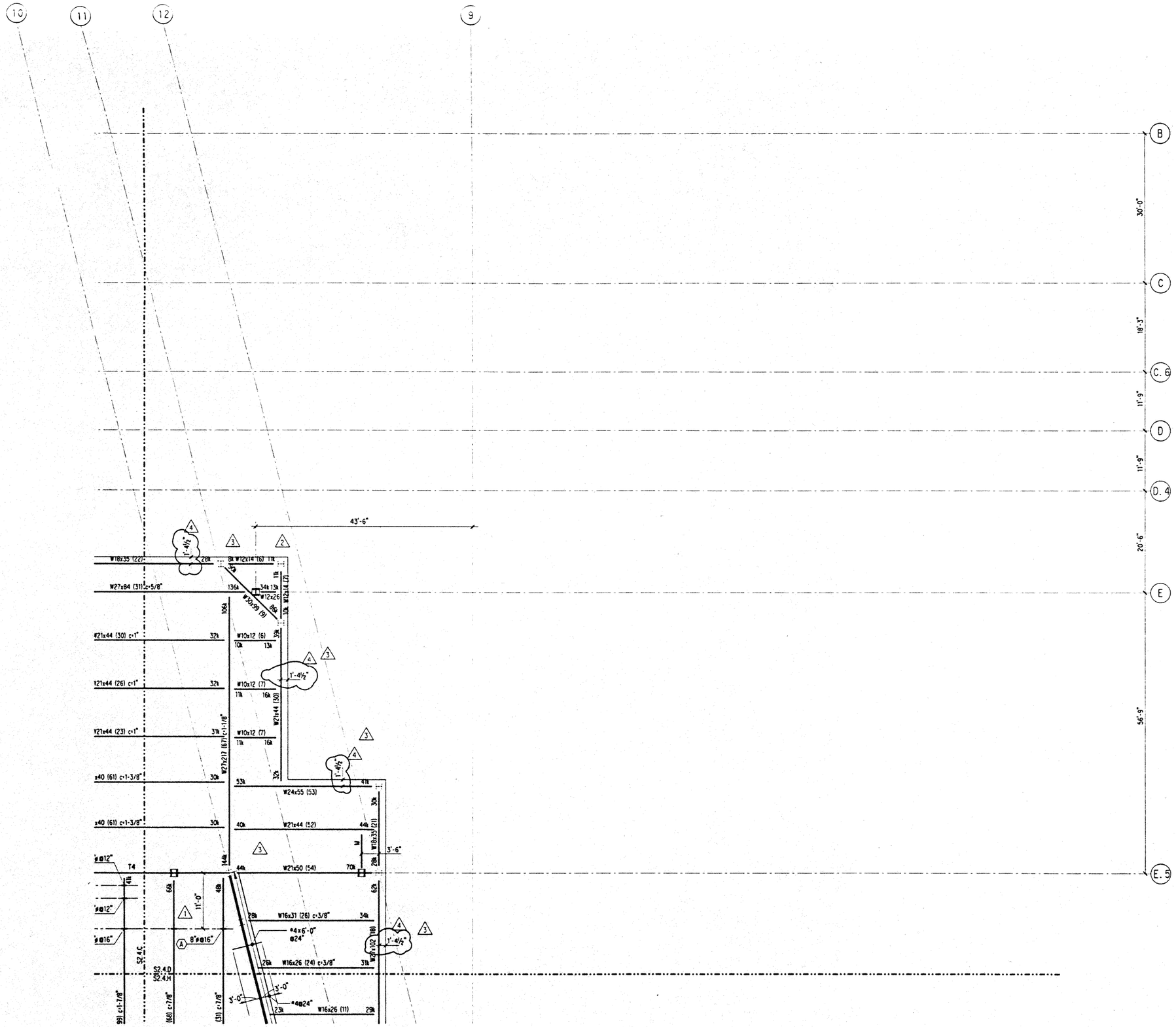

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
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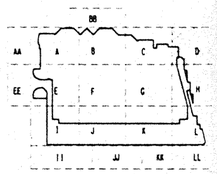
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11773 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervised by derived in Title 16 Chapter 18 Section 1.2.2.a of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

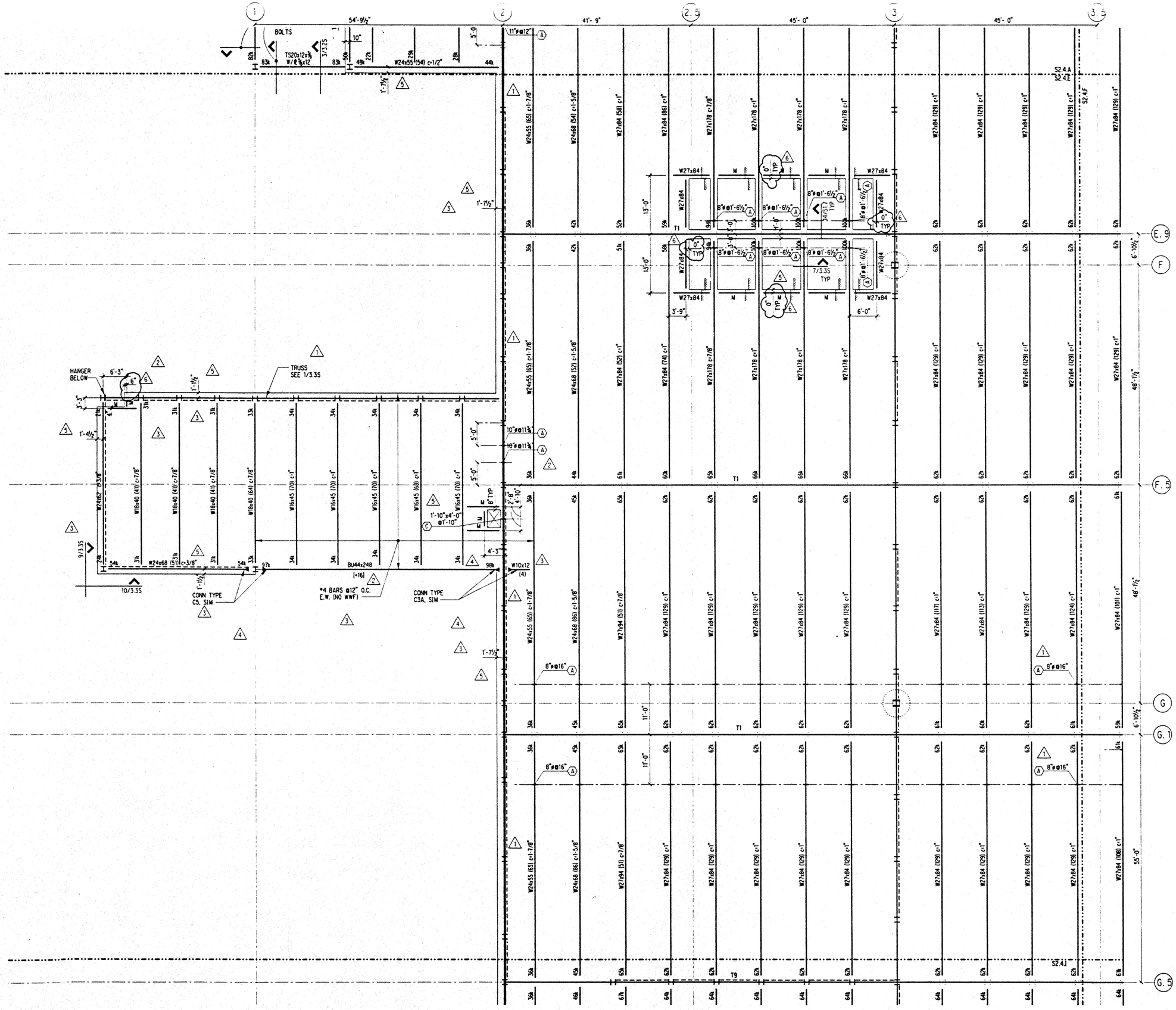
MEETING
FRAMING PLAN
SECTOR D

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.D

Thu Nov 30 18:11:02 1995 jthccmeet.d.dgn pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

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Seattle, Washington

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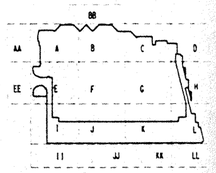
RON S. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11773
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision signed in Kona on December 12 1995 in the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

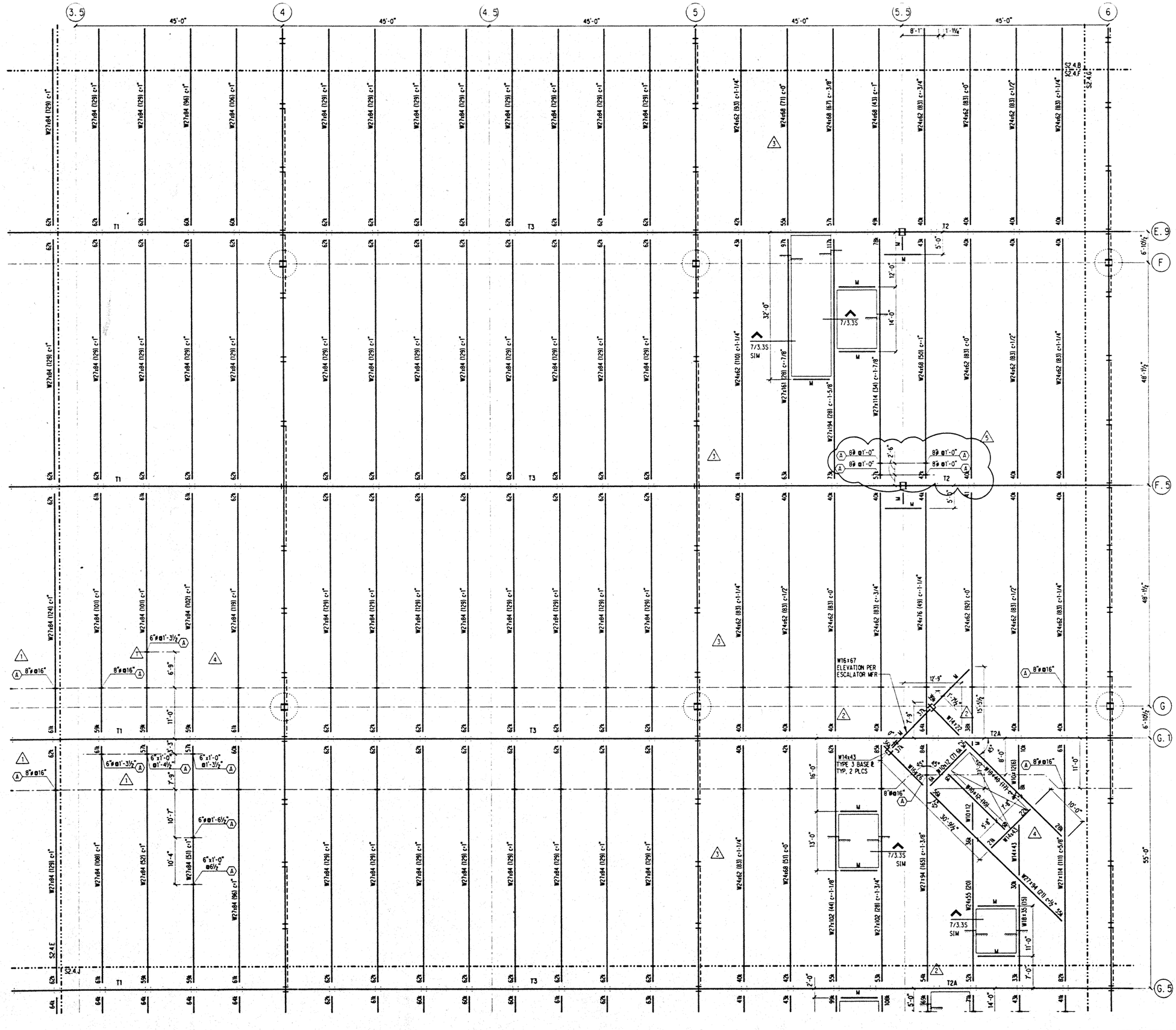
MEETING
FRAMING PLAN
SECTOR E

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.E

Fri Dec 1 15:37:41 1995 jyhccmeet.e.dgn plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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SKILLING WARD MAGNUSSON BARKSHIRE INC.
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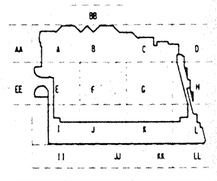
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervised by James A. Fox in Chapter 16 Section 1.29 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

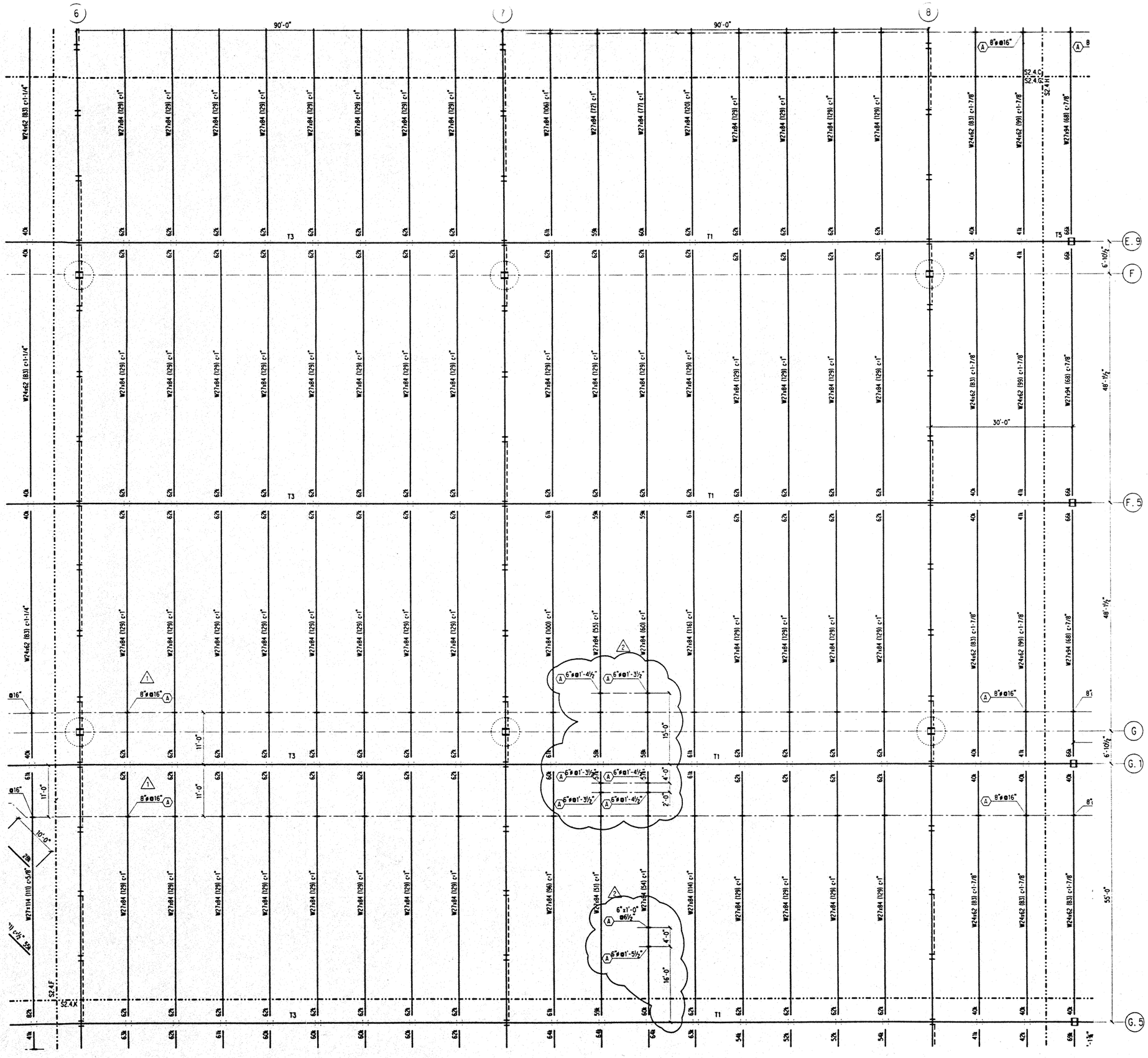
MEETING
FRAMING PLAN
SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.F

jxtccmeet.f.dgn Thu Nov 30 18:16:03 1995 pltdrws



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

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Seattle, Washington

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Consultants

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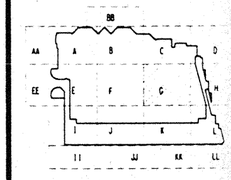
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Licensed in accordance with Title 16 Chapter 476 Subpart 1 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
1	5-8-95		ADDENDUM #3

Sheet Title

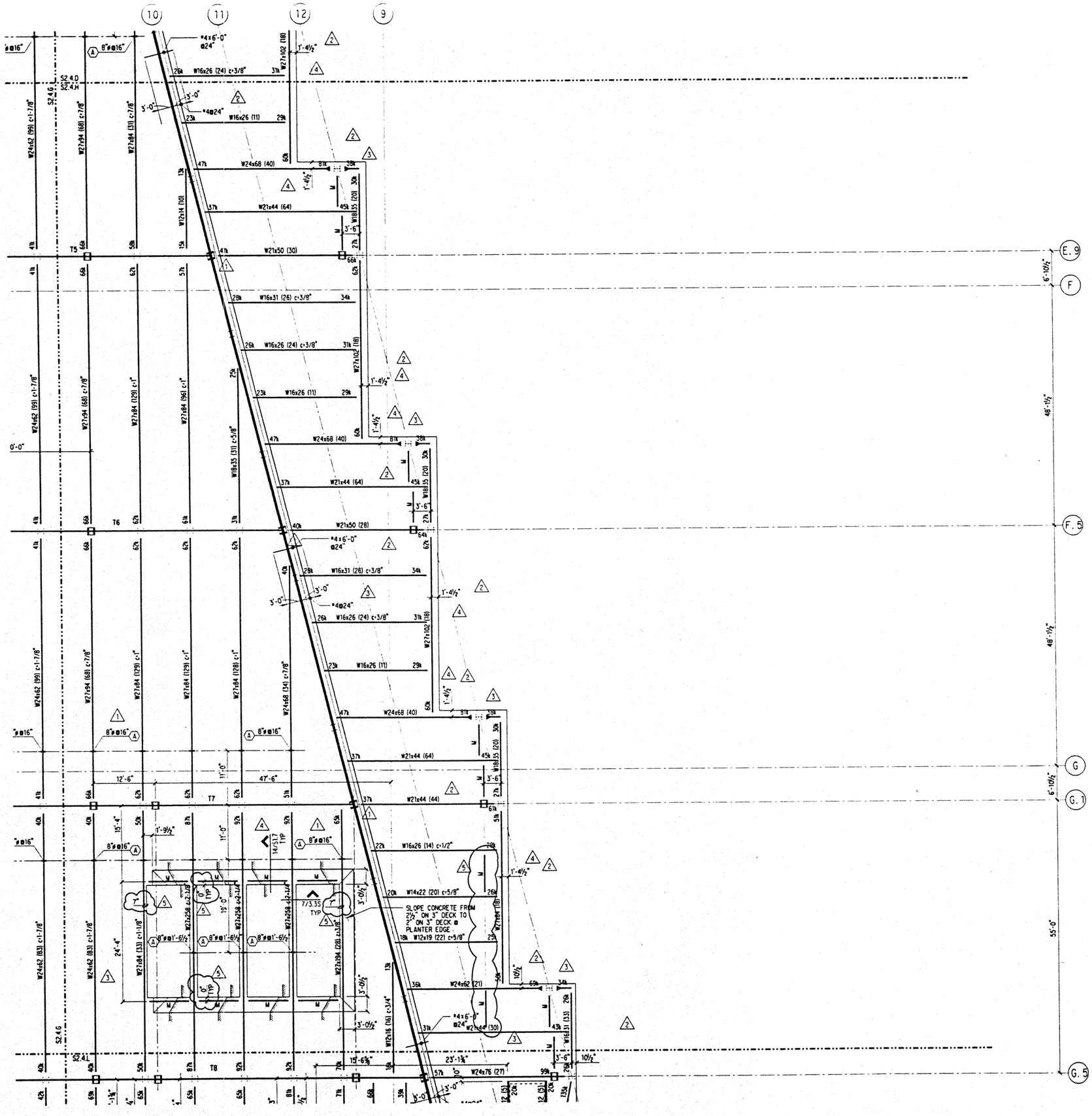
MEETING FRAMING PLAN SECTOR G

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.G

Thu Nov 30 18:18:35 1995
jatccmeet.g.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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A Joint Venture
Honolulu, HI

Consultants

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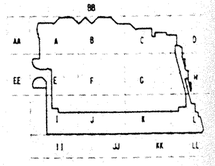
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 71779 JOHN D. MAGNUSSON HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer in Total in Charge in accordance with the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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3	8-31-95		
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1	5-8-95		ADDENDUM #3

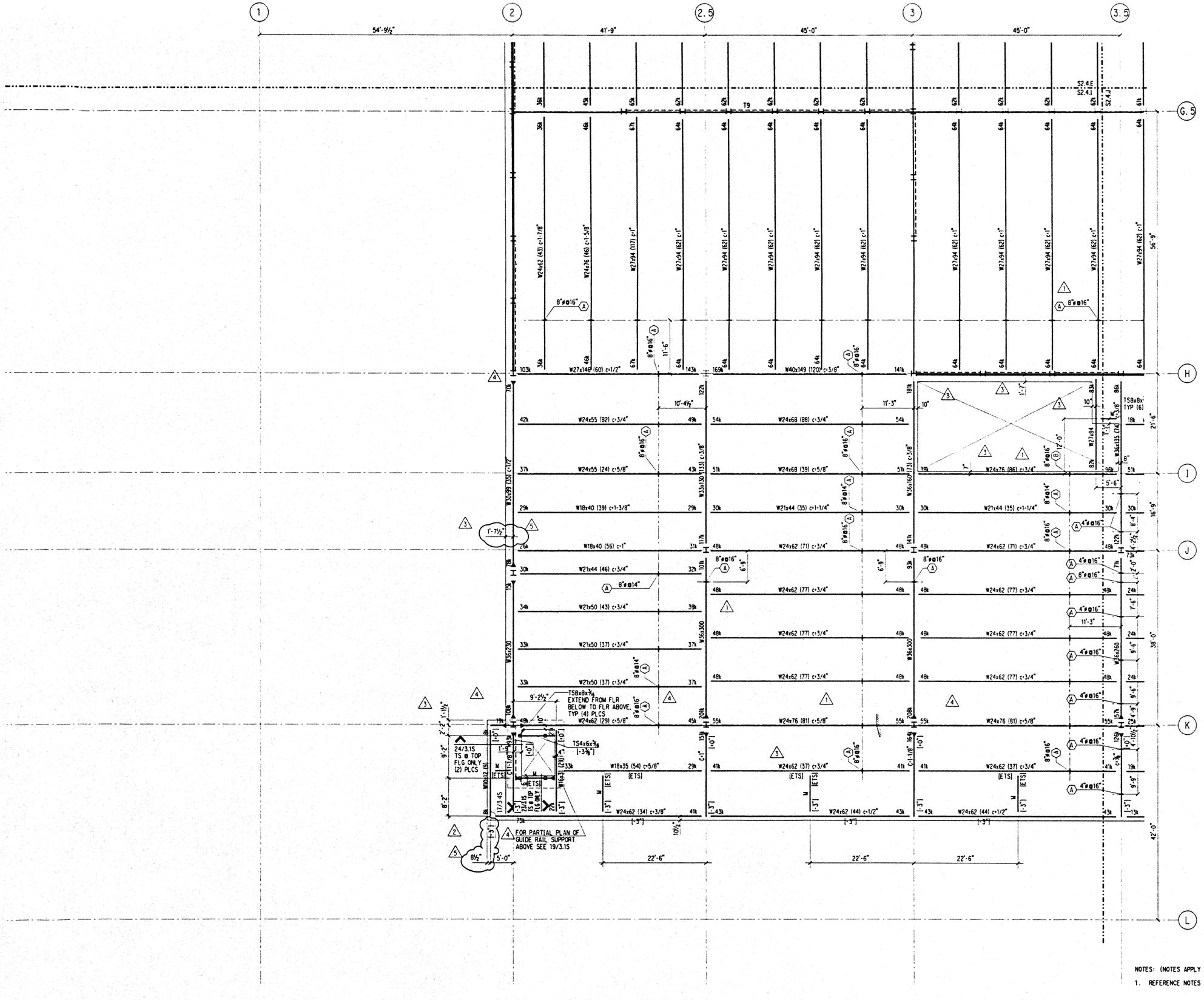
Sheet Title

MEETING
FRAMING PLAN
SECTOR H

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.4.H

Fri Dec 1 15:40:03 1995 jxhccvmeet_h.dgn
MS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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Consultants

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JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER NO. 11773
HAWAII, U.S.A.

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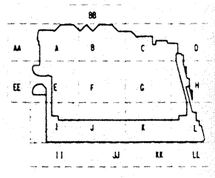
Supervision by licensed in Title 16 Chapter 18 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

MEETING FRAMING PLAN SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.I

Thu Nov 30 18:23:17 1995

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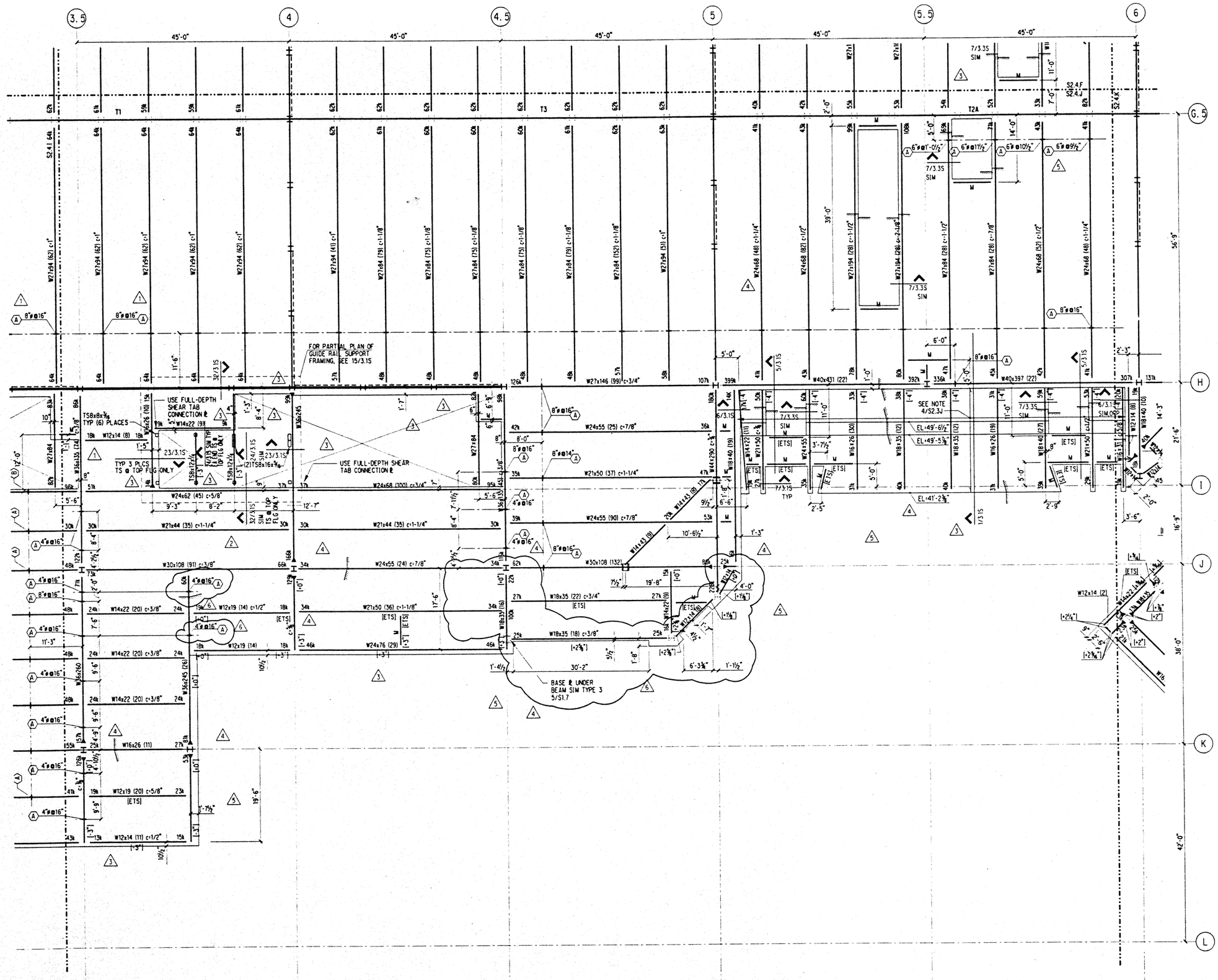

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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Honolulu, HI

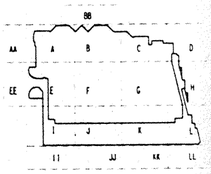
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 292-1201

JOB D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11773
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising engineer in Title III Chapter 16 Subpart 3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

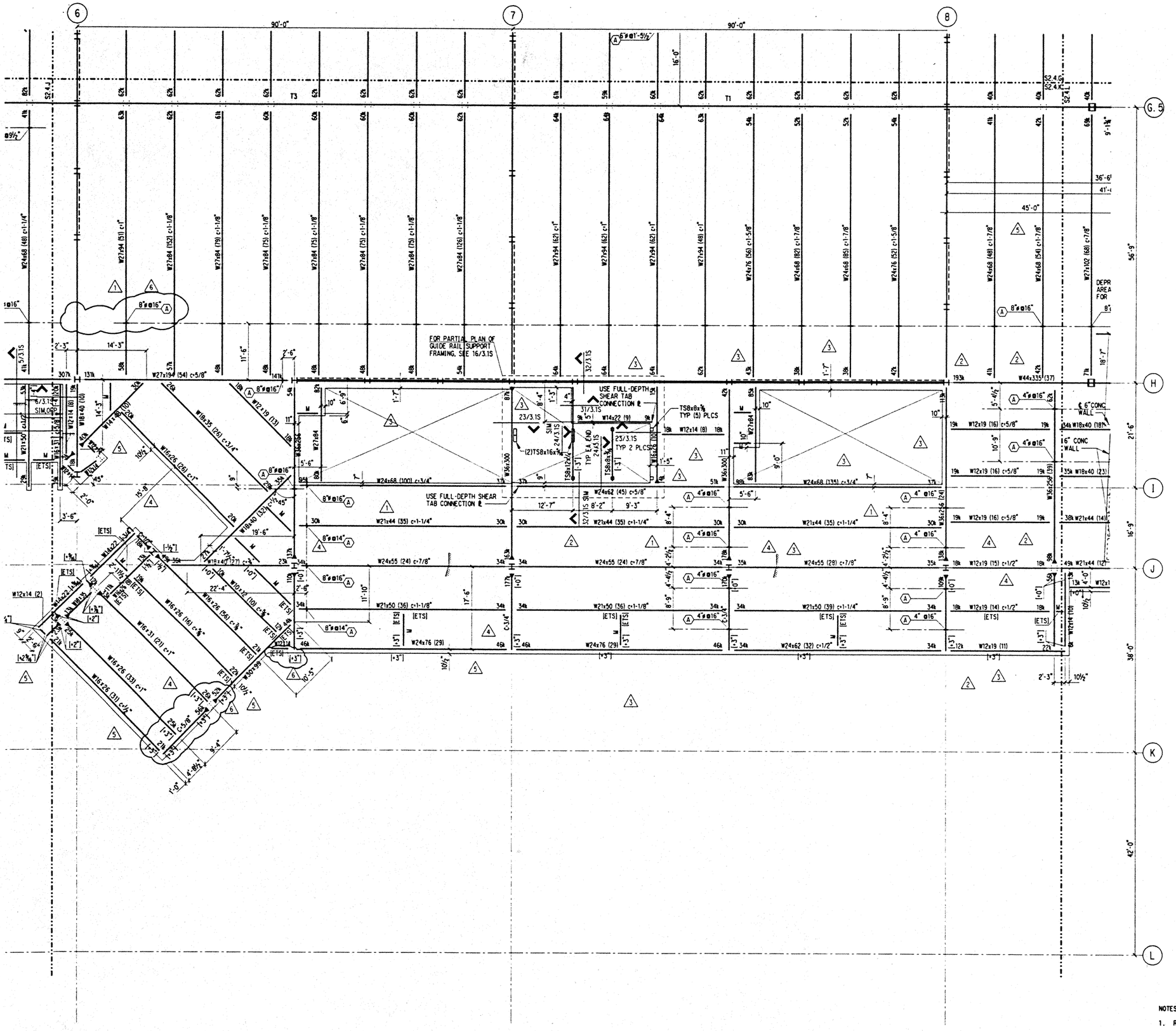
MEETING
FRAMING PLAN
SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.J

Sat Dec 2 12:21:03 1995 jxhccmeet_j.dgn ws pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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in association with

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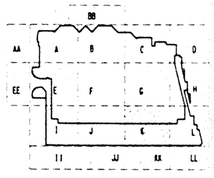
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 292-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7777S
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision defined in Title 16 Chapter 18 Section 1.3 of the Rules of Regulation of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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1	5-8-95		ADDENDUM #3

Sheet Title

MEETING
FRAMING PLAN
SECTOR K

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.K

Thu Nov 30 18:28:24 1995
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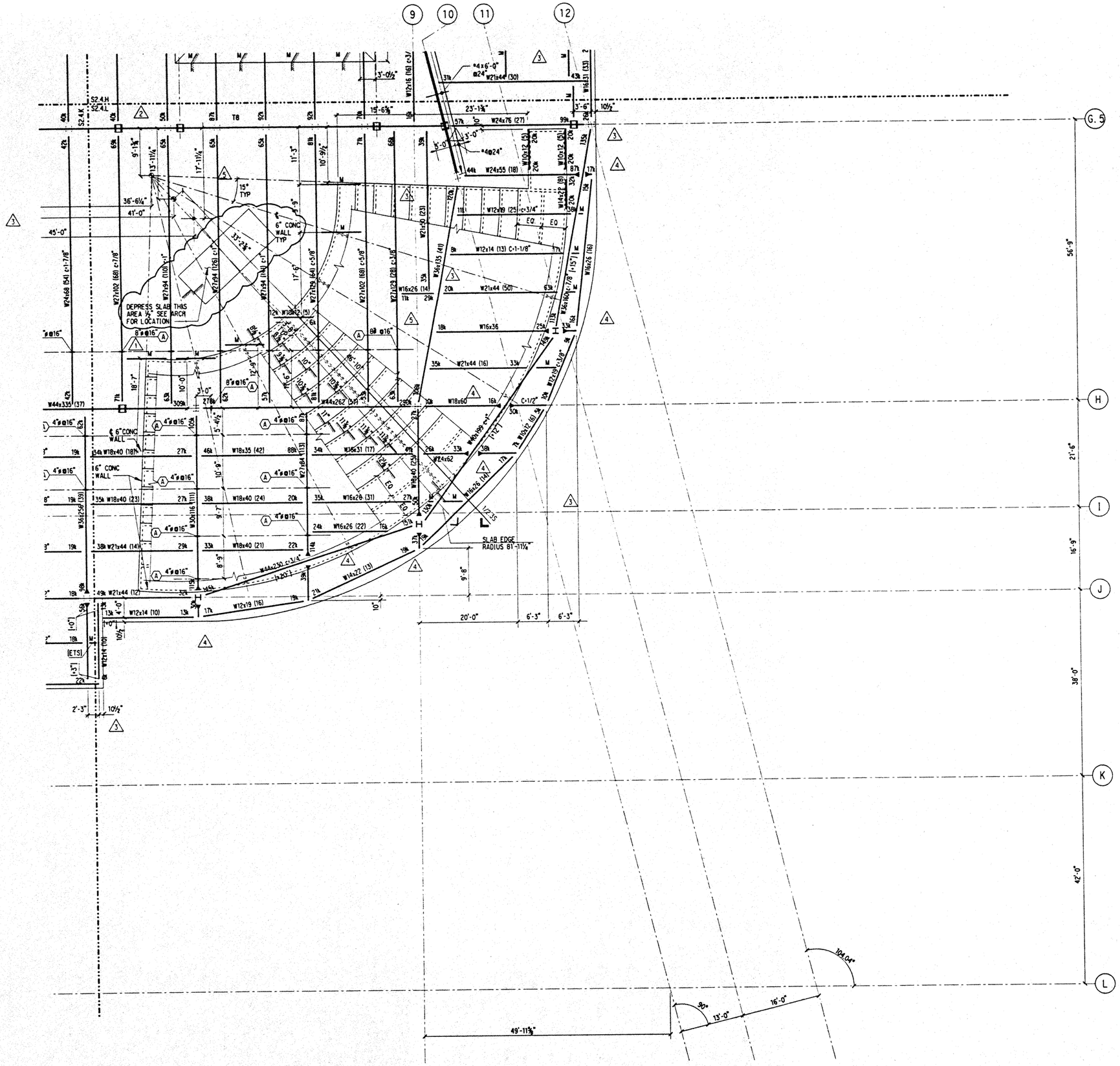

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.4.A

Thu Nov 30 18:30:56 1995

jshccmeet.i.dgn

ws_plt

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

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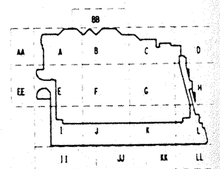
Supervision as defined in Title 16 Chapter 8 Subtitle 1.2.a of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
6	12-15-95		FINAL DOCUMENT
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

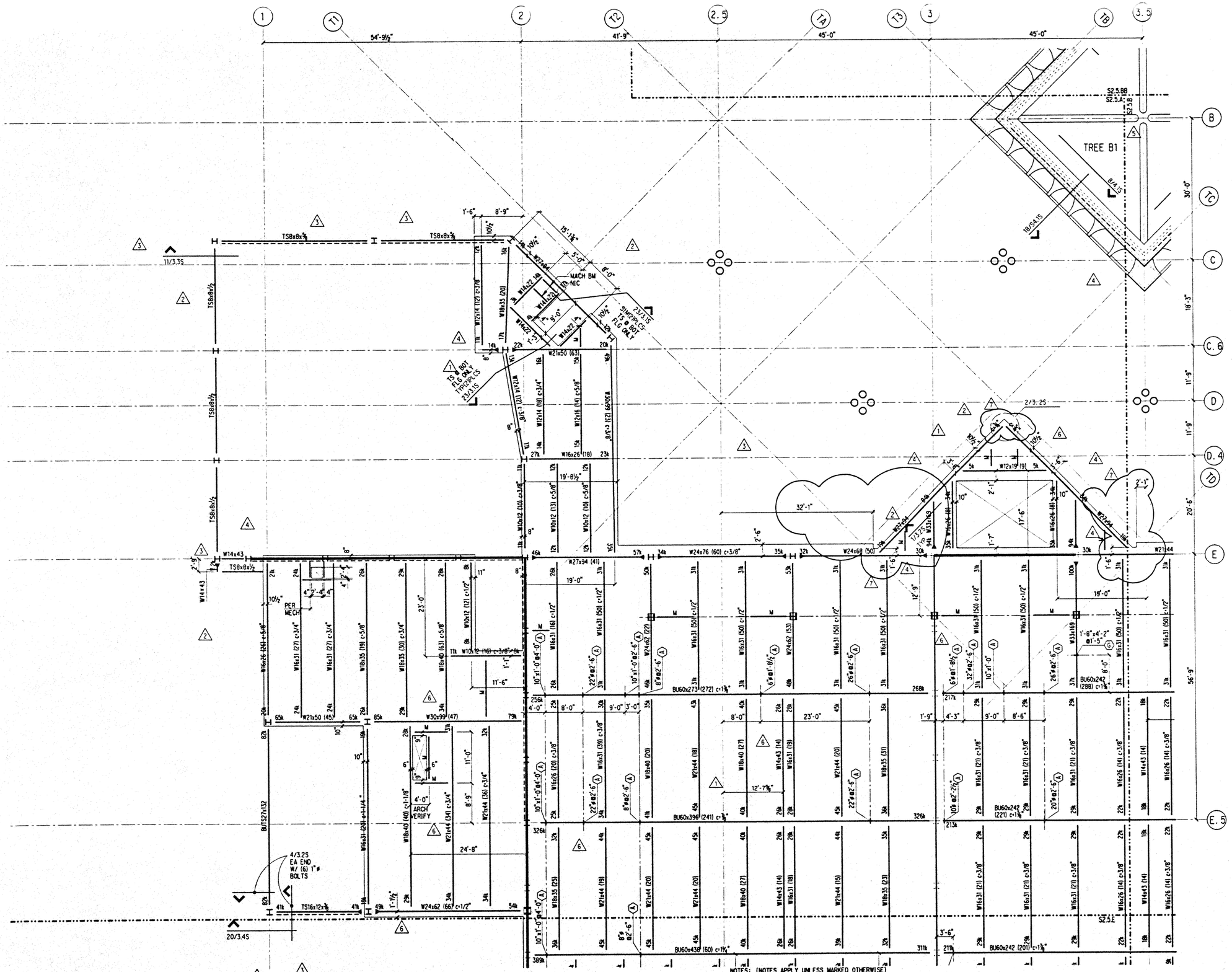
Sheet Title

MEETING
FRAMING PLAN
SECTOR L

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.4.L



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.• SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.• TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
   - S1.20 CONCRETE SCHEDULES
   - S1.21 S1.22 STEEL SCHEDULES
   - S2.• PLANS
   - S3.• TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
   - S4.• BRACED FRAME ELEVATIONS
   - S5.• TRUSS ELEVATIONS
   - S6.• BRACED FRAME & TRUSS DETAILS
   - DETAIL BOOK STRUCTURAL DETAILS
2. REFERENCE FLOOR ELEVATION = 77'-6"
3. TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
4. REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION, UNO.
5. WHERE NOTED THUS [+/- 8"], TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
6. STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
7. REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
8. "M" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
9. INDICATES BEAM PENETRATION, SEE S1.10
10. INDICATES HAUNCHED BEAM, SEE S1.10
11. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

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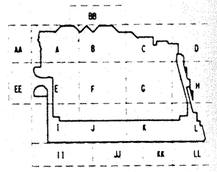

Revisions

No.	Date	By	Description
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6	11-7-95		
5	9-20-95		TREE REVISIONS
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-14-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

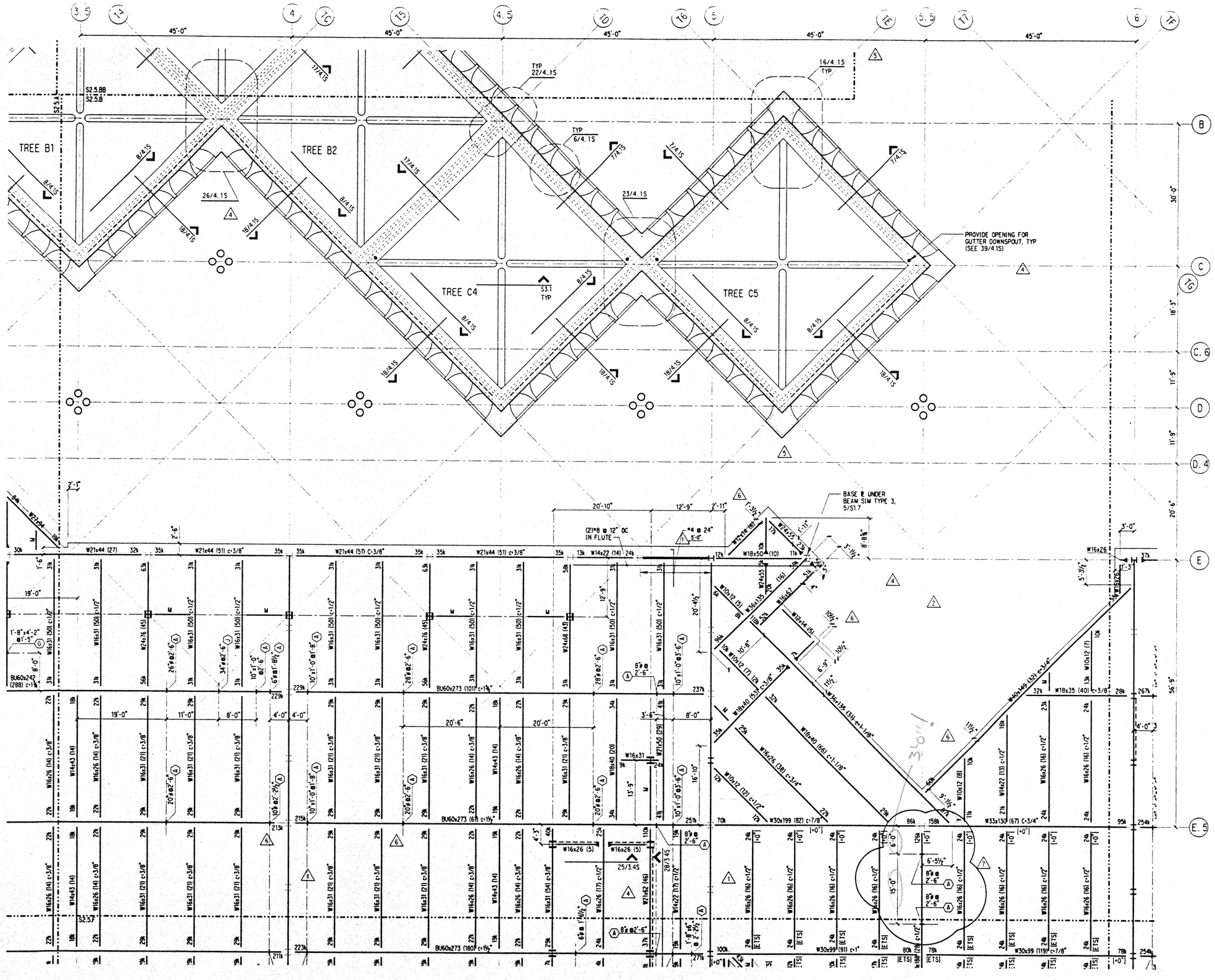
BALLROOM/TERRACE FRAMING PLAN SECTOR A

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.A

Thu Nov 30 17:24:59 1995 jthecball.a.dgn pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

Hawai'i Convention Center

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LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII U.S.A.

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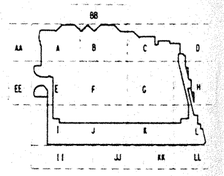
Supervision as defined in Title 16 Chapter 18 Section 1.2.5a of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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6	11-7-95		
5	9-20-95		TREE REVISIONS
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.5.B

Thu Nov 30 17:27:46 1995
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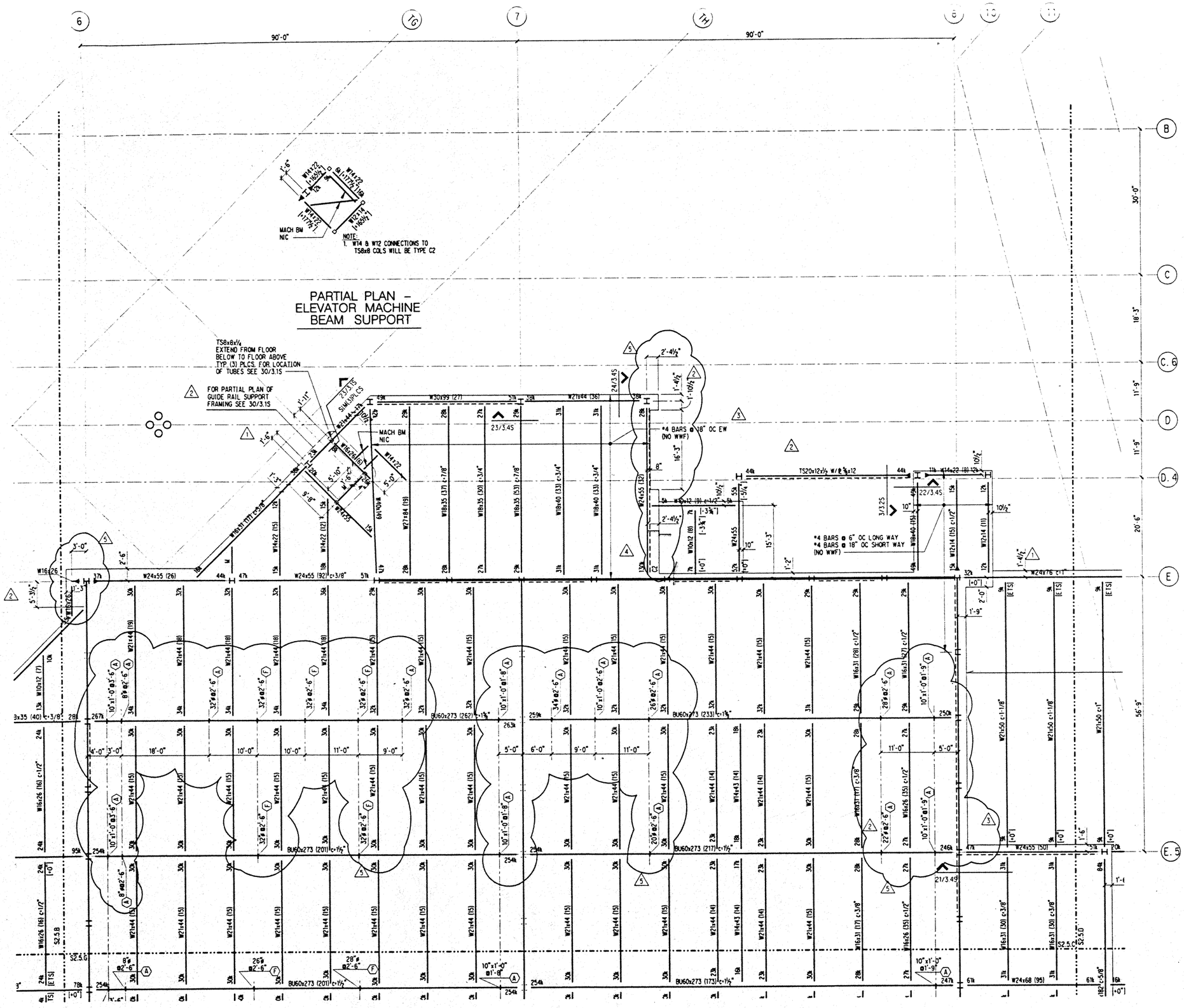
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

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LICENSED PROFESSIONAL ENGINEER NO. 71773 HAWAII, U.S.A.

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Supervision as defined in Title 16 Chapter 8 Subtitle 1.2.A of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.

Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE FRAMING PLAN SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.C

Thu Nov 30 17:30:17 1995

jyhccball.c.dgn

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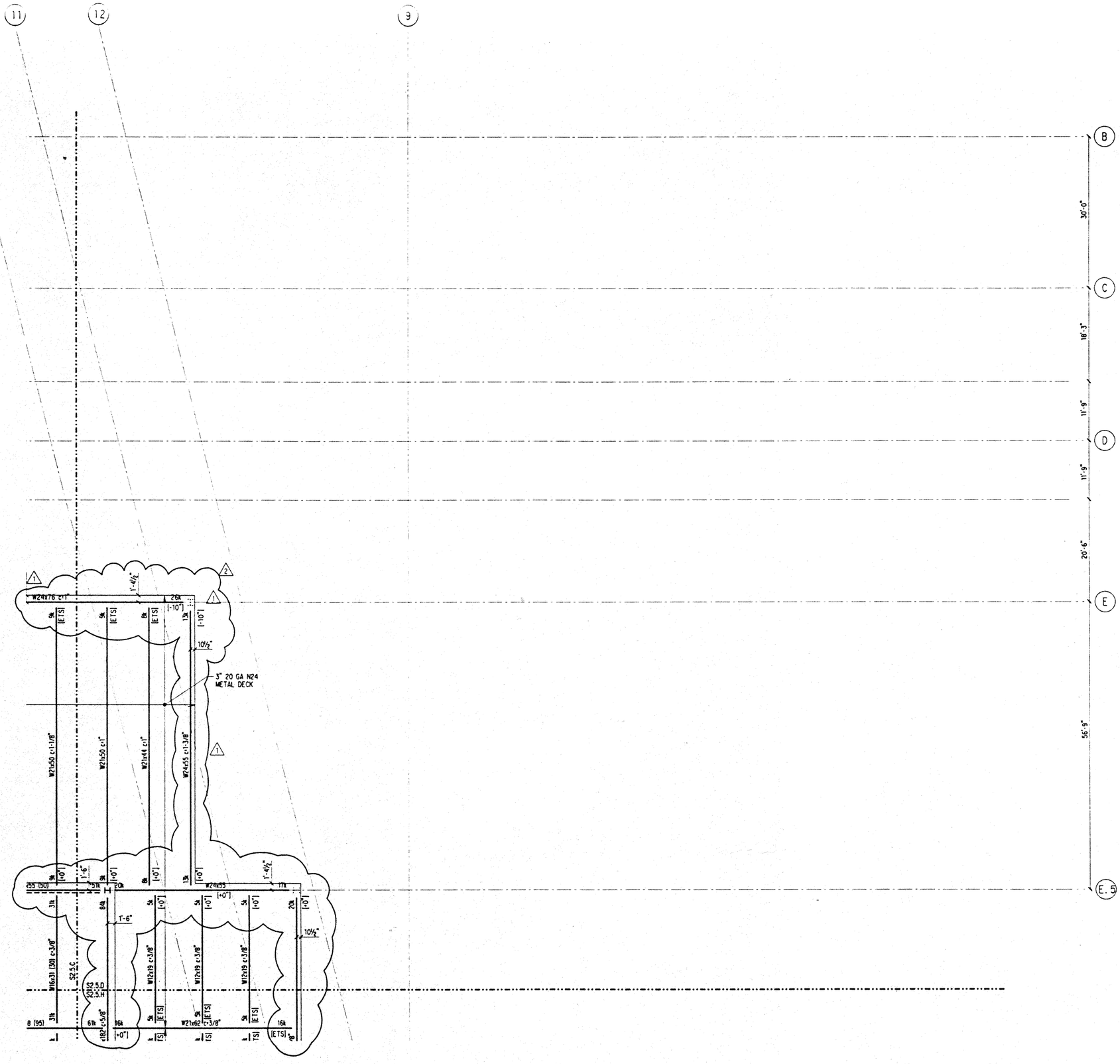

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

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JOHN D. MAGNUSSON
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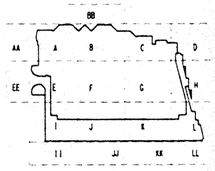
Supervision by licensed in Title 16 Chapter 53 Subchapter 3 Sec of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE
FRAMING PLAN
SECTOR D

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.D

Thu Nov 30 17:32:44 1995
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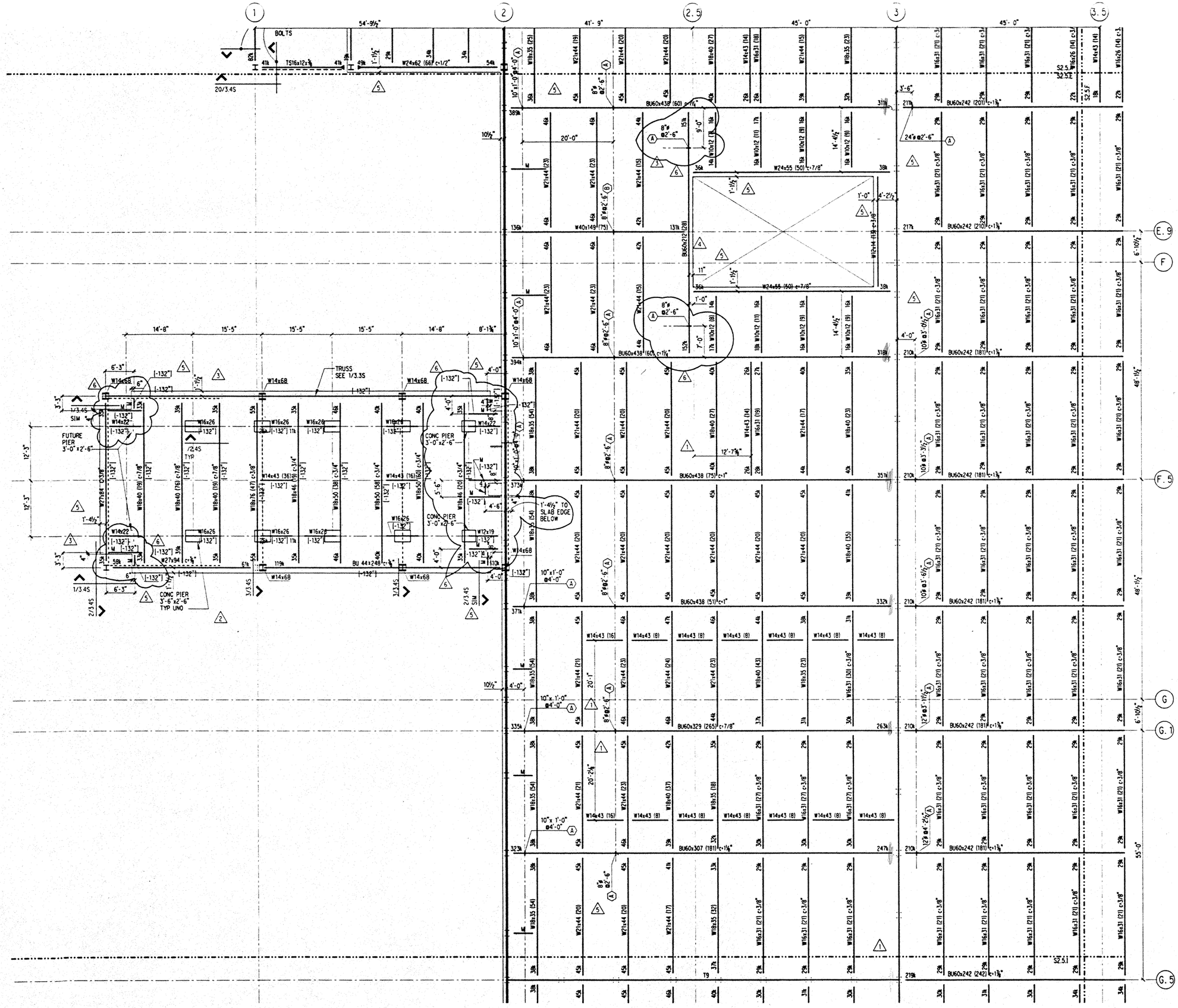
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

Hawai'i Convention Center

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LICENSED PROFESSIONAL ENGINEER NO. 71773 HAWAII, U.S.A.

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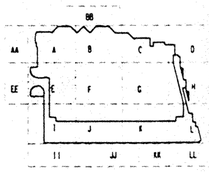

Revisions

No.	Date	By	Description
6	12-15-95		FINAL DOCUMENT
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE
FRAMING PLAN
SECTOR E

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.5.E

Thu Nov 30 17:35:10 1995

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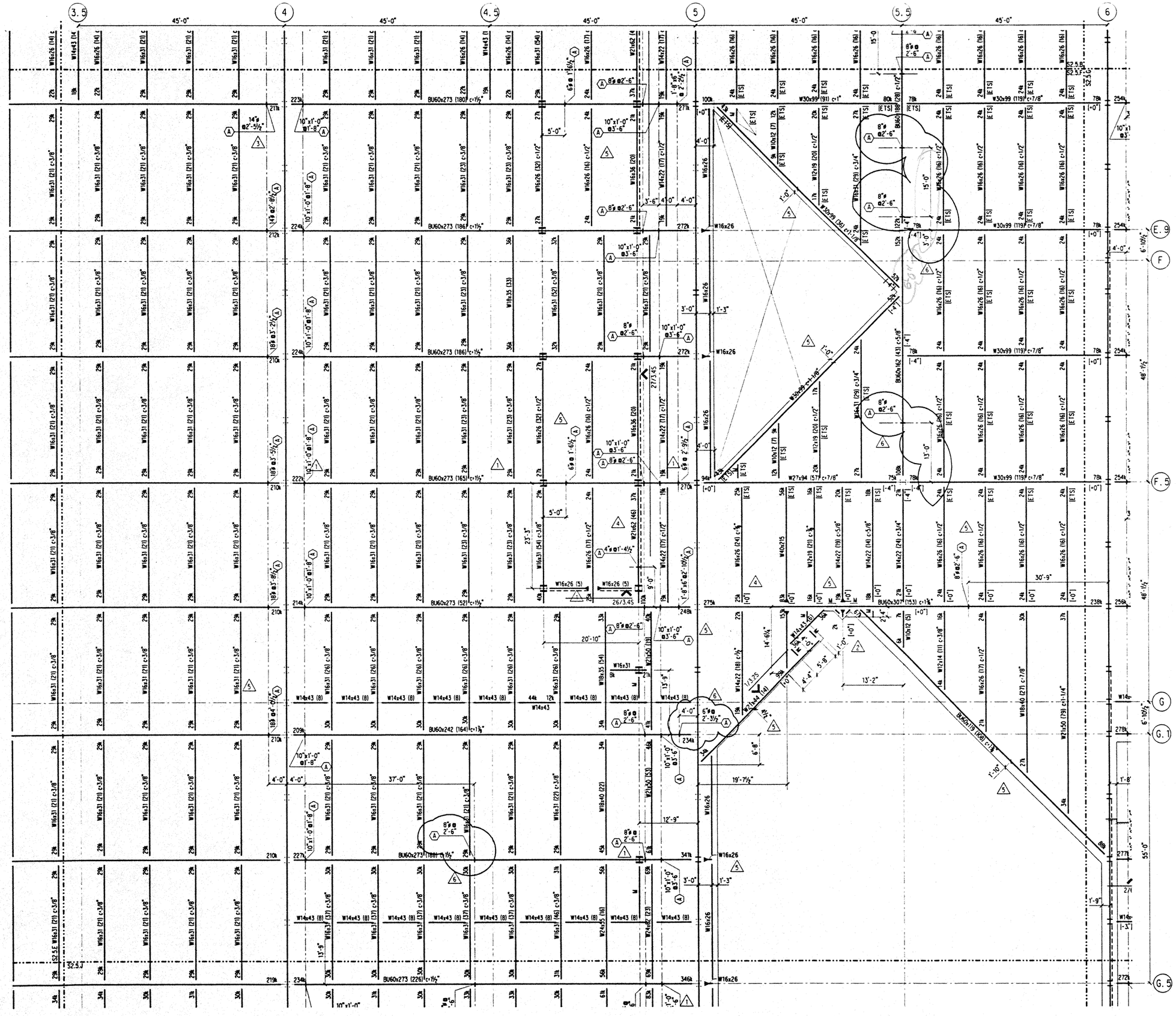

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

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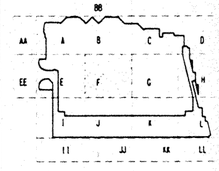
ROY D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11773
HAWAII, U.S.A.

Supervision, defined in Title 16 Chapter 8 Section 1-3 (h) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
6	12-15-95		FINAL DOCUMENT
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4	9-15-95		BUILDING PERMIT SET
3	8-31-95		
2	8-4-95		
1	6-19-95		
0	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE
FRAMING PLAN
SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.F

Thu Nov 30 17:37:43 1995

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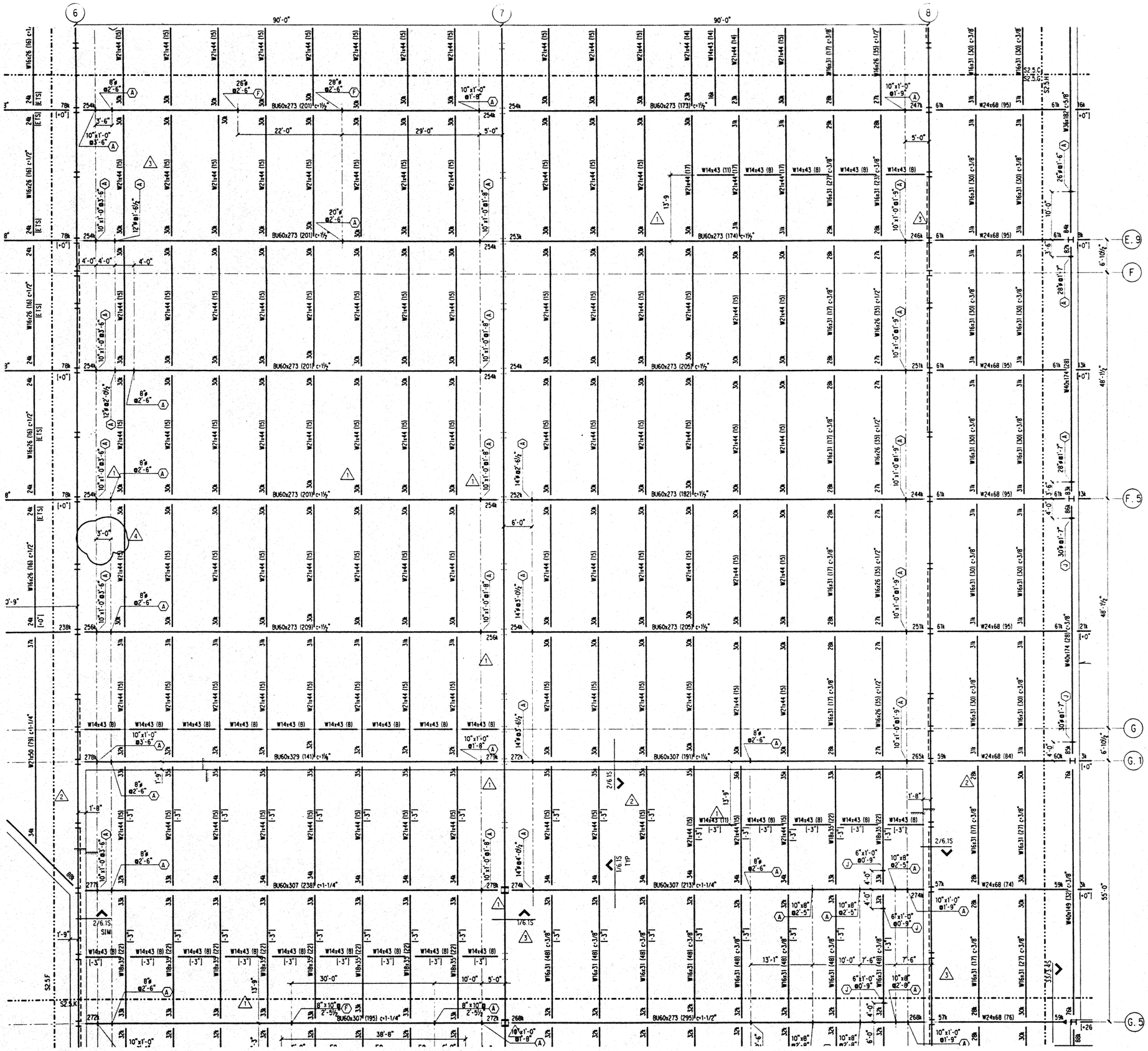
MS plt



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LICENSED PROFESSIONAL ENGINEER NO. 7773 — JOHN D. MAGNUSSON — HAWAII, U.S.A.

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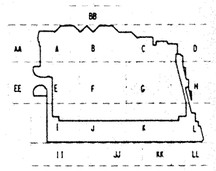
Supervision as defined in Title 16 Chapter 18 Subpart 1-2.0 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
4	12-15-95		FINAL DOCUMENT
3	11-7-95		
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	5-8-95		ADDENDUM #3

Sheet Title

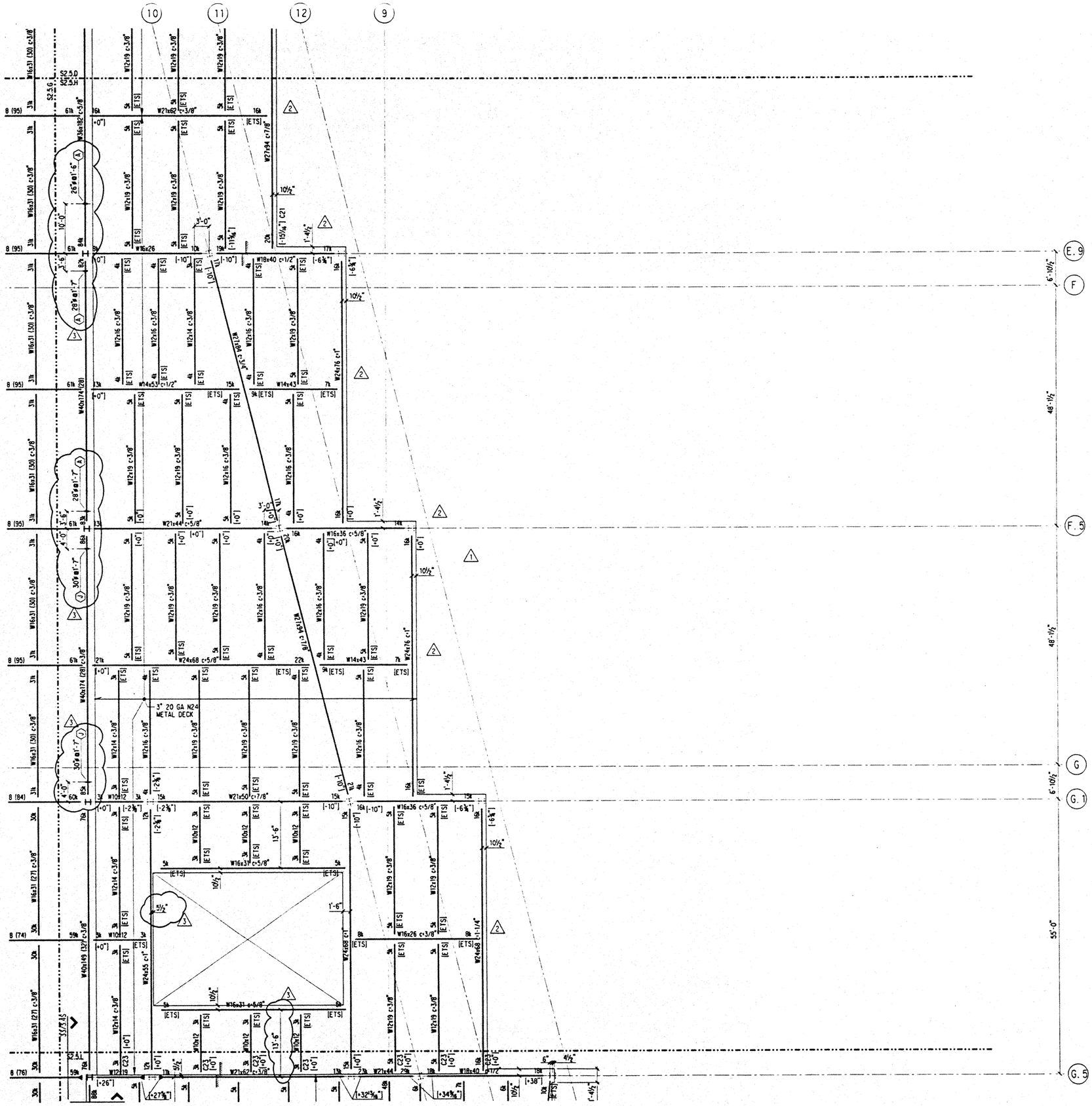
BALLROOM/TERRACE
FRAMING PLAN
SECTOR G

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.G

Thu Nov 30 17:40:14 1995 jhncball_g.dgn pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

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LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

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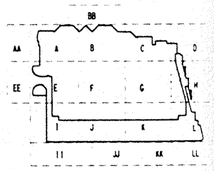

Revisions

No.	Date	By	Description
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1	11-7-95		
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE
FRAMING PLAN
SECTOR H

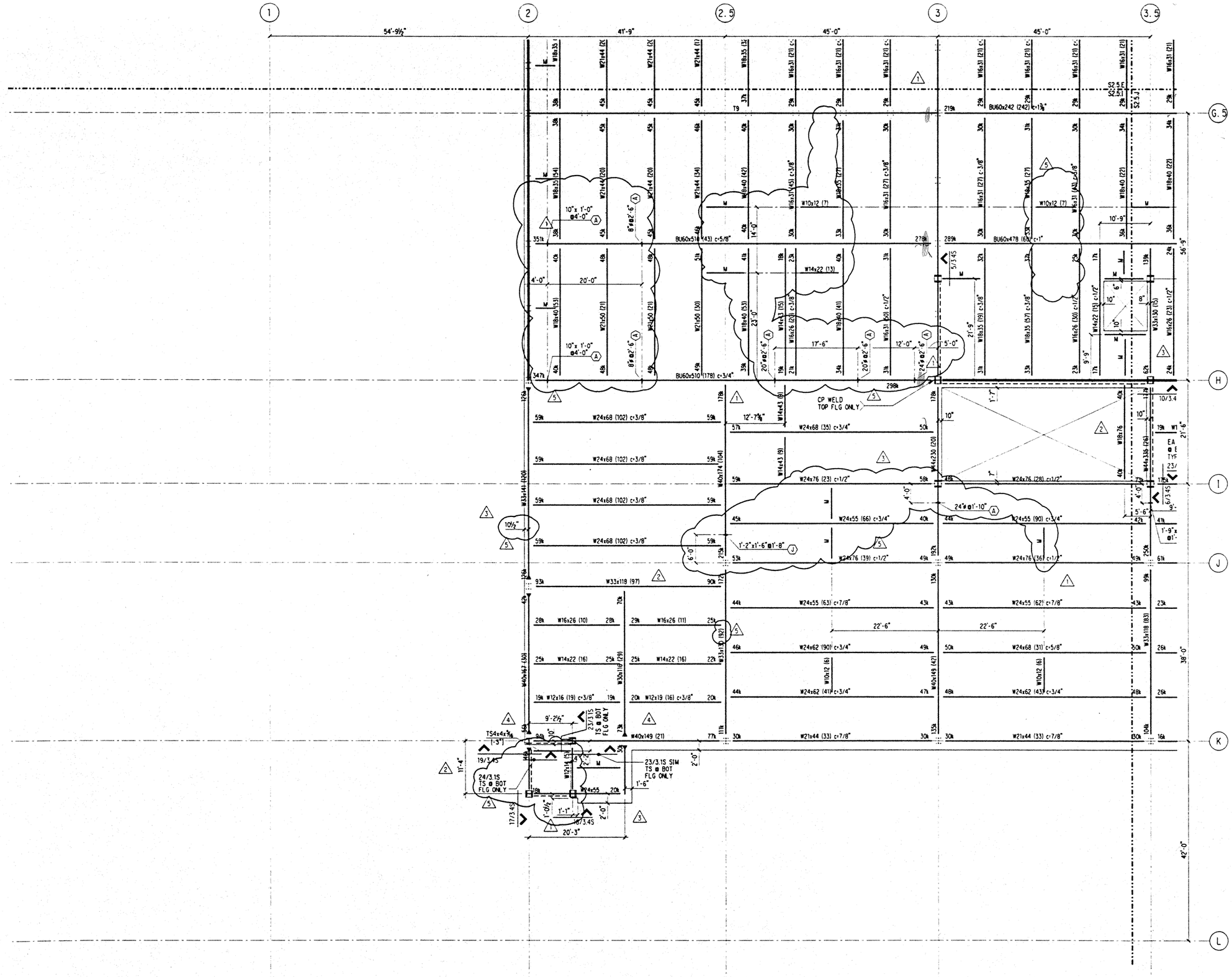
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.H

Thu Nov 30 17:42:44 1995

j:\hcc\ball_h.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

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JON S. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII, U.S.A.

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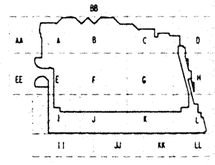
Supervision as defined in Title 16 Chapter 18 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE
FRAMING PLAN
SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

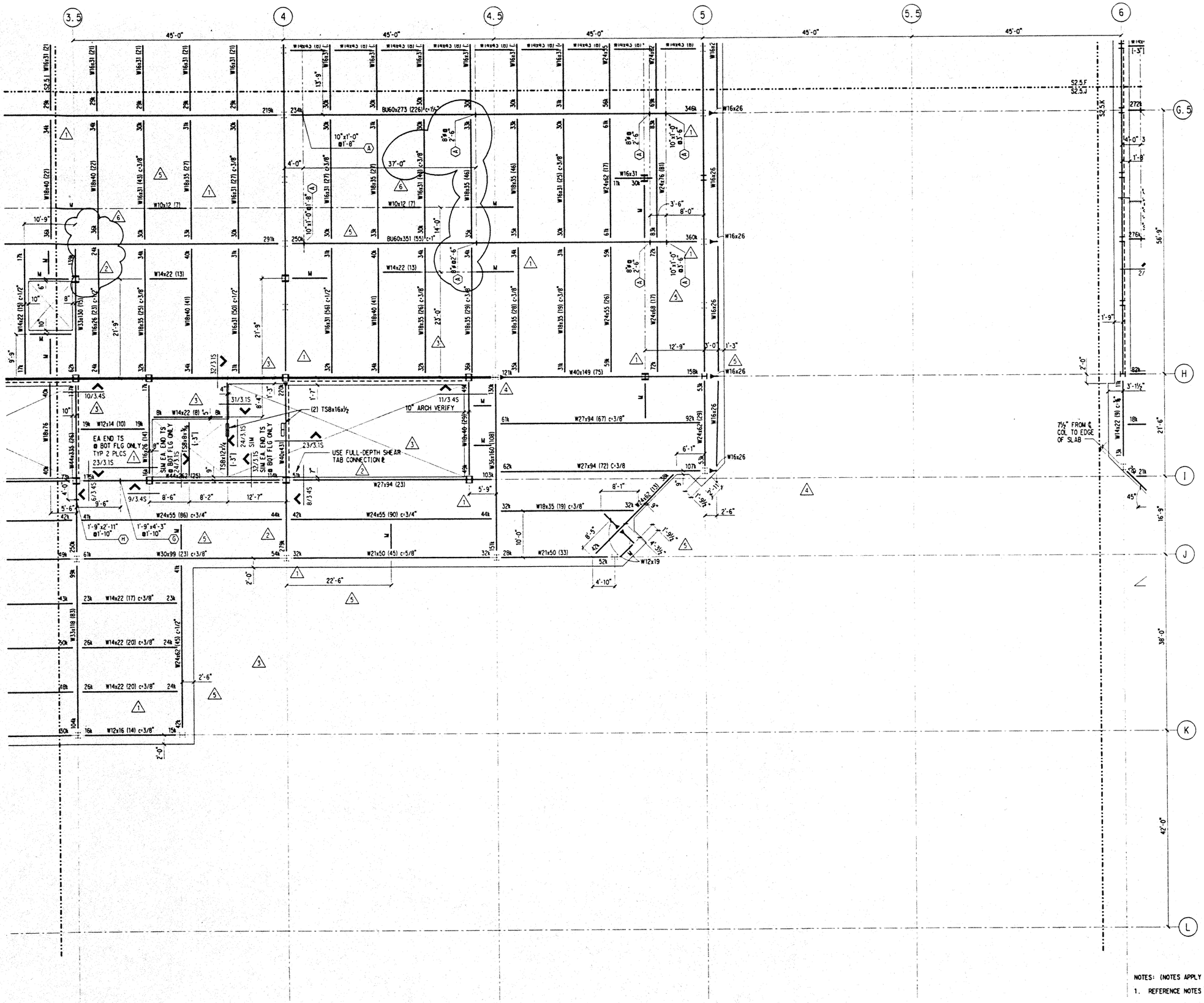
Sheet No.

S2.5.I

Thu Nov 30 17:44:51 1995

jnhccball.1.dgn

pit



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RON S. MAGNUSSON
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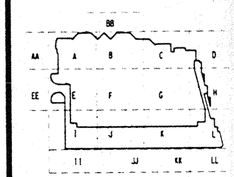
This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Surveyors are defined in Title 16 Chapter 18 Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
4	12-15-95		FINAL DOCUMENTS
3	11-7-95		
2	9-15-95		BUILDING PERMIT SET
1	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BALLROOM/TERRACE
FRAMING PLAN
SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

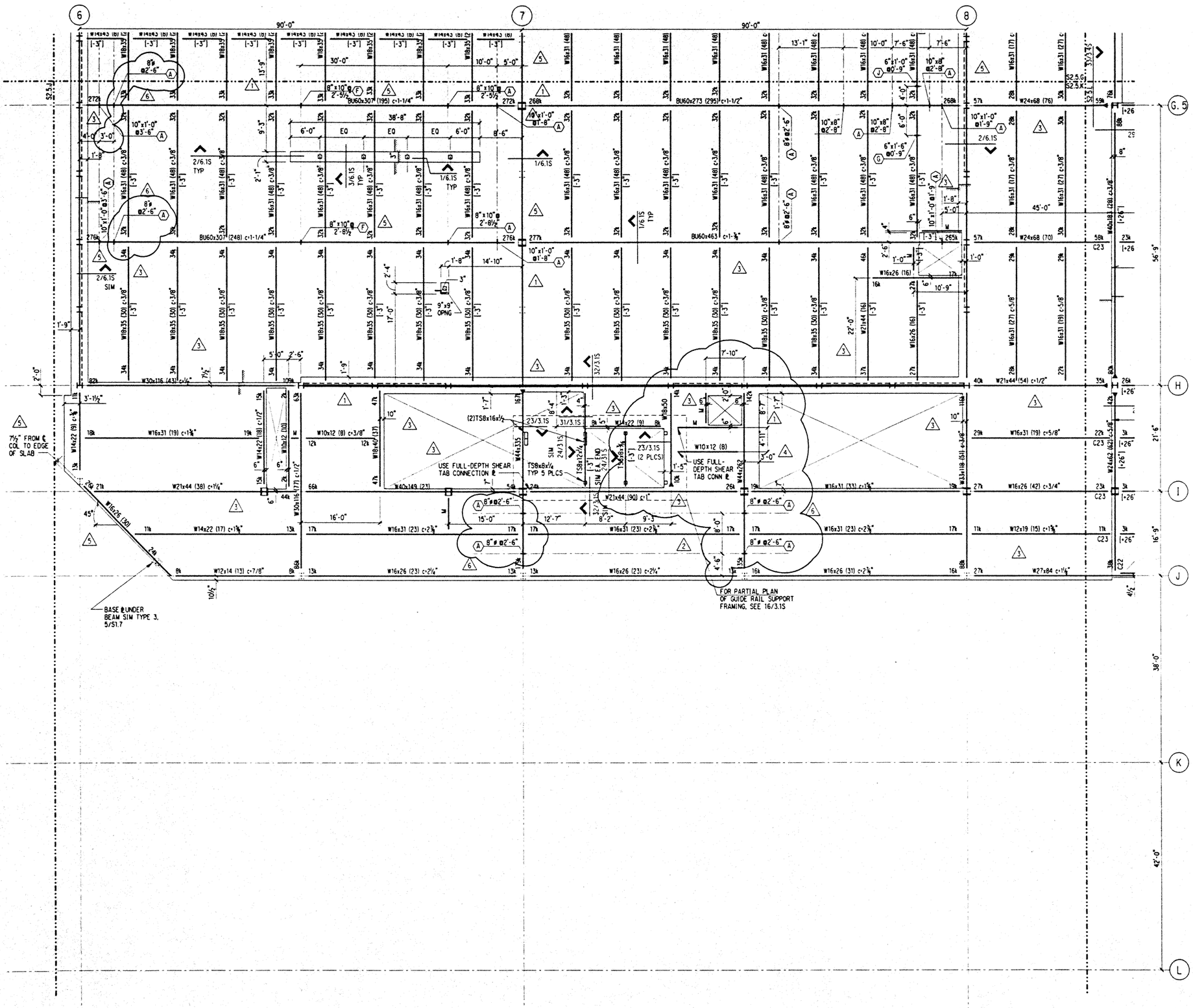
Sheet No.

S2.5.J

Thu Nov 30 17:47:20 1995

j:hccball.j.dgn

pt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11773
HAWAII, U.S.A.

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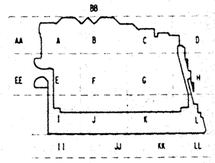
Supervision defined in Title 16 Chapter 18 Subpart 1.2 in of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

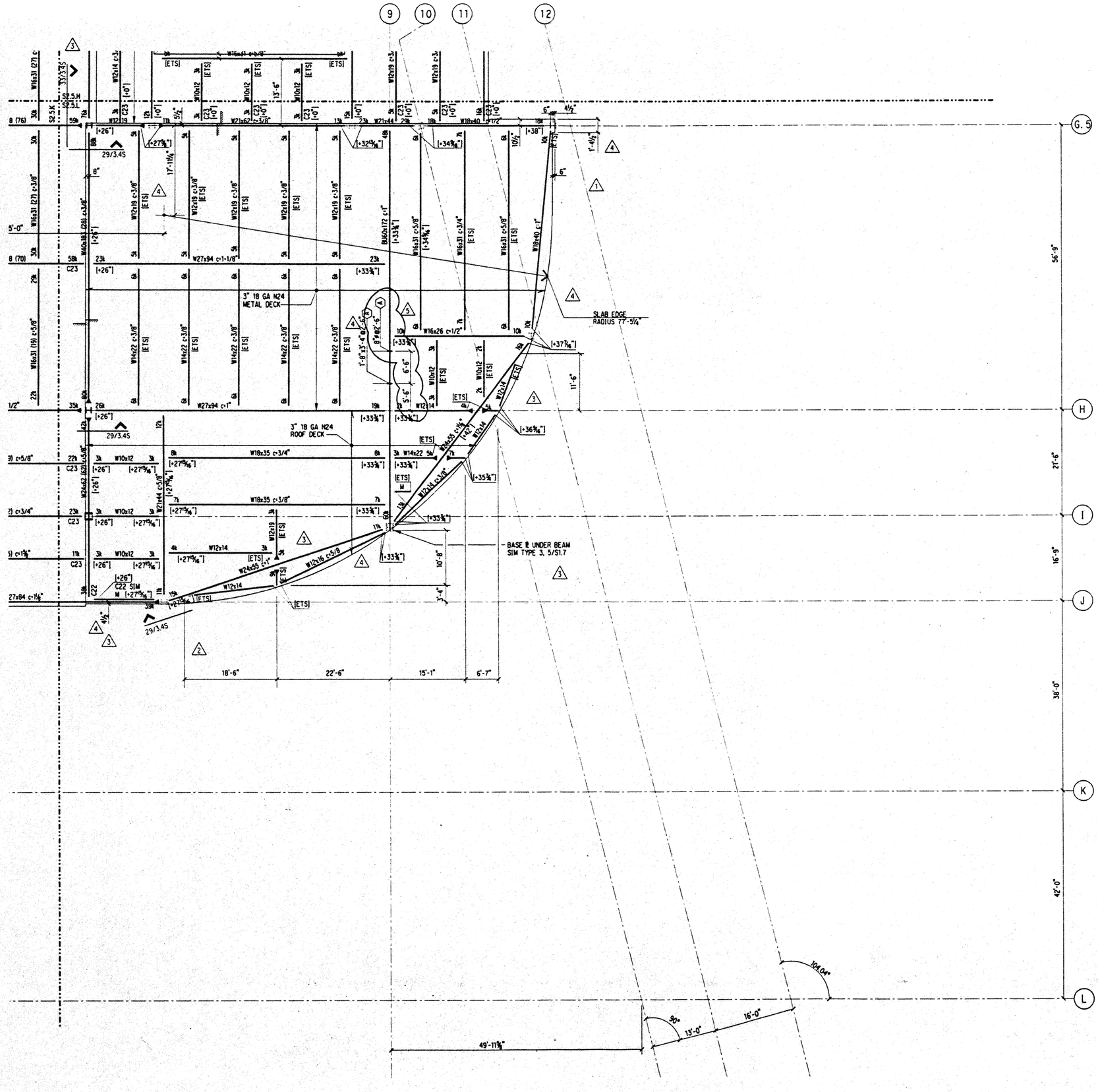
BALLROOM/TERRACE FRAMING PLAN SECTOR K

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.K

Thu Nov 30 17:49:47 1995 j:hccball_k.dgn pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

Hawai'i Convention Center

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Honolulu, Hawaii

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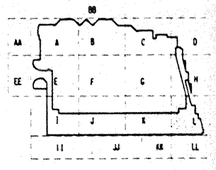
Supervision exempted in lieu of Chapter 16 Section L.2.(a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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3	9-15-95		BUILDING PERMIT SET
2	8-31-95		
1	8-4-95		
0	5-8-95		ADDENDUM #3

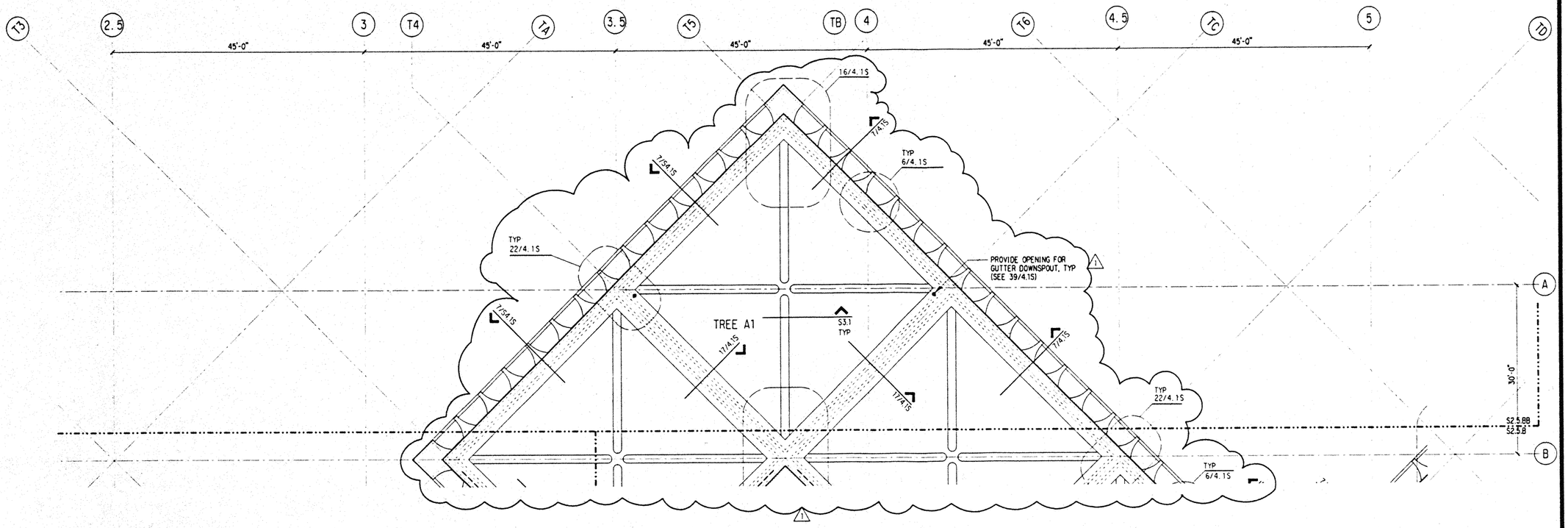
Sheet Title

BALLROOM/TERRACE
FRAMING PLAN
SECTOR L

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.5.L

DMS plt j:\hccball.l.dgn Thu Nov 30 17:52:16 1995



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.5.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
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JOHN D. MAGNUSSON
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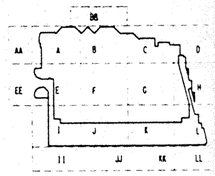
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 16 Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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	9-15-95		BUILDING PERMIT SET
1	8-31-95		

Sheet Title

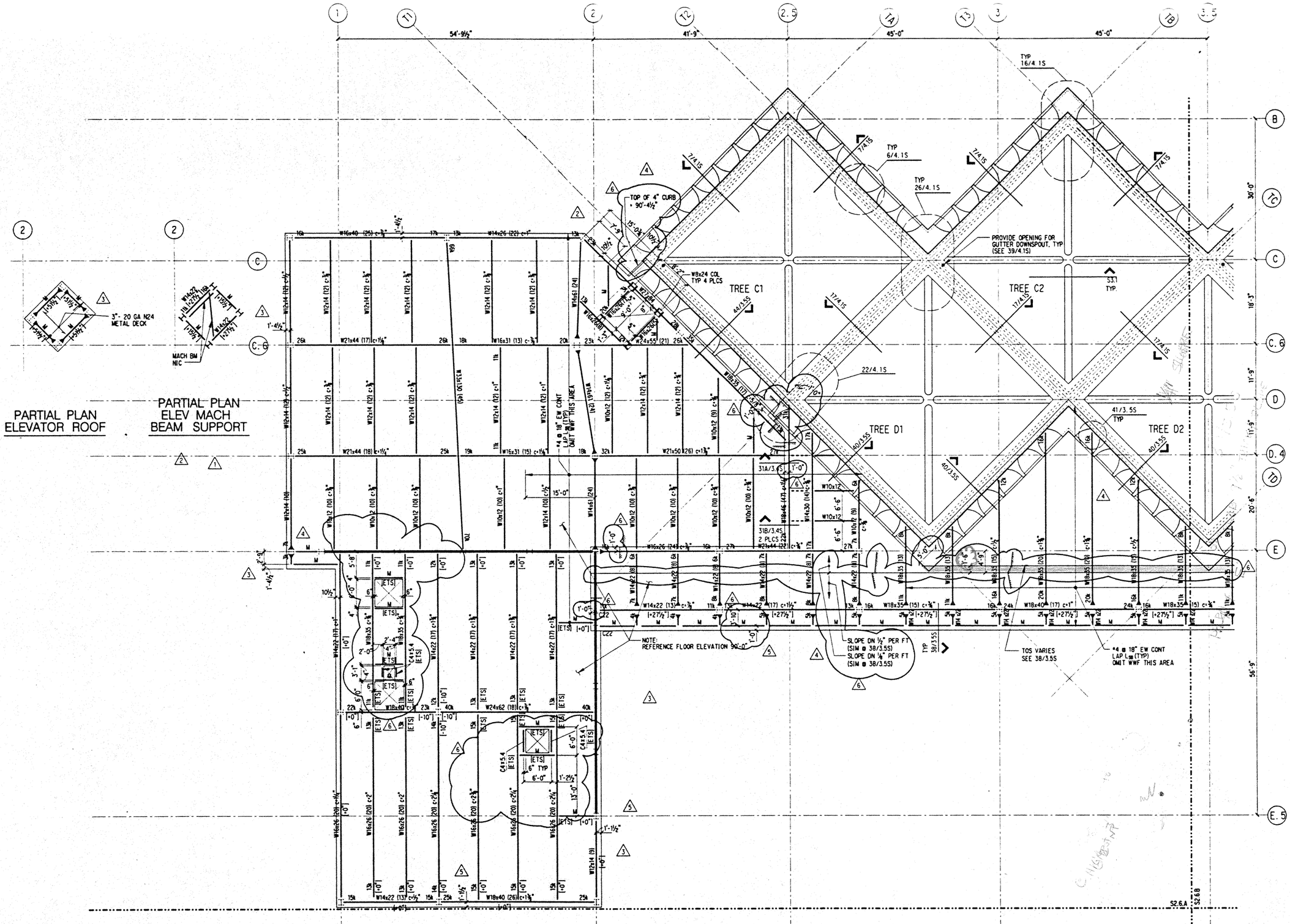
BALLROOM/TERRACE
FRAMING PLAN
SECTOR BB

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.5.BB

j:\hcc\ball_bb.dgn Thu Nov 30 17:54:41 1995 pt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.• SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.• TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
   - S1.20 CONCRETE SCHEDULES
   - S1.21 S1.22 STEEL SCHEDULES
   - S2.• PLANS
   - S3.• TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
   - S4.• BRACED FRAME ELEVATIONS
   - S5.• TRUSS ELEVATIONS
   - S6.• BRACED FRAME & TRUSS DETAILS
   - ~~DETAIL BOOK~~ STRUCTURAL DETAILS
2. REFERENCE FLOOR ELEVATION = 97'-0" UNO.
3. TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
4. ROOF DECK, 3", 20 GAGE, N24 METAL DECK, UNO.
5.
6. REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION AT SLAB AND ROOF DECK LOCATIONS, UNO.
7. WHERE NOTED THUS [+/- 8"], TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
8. STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
9. REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
10. "M" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
11. INDICATES BEAM PENETRATION, SEE S1.10
12. INDICATES HAUNCHED BEAM, SEE S1.10
13. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

Hawai'i Convention Center

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JOHN D. MAGNUSSON
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NO. 11773
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Revisions

No.	Date	By	Description
4	12-15-95		FINAL DOCUMENT
3	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-14-95		
2	6-19-95		
1	5-8-95		ADDENDUM #4

Sheet Title

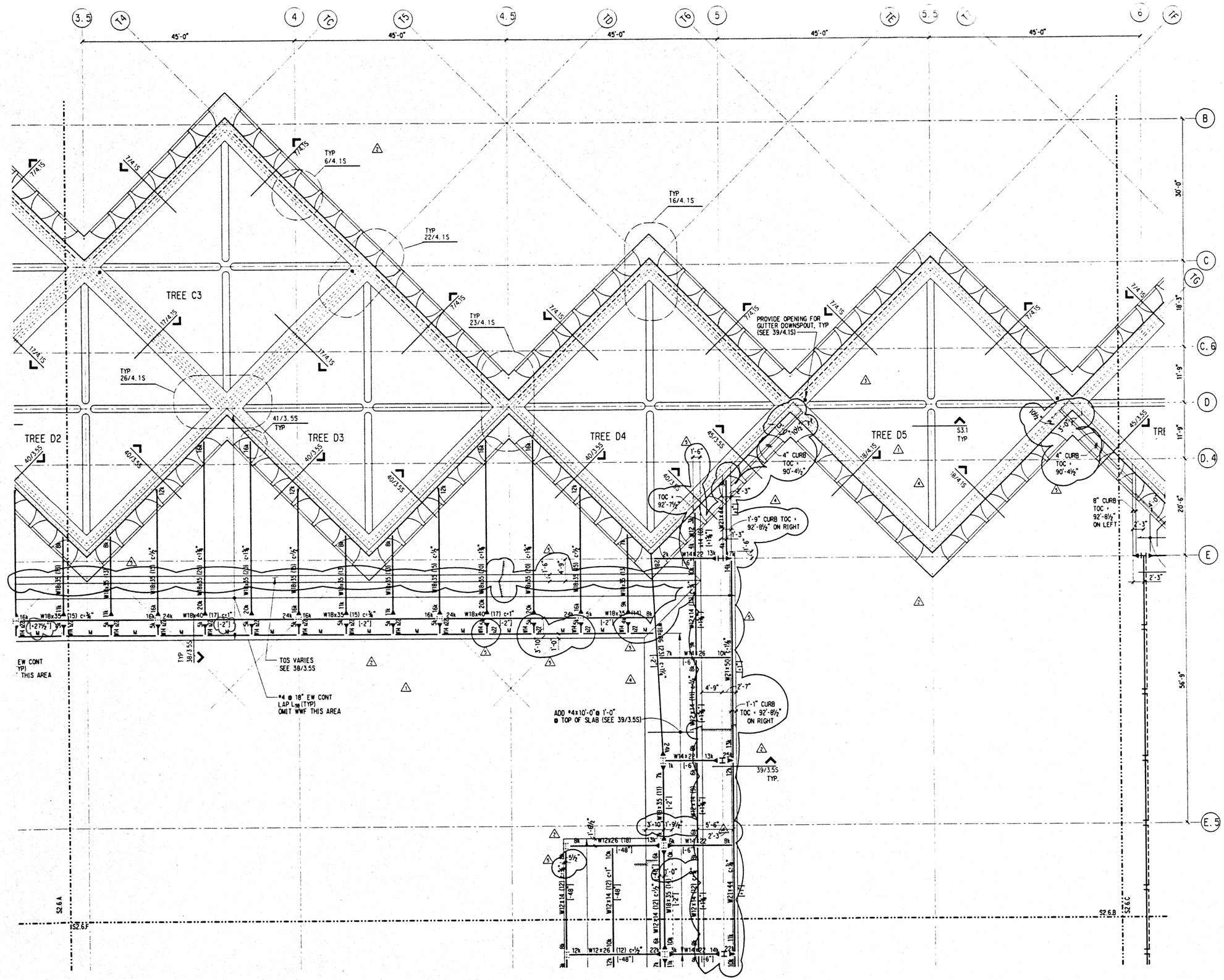
MECHANICAL PENTHOUSE FRAMING PLAN SECTOR A

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.6.A

Thu Nov 30 16:26:51 1995 jxhccmech.a.dgn pit DMS



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.6.A
2. REFERENCE FLOOR ELEVATION = 92'-5½".

Hawai'i Convention Center

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Honolulu, Hawaii

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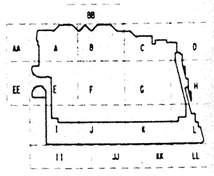
LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer in Title of Chapter 16 Subdivision 1, 2 or 3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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4	11-7-95		
3	9-20-95		TREE REVISIONS
	9-15-95		BUILDING PERMIT SET
2	8-31-95		
1	8-4-95		

Sheet Title

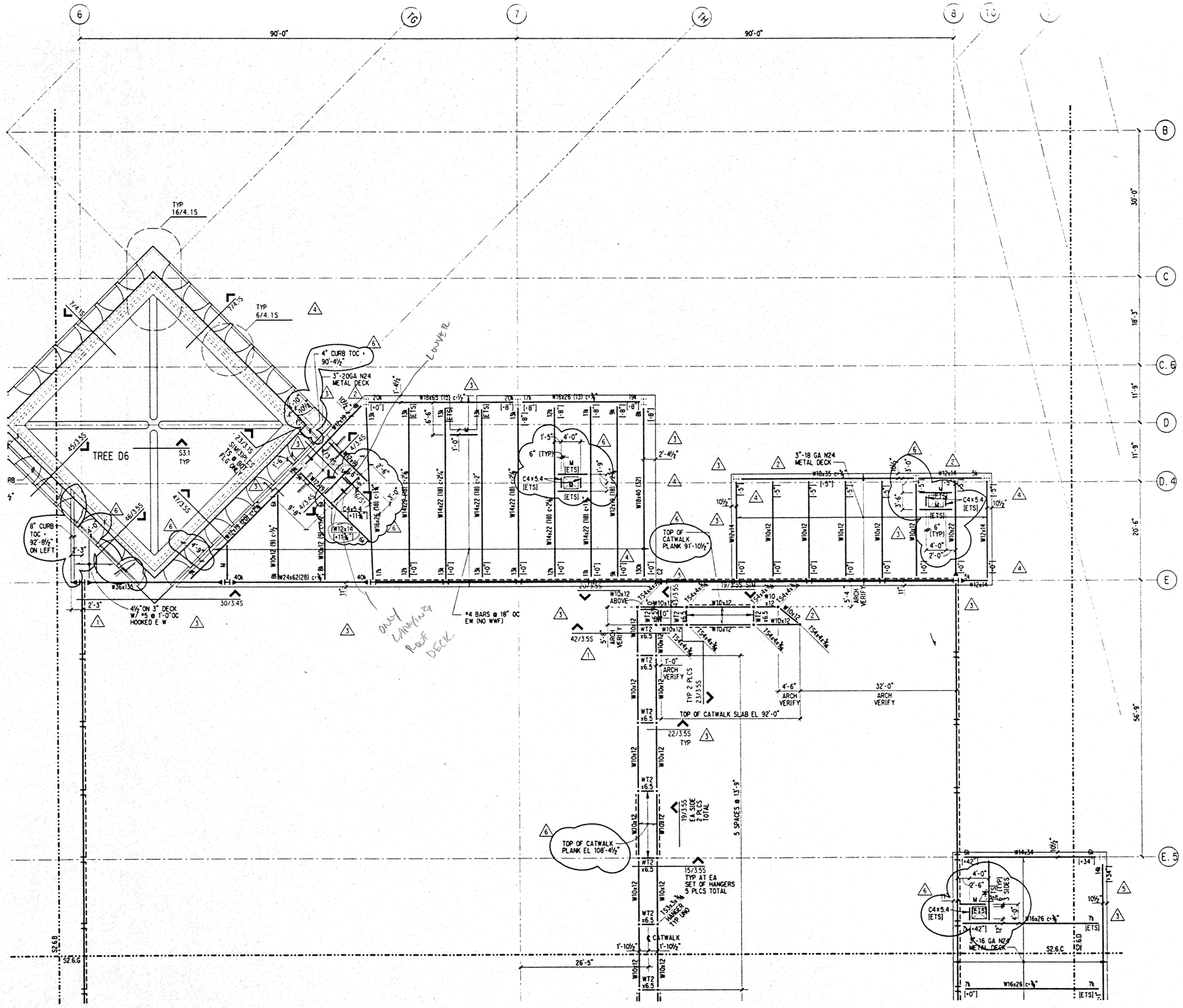
MECHANICAL PENTHOUSE FRAMING PLAN SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.6.B

Thu Nov 30 16:29:51 1995 jhccmech.b.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.6.A
2. REFERENCE FLOOR ELEVATION = 93'-6".

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Honolulu, Hawaii
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Seattle, Washington

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Honolulu, HI

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JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER NO. 11773
HAWAII, U.S.A.

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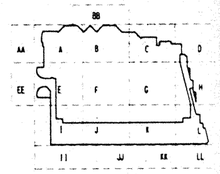
Supervision is exercised in full in Chapter 16 Subtitle 1 Title 16 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

MECHANICAL PENTHOUSE FRAMING PLAN SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.6.C

Thu Nov 30 16:32:20 1995

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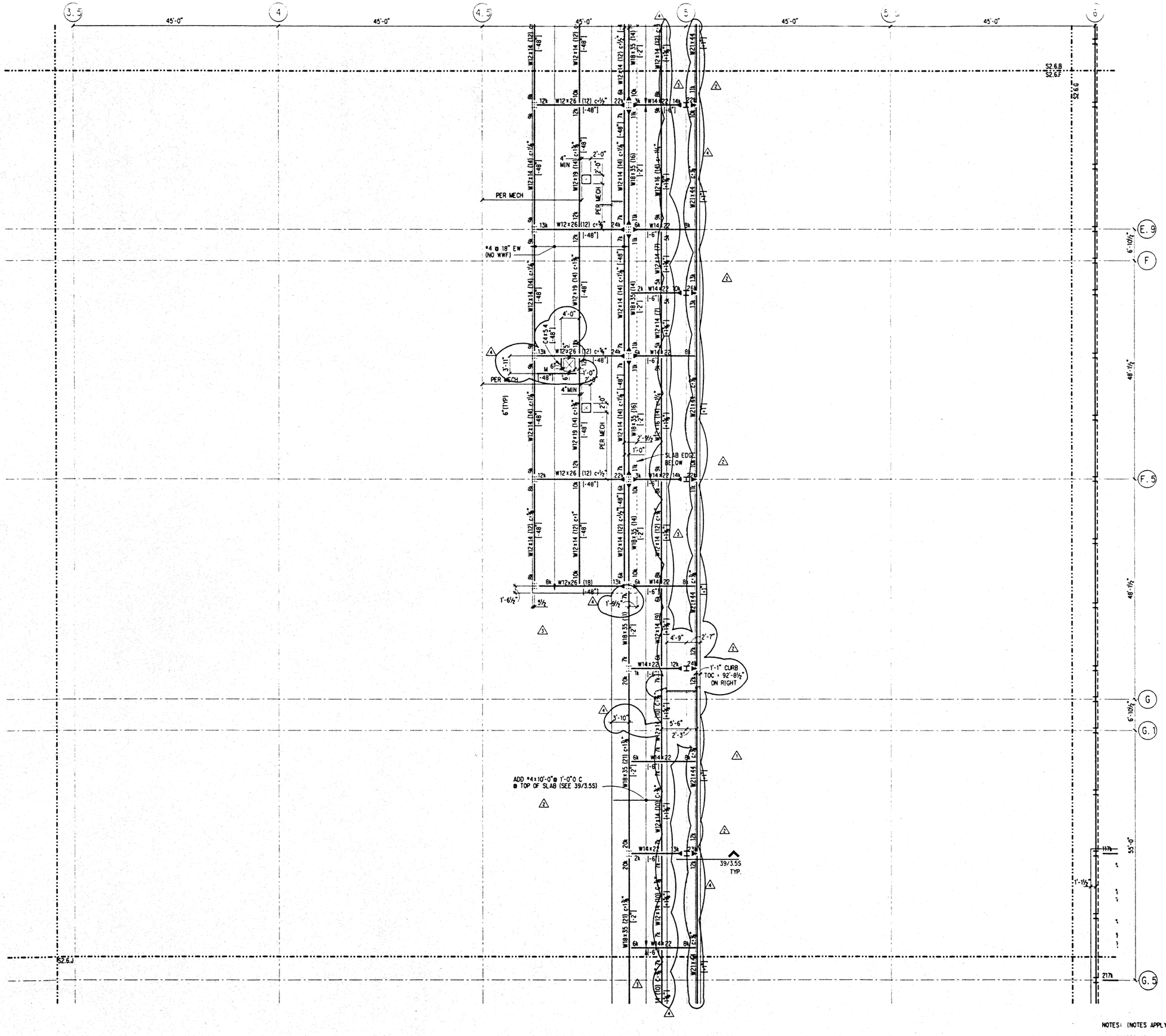
pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.6.A
2. REFERENCE FLOOR ELEVATION = 92'-5½". △1

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

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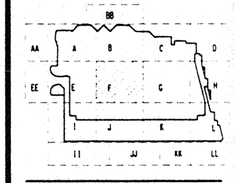
LICENSED PROFESSIONAL ENGINEER NO. 7177S
JON D. MAGNUSSON
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 15 Section 1.3.H of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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3	11-7-95		
	9-15-95		BUILDING PERMIT SET
2	9-31-95		
1	8-4-95		

Sheet Title

MECHANICAL PENTHOUSE FRAMING PLAN SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.6.F

Thu Nov 30 16:34:45 1995
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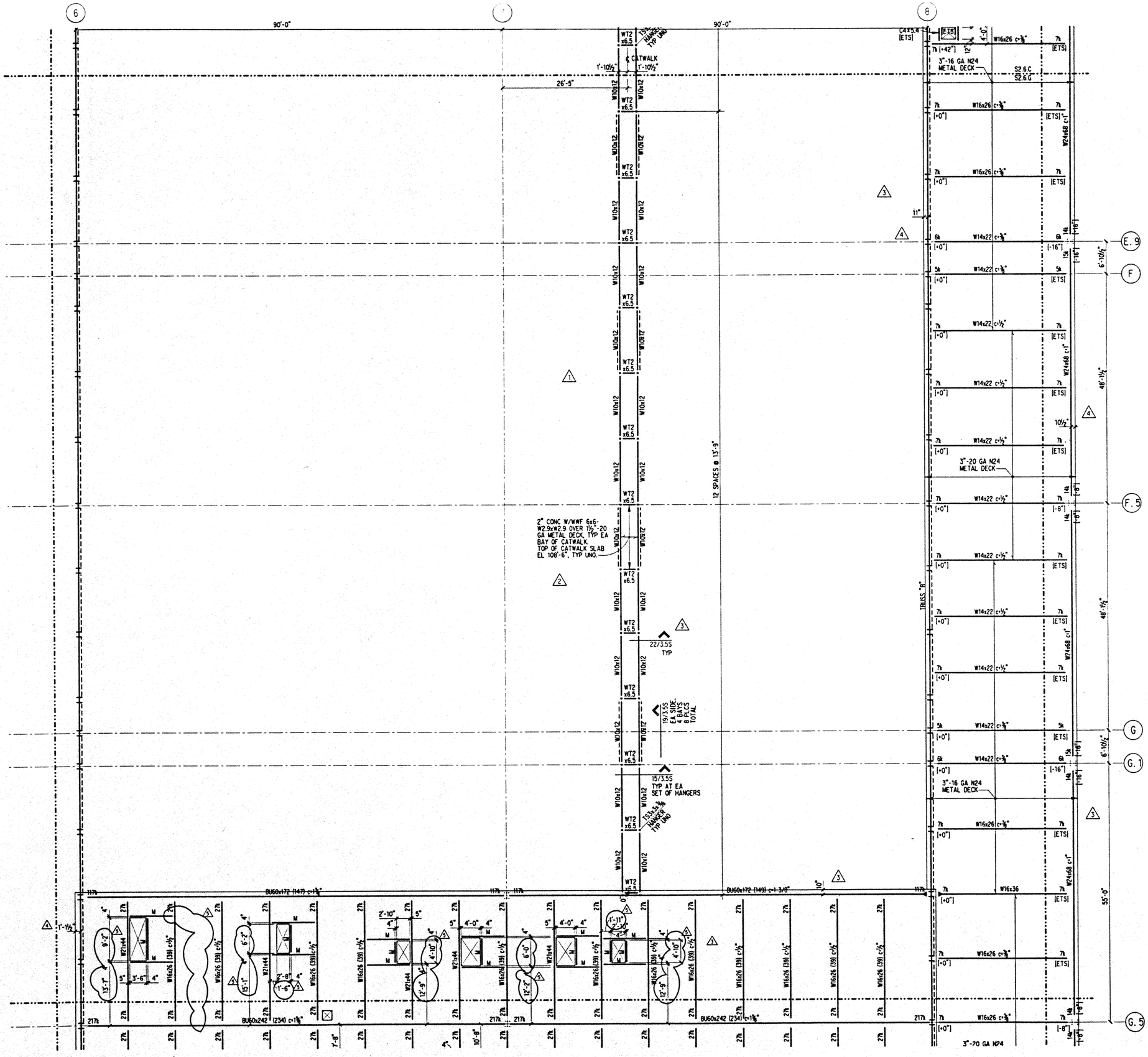
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.6.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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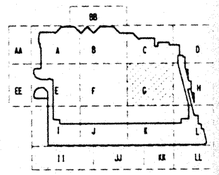
LICENSED PROFESSIONAL ENGINEER NO. 11773 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title III Chapter 16 Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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4	11-7-95		
	9-15-95		BUILDING PERMIT SET
1	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

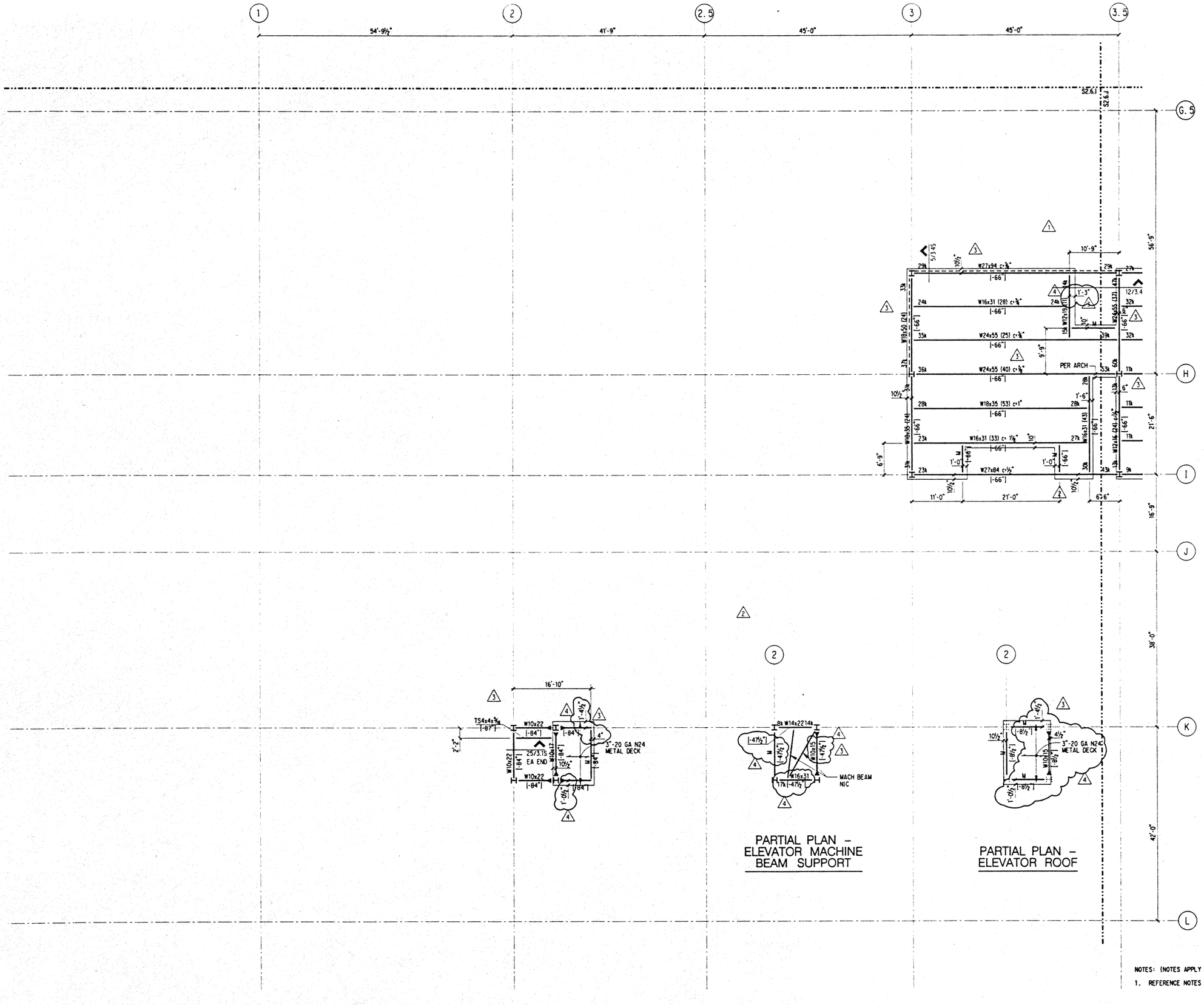
Sheet Title

MECHANICAL PENTHOUSE FRAMING PLAN SECTOR G

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.6.G

Thu Nov 30 16:37:12 1995 jyhccmech.g.dgn DMS pit



PARTIAL PLAN – ELEVATOR MACHINE BEAM SUPPORT

PARTIAL PLAN – ELEVATOR ROOF

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.6.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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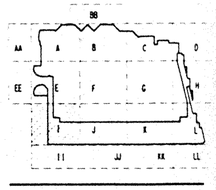
LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 53, Section 2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



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No.	Date	By	Description
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4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

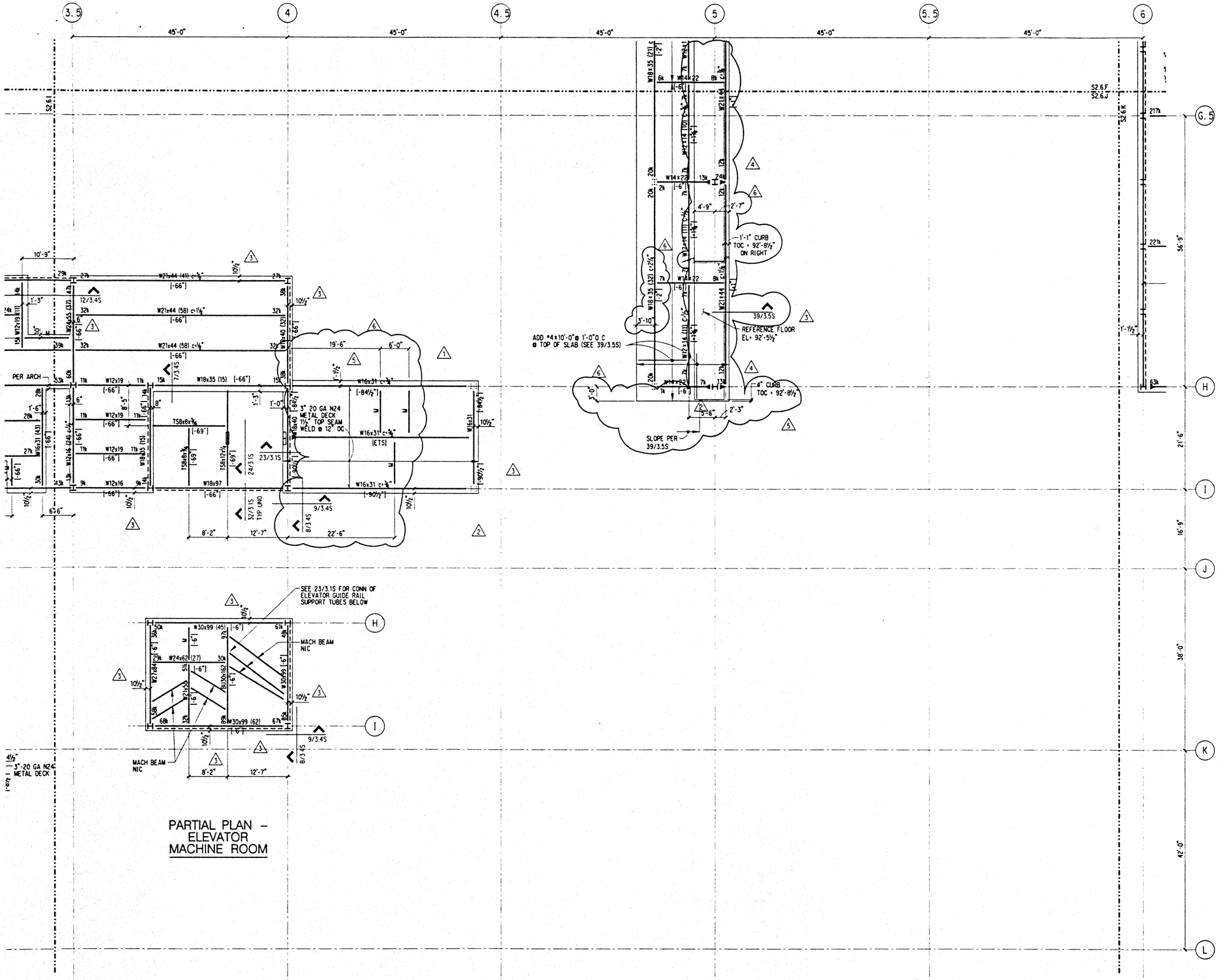
MECHANICAL PENTHOUSE FRAMING PLAN SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.6.I

Thu Nov 30 16:39:21 1995 hccmech.l.dgn pit



PARTIAL PLAN - ELEVATOR MACHINE ROOM

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.6.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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in association with

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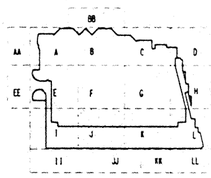
Supervision is defined in Title 16 Chapter 53 Subtitle 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
5	12-15-95		FINAL DOCUMENT
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

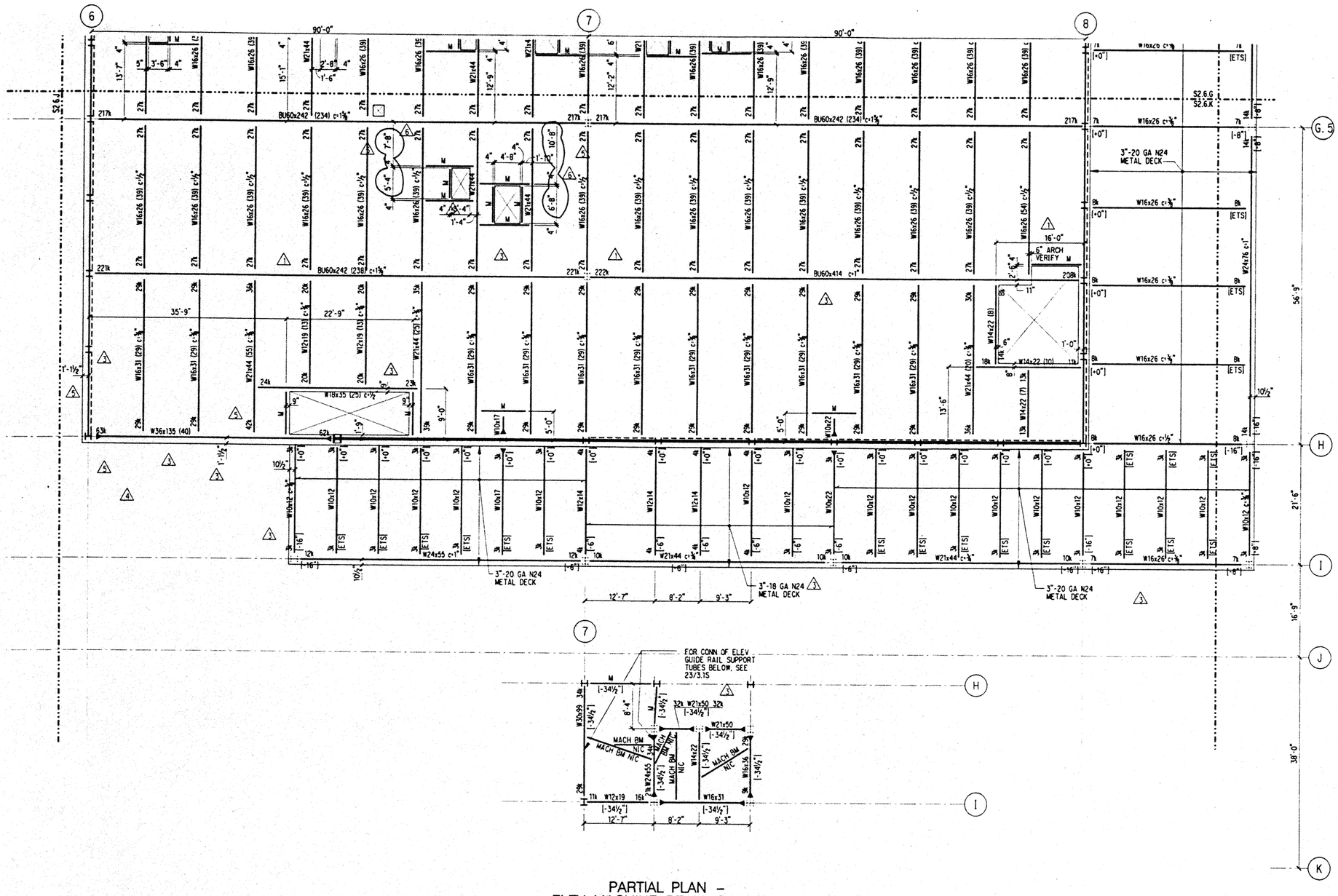
MECHANICAL PENTHOUSE FRAMING PLAN SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

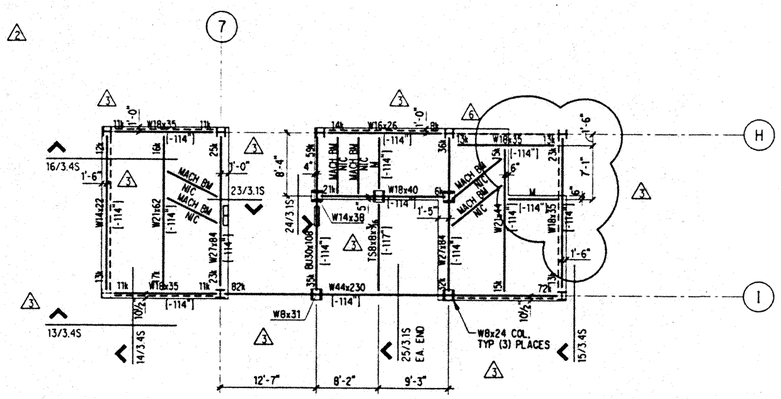
Sheet No.

S2.6.J

Thu Nov 30 16:41:45 1995 jhccmech_j.dgn pif



PARTIAL PLAN –
ELEV MACHINE BEAM SUPPORT



PARTIAL PLAN –
ELEVATOR MACHINE ROOM

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.6.A

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
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LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

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Supervision is defined in Title of Chapter 16 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.

Revisions

No.	Date	By	Description
4	12-15-95		FINAL DOCUMENT
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

MECHANICAL
PENTHOUSE
FRAMING PLAN
SECTOR K

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

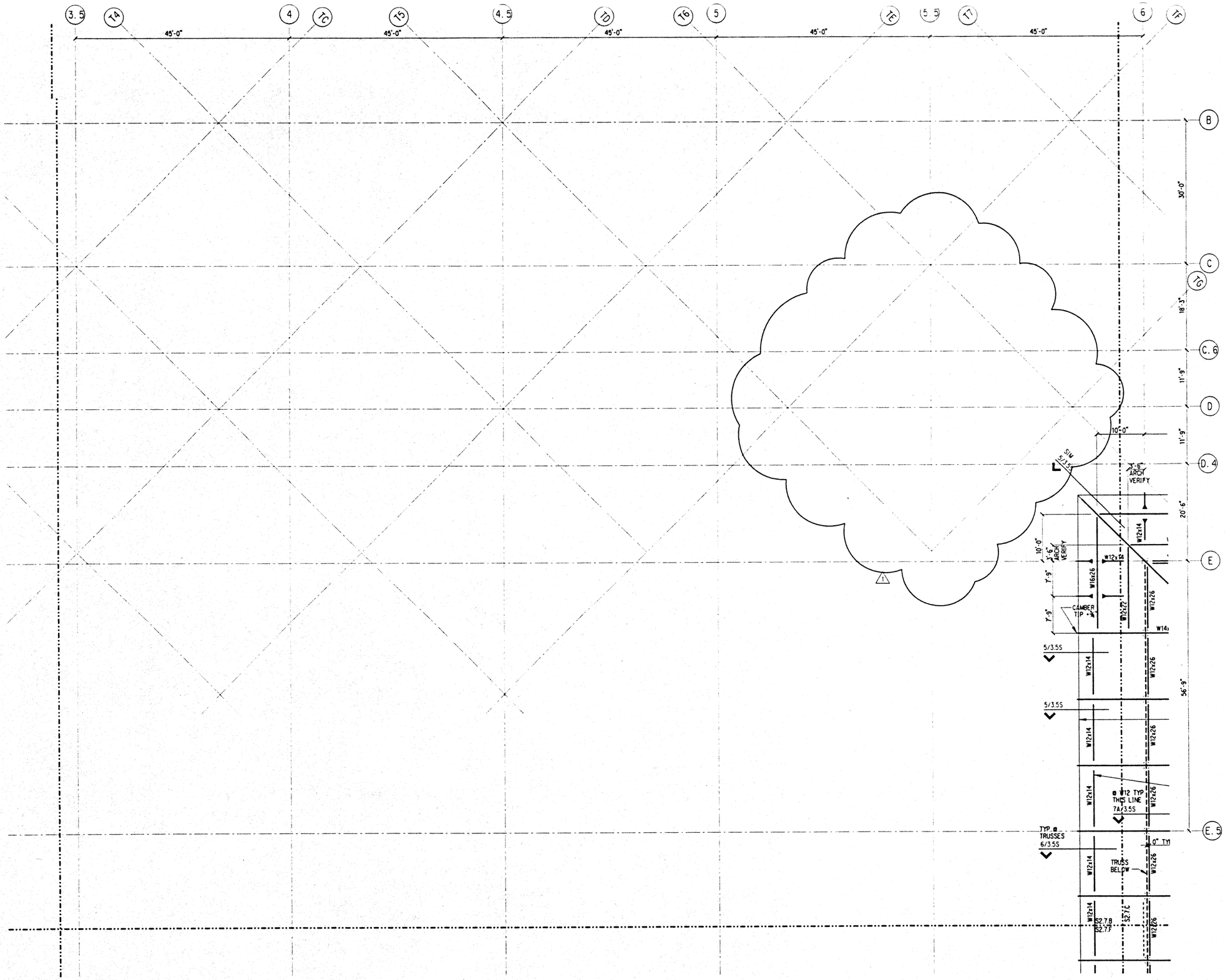
Sheet No.

S2.6.K

Thu Nov 30 16:44:10 1995

j:\hccmech_k.dgn

DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE DRAWINGS:

S0.*	SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
S1.*	TYPICAL FOUNDATION, CONCRETE, STEEL AND CMU DETAILS
S1.20	CONCRETE SCHEDULES
S1.21 S1.22	STEEL SCHEDULES
S2.*	PLANS
S3.*	TYPICAL STEEL COLUMN TREE ELEVATIONS AND PLANS
S4.*	BRACED FRAME ELEVATIONS
S5.*	TRUSS ELEVATIONS
S6.*	BRACED FRAME & TRUSS DETAILS
DETAIL BOOK	STRUCTURAL DETAILS

2. REFERENCE FLOOR ELEVATION, SEE PLAN.
3. ROOF DECK, 3", 20 GAGE, N24 METAL DECK, UNO.
4. TOP OF SLAB IS AT REFERENCE FLOOR ELEVATION, UNO.
5. REFERENCE TOP OF STRUCTURAL STEEL IS 5-1/2" BELOW REFERENCE ELEVATION AT SLAB AND ROOF DECK LOCATIONS, UNO.
6. WHERE NOTED THUS [+/- 8"], TOP OF STEEL IS 8" ABOVE/BELOW REFERENCE TOP OF STRUCTURAL STEEL.
7. STRUCTURAL SLAB IS 2-1/2" OF HARDROCK CONCRETE ON 3" STEEL DECK, UNO.
8. REINFORCE SLAB WITH WWF 6X6 - W2.9 X W2.9, UNO.
9. "W" DENOTES W10X12 BEAM, FY=50 KSI, MIN. STUDS.
10. INDICATES BEAM PENETRATION, SEE S1.10
11. INDICATES HAUNCHED BEAM, SEE S1.10
12. REFERENCE ALL CONSTRUCTION DOCUMENTS FOR SIZE, EXTENT AND LOCATION OF CONCRETE CURBS, HOUSEKEEPING PADS, CMU WALLS, PLANTER WALLS AND SLAB PENETRATIONS. REINFORCE PER TYPICAL DETAILS.

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GIR. S. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII, U.S.A.

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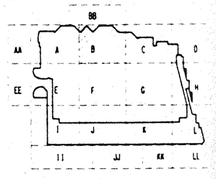
Supervision as defined in Title 16 Chapter 18 Section L2-4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	8-4-95		

Sheet Title

ROOF FRAMING PLAN SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.7.B

Thu Nov 30 15:07:25 1995

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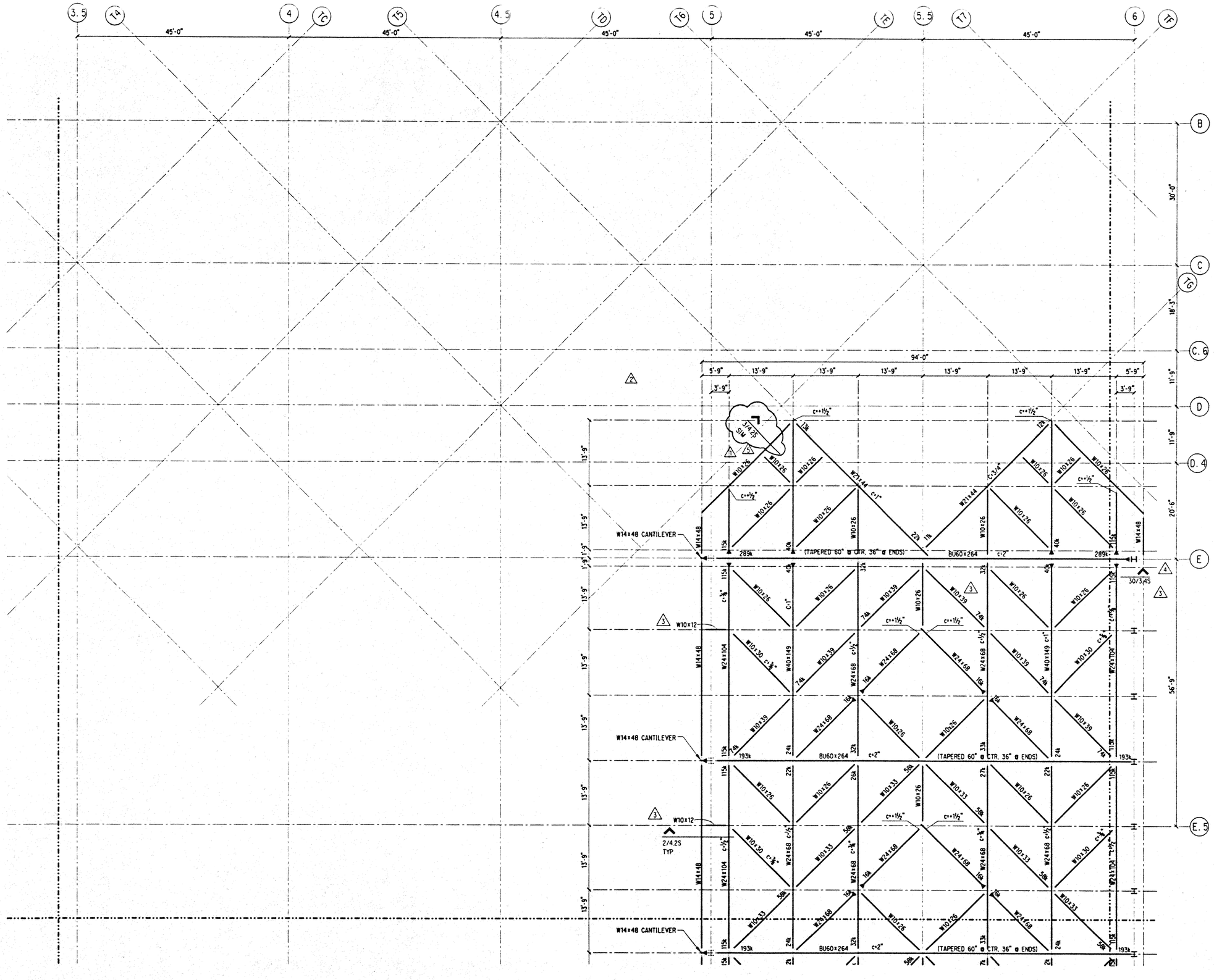
us pit



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B
2. ALL BEAMS ON THE TRELLIS FRAMING PLAN, FRAMING INTO SLOPING MEMBERS, SHALL HAVE THE SAME TOP OF STEEL ELEVATION AT THE ENDS AS THEIR SUPPORTING BEAMS, UNO.

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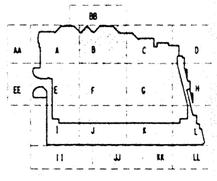
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is optional in Title 16, Chapter 18, Section 1.24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
5	12-15-95		FINAL DOCUMENT
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-31-95		
2	8-4-95		
1	6-19-95		

Sheet Title

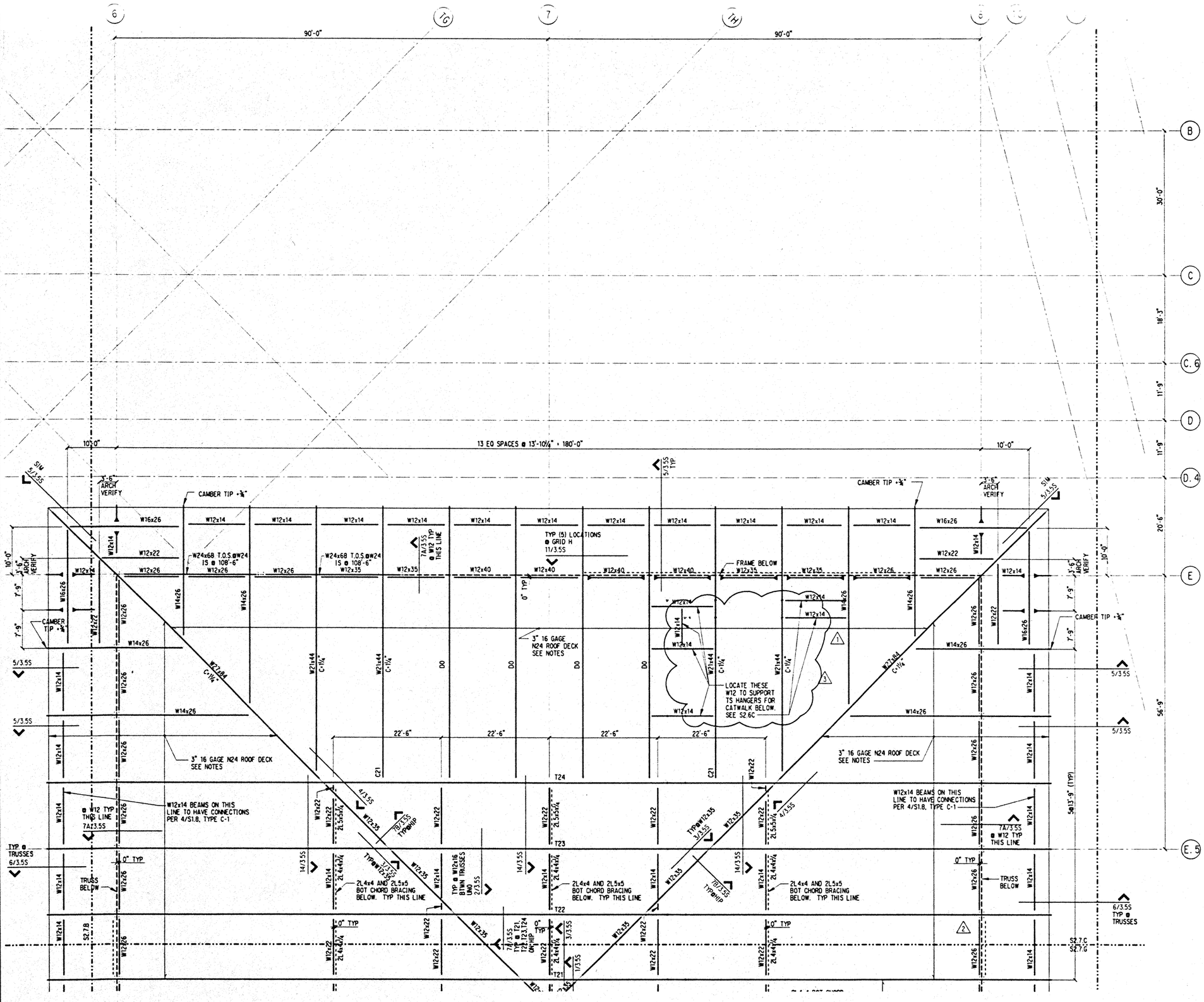
ROOF TRELLIS
FRAMING PLAN
SECTOR B

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.7.BT

Thu Nov 30 15:09:41 1995
j:\hccroof_bt.dgn
DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B
2. METAL ROOF DECK IS TO BE THE SIZE AND GAGE SHOWN ON THE PLAN. ALL UNITS ARE TO BE FASTENED TO EACH SUPPORT WITH (4) ¾"ø PUDDLE WELDS ALONG THE WIDTH AND 1½" TOP SEAM WELDS AT 1'-0" ON SIDE BOUNDARIES.
3. FOR TOP OF STEEL ELEVATIONS AT TRUSSES REFER TO TRUSS ELEVATIONS ON S5.9.
4. FOR TOP OF STEEL ELEVATIONS @ BEAMS REFER TO DETAILS.
5. FOR LOCATION OF CATWALK HANGERS AND BRACES HUNG FROM BOTTOM CHORD OF TRUSSES REFER TO S2.6C AND S2.6G.

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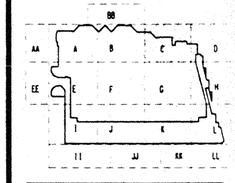
LICENSED PROFESSIONAL ENGINEER NO. 7173 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer as defined in Title 16 Chapter 18 Section 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
3	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	5-8-95		ADDENDUM #3

Sheet Title

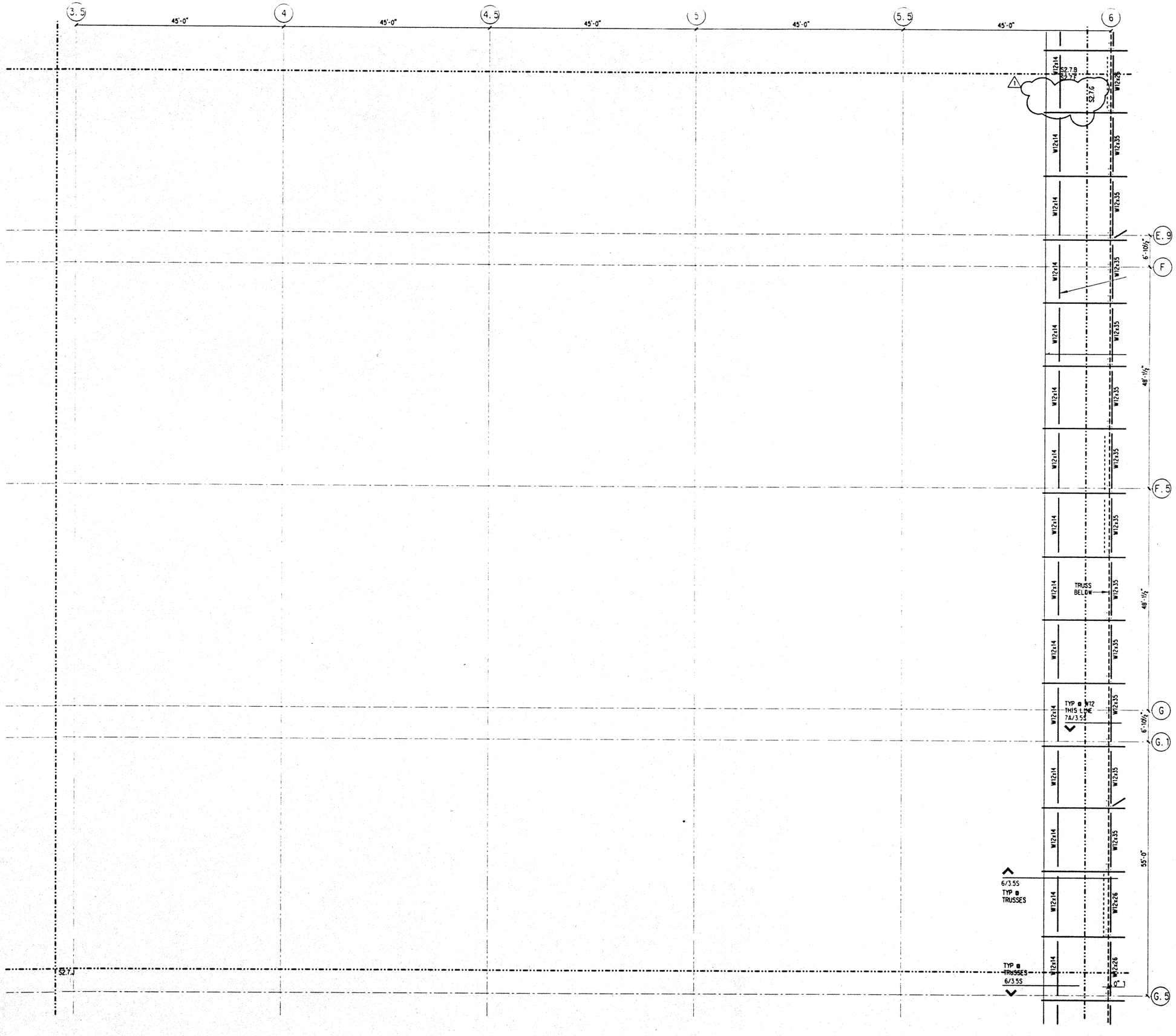
ROOF FRAMING PLAN SECTOR C

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.7.C

Thu Nov 30 15:11:59 1995 j:\hccr\roof_c.dgn DMS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B

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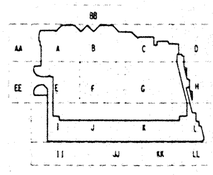
JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7177S
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer Certified on Files in Chapter 16 Subpart 1.3.4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	8-4-95		

Sheet Title

ROOF FRAMING PLAN SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

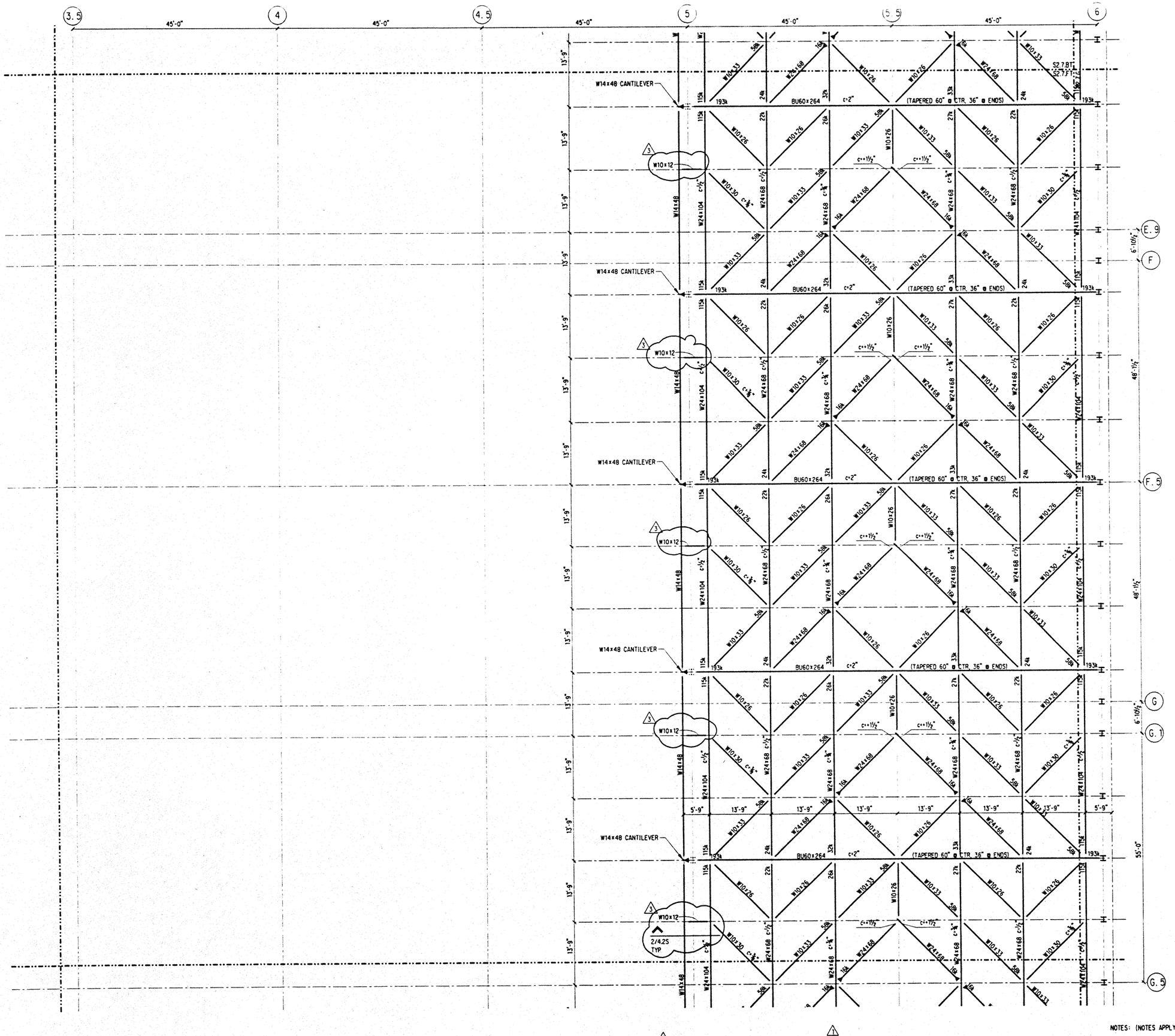
Sheet No.

S2.7.F

Thu Nov 30 15:14:11 1995

j:\hccroof-f.dgn

DWG: plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B
2. SEE NOTE 2/S2.7.BT

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JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 71775
HAWAII, U.S.A.

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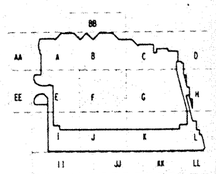
Supervision is defined in Title 16, Chapter 18 Subtitle 1, Part of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
3	8-31-95		
2	8-4-95		
1	6-19-95		

Sheet Title

ROOF TRELLIS
FRAMING PLAN
SECTOR F

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.7.FT

Thu Nov 30 15:16:16 1995

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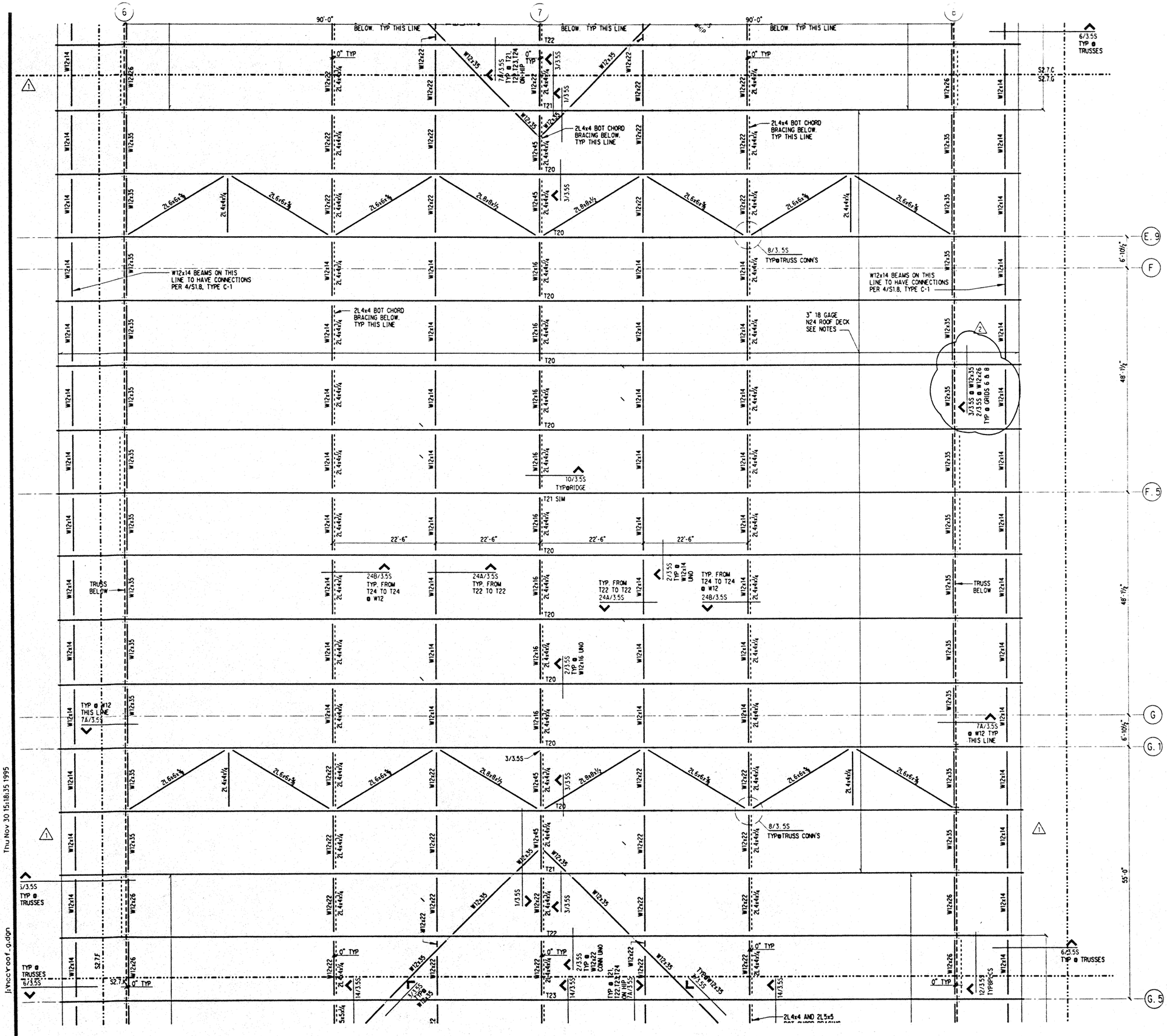
pt1



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B
2. METAL ROOF DECK IS TO BE THE SIZE AND GAGE SHOWN ON THE PLAN. ALL UNITS ARE TO BE FASTENED TO EACH SUPPORT WITH (4) ⅜"ø PUDDLE WELDS ALONG THE WIDTH AND 1½" TOP SEAM WELDS AT 1'-0" ON SIDE BOUNDARIES.
3. FOR TOP OF STEEL ELEVATIONS @ TRUSSES REFER TO TRUSS ELEVATIONS ON S5.9.
4. FOR TOP OF STEEL ELEVATIONS @ BEAMS REFER TO DETAILS.
5. FOR LOCATION OF CATWALK HANGERS AND BRACES HUNG FROM BOTTOM CHORD OF TRUSSES REFER TO S2.6C AND S2.6G.

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Honolulu, Hawaii

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Seattle, Washington

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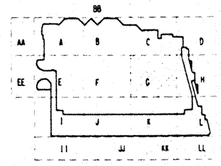
JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7173
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 8, Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
2	12-15-95		FINAL DOCUMENT
	9-15-95		BUILDING PERMIT SET
1	8-4-95		

Sheet Title

ROOF FRAMING PLAN SECTOR G

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.7.G

Thu Nov 30 15:18:35 1995
jhnccroof_g.dgn
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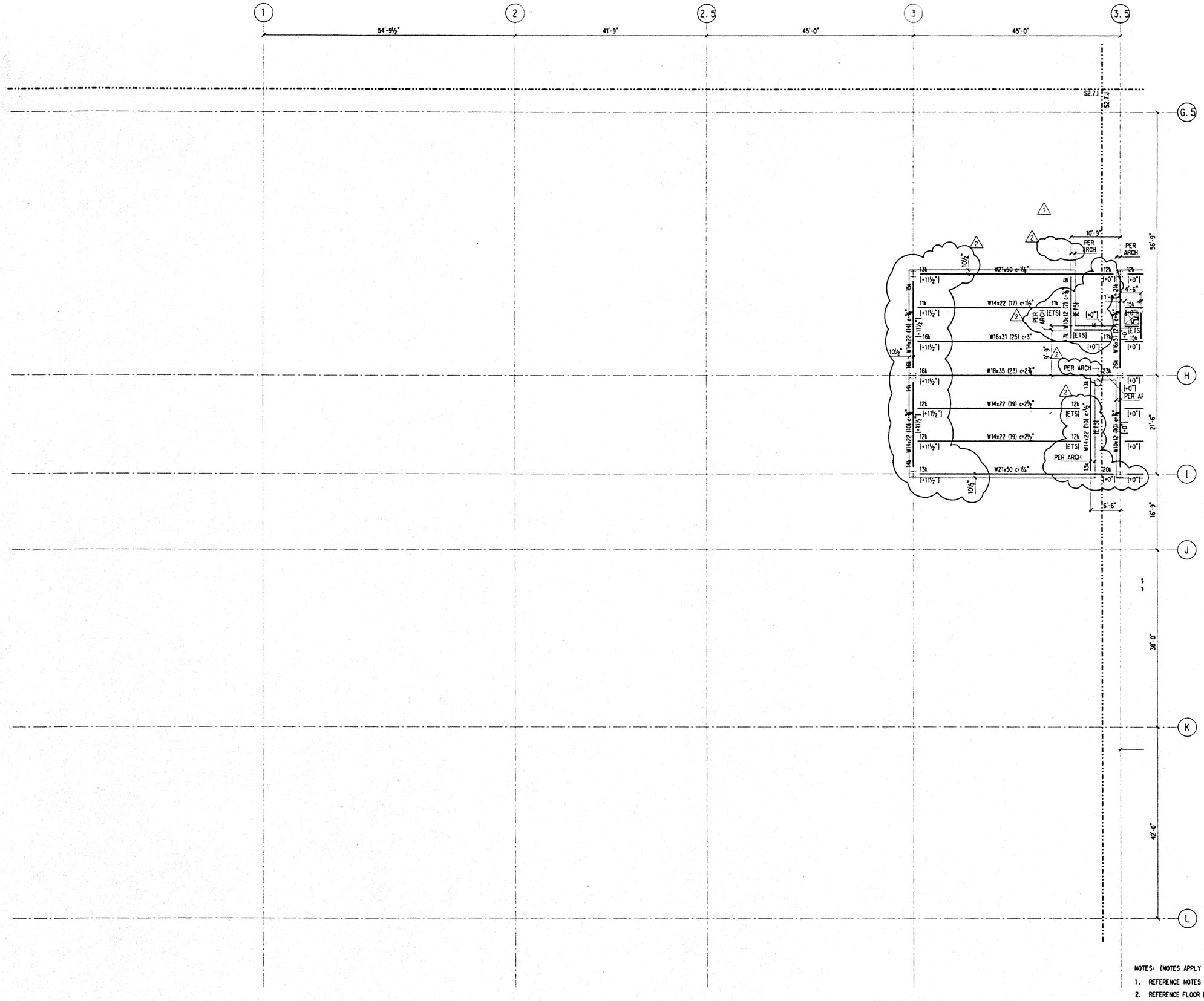

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B
2. REFERENCE FLOOR ELEVATION = 102'-6".

Hawai'i Convention Center

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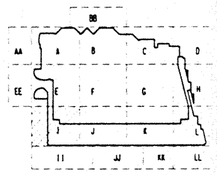
LICENSED PROFESSIONAL ENGINEER NO. 71775 JOHN D. MAGNUSSON HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision as defined in Title 16 Chapter 16 Subpart 1.2.4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	5-8-95		ADDENDUM #3

Sheet Title

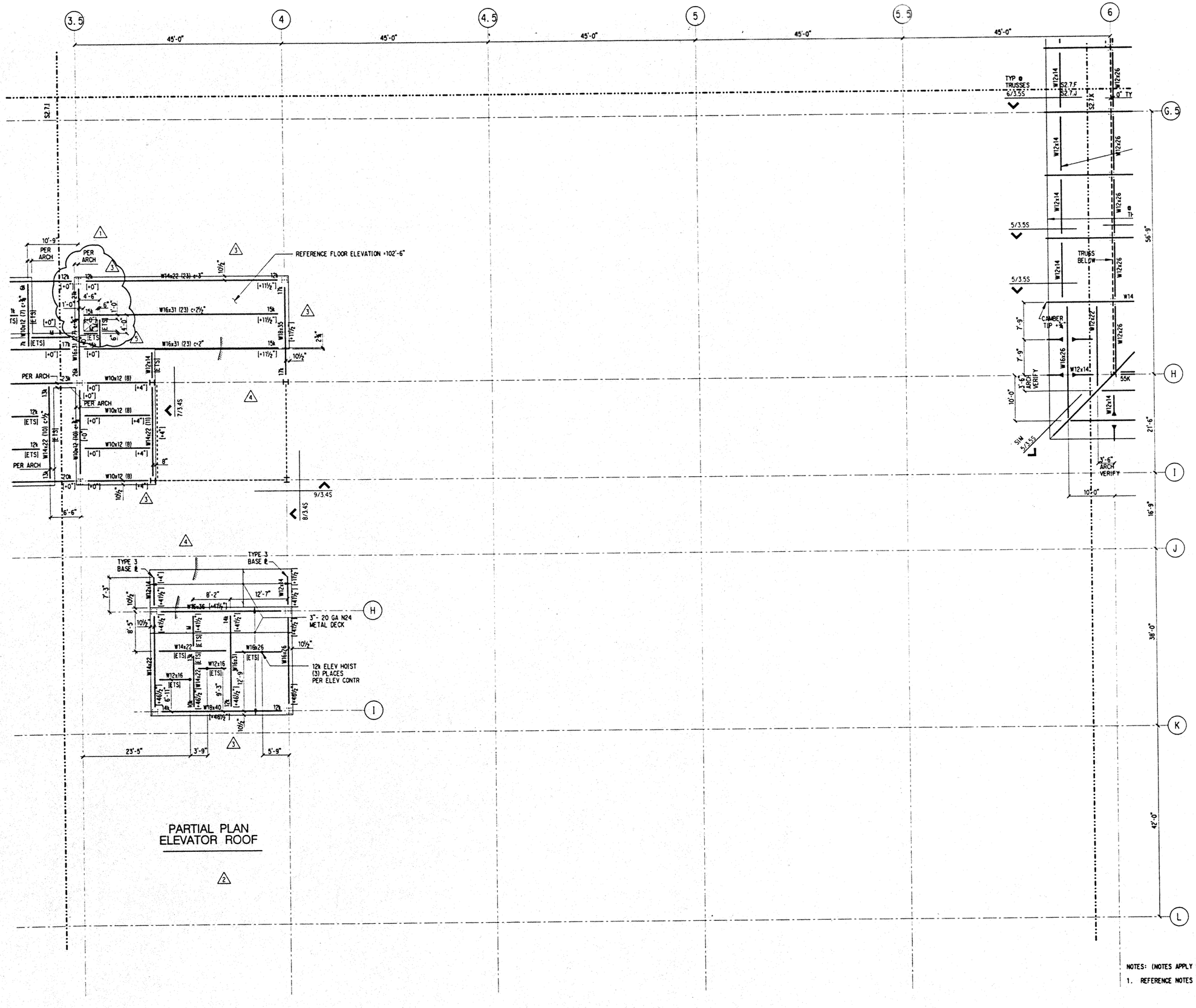
ROOF FRAMING PLAN SECTOR I

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.7.I

Thu Nov 30 15:20:42 1995
jhccroof_1.dgn
DMS plt



PARTIAL PLAN ELEVATOR ROOF

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B

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Honolulu, Hawaii

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Honolulu, Hawaii
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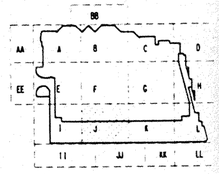
(206) 292-1200 FAX: (206) 623-1201

JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11779
HAWAII U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Signature is affirmed in Title III Chapter 16 Subpart 1.2 et al of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

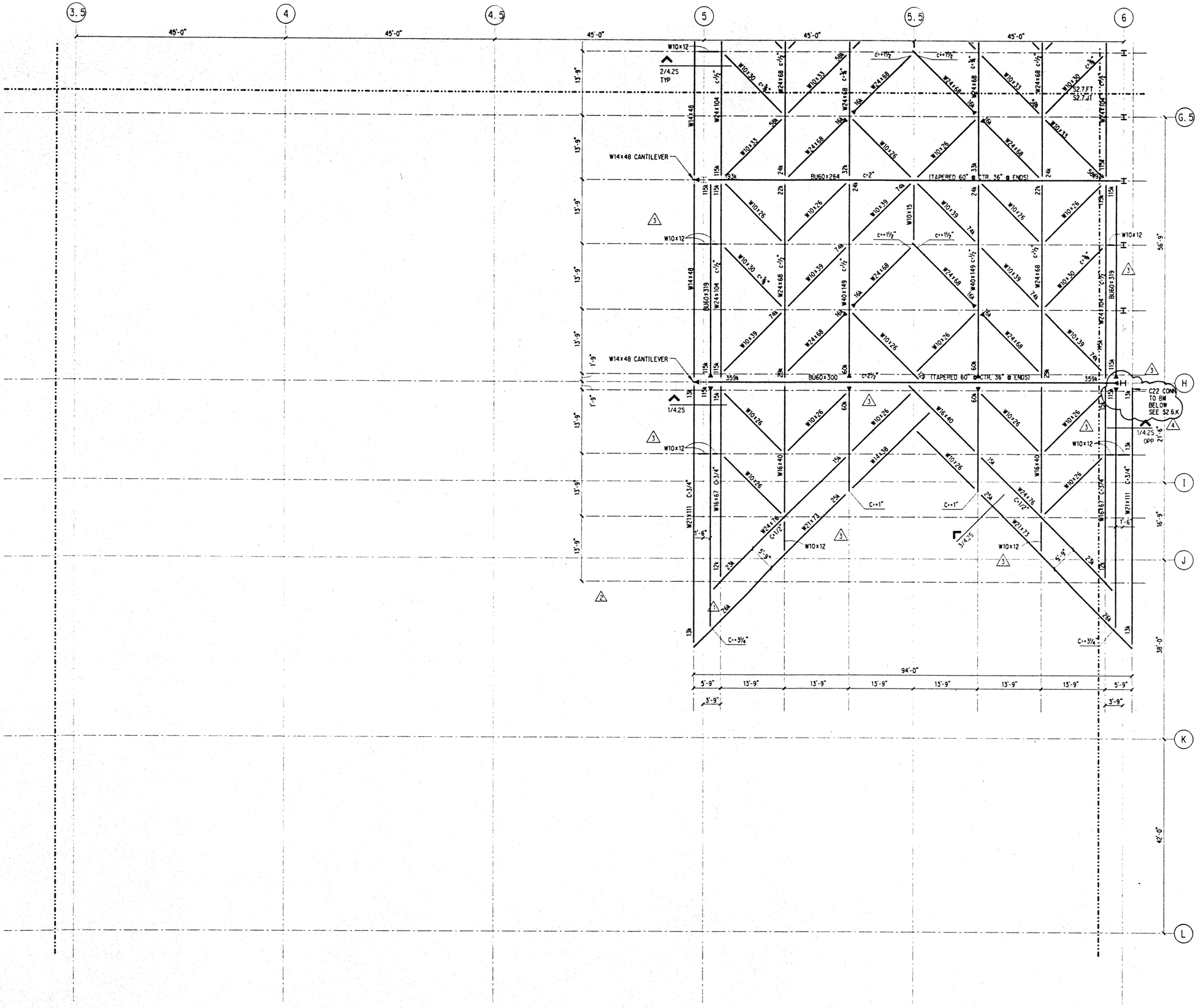
ROOF FRAMING PLAN SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S2.7.J

Thu Nov 30 15:22:53 1995

jhccroof_j.dgn



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B
2. SEE NOTE 2/S2.7.BT

Hawai'i Convention Center

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JON D. MAGNUSSON
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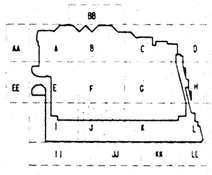
Supervision as defined in Title 16 Chapter 18 Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-31-95		
2	8-4-95		
1	6-19-95		

Sheet Title

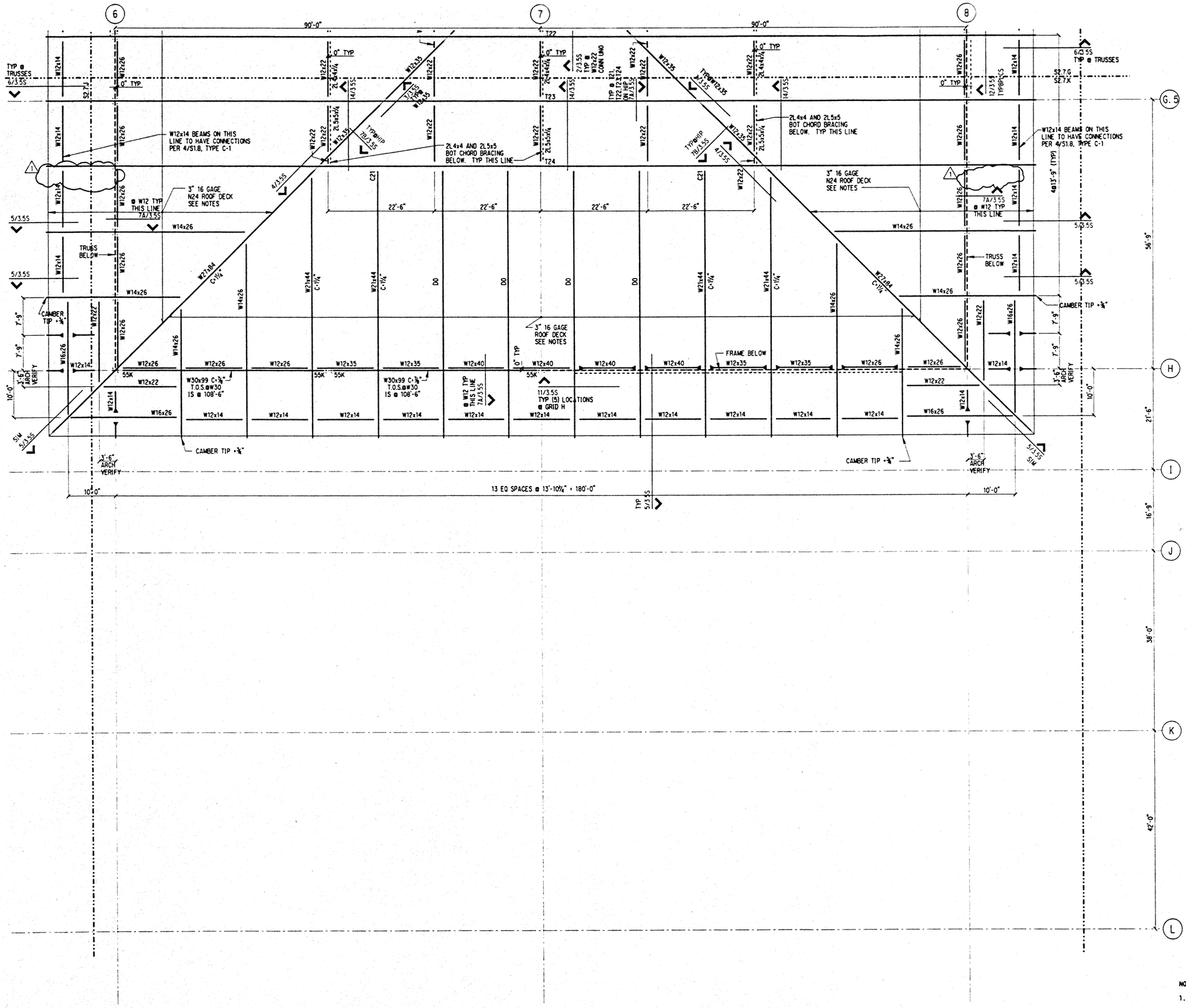
ROOF TRELLIS
FRAMING PLAN
SECTOR J

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S2.7.JT

Thu Nov 30 15:25:05 1995
j:\hccr\roof_jt.dgn
PWS plt



NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE NOTES ON S2.7.B
2. METAL ROOF DECK IS TO BE THE SIZE AND GAGE SHOWN ON THE PLAN. ALL UNITS ARE TO BE FASTENED TO EACH SUPPORT WITH (4) ⅜"# PUDDLE WELDS ALONG THE WIDTH AND 1½" TOP SEAM WELDS AT 1'-0" ON SIDE BOUNDARIES.
3. FOR TOP OF STEEL ELEVATIONS AT TRUSSES REFER TO TRUSS ELEVATIONS ON S5.9.
4. FOR TOP OF STEEL ELEVATIONS @ BEAMS REFER TO DETAILS.

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JOHN D. MAGNUSSON LICENSED PROFESSIONAL ENGINEER NO. 7175 HAWAII, U.S.A.

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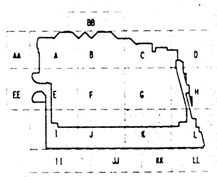
Supervision as defined in Title III Chapter 16 Section 1.3 of of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	8-4-95		

Sheet Title

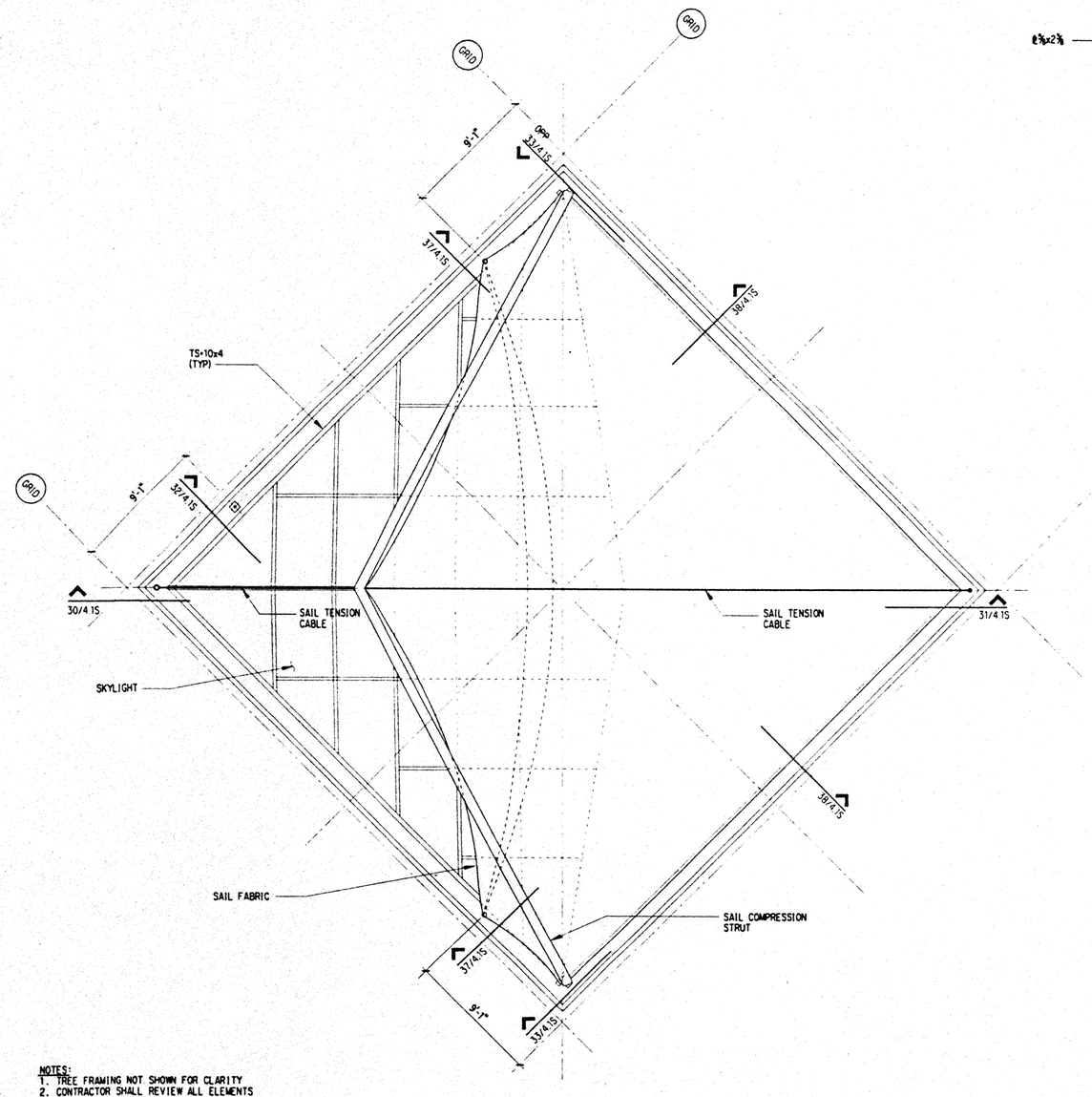
ROOF FRAMING PLAN SECTOR K

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

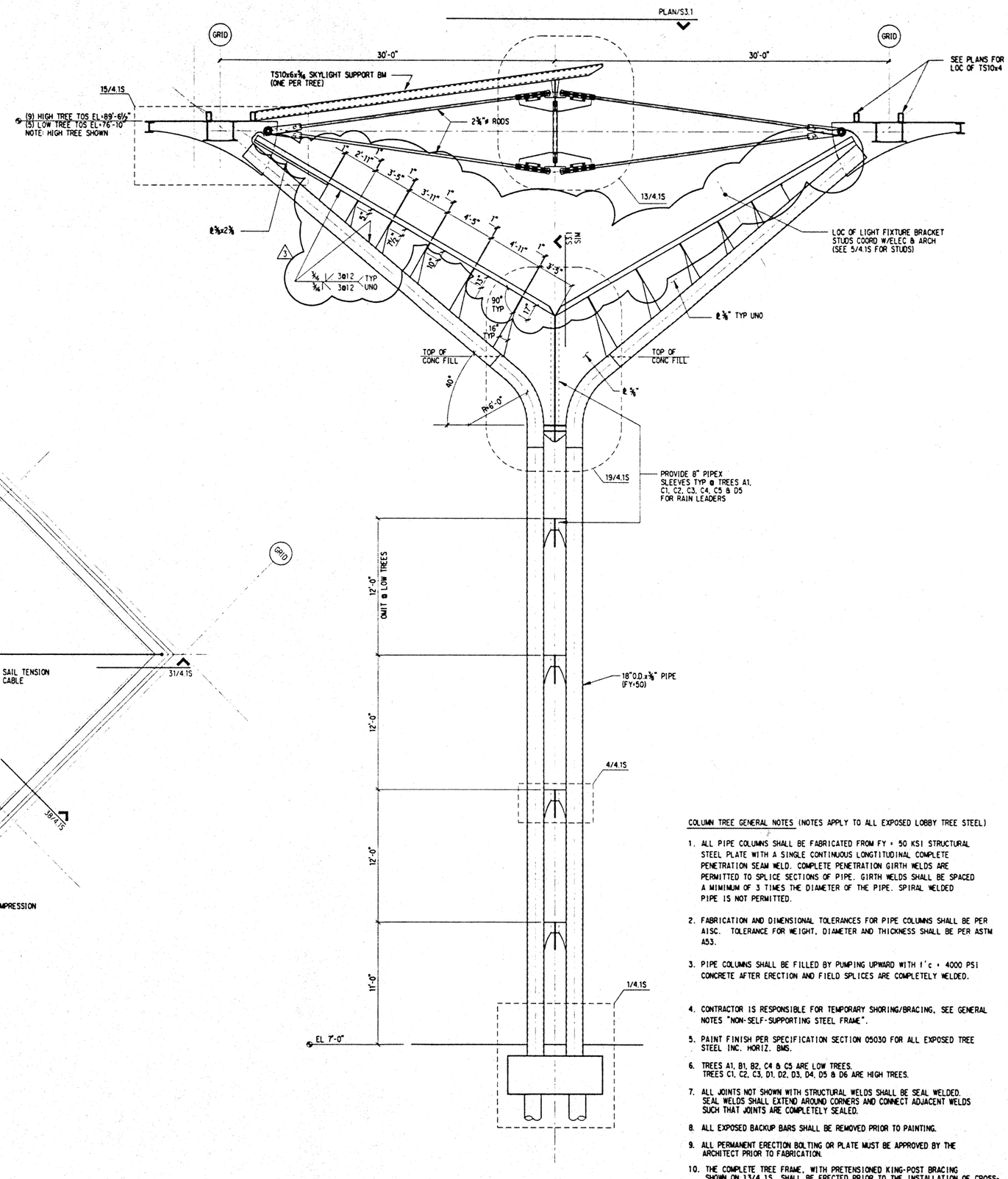
S2.7.K

Thu Nov 30 15:27:13 1995 jyncovroof_k.dgn DMS pit



NOTES:
1. TREE FRAMING NOT SHOWN FOR CLARITY
2. CONTRACTOR SHALL REVIEW ALL ELEMENTS SHOWN FOR COMPATIBILITY W/ SAIL & SKYLIGHT.

TYPICAL PLAN OF SAIL & SKYLIGHT
(NOT SHOWN ON ELEVATION)



ELEVATION
¼"=1'-0"

COLUMN TREE GENERAL NOTES (NOTES APPLY TO ALL EXPOSED LOBBY TREE STEEL)

1. ALL PIPE COLUMNS SHALL BE FABRICATED FROM FY = 50 KSI STRUCTURAL STEEL PLATE WITH A SINGLE CONTINUOUS LONGITUDINAL COMPLETE PENETRATION SEAM WELD. COMPLETE PENETRATION GIRTH WELDS ARE PERMITTED TO SPLICE SECTIONS OF PIPE. GIRTH WELDS SHALL BE SPACED A MIMIMUM OF 3 TIMES THE DIAMETER OF THE PIPE. SPIRAL WELDED PIPE IS NOT PERMITTED.
2. FABRICATION AND DIMENSIONAL TOLERANCES FOR PIPE COLUMNS SHALL BE PER AISC. TOLERANCE FOR WEIGHT, DIAMETER AND THICKNESS SHALL BE PER ASTM A53.
3. PIPE COLUMNS SHALL BE FILLED BY PUMPING UPWARD WITH f'c = 4000 PSI CONCRETE AFTER ERECTION AND FIELD SPLICES ARE COMPLETELY WELDED.
4. CONTRACTOR IS RESPONSIBLE FOR TEMPORARY SHORING/BRACING. SEE GENERAL NOTES "NON-SELF-SUPPORTING STEEL FRAME".
5. PAINT FINISH PER SPECIFICATION SECTION 05030 FOR ALL EXPOSED TREE STEEL INC. HORIZ. BMS.
6. TREES A1, B1, B2, C4 & C5 ARE LOW TREES. TREES C1, C2, C3, D1, D2, D3, D4, D5 & D6 ARE HIGH TREES.
7. ALL JOINTS NOT SHOWN WITH STRUCTURAL WELDS SHALL BE SEAL WELDED. SEAL WELDS SHALL EXTEND AROUND CORNERS AND CONNECT ADJACENT WELDS SUCH THAT JOINTS ARE COMPLETELY SEALED.
8. ALL EXPOSED BACKUP BARS SHALL BE REMOVED PRIOR TO PAINTING.
9. ALL PERMANENT ERECTION BOLTING OR PLATE MUST BE APPROVED BY THE ARCHITECT PRIOR TO FABRICATION.
10. THE COMPLETE TREE FRAME, WITH PRETENSIONED KING-POST BRACING SHOWN ON 13/4.1S, SHALL BE ERECTED PRIOR TO THE INSTALLATION OF CROSS-BRACING SHOWN ON 8/4.1S AND PRIOR TO THE FINAL CONNECTION OF THE TREE BEAMS TO MAIN BUILDING STRUCTURE.

2

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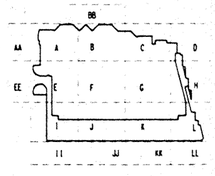
Supervision is defined in Title 16 Chapter 18 Subpart 1.2 in of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
1	11-7-95		
2	9-20-95		TREE REVISIONS
	9-15-95		BUILDING PERMIT SET
1	8-31-95		

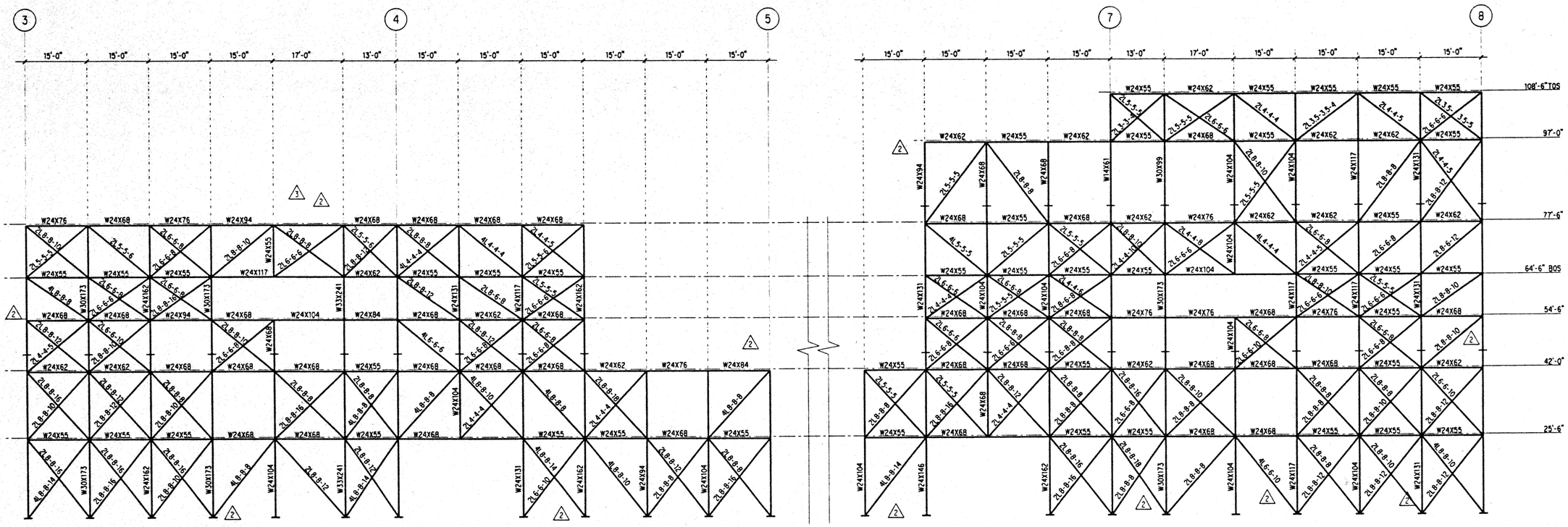
Sheet Title

TYPICAL STEEL
COLUMN TREE
ELEVATION & PLAN

Designed BJM
Drawn DMS
Checked JDM.
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

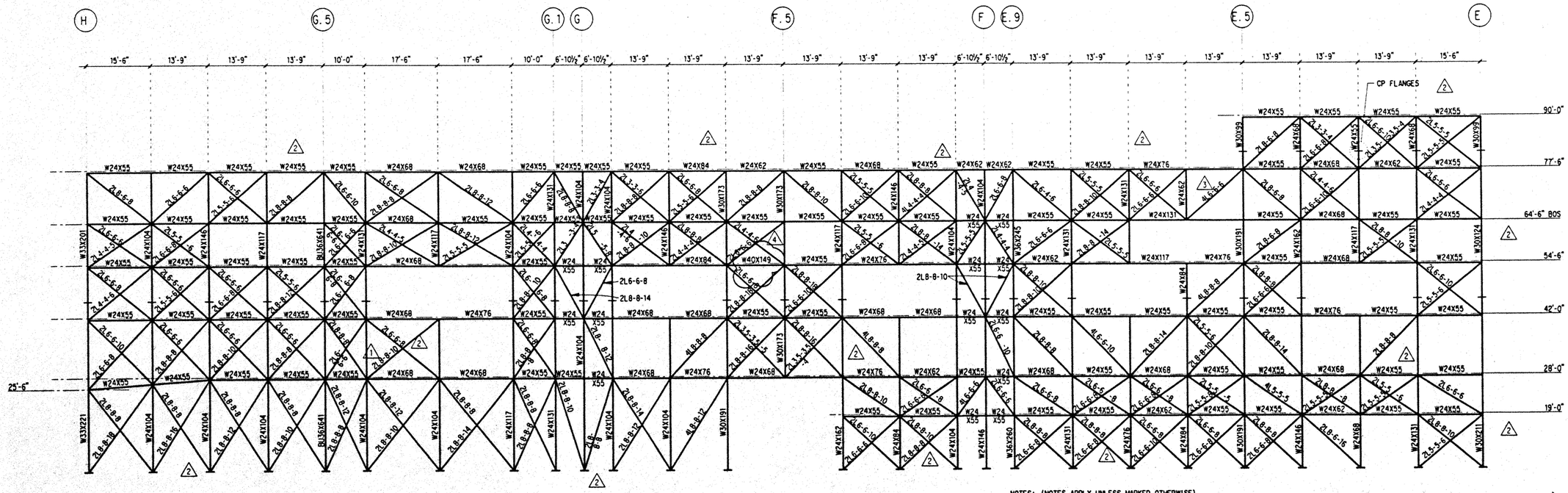
S3.1 1

Thu Nov 30 12:56:19 1995
jkhccsd.1.dgn
DMS plt



FRAME ON GRID H

SCALE: 3/32" = 1'-0"



FRAME ON GRID 2

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C=⅜". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. ─┼─ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. ┼ DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.
6. SEE S6.4 FOR BRACED FRAME COLUMN BASE PLATE SCHEDULE AND DETAILS.

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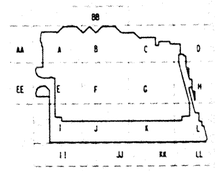
Supervision performed in lieu of Chapter 16 Section 1.3 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
4	11-7-95		
	9-15-95		BUILDING PERMIT SET
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

BRACED FRAME ELEVATIONS – GRIDS H & 2

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S4.1

Thu Nov 30 12:58:32 1995
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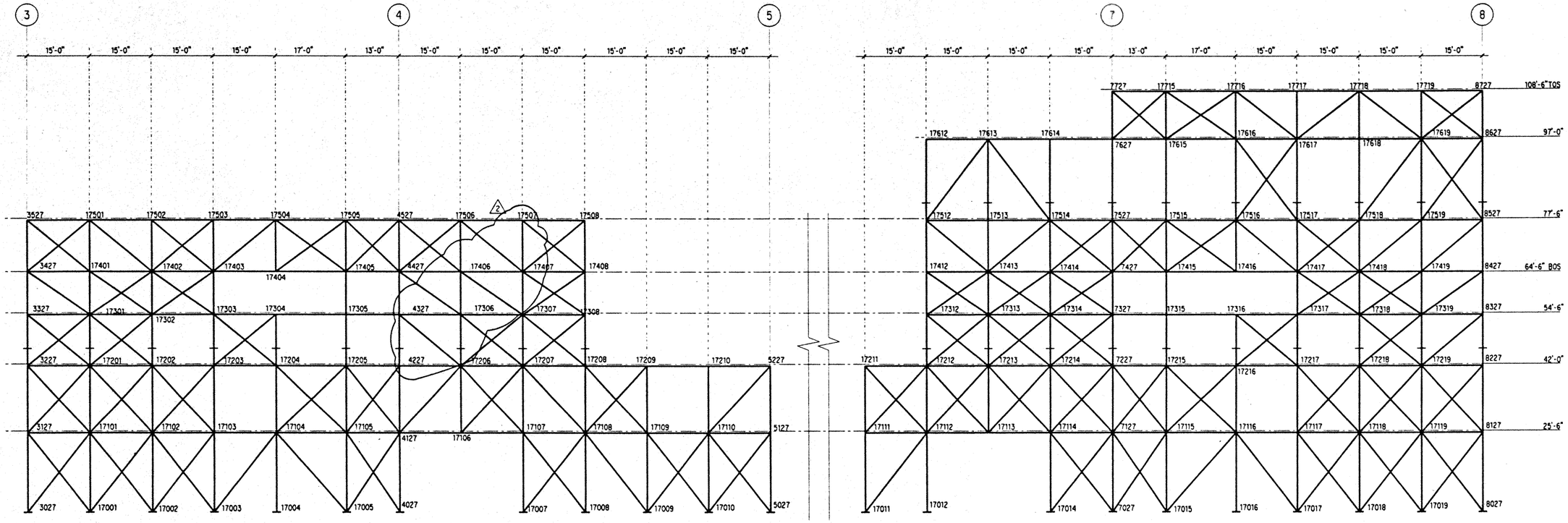
DMS pl4



FRAME ON GRID H

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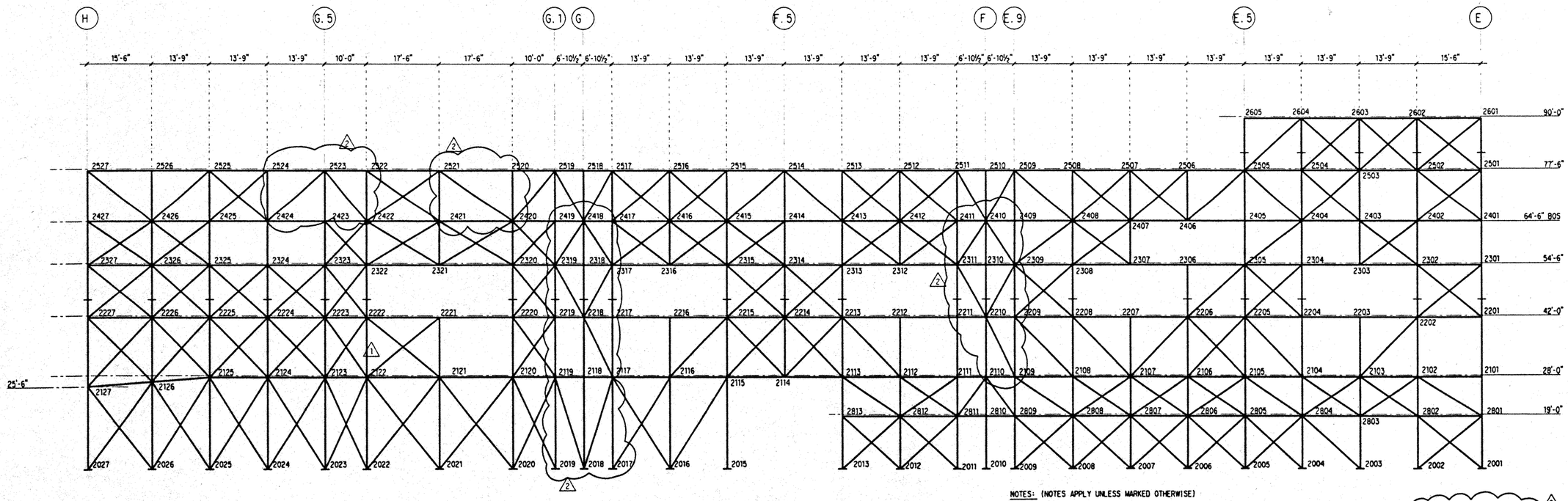

FRAME ON GRID 2

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS. UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C=⅜". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. ┼ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. ┼ DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.
6. SEE S6.4 FOR BRACED FRAME COLUMN BASE PLATE SCHEDULE AND DETAILS.

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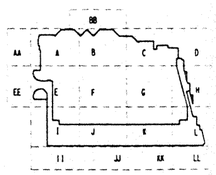


Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

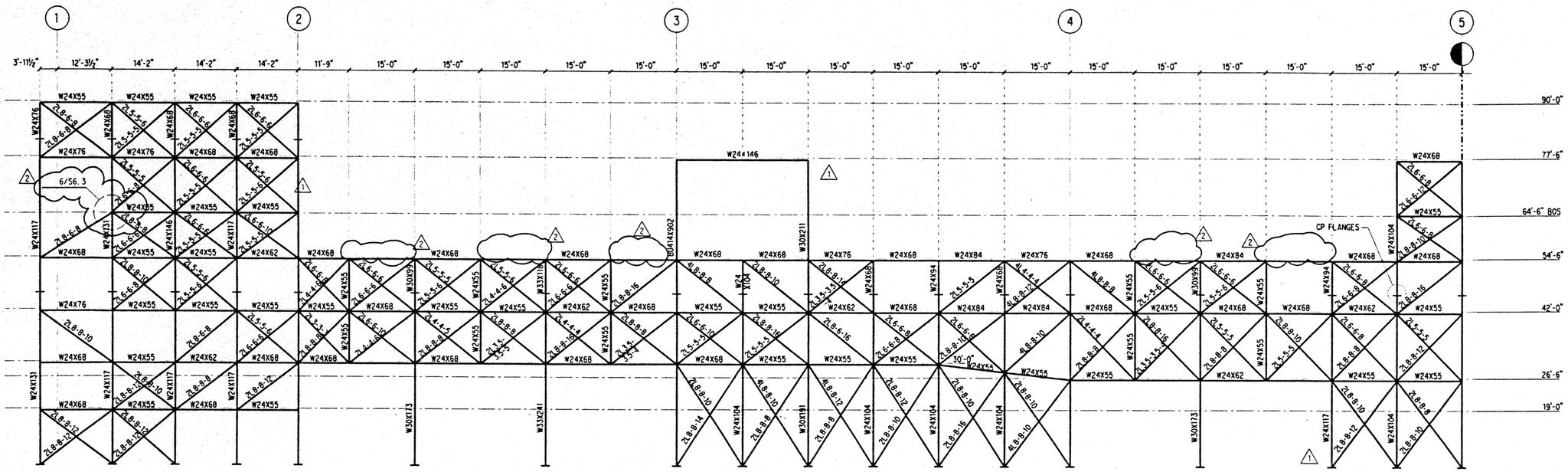
BRACED FRAME ELEVATIONS – GRIDS H & 2

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

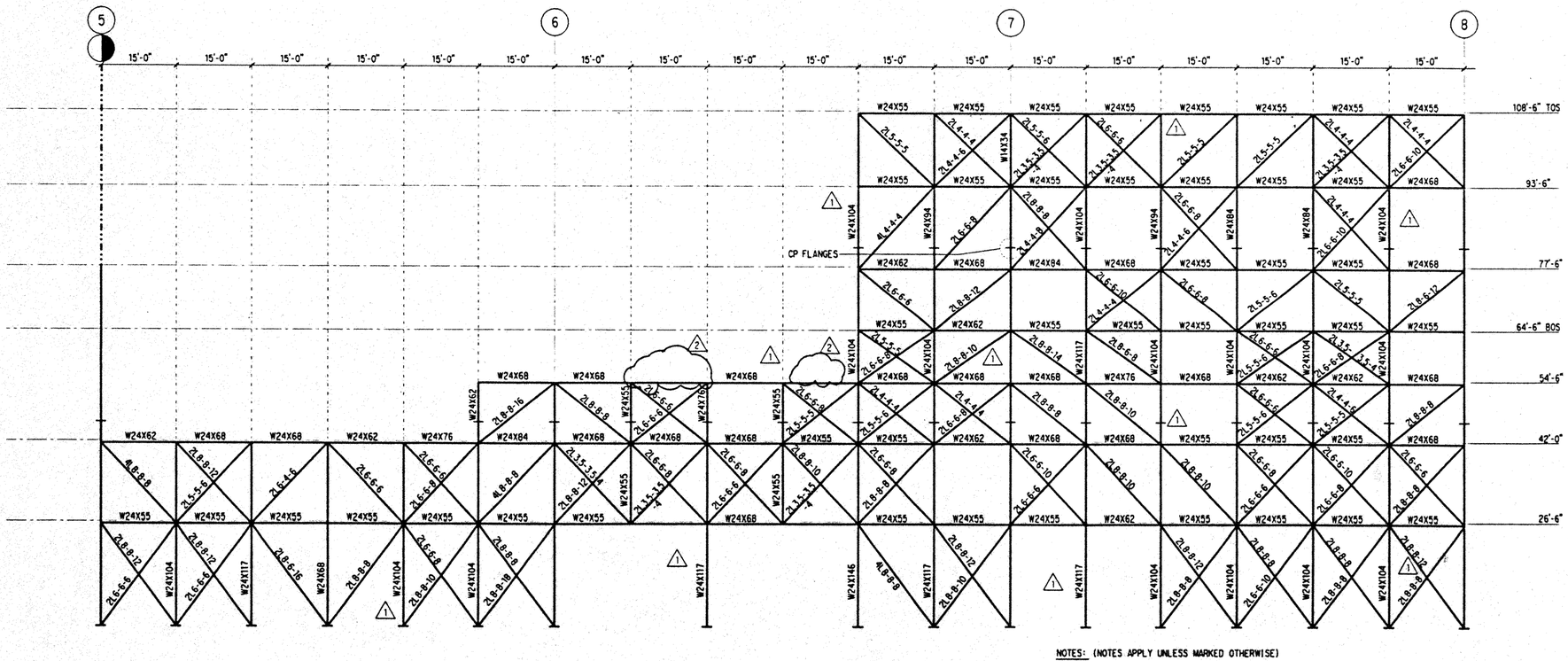
S4.2

Thu Nov 30 13:00:42 1995 jjhocbrfr_2.dgn



FRAME ON GRID E

SCALE: 3/32" = 1'-0"



FRAME ON GRID E

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C+⅜". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. ┼ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. ┼ DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.
6. SEE S6.4 FOR BRACED FRAME COLUMN BASE PLATE SCHEDULE AND DETAILS.

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LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII, U.S.A.

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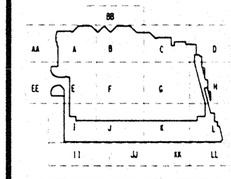

Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		

Sheet Title

BRACED FRAME ELEVATION – GRID E

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S4.3

Thu Nov 30 13:02:57 1995
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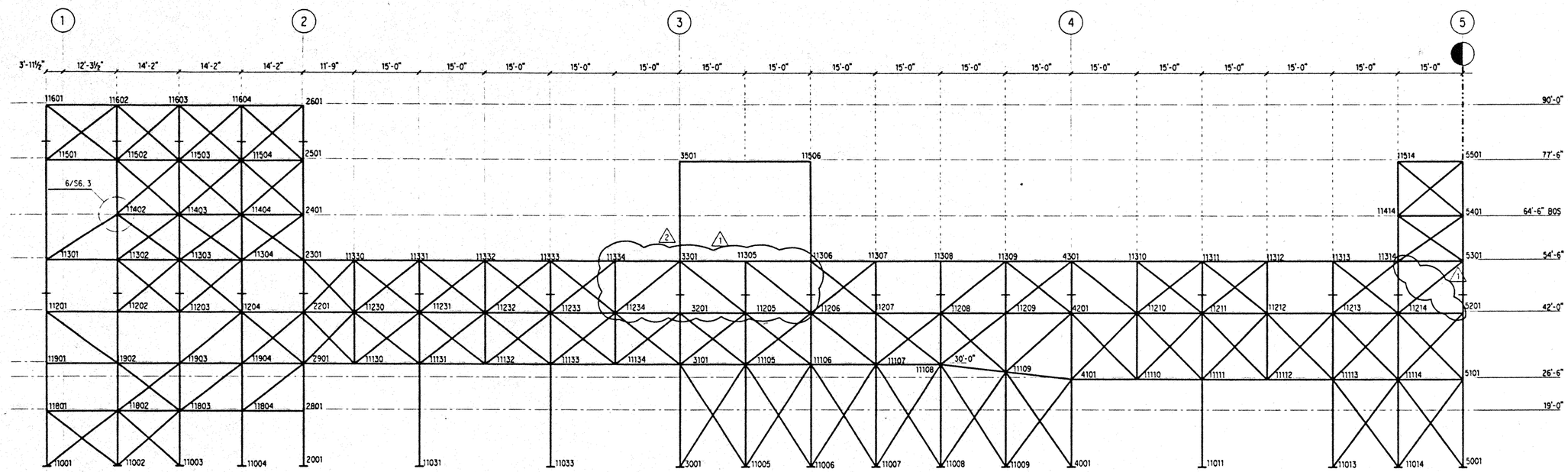
DMS plt



FRAME ON GRID E

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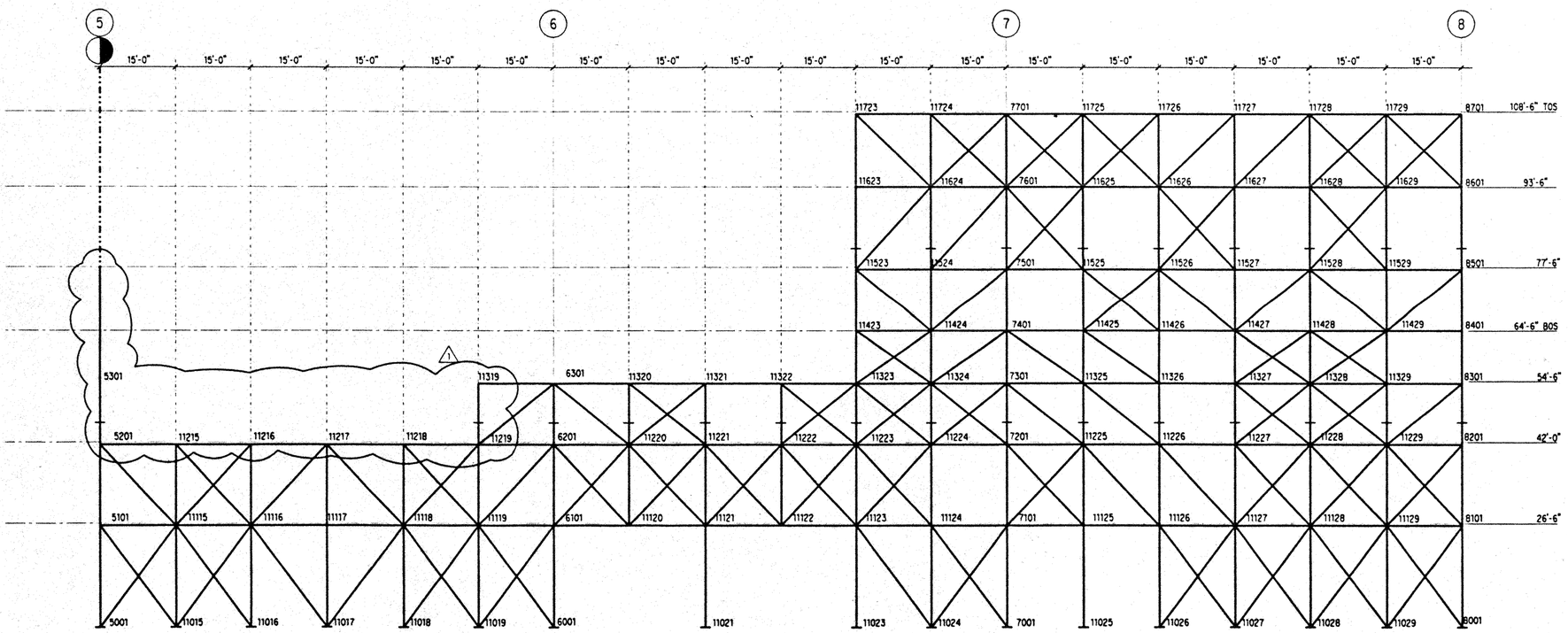

FRAME ON GRID E

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C=⅞". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. ⊥⊥ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. ⊥. DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.
6. SEE S6.4 FOR BRACED FRAME COLUMN BASE PLATE SCHEDULE AND DETAILS.

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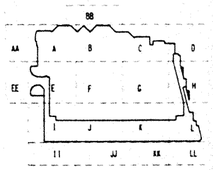

Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	6-19-95		

Sheet Title

BRACED FRAME ELEVATION - GRID E

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S4.4

Thu Nov 30 13:05:06 1995

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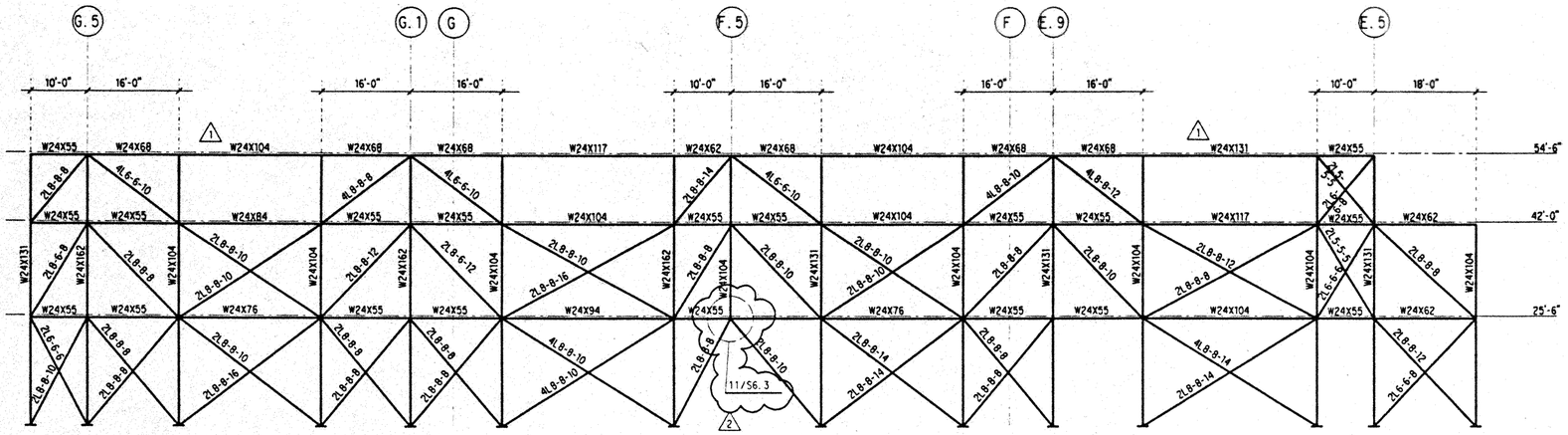
pit



FRAME ON GRID 10

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C+3/8". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS, EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS. △1
4. —+— DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.
6. SEE S6.4 FOR BRACED FRAME COLUMN BASE PLATE SCHEDULE AND DETAILS.

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LICENSED PROFESSIONAL ENGINEER
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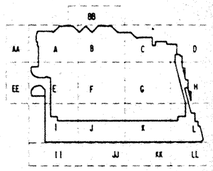
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision defined in Title 16 Chapter 18 Section 1.2.H of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		

Sheet Title

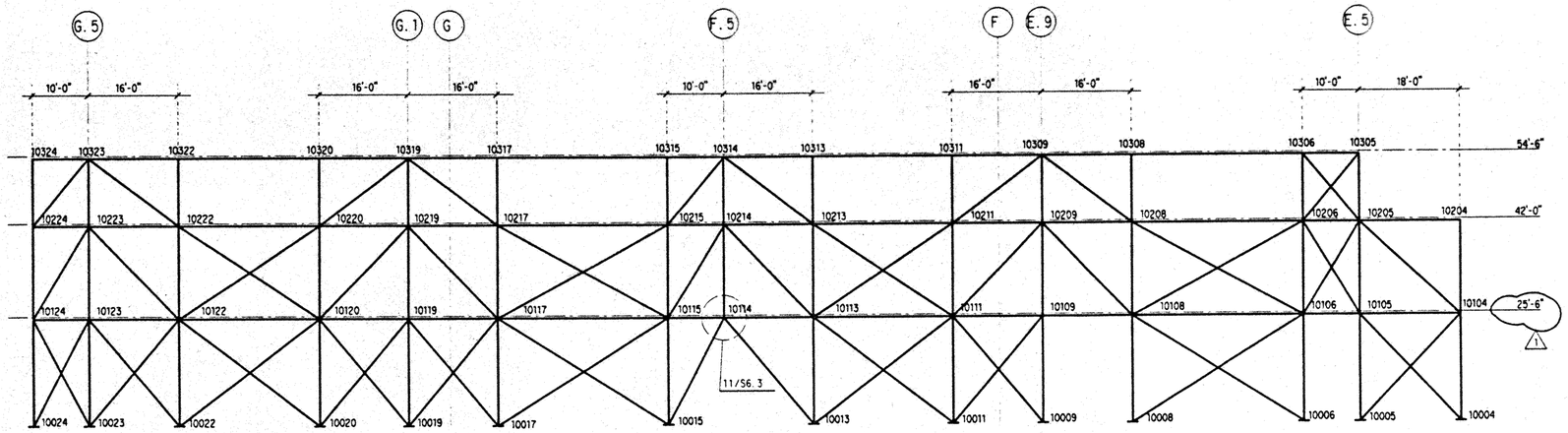
BRACED FRAME ELEVATION – GRID 10

Designed	BJM
Drawn	DMS
Checked	JDM
Date	4 / 12 / 95
Job No.	93036.02.9

Sheet No.

S4.5

Thu Nov 30 13:07:16 1995
j:\hcc\br fr_5.dgn
MWS plt



SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C=⅜". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS, EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. ⊥ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2. 1
5. + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.
6. SEE S6.4 FOR BRACED FRAME COLUMN BASE PLATE SCHEDULE AND DETAILS.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

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Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 623-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

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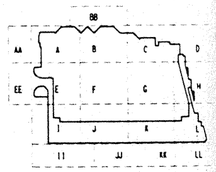
Supervision as defined in Title 16 Chapter 16 Section 1-2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	6-19-95		

Sheet Title

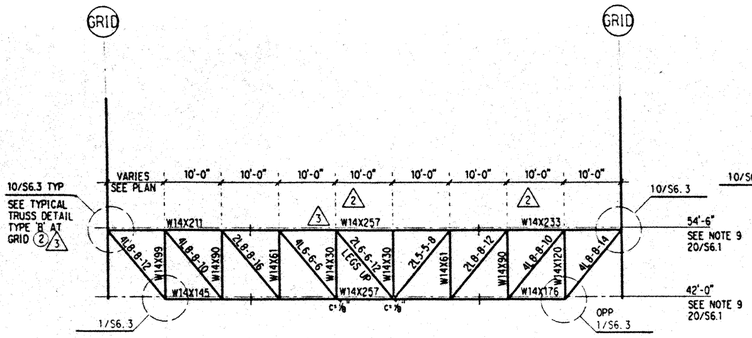
BRACED FRAME ELEVATION – GRID 10

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

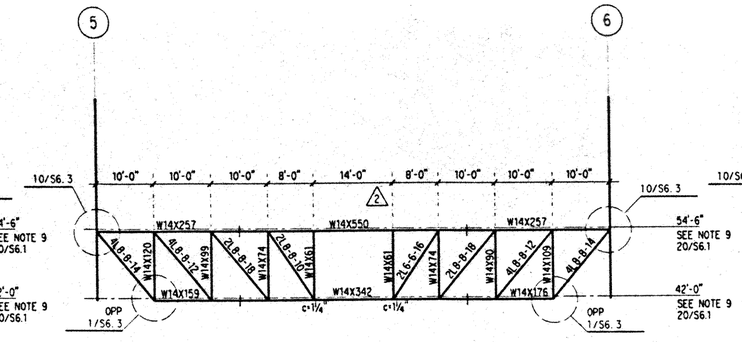
S4.6

Thu Nov 30 13:09:21 1995
jwhccbrfr_6.dgn
DMS plt



TRUSS T1

SCALE: 3/32" = 1'-0"



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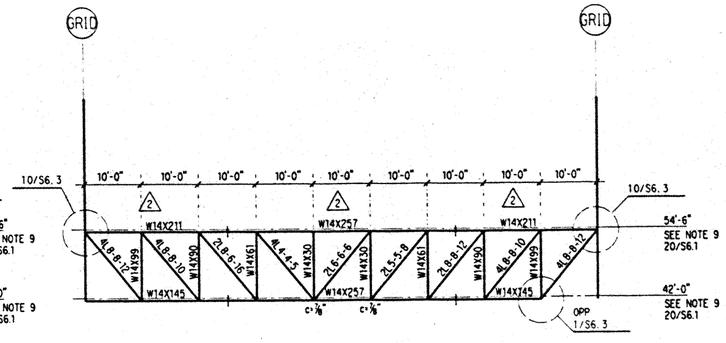
SCALE: 3/32" = 1'-0"



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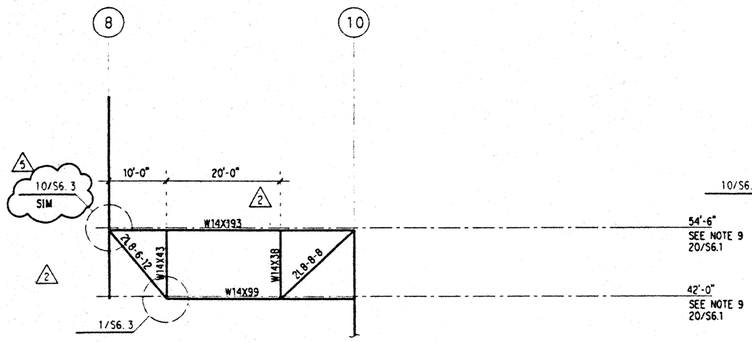
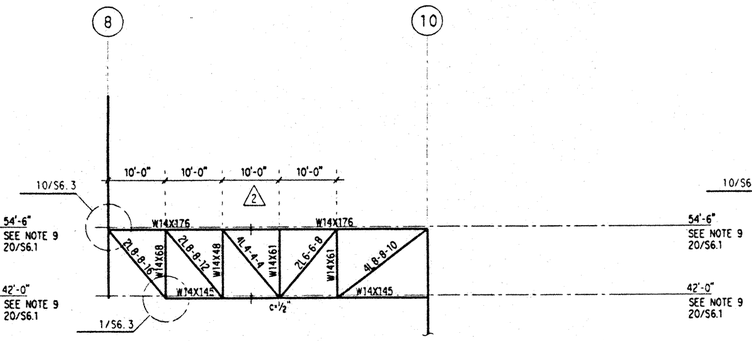
SCALE: 3/32" = 1'-0"



TRUSS T4

SCALE: 3/32" = 1'-0"



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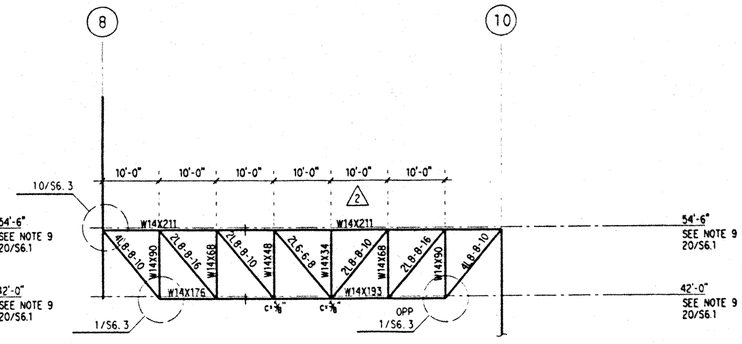
SCALE: 3/32" = 1'-0"



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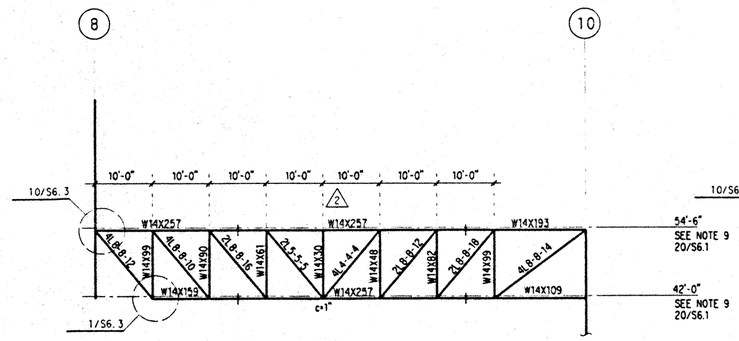
SCALE: 3/32" = 1'-0"



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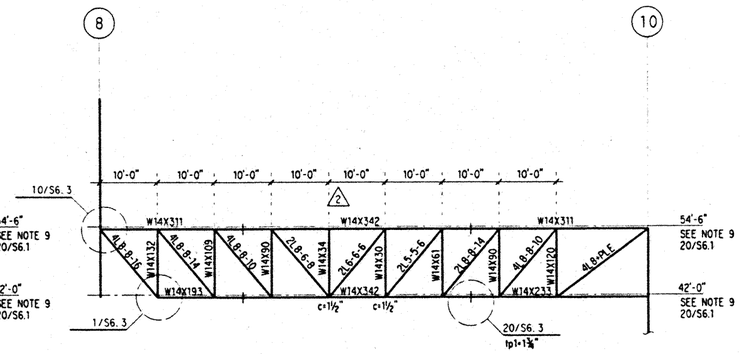
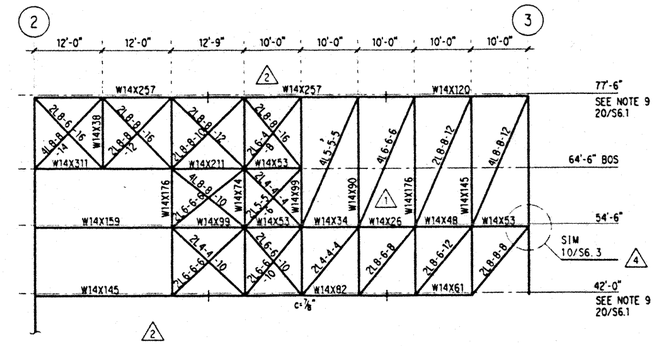
SCALE: 3/32" = 1'-0"



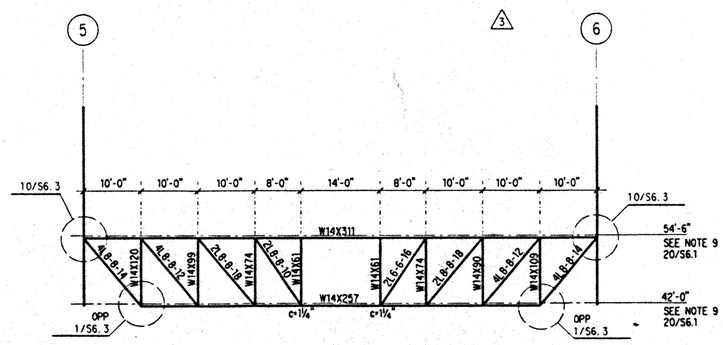
TRUSS T8

SCALE: 3/32" = 1'-0"



TRUSS T9

SCALE: 3/32" = 1'-0"



TRUSS T2A

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C+⅞". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS, EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS. 2
4. —+— DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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JON D. MAGNUSSON
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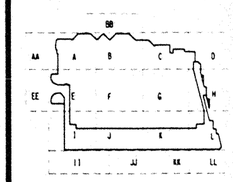
Supervision as defined in Title 16 Chapter 10 Section 1-3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
5	11-7-95		
	9-15-95		BUILDING PERMIT SET
4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

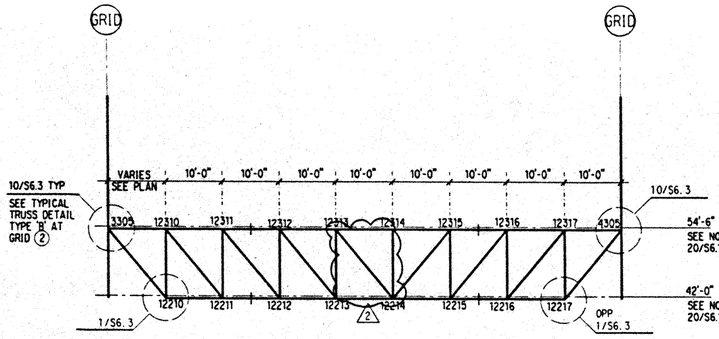
Sheet Title

TRUSS ELEVATIONS – PARKING

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S5.1

Thu Nov 30 13:11:31 1995
jyhccttruss.1.dgn
DMS plt



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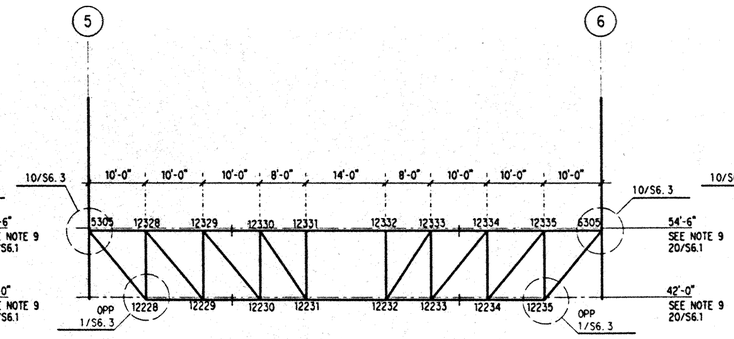
SCALE: 3/32" = 1'-0"



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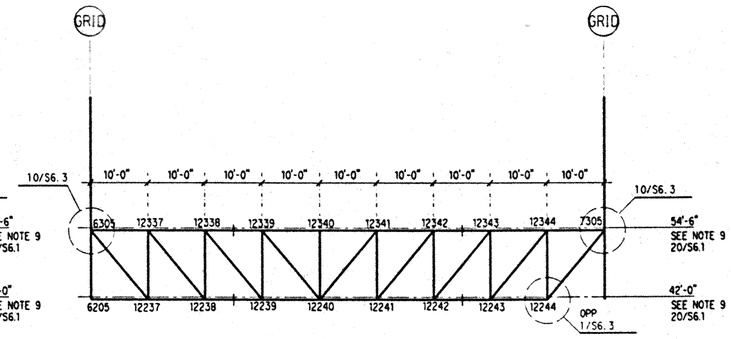
SCALE: 3/32" = 1'-0"



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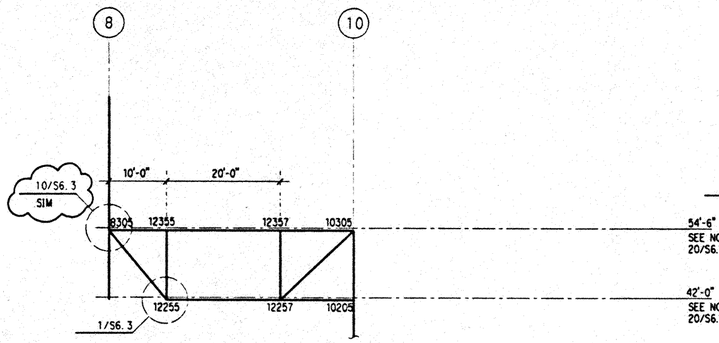
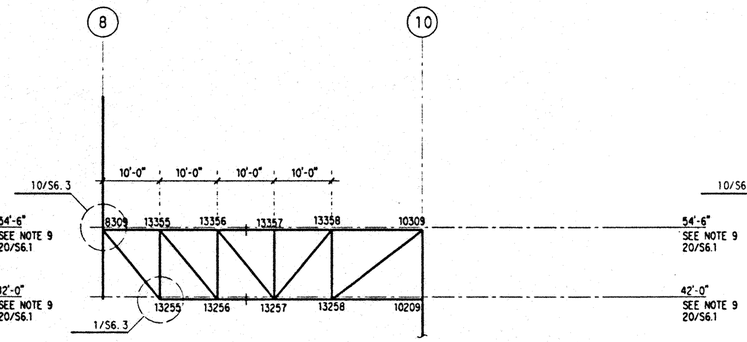


TRUSS T4

SCALE: 3/32" = 1'-0"



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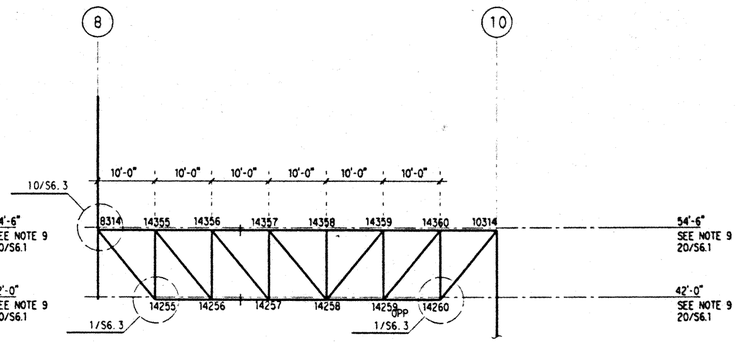
SCALE: 3/32" = 1'-0"



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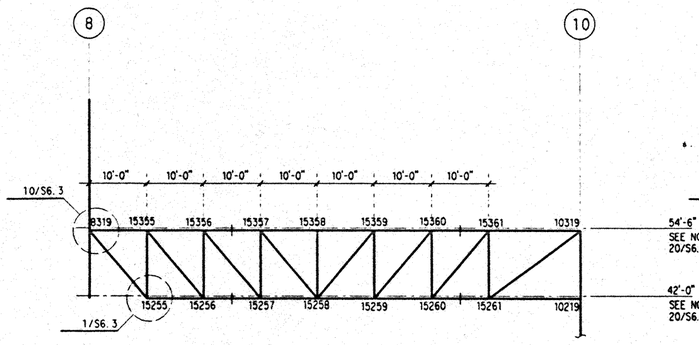
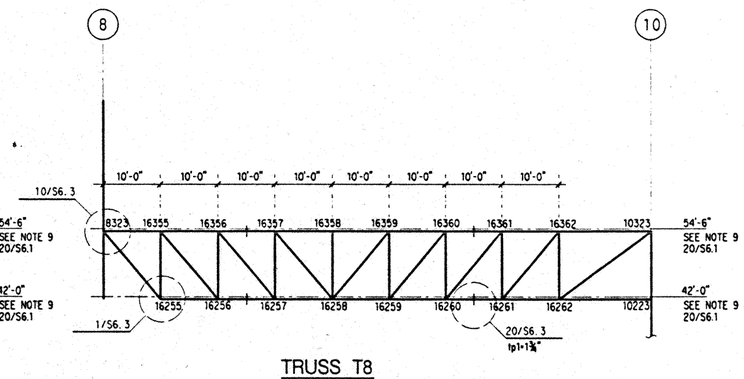
SCALE: 3/32" = 1'-0"



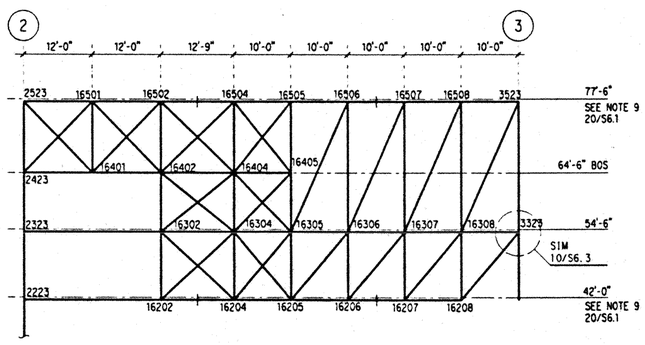
TRUSS T7

SCALE: 3/32" = 1'-0"



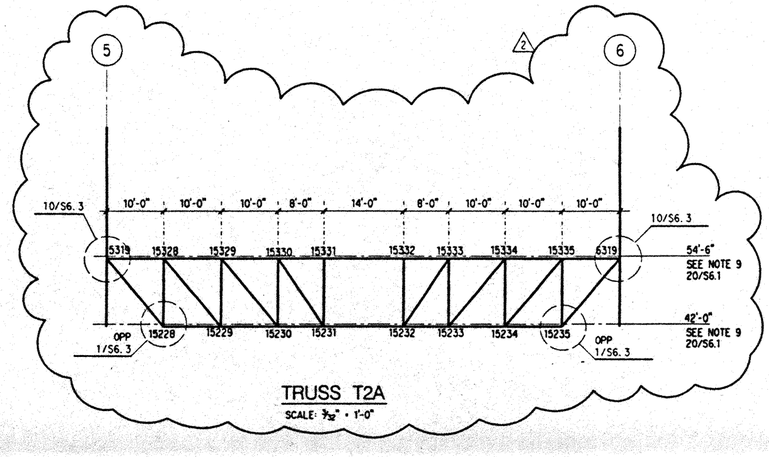
TRUSS T8

SCALE: 3/32" = 1'-0"



TRUSS T9

SCALE: 3/32" = 1'-0"



TRUSS T2A

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C+⅞". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS, EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS. 1
4. ┼┼ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. ┼ DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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Seattle, Washington

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Consultants

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LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

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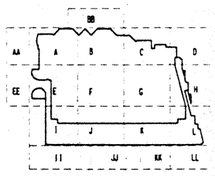
Supervision prepared in lieu of Chapter 10 Section 1.3 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		

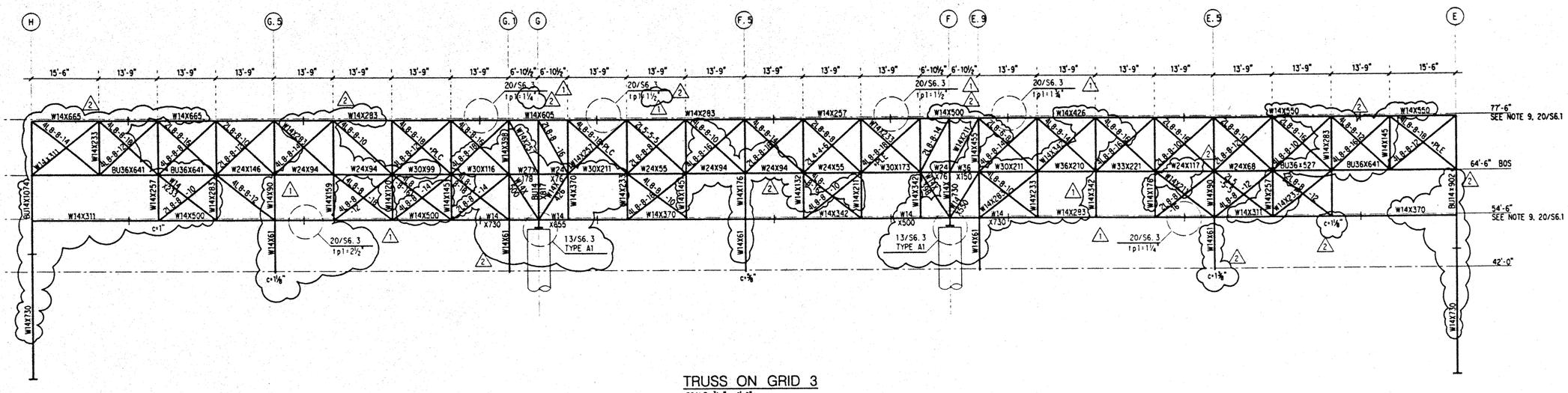
Sheet Title

TRUSS ELEVATIONS – PARKING

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

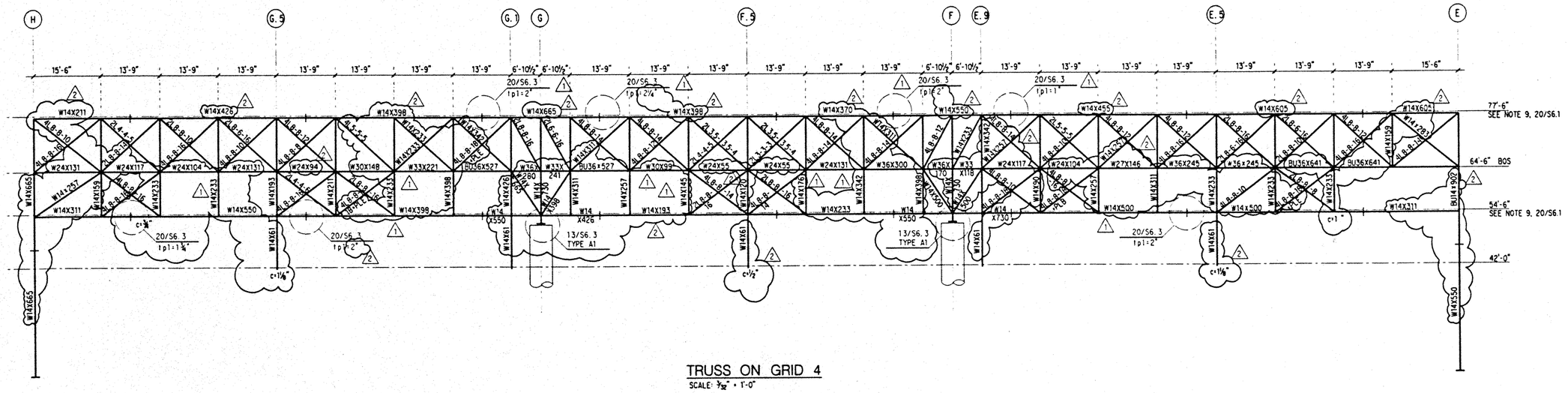
S5.2

Thu Nov 30 13:13:38 1995
j:\hcc\truss_2.dgn
DMS plt



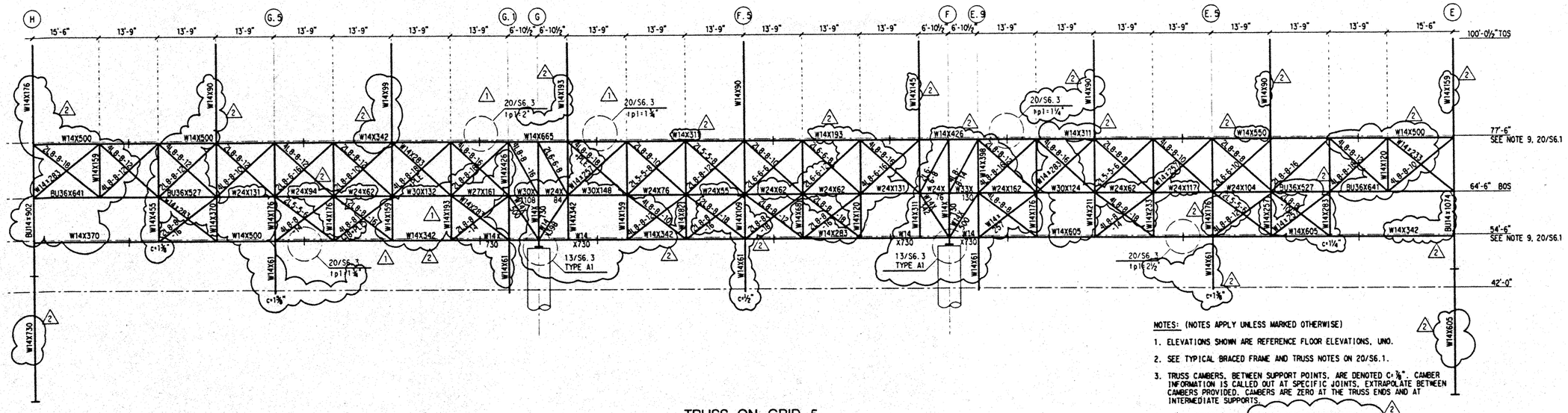
TRUSS ON GRID 3

SCALE: 3/32" = 1'-0"



TRUSS ON GRID 4

SCALE: 3/32" = 1'-0"



TRUSS ON GRID 5

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
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4. ┼ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. ┼ DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

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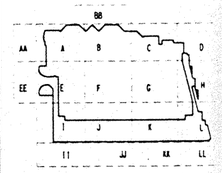
LICENSED PROFESSIONAL ENGINEER NO. 11775 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision performed in lieu of Chapter 8 Section 1.3.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TRUSS ELEVATIONS – GRIDS 3, 4, 5

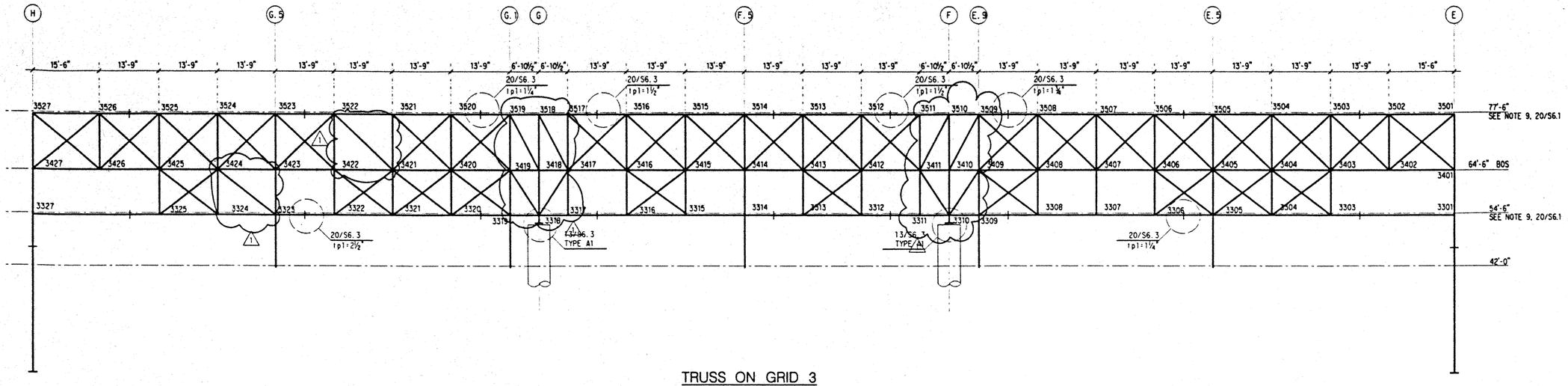
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S5.3

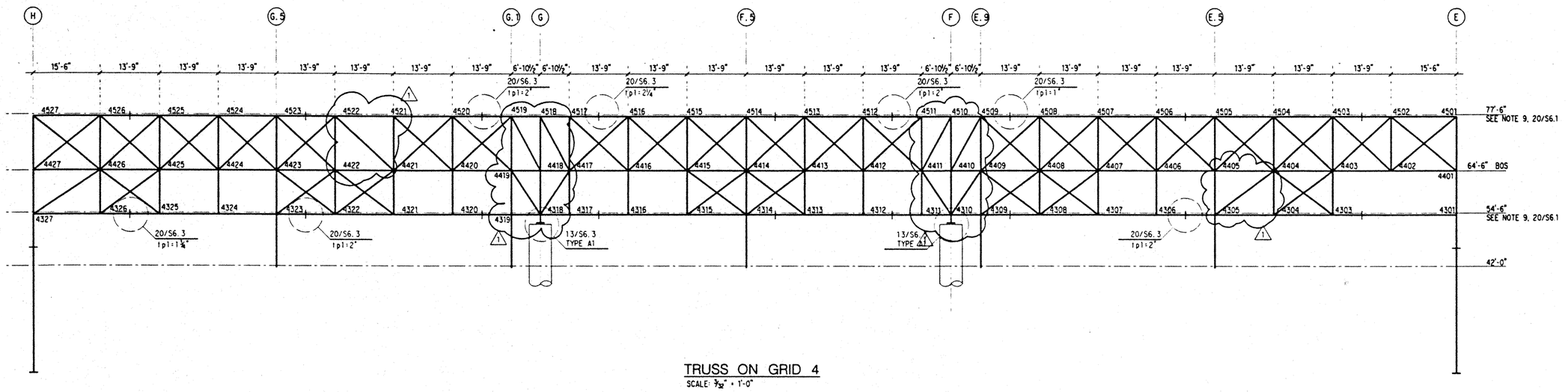
Thu Nov 30 13:16:02 1995 jnhcc\truss_3.dgn

DMS plt



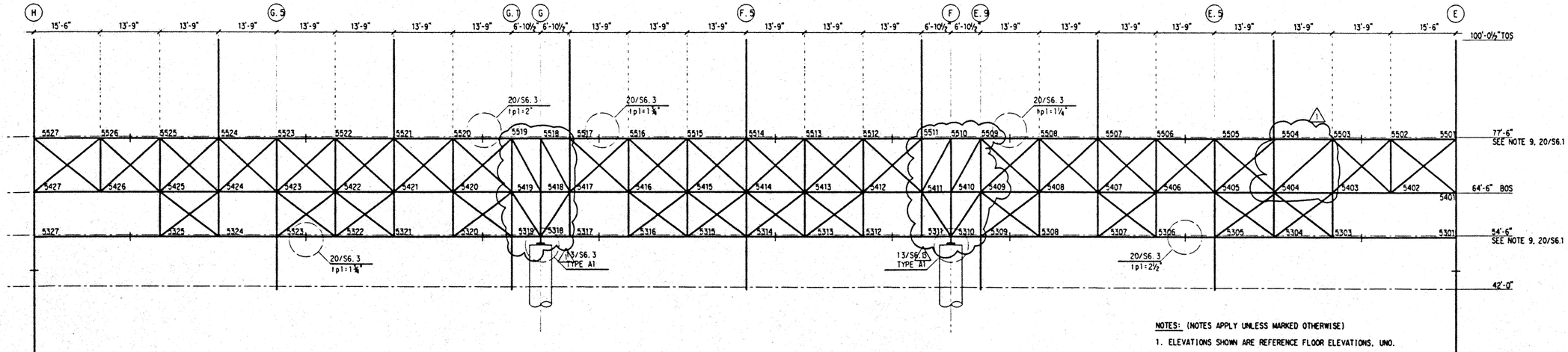
TRUSS ON GRID 3

SCALE: 3/32" = 1'-0"



TRUSS ON GRID 4

SCALE: 3/32" = 1'-0"



TRUSS ON GRID 5

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

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4. ┼ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
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JON D. MAGNUSSON
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HAWAII, U.S.A.

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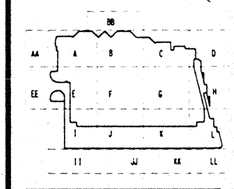
Signature and seal shall be in Title 16 Chapter 18 Subpart 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	6-19-95		

Sheet Title

TRUSS ELEVATIONS – GRIDS 3, 4, 5

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

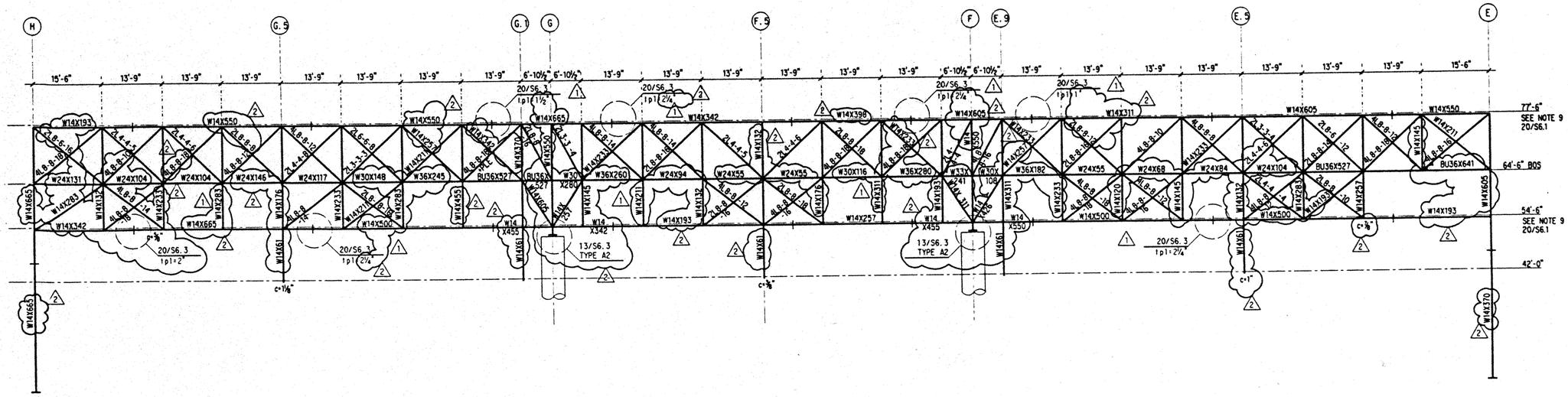
Sheet No.

S5.4

Thu Nov 30 13:18:13 1995

jhccttruss_4.dgn

DMS plt



TRUSS ON GRID 7

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C=3/8". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. ⊥⊥ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. ⊥ DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

2

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

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(206) 292-0200 FAX (206) 628-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7177S
HAWAII, U.S.A.

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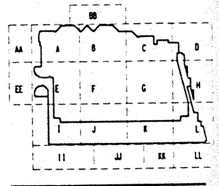
Supervision is defined in Title VII Chapter 16 Subpart 1.2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

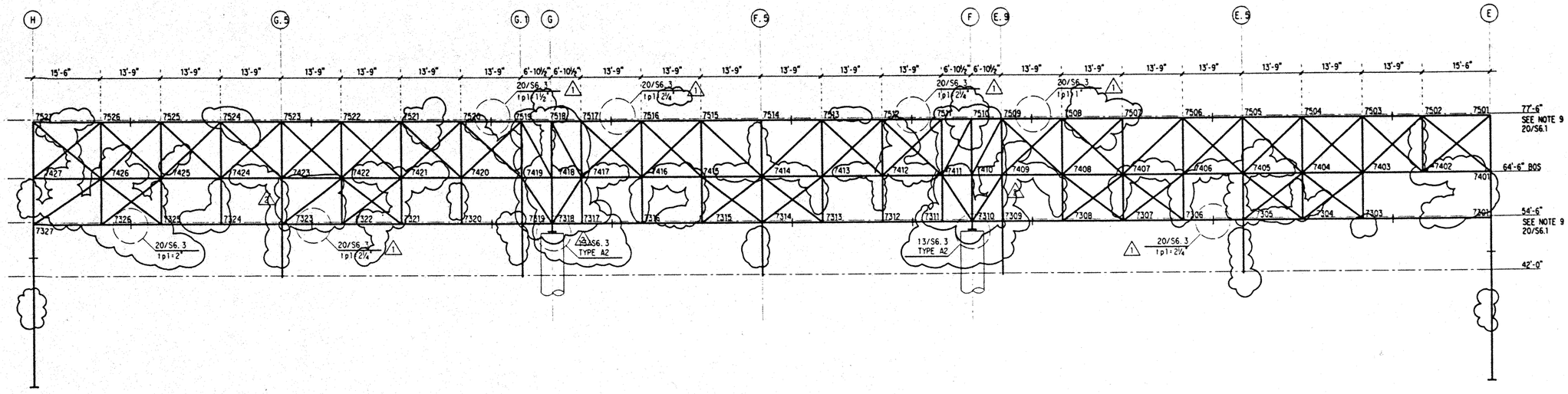
TRUSS ELEVATION – GRID 7

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S5.5

Thu Nov 30 13:20:32 1995
j:\hcc\truss_5.dgn
ws pit



TRUSS ON GRID 7

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C=⅞". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. —+— DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2. (2)
5. + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

Hawai'i Convention Center

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JON D. MAGNUSSON
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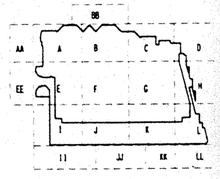
This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is defined in Title 16 Chapter 18 Subsection 1.3 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TRUSS
ELEVATION
– GRID 7

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S5.6

Thu Nov 30 13:22:36 1995
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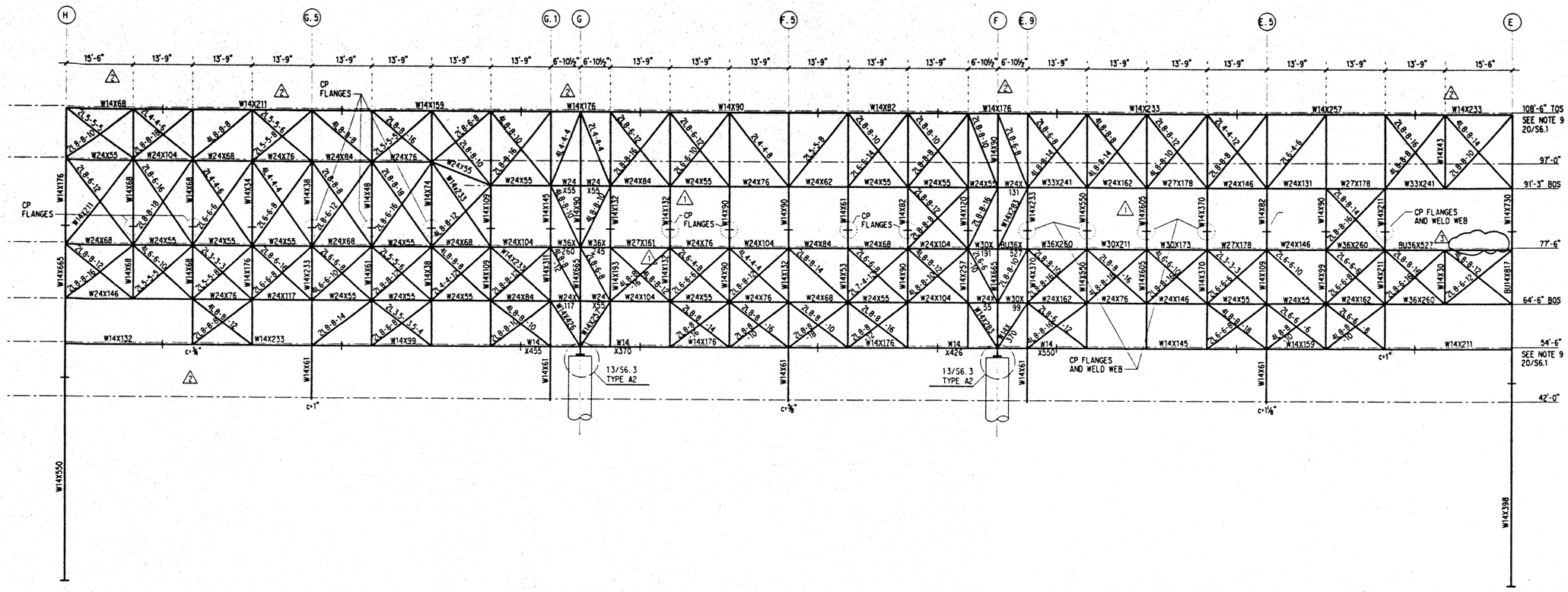
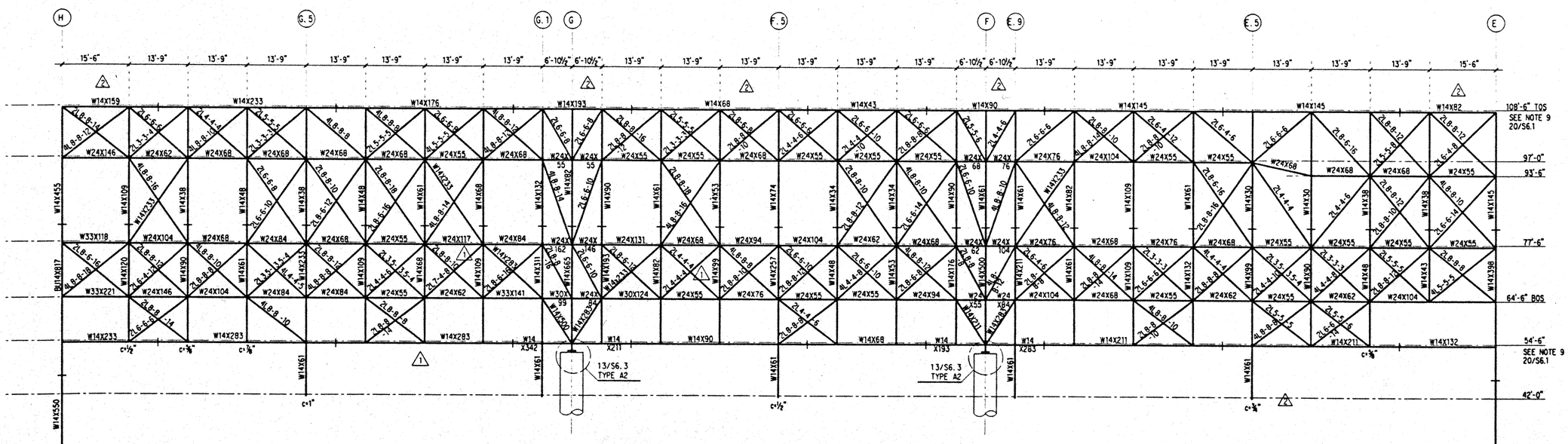
pl+



TRUSS ON GRID 6

SCALE: 3/32" = 1'-0"



TRUSS ON GRID 8

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C+⅞". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. —+— DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

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JOHN D. MAGNUSSON
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NO. 7775
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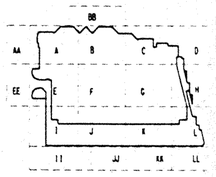
This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervised by signed in lieu of Chapter 16 Section 1.20 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

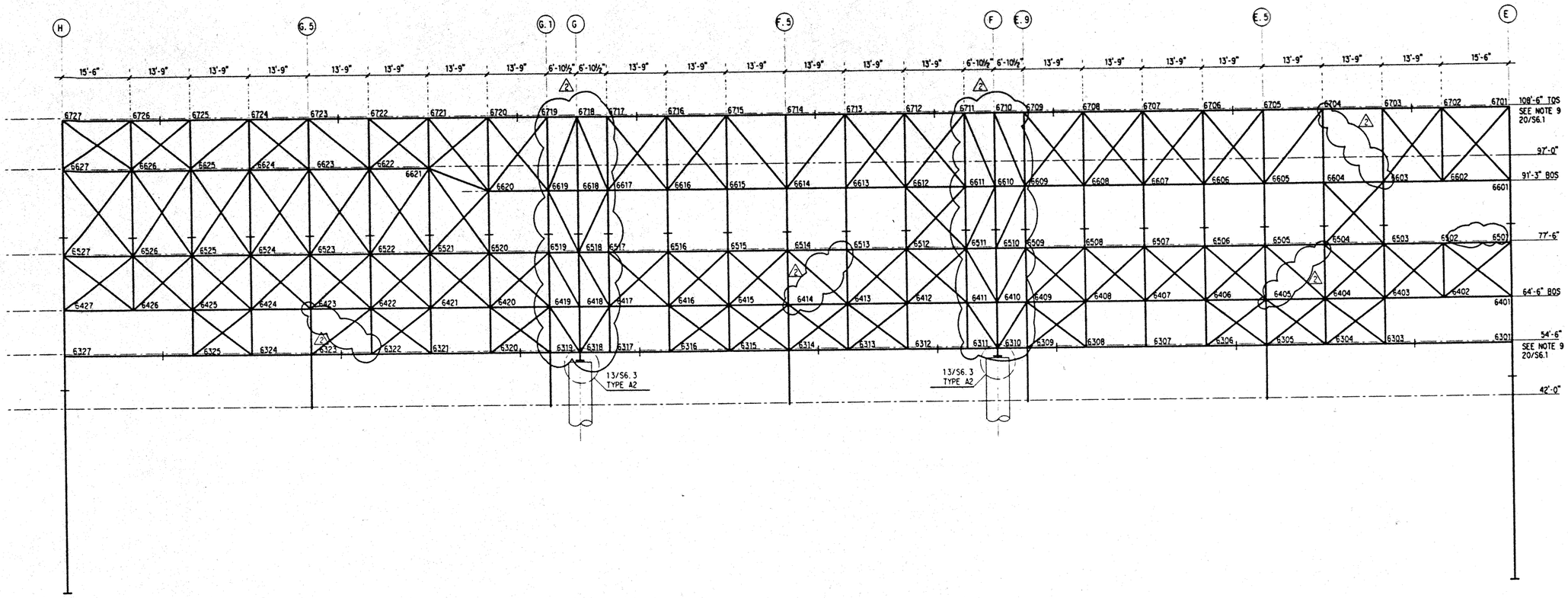
TRUSS ELEVATIONS – GRIDS 6 & 8

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

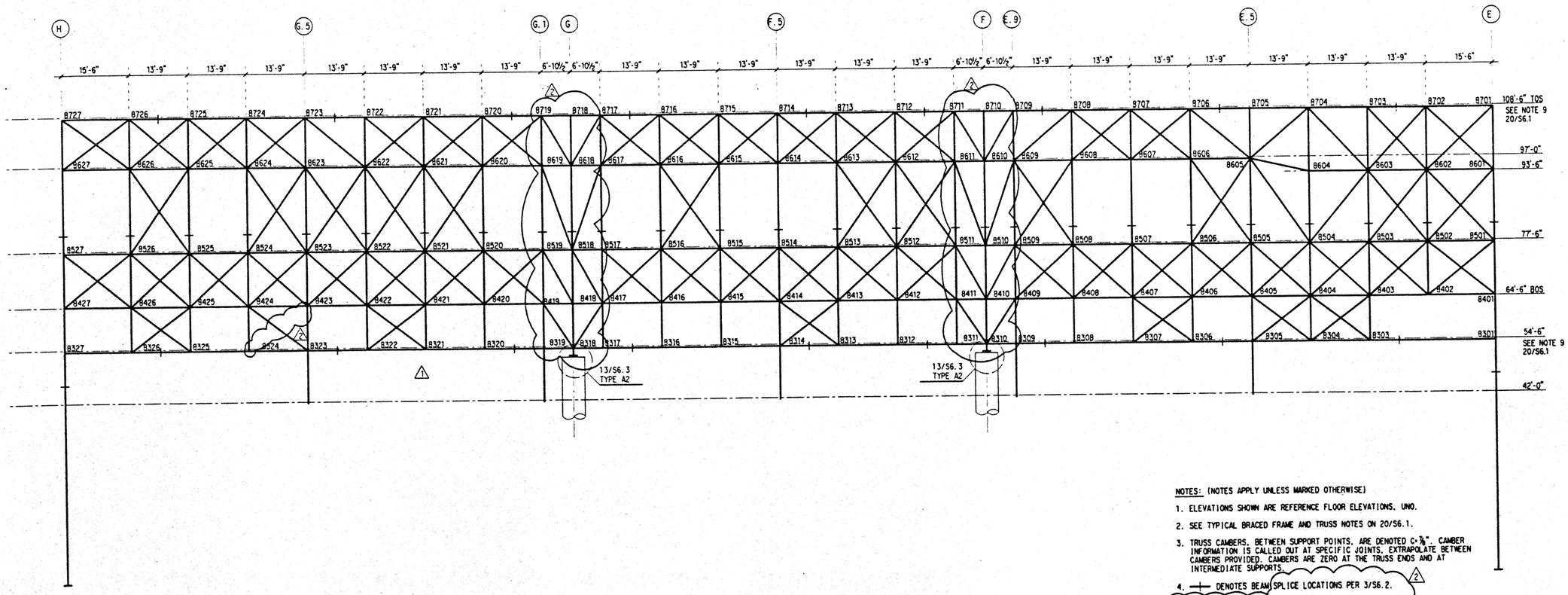
S5.7

Thu Nov 30 13:24:52 1995 jjnoctruss_7.dgn DMS plt



TRUSS ON GRID 6

SCALE: 3/32" = 1'-0"



TRUSS ON GRID 8

SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. ELEVATIONS SHOWN ARE REFERENCE FLOOR ELEVATIONS, UNO.
2. SEE TYPICAL BRACED FRAME AND TRUSS NOTES ON 20/S6.1.
3. TRUSS CAMBERS, BETWEEN SUPPORT POINTS, ARE DENOTED C+⅞". CAMBER INFORMATION IS CALLED OUT AT SPECIFIC JOINTS. EXTRAPOLATE BETWEEN CAMBERS PROVIDED. CAMBERS ARE ZERO AT THE TRUSS ENDS AND AT INTERMEDIATE SUPPORTS.
4. ⊥⊥ DENOTES BEAM SPLICE LOCATIONS PER 3/S6.2.
5. + DENOTES COLUMN SPLICE LOCATIONS PER 20/S6.4.

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Seattle, Washington

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LICENSED PROFESSIONAL ENGINEER NO. 7177S
JON D. MAGNUSSON
HAWAII, U.S.A.

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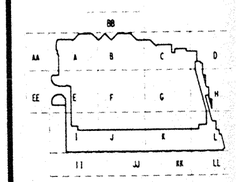
Supervision defined in Title II Chapter 16 Section 1.3.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
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	9-15-95		BUILDING PERMIT SET
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TRUSS ELEVATIONS – GRIDS 6 & 8

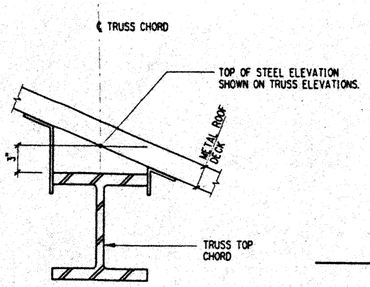
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

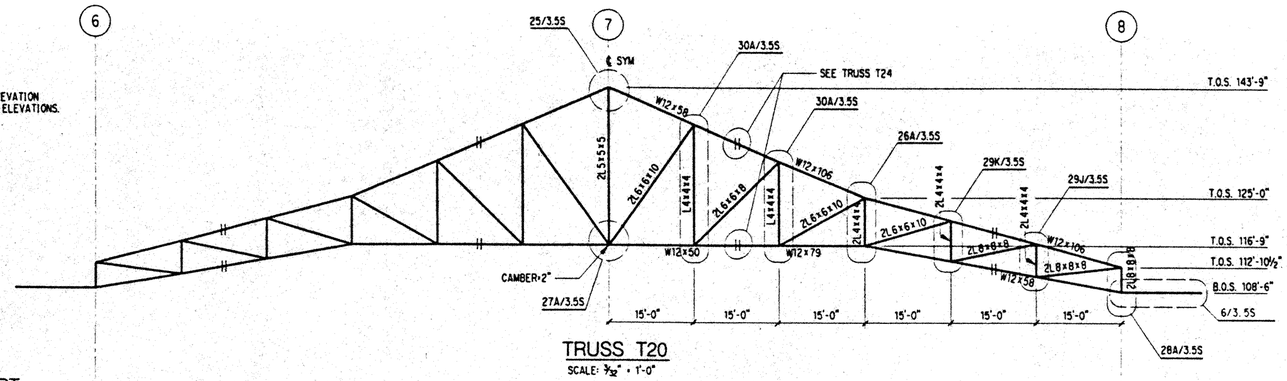
S5.8

Thu Nov 30 13:27:03 1995

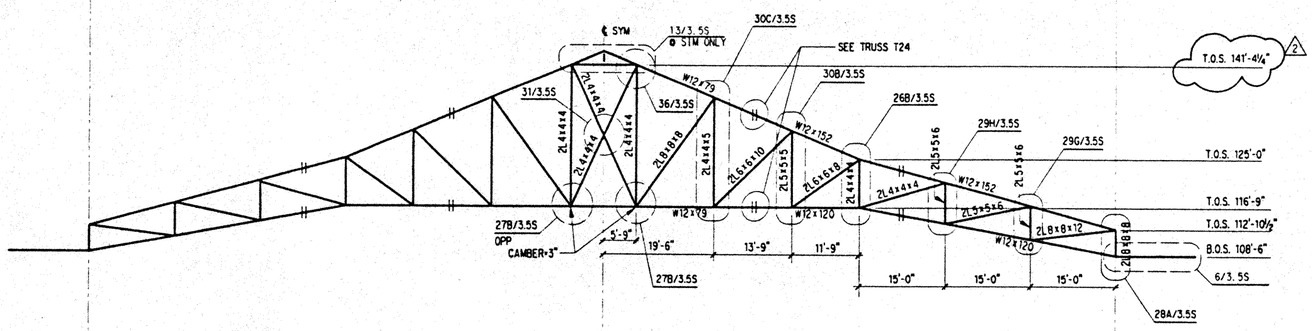
jthcc\truss_8.dgn



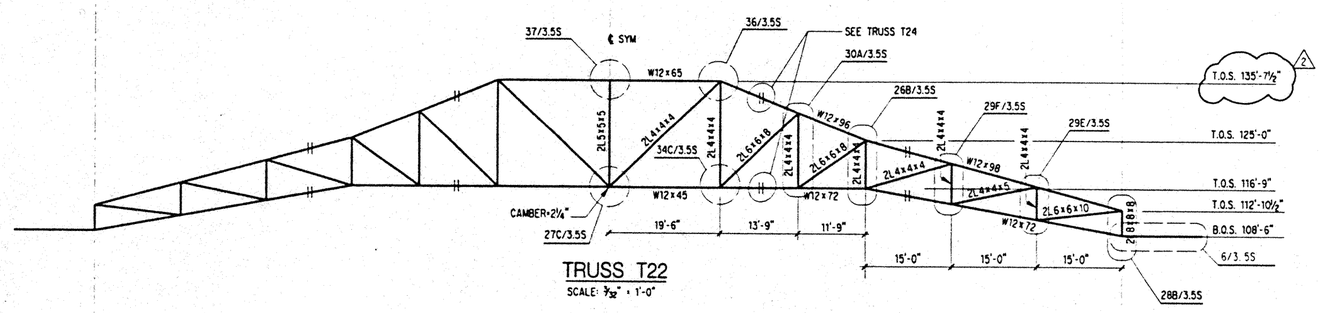
T.O.S. ORIENTATION @ ROOF HIP SUPPORT
SCALE: 1½" = 1'-0"



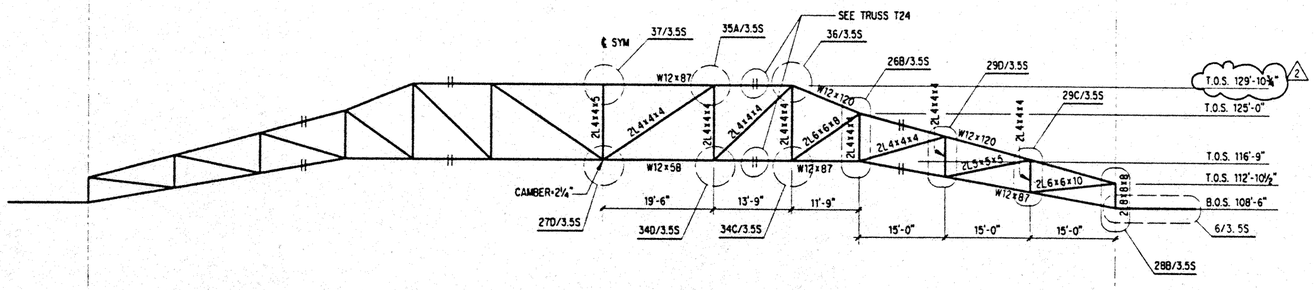
TRUSS T20
SCALE: 3/32" = 1'-0"



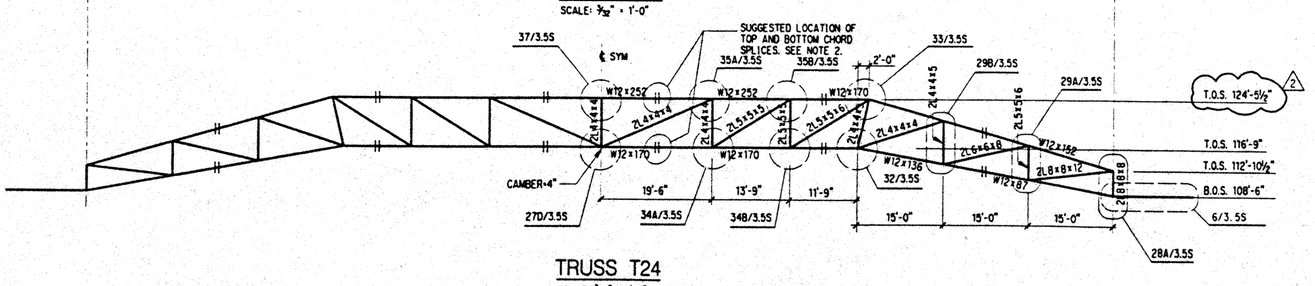
TRUSS T21
SCALE: 3/32" = 1'-0"



TRUSS T22
SCALE: 3/32" = 1'-0"



TRUSS T23
SCALE: 3/32" = 1'-0"



TRUSS T24
SCALE: 3/32" = 1'-0"

NOTES: (NOTES APPLY UNLESS NOTED OTHERWISE)

1. REFERENCE DRAWINGS:
   - S0.* SYMBOLS, INDEX, ABBREVIATIONS, GENERAL NOTES, LOAD MAPS
   - S1.7-S1.9 TYPICAL STEEL DETAILS
   - S4.* BRACED FRAME ELEVATIONS
   - DETAIL BOOK STRUCTURAL DETAIL
2. TOP AND BOTTOM CHORDS ARE TO BE SPLICED PER 3/S6.2 .
3. CAMBERS ARE MAXIMUM AT THE CENTER OF SPAN OF THE TRUSS AND WILL VARY LINERALY TO 0.0 AT THE SUPPORTS.
4. TYPICAL ANGLE BRACING DETAILS 15/S6.2, 18/S6.2, 20/S6.2 .
5. TOP OF STEEL ELEVATIONS ARE SHOWN.

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LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

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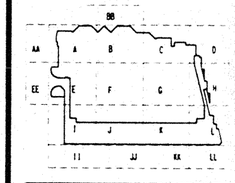
Supervision as defined in Title 16 Chapter 18 Section 1.2 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	5-8-95		ADDENDUM #3

Sheet Title

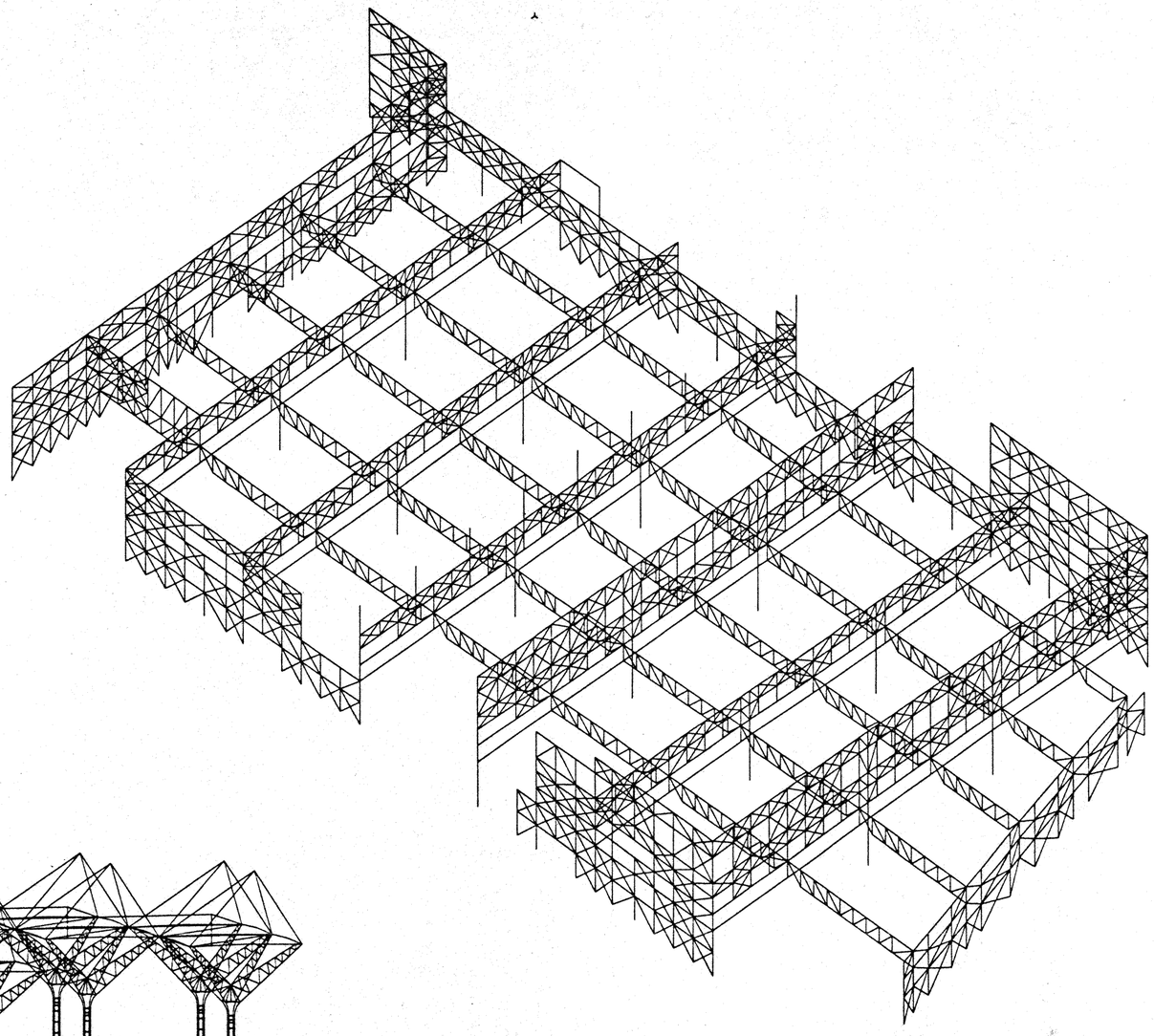
TRUSS ELEVATIONS – BALLROOM

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

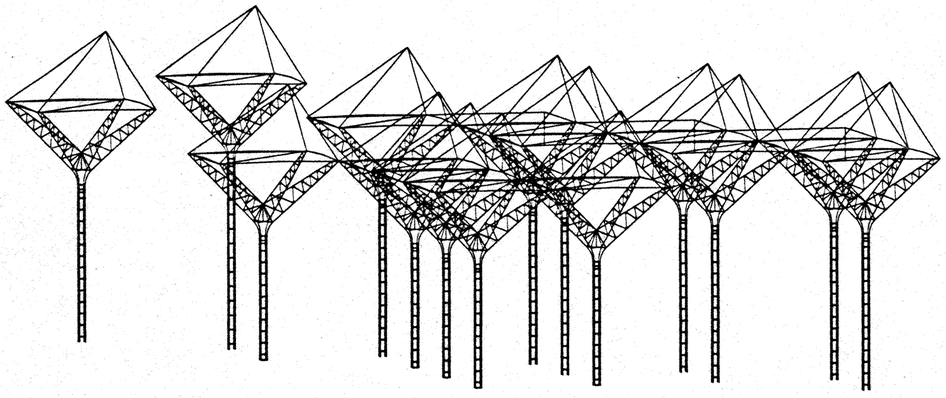
Sheet No.

S5.9

JWS pit j:\hcc\truss_9.dgn Thu Nov 30 13:29:20 1995



STRUCTURAL FRAME ISOMETRIC



LOBBY ISOMETRIC

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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in association with

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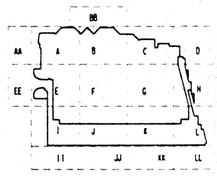
JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 71775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision performed in Title 16 Chapter 18 Section 1-3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	6-19-95		

Sheet Title

STRUCTURAL FRAME ISOMETRIC

Designed BJM
Drawn DMS
Checked JDM
Date 6 / 19 / 95
Job No. 93036.02.9

Sheet No.

S6.0

Thu Nov 30 13:31:51 1995
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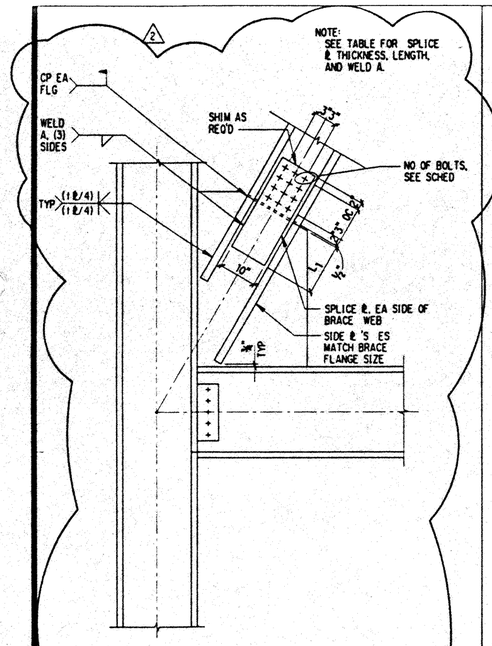
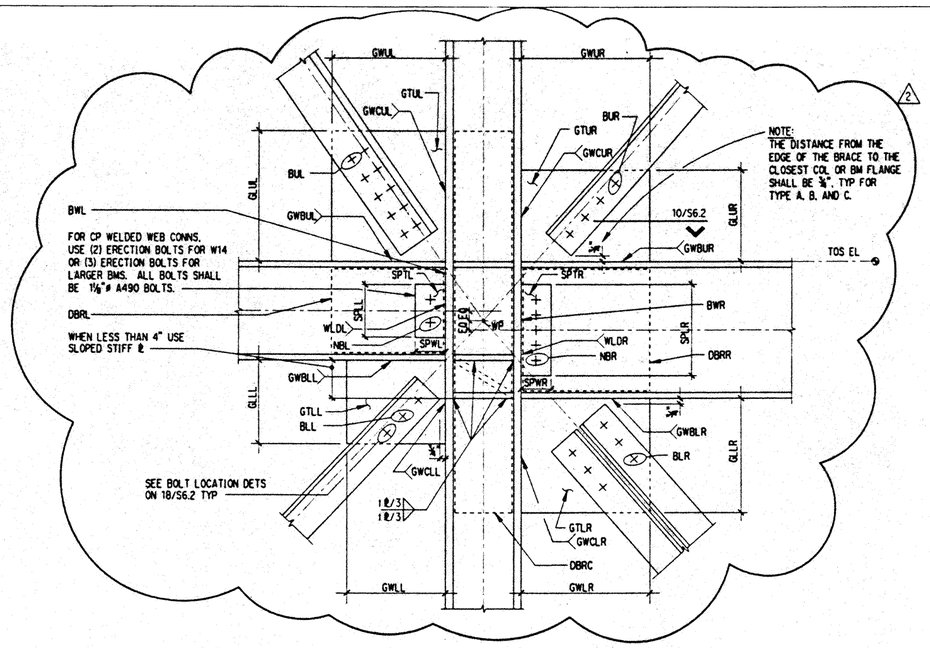
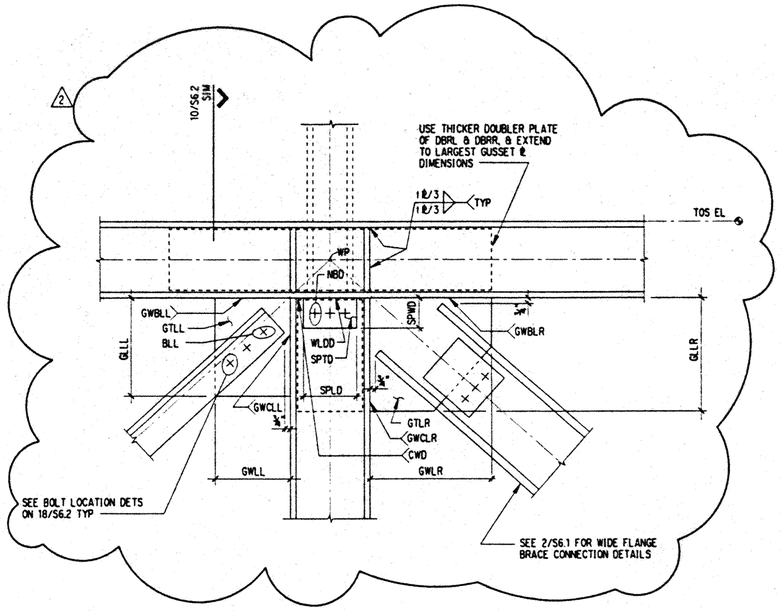


NO OF BOLTS	SPLICE ℓ THICKNESS	L_1	WELD A	MIN BRACE WEB THICKNESS
3	½"	4"	3/16	¾"
6	⅜"	7"	⅜	1"
9	1"	10"	½	1¼"
12	1¼"	13"	½	1⅜"

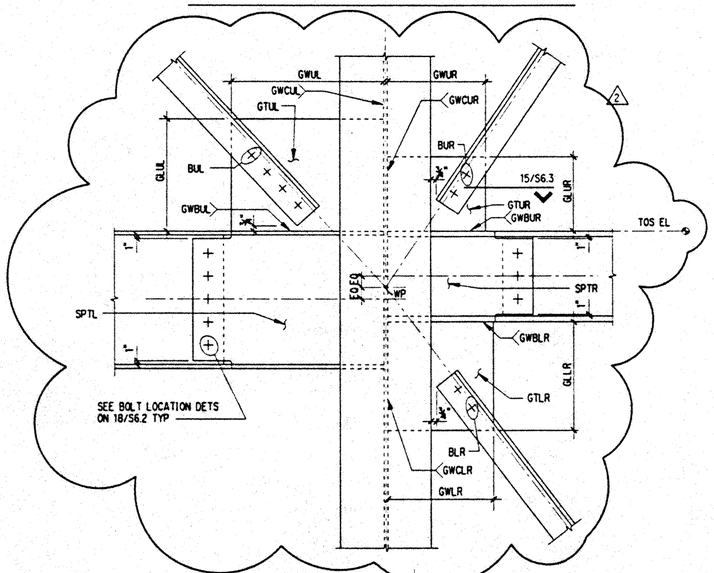
TYP WIDE FLANGE BRACE CONN | 6



DETAIL KEY FOR TYPE 'A' CONNECTION



DETAIL KEY FOR TYPE 'C' CONNECTION



DETAIL KEY FOR TYPE 'B' CONNECTION

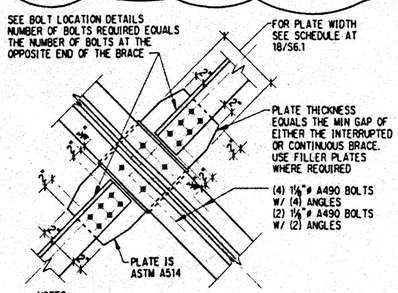
KEY TO CONNECTIONS

CONNECTION PIECE	ACRONYM	DESCRIPTION
LEFT BEAM SHEAR PLATE	NBL	NUMBER BOLTS LEFT
	SPTL	SHEAR PLATE THICKNESS LEFT
	SPWL	SHEAR PLATE WIDTH LEFT
	SPLL	SHEAR PLATE LENGTH LEFT
	WLDL	SHEAR PLATE FILLET WELD LEFT
	CWL (2)	LEFT BEAM WEB CP REQUIREMENT
RIGHT BEAM SHEAR PLATE	NBR	NUMBER BOLTS RIGHT
	SPTR	SHEAR PLATE THICKNESS RIGHT
	SPWR	SHEAR PLATE WIDTH RIGHT
	SPLR	SHEAR PLATE LENGTH RIGHT
	WLDR (2)	SHEAR PLATE FILLET WELD RIGHT
	CWR (2)	RIGHT BEAM WEB CP REQUIREMENT
UPPER COLUMN SHEAR PLATE	NBU	NUMBER BOLTS UPPER
	SPTU	SHEAR PLATE THICKNESS UPPER
	SPWU	SHEAR PLATE WIDTH UPPER
	SPLU	SHEAR PLATE LENGTH UPPER
	WLDU (2)	SHEAR PLATE FILLET WELD UPPER
	CWU (2)	UPPER COLUMN WEB CP REQUIREMENT
LOWER COLUMN SHEAR PLATE	NBD	NUMBER BOLTS LOWER
	SPTD	SHEAR PLATE THICKNESS LOWER
	SPWD	SHEAR PLATE WIDTH LOWER
	SPLD	SHEAR PLATE LENGTH LOWER
	WLDD (2)	SHEAR PLATE FILLET WELD LOWER
	CWD (2)	LOWER COLUMN WEB CP REQUIREMENT
BRACE BOLTS	BUL	NUMBER OF BRACE BOLTS UPPER LEFT
	BUR	NUMBER OF BRACE BOLTS UPPER RIGHT
	BLL	NUMBER OF BRACE BOLTS LOWER LEFT
	BLR	NUMBER OF BRACE BOLTS LOWER RIGHT
UPPER LEFT GUSSET PLATE	GTUL	GUSSET THICKNESS UPPER LEFT
	GWUL	GUSSET WIDTH UPPER LEFT
	GLUL	GUSSET LENGTH UPPER LEFT
	GWCUL	GUSSET WELD COLUMN UPPER LEFT
	GWBUL	GUSSET WELD BEAM UPPER LEFT
UPPER RIGHT GUSSET PLATE	GTUR	GUSSET THICKNESS UPPER RIGHT
	GWUR	GUSSET WIDTH UPPER RIGHT
	GLUR	GUSSET LENGTH UPPER RIGHT
	GWCUR	GUSSET WELD COLUMN UPPER RIGHT
	GWBUR	GUSSET WELD BEAM UPPER RIGHT
LOWER LEFT GUSSET PLATE	GTLL	GUSSET THICKNESS LOWER LEFT
	GWLL	GUSSET WIDTH LOWER LEFT
	GLLL	GUSSET LENGTH LOWER LEFT
	GWCLL	GUSSET WELD COLUMN LOWER LEFT
	GWBLL	GUSSET WELD BEAM LOWER LEFT
LOWER RIGHT GUSSET PLATE	GTLR	GUSSET THICKNESS LOWER RIGHT
	GWLR	GUSSET WIDTH LOWER RIGHT
	GLLR	GUSSET LENGTH LOWER RIGHT
	GWCLR	GUSSET WELD COLUMN LOWER RIGHT
	GWBLR	GUSSET WELD BEAM LOWER RIGHT
MISCELLANEOUS	PL	FRAME GEOMETRIC PLANE
WEB DOUBLER PLATE (SEE 17/S6.3 FOR DETAILS)	DBRL	BEAM WEB DOUBLER PLATE THICKNESS LEFT
	DBRR	BEAM WEB DOUBLER PLATE THICKNESS RIGHT
	DBRC	COLUMN WEB DOUBLER PLATE THICKNESS

NOTES:

1. SEE SCHEDULE ON SHEETS S6.5 THRU S6.16 FOR SPECIFIC JOINT INFORMATION.
2. SEE FRAME AND TRUSS ELEVATIONS FOR JOINT NUMBERS.

BRACED FRAME AND TRUSS DETAIL KEY | 15



NOTES:

1. THE BRACE WHICH HAS THE GREATER NO. OF BOLTS AT EACH END SHALL BE CONTINUOUS THRU THE JOINT. IF THE BRACES HAVE EQUAL NO. OF BOLTS, THEN THE BRACE W/ THE LARGER AREA SHALL BE CONTINUOUS THRU THE JOINT. IF THE BRACES HAVE AN EQUAL NO. OF BOLTS AND EQUAL AREA, THEN THE BRACE W/THE LARGER GAP SHALL BE CONTINUOUS.
2. FOR W SECTION BRACE CONNECTION DETAILS, SEE 12 & 13/S6.2

BRACE CONNECTION DETAIL | 11

BRACE CONNECTION PLATE WIDTH (INCHES)

PLATE THICKNESS \ BRACE SIZE	4L8-8-1/8-PLA	4L8-8-1/8-PLB	4L8-8-1/8-PLC	4L8-8-1/8-PLD	4L8-8-1/8-PLE	4L8-8-1/8	4L8-8-16	4L8-8-14	4L8-8-12	4L8-8-10	4L8-8-8	4L6-6-10	4L6-6-6	4L5-5-5	4L4-4-8	4L4-4-5	4L4-4-4	2L8-8-18	2L8-8-16	2L8-8-14	2L8-8-12	2L8-8-10	2L8-8-8	2L8-6-16	2L8-6-12	2L8-6-8	2L7-4-12	2L7-4-10	2L7-4-8	2L6-6-16	2L6-6-14	2L6-6-12	2L6-6-10	2L6-6-8	2L6-6-6	2L6-4-12	2L6-4-8	2L6-4-6	2L5-5-8	2L5-5-6	2L5-5-5	2L4-4-12	2L4-4-10	2L4-4-8	2L4-4-6	2L4-4-5	2L4-4-4	2L3.5-3.5-6	2L3.5-3.5-5	2L3.5-3.5-4	2L3-3-8	2L3-3-6	2L3-3-5	2L3-3-4
1/2	85	82	79	76	72	66	61	55	49	44	38	36	27	16	19	14	14	33	30	28	25	22	19	27	22	17	19	17	13	24	22	20	18	16	14	18	14	12	11	11	11	12	11	10	10	10	10	10	10	10	10	9	9	9	9
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SCHEDULE | 18

NOTES: (NOTES APPLY UNLESS MARKED OTHERWISE)

1. REFERENCE GENERAL NOTES ON DRAWING S0.2 FOR ITEMS NOT COVERED BELOW. THESE NOTES SUPERSEDE THE GENERAL NOTES WHERE A DIFFERENCE EXISTS.
2. ALL STRUCTURAL STEEL CALLED OUT ON DRAWINGS S4.•, S5.• AND S6.• IS AS FOLLOWS:
   ALL STEEL, UNO - ASTM A572 GRADE 50, A441 FY=50 KSI, A588 FY=50 KSI
   MATERIAL CALLED OUT AS (FY=36) - ASTM A36 FY=36 KSI
   ALL PLATE, UNO - ASTM A572 GRADE 50, A441 FY=50 KSI, A588 FY=50 KSI
   MATERIAL CALLED OUT AS (ASTM A514) - ASTM A514 FY=100 KSI
   ALL ANGLES - ASTM A572 GRADE 50, A441 Fy=50 KSI, A588 Fy=50 KSI. (1)
3. WORK POINT (W.P.) IS ALWAYS MIDWAY BETWEEN THE CENTERLINES OF BEAMS FRAMING INTO THE JOINT (APPLIES TO BEAMS IN THE SAME PLANE ONLY).
4. SHEAR STUDS SHALL BE PLACED ON ALL HORIZONTAL STEEL MEMBERS RECEIVING A CONCRETE SLAB. ONE ROW OF STUDS AT 1'-0" ON CENTER IS REQUIRED, UNO.
5. INDIVIDUAL JOINT DETAILS INCLUDE DETAILING INFORMATION ONLY IN THE PLANE OF THE BRACING ELEVATION. FOR DETAILING INFORMATION IN OTHER PLANES, SEE JOINT DETAILS OF THE ELEVATION OR GRAVITY FRAMING DETAILS.
6. ALL BOLTS SHALL BE 1-1/8" DIAMETER A490 BOLTS WITH STANDARD SIZE HOLES, UNO.
7. FOR PARTIAL PENETRATION GROOVE WELDS SHOWN ON THE DRAWINGS, ALL WELD SIZES REFER TO EFFECTIVE THROAT THICKNESS.
8. DIMENSIONAL SIZES CALLED OUT IN DECIMAL NOTATION SHALL BE CONSIDERED TO THE NEAREST 1/16 INCH FRACTIONAL NOTATION.
9. WHERE NOTED, CENTERLINES OF STEEL LINE UP. CENTERLINE IS SET BY DEEPEST MEMBER BEING PLACED IMMEDIATELY UNDER STEEL DECK, UNDER TOP OF STEEL (T.O.S.) ELEVATION OR ABOVE BOTTOM OF STEEL (B.O.S.) ELEVATION. TYPICALLY SEE FLOOR PLANS AND ELEVATIONS FOR TOP OF STEEL ELEVATIONS. SEE "TYPICAL STEEL DECK SUPPORT AT CHORD MEMBERS" DETAIL FOR DECK SUPPORT. (2)
10. ASTM A6 GROUP 4 AND 5 ROLLED SHAPES AND PLATES EXCEEDING 2-INCH THICKNESS USED FOR BUILT-UP MEMBERS, REQUIRES CHARPY V-NOTCH TESTING IN ACCORDANCE WITH ASTM A6 SUPPLEMENTARY REQUIREMENT S5, WHERE FULL-PENETRATION WELDS ARE CALLED OUT AT EITHER END OF THE MEMBER ON THE DRAWINGS. REFER TO AISC SPECIFICATION PARAGRAPH A3.C FOR TESTING STANDARDS. (1)

TYPICAL BRACED FRAME AND TRUSS NOTES | 20

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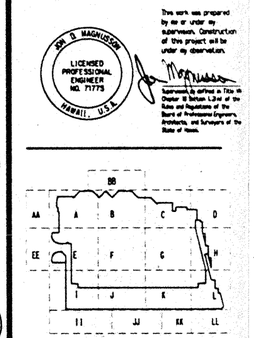


Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TYPICAL BRACED FRAME & TRUSS DETAILS

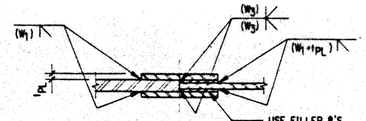
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S6.1

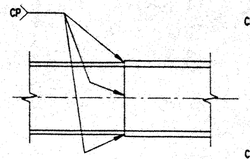
Thu Nov 30 13:34:19 1995
jhncc\brfrdt_1.dgn
DMS plt

NOTES:

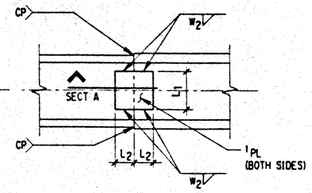
1. SEE AISC AND AWS FOR FULL PENETRATION SPLICE REQUIREMENTS.
2. ALL BEAM SPLICES HAVE BEEN DESIGNED AS TENSION SPLICES.
3. BEAM SPLICES SHALL BE LOCATED AT MID-SPAN UNLESS OTHERWISE NOTED.



SECTION A



AISC GROUP 1, 2 AND 3



AISC GROUP 4 AND 5

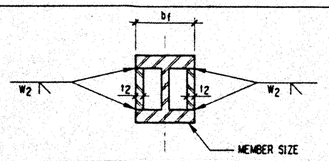
BEAM SPLICE SCHEDULE

BEAM SIZE	L1 (IN)	L2 (IN)	tPL (IN)	W1 (IN)	W2 (IN)	W3 (IN)
W14x730	10	5	1½	1¼	½	½
W14x665	10	5	1½	1¼	½	½
W14x605	10	5	1½	1¼	½	½
W14x550	10	5	1¼	1	½	½
W14x500	10	5	1¼	1	½	½
W14x455	10	5	1	¾	⅜	½
W14x426	10	5	1	¾	⅜	½
W14x398	10	5	1	¾	⅜	½
W14x370	10	5	¾	½	¼	½
W14x342	10	5	¾	½	¼	½
W14x311	10	5	¾	½	¼	½
W14x283	10	5	½	⅜	¼	⅜
W14x257	10	5	½	⅜	¼	⅜
W14x233	10	5	½	⅜	¼	⅜
W12x252	8	5	½	⅜	¼	⅜

BEAM SPLICE DETAILS 3

4

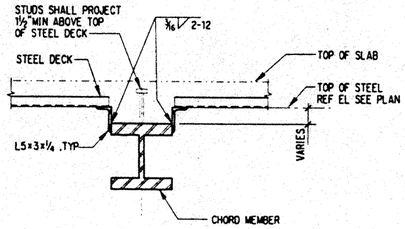
5



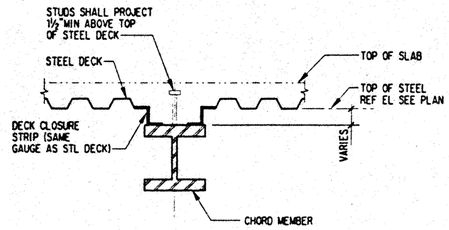
SECTION	MEMBER SIZE	bf (IN)	t2 (IN)	W2 (IN)
BU14x1074	W14x730	17.89	4	⅜
BU14x988	W14x730	17.89	3	½
BU14x902	W14x730	17.89	2	½
BU14x817	W14x730	17.89	1	½
BU36x527	W36x300	--	1	½
BU36x641	W36x300	16.65	1½	½

NOTE:
ADDED PLATES EXTEND 3'-0" ABOVE TOP OF STEEL @ BU14, UNLESS NOTED OTHERWISE.

BUILT-UP MEMBER 6

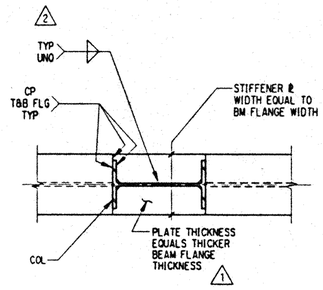


DECK SPAN NORMAL TO TRUSS CHORD

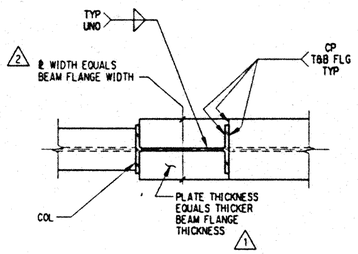


DECK SPAN PARALLEL TO TRUSS CHORD

TYPICAL STEEL DECK SUPPORT AT CHORD MEMBERS 8



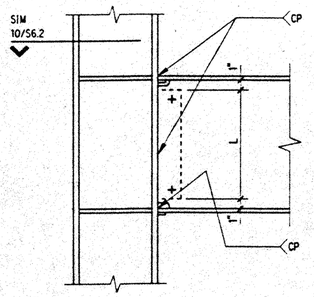
BEAM FLANGES WIDER THAN COLUMN FLANGES BOTH SIDES



BEAM FLANGES WIDER THAN COLUMN FLANGES ON ONE SIDE

NOTE:
1. BRACE GUSSET PL'S NOT SHOWN FOR CLARITY.

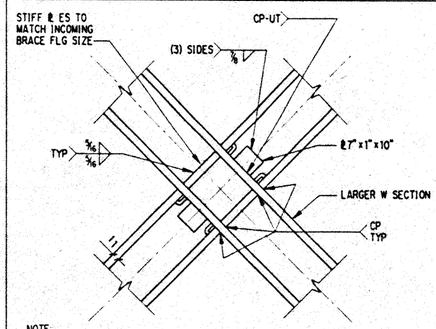
BEAM FLANGE DETAILS - TYPE A & C 10



NOTE:
PROVIDE ⅜"x5"xL ERECTION PL W/ (2) 1⅛"ø A490 BOLTS FOR W14'S OR (3) 1⅛"ø A490 BOLTS FOR LARGER BEAMS. WELD ERECTION PL TO COL SECTION W/ ⅜"FILLET WELD BOTH SIDES.

DETAIL KEY FOR TYPE A & C

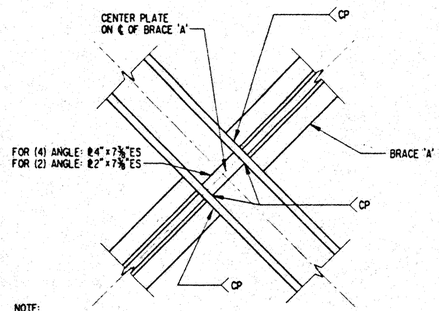
MOMENT CONNECTIONS 11



NOTE:
1. THE LARGEST WIDE FLANGE BRACE SHALL BE CONTINUOUS THRU JOINT.

DOUBLE W-SECTION

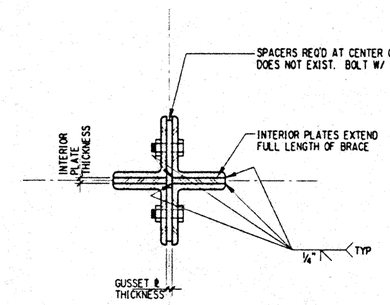
BRACE CONNECTION DETAIL 12



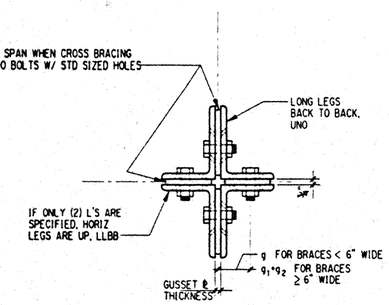
NOTE:
BRACE 'A' SHALL HAVE A COMPLETE PENETRATION WELD TO THE W-SECTION.

W-SECTION

BRACE CONNECTION DETAIL 13

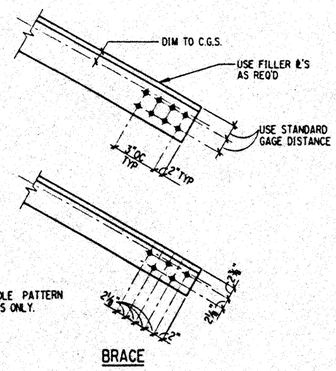


WITH PLATES



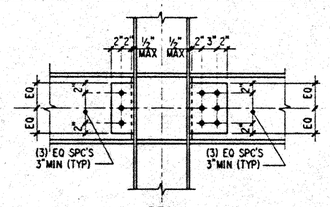
WITHOUT PLATES

ANGLE BRACING DETAILS 15



NOTE:
STAGGERED HOLE PATTERN FOR 6" ANGLES ONLY.

BRACE



BEAM

STANDARD GAGE DISTANCE

LEG LENGTH	8"	7"	6"	5"	4"	3½"	3"
g	4.5	4.0	3.5	3.0	2.5	2.0	1.75
g1	3.0	2.5	2.375	-	-	-	-
g2	3.0	3.0	2.125	-	-	-	-

NOTE: ALL BOLT HOLES IN BRACES ARE STANDARD SIZE HOLES.

BOLT LOCATION DETAILS 18

EL BRACE MARK	ACTUAL BUILT-UP SHAPE	EL BRACE MARK	ACTUAL BUILT-UP SHAPE
4L8-8-18+PLA	(4) L8x8x1-1/8 with (2) 1-1/2x8 PLS	2L6-6-16	(2) L6x6x1
4L8-8-18+PLB	(4) L8x8x1-1/8 with (2) 1-1/4x8 PLS	2L6-6-14	(2) L6x6x7/8
4L8-8-18+PLC	(4) L8x8x1-1/8 with (2) 1x8 PLS	2L6-6-12	(2) L6x6x3/4
4L8-8-18+PLD	(4) L8x8x1-1/8 with (2) 3/4x8 PLS	2L6-6-10	(2) L6x6x5/8
4L8-8-18+PLE	(4) L8x8x1-1/8 with (2) 1/2x8 PLS	2L6-6-8	(2) L6x6x1/2
4L8-8-18	(4) L8x8x1-1/8	2L6-6-6	(2) L6x6x3/8
4L8-8-16	(4) L8x8x1	2L6-4-12	(2) L6x4x3/4
4L8-8-14	(4) L8x8x7/8	2L6-4-8	(2) L6x4x1/2
4L8-8-12	(4) L8x8x3/4	2L6-4-6	(2) L6x4x3/8
4L8-8-10	(4) L8x8x5/8	2L5-5-8	(2) L5x5x1/2
4L8-8-8	(4) L8x8x1/2	2L5-5-6	(2) L5x5x3/8
4L6-6-10	(4) L6x6x5/8	2L5-5-5	(2) L5x5x5/16
4L6-6-6	(4) L6x6x3/8	2L4-4-12	(2) L4x4x3/4
4L5-5-5	(4) L5x5x5/16	2L4-4-10	(2) L4x4x5/8
4L4-4-8	(4) L4x4x1/2	2L4-4-8	(2) L4x4x1/2
4L4-4-5	(4) L4x4x5/16	2L4-4-6	(2) L4x4x3/8
4L4-4-4	(4) L4x4x1/4	2L4-4-5	(2) L4x4x5/16
2L8-8-18	(2) L8x8x1-1/8	2L4-4-4	(2) L4x4x1/4
2L8-8-16	(2) L8x8x1	2L3.5-3.5-6	(2) L3-1/2x3-1/2x3/8
2L8-8-14	(2) L8x8x7/8	2L3.5-3.5-5	(2) L3-1/2x3-1/2x5/16
2L8-8-12	(2) L8x8x3/4	2L3.5-3.5-4	(2) L3-1/2x3-1/2x1/4
2L8-8-10	(2) L8x8x5/8	2L3-3-8	(2) L3x3x1/2
2L8-8-8	(2) L8x8x1/2	2L3-3-6	(2) L3x3x3/8
2L8-6-16	(2) L8x6x1	2L3-3-5	(2) L3x3x5/16
2L8-6-12	(2) L8x6x3/4	2L3-3-4	(2) L3x3x1/4
2L8-6-8	(2) L8x6x1/2		
2L7-4-12	(2) L7x4x3/4		
2L7-4-10	(2) L7x4x5/8		
2L7-4-8	(2) L7x4x1/2		

ANGLE BRACE CONVERSION TABLE 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

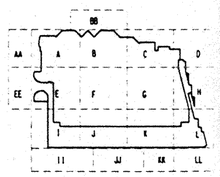
Consultants

Skilling Ward Magnusson Barkshire Inc.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 623-1201

JAM D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervised by defined in Title 16 Chapter 18 Subtitle 1 Part of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

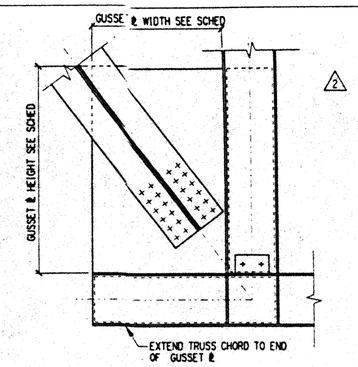
Sheet Title

TYPICAL BRACED FRAME & TRUSS DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

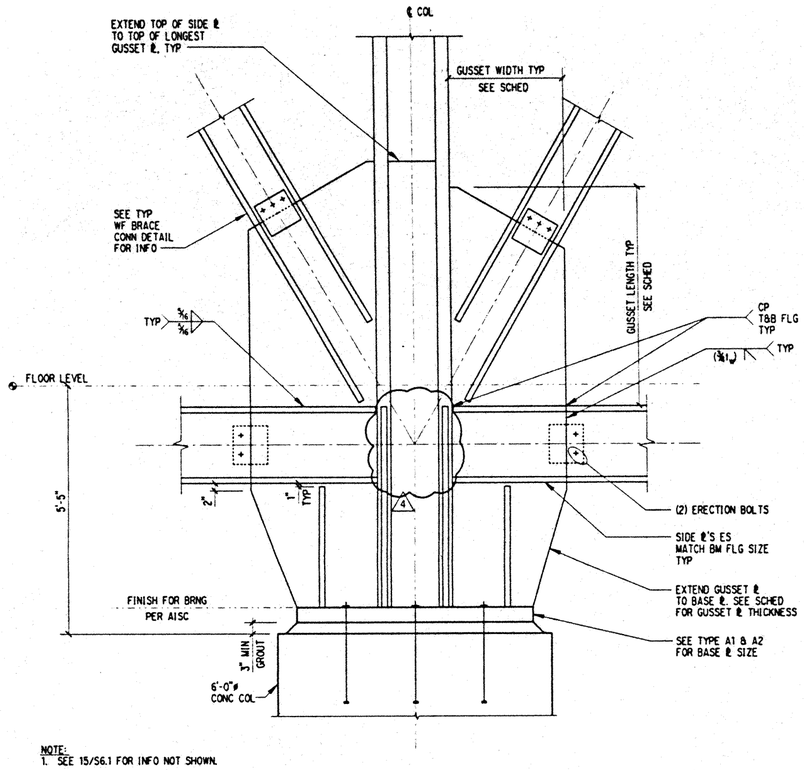
S6.2

Thu Nov 30 13:36:43 1995 j:\hccbr\frdt_2.dgn DMS plt

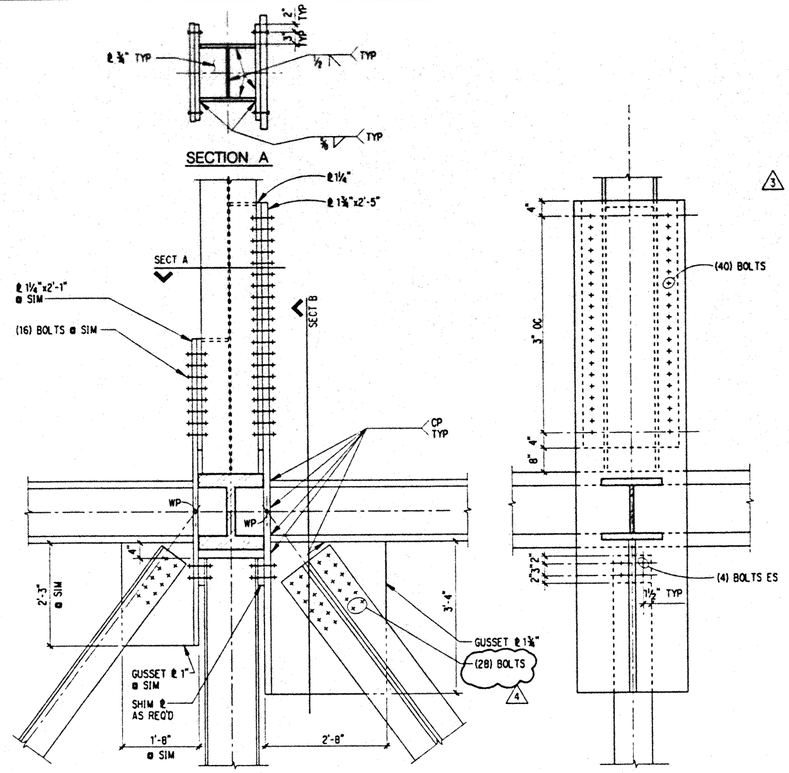


NOTE:
SEE TYPICAL TRUSS DETAIL TYPE "C" CONNECTION FOR INFO NOT SHOWN.

DETAIL 1



NOTE:
1. SEE 15/S6.1 FOR INFO NOT SHOWN.

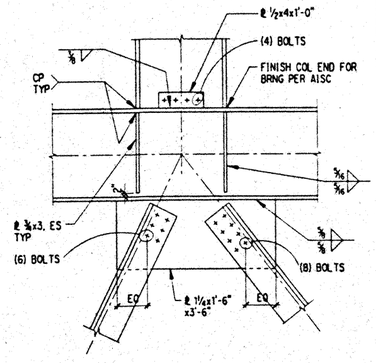


NOTE:
ALL BOLTS 1⅛"ø A490

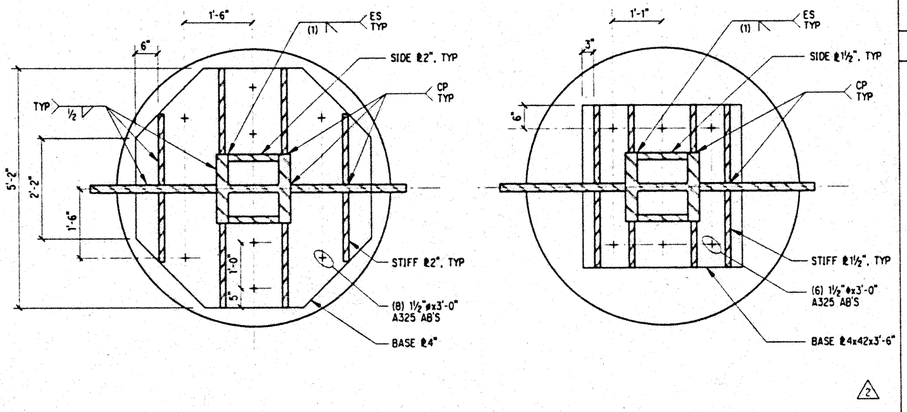
SECTION B

DETAIL 10

DETAIL 6

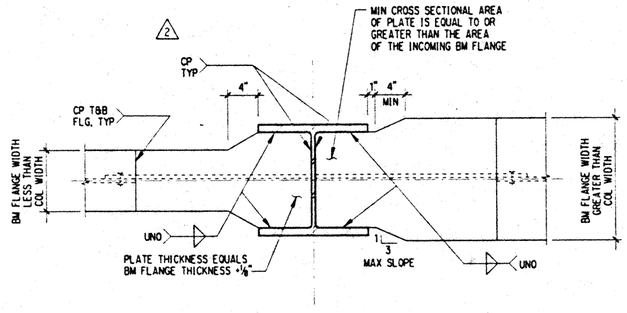


DETAIL 11



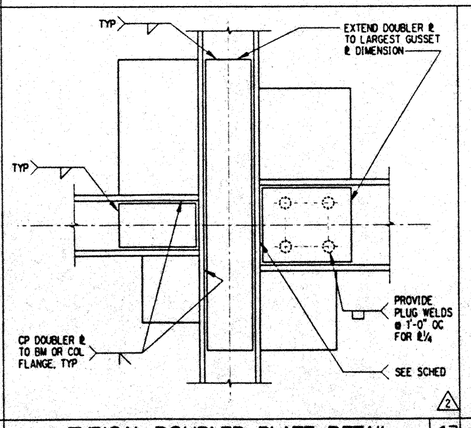
TYPE A1 TYPE A2

BASE PLATE - TYPE A 13

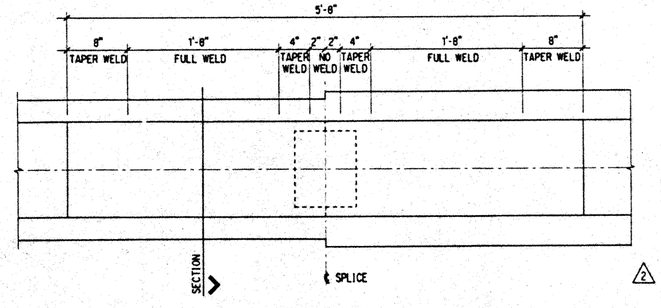


NOTE:
BRACE GUSSET PLATES NOT SHOWN FOR CLARITY.

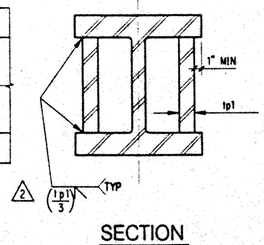
BEAM FLANGE DETAILS - TYPE B 15



TYPICAL DOUBLER PLATE DETAIL 17



ELEVATION



SECTION

NOTES:

1. SPLICE PLATES SHALL BE INSTALLED AFTER BEAM SPLICE DETAIL 3/S6.2 IS COMPLETE AND HAS BEEN FULLY INSPECTED.
2. ALL PLATES SHALL BE Fy=50 KSI, ASTM A572 GR 50, A441 Fy=50 OR A588 Fy=50.
3. ALL PLATE MATERIAL TO BE TESTED FOR CHARPY V-NOTCH TESTING IN ACCORDANCE WITH ASTM A6, SUPPLEMENTARY REQUIREMENT S5. 2
4. ALL BEVELS SHALL HAVE A MILLED FACE.
5. PARTIAL PENETRATION WELD SIZES ARE CALLED OUT AS EFFECTIVE THROAT THICKNESS.
6. ALL PLATES SHALL BE ULTRASONICALLY TESTED FOR LAMINAR DEFECTS USING ASTM A578 - LEVEL 1.
7. ALL WELDS SHALL BE M-T TESTED AFTER THEY ARE COMPLETE.
8. SEE FRAME AND TRUSS ELEVATIONS FOR LOCATIONS AND SIZE OF TENSION SPLICE PLATES.

TENSION SPLICE PLATES 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
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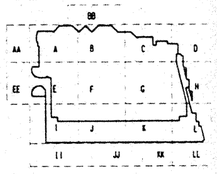
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 623-1201

LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising Engineer in Title in Chapter 16 Subpt 1-2 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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4	8-31-95		
3	8-4-95		
2	6-19-95		
1	5-8-95		ADDENDUM #3

Sheet Title

TYPICAL BRACED FRAME & TRUSS DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S6.3

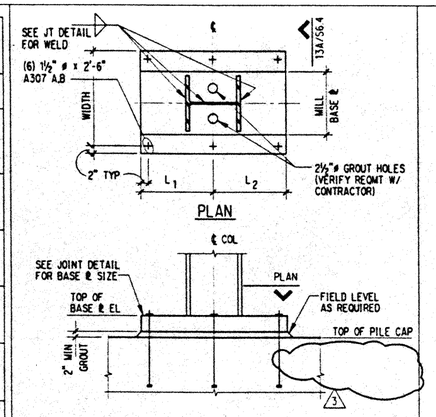
Thu Nov 30 13:39:04 1995
j:\hccbr\frdt\3.dgn
MS pit

BRACED FRAME BASE PLATE SCHEDULE

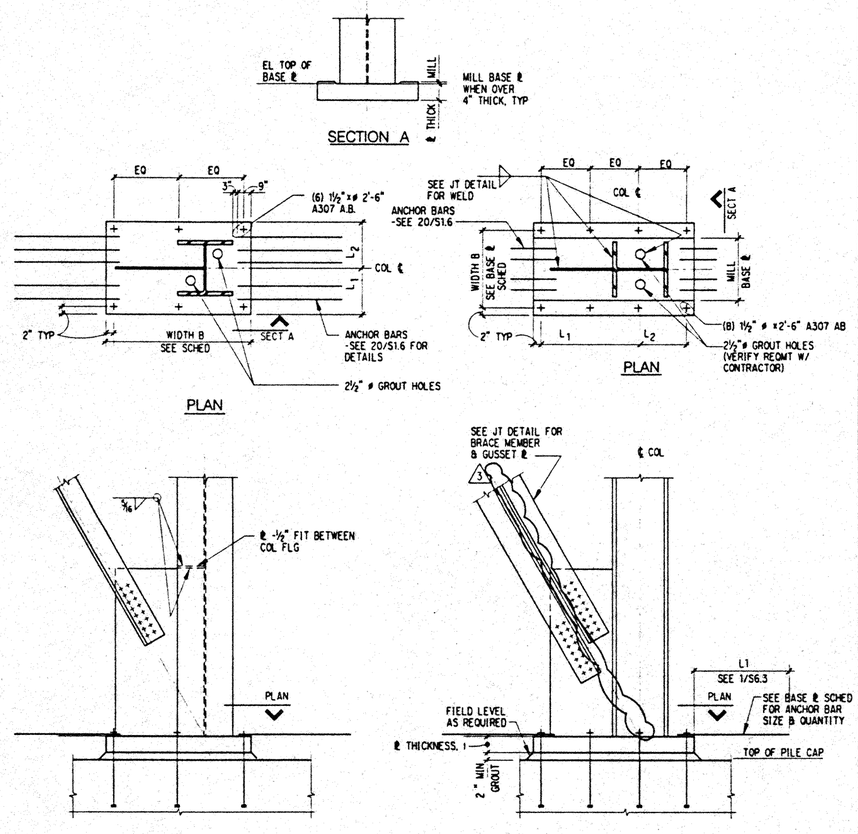
NODE NO	L1 (in)	L2 (in)	B (in)	t (in)	ANCHOR BARS NO	ANCHOR BARS SIZE	REMARKS
2001	17	17	50	1.5	5	11	
2002	14	28	16	1			
2003	31	24	24	1	6	11	
2004	32	25	16	2	4	11	
2005	33	33	20	1.5	4	11	
2006	30	28	16	1	4	11	
2007	33	31	20	1	5	11	
2008	32	30	20	1.75	5	11	
2009	31	36	20	2	4	11	
2010	14	14	16	1.25			
2011	29	25	20	1.25	5	11	
2012	30	36	20	1	5	11	
2013	14	28	16	2			
2015	17	17	18	1.5			
2016	30	35	24	1.75	5	14	
2017	25	30	16	1.25	4	11	
2018	26	25	16	2			
2019	25	25	16	1.25	2	11	
2020	31	26	16	1.25	4	11	
2021	33	31	24	0.75	6	11	
2022	27	29	16	1	4	11	
2023	36	31	20	1.5	3	11	
2024	30	28	16	1.25	4	11	
2025	28	30	16	1.25	4	11	
2026	29	31	20	1.5	5	11	
2027	29	38	24	2	6	11	
3001	24	29	32	4	4	11	
3027	27	34	36	5.25	6	14	
4001	26	23	30	4	4	11	
4027	26	23	28	3.5	4	11	
5001	23	25	28	3.25			
5027	24	24	34	4.75	2	11	
6001	22	16	28	3.5			
6027	19	19	34	4.75			
7001	25	21	22	2.25	4	11	
7027	29	25	26	2.5	5	11	
8001	25	21	24	2.5	5	11	
8027	31	23	30	3.75	6	11	
10004	33	25	16	1	4	11	
10005	14	27	16	1			
10006	34	28	24	1	5	14	
10008	25	32	16	0.75	4	11	
10009	33	25	16	1	3	11	
10011	34	33	24	0.75	6	11	
10013	29	37	24	1	5	14	
10015	34	26	20	1.5	5	11	
10017	29	34	24	1	6	11	
10019	31	31	16	1.25	4	11	
10020	34	29	20	1	5	11	
10022	29	32	24	1	6	11	
10023	25	31	16	1.25	3	11	
10024	25	28	16	1.5	4	11	
11001	25	34	24	1.75	6	11	
11002	29	32	24	1.5	6	11	
11003	29	25	16	1.5	4	11	
11004	14	14	16	1.25			
11005	28	25	16	1.25	3	11	3
11006	37	28	20	1.5	5	11	
11007	35	28	24	1.5	6	11	
11008	28	30	20	1.25	5	11	
11009	28	34	24	1.5	6	11	
11011	31	28	18	1.75	4	11	
11013	25	29	16	1.25	4	11	
11014	29	29	16	1.25	3	11	
11015	29	27	16	1	3	11	
11016	29	25	16	0.75	4	11	
11017	29	24	14	0.75	3	11	
11018	25	29	16	1	4	11	
11019	27	32	24	1.25	6	11	
11021	14	14	16	1.25			
11023	14	14	16	1.25			
11024	32	29	16	1.75	4	11	
11025	14	14	16	1.25			
11026	25	27	16	1	3	11	
11027	29	27	20	1.25	5	11	
11028	27	25	16	1	3	11	
11029	27	25	16	1.25			
11031	17	17	18	1.25			
11033	19	19	20	1.5			
17001	34	34	18	2	4	11	
17002	31	29	16	2	4	11	
17003	35	36	20	2	5	11	
17004	14	14	14	0.75			
17005	34	42	30	2.75	7	14	SPACE ANCHOR BOLTS @ 4"
17007	14	27	16	1.5			
17008	35	28	34	2	7	14	
17009	33	28	24	1.25	5	14	
17010	29	31	16	1.5	4	11	
17011	28	37	34	1.75	7	14	
17012	14	14	16	1.25			
17014	25	31	20	2	5	11	
17015	35	34	24	1.75	5	14	
17016	14	14	16	0.75			
17017	34	29	16	1.5	4	11	
17018	25	29	16	1.25	3	11	
17019	29	29	16	1.75	3	11	

NOTE:

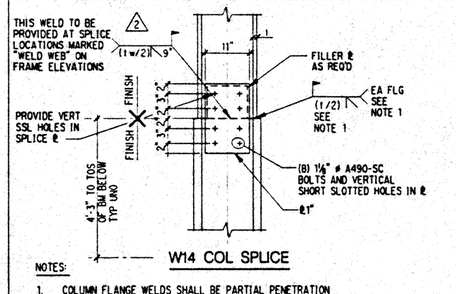
1. SEE DETS 2, 13/S6.4 FOR PLATE CONFIGURATION.
2. REFER TO 20/S1.6 FOR ANCHOR BAR DETAILS



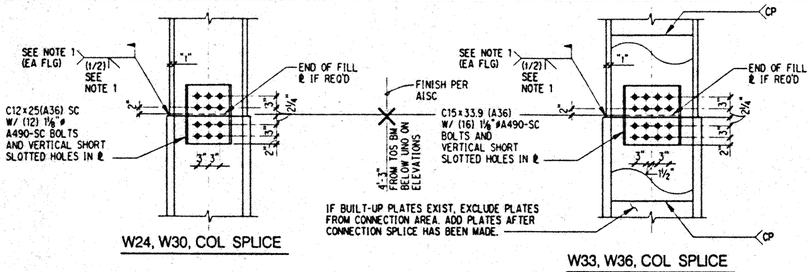
BASE PLATE – TYPE B | 2 | 3



BASE PLATE – TYPE C | 13 | 14 | 15



W14 COL SPLICE



W24, W30, COL SPLICE

W33, W36, COL SPLICE

NOTES:

1. COLUMN FLANGE WELDS SHALL BE PARTIAL PENETRATION AS SHOWN ABOVE UNLESS NOTED OTHERWISE ON ELEVATIONS BY CP NEXT TO THE CORRESPONDING COLUMN SPLICE. CP DENOTES A COMPLETE PENETRATION WELD IS REQ'D AT EACH FLANGE FOR THAT PARTICULAR SPLICE. THE (1/2) SHOWN ABOVE INDICATES THAT A PARTIAL PENETRATION WELD IS REQUIRED WITH AN EFFECTIVE THROAT EQUAL TO "1/2". IF "1/2" IS LESS THAN ¼" THEN FLANGES SHALL HAVE COMPLETE PENETRATION WELDS.
2. CONTRACTOR SHALL PROVIDE ALL ERECTION AND STABILITY AIDS.

16 | 17 | COLUMN SPLICE DETAIL, TYPE 3 | 20

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

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JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

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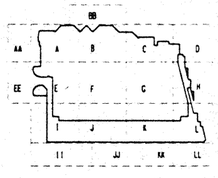
Supervision fulfilled in Title 16 Chapter 18 Section 1.2.H of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

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3	11-7-95		
	9-15-95		BUILDING PERMIT SET
2	8-4-95		
1	6-19-95		

Sheet Title

TYPICAL BRACED FRAME SCHEDULE & TRUSS DETAILS

Designed BJM
Drawn DMS
Checked JDM
Date 6 / 19 / 95
Job No. 93036.02.9
Sheet No.

1 S6.4

Thu Nov 30 13:41:18 1995
jxhccbrfrdt_4.dgn

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEL	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWER	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEU	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWED	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
GRID 2																																																							
2001 YZ B	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	-	-	-	.75	29.0	19.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2801 YZ B	5	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	-	.50	42.0	12.0	.375	.250	-	-	-	-	-	-	.50	31.0	13.0	.438	.313	-	-	-	-	-	-	-	-	-
2101 YZ B	4	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2201 YZ B	6	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	.75	32.0	13.0	.500	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2301 YZ B	4	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	27.0	13.0	.313	.313	-	-	-	-	-	-	-	-
2401 YZ B	4	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	.50	37.0	12.0	.313	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2501 YZ B	4	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	3	-	.50	32.0	12.0	.188	.250	-	-	-	-	-	-	.50	29.0	11.0	.250	.188	-	-	-	-	-	-	-	-
2601 YZ B	4	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	27.0	13.0	.188	.188	-	-	-	-	-	-	-	-
2002 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	.50	15.0	17.0	CP	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2802 YZ A	5	.500	4.63	16.00	.250		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	12	.75	29.0	12.0	.313	.250	-	-	-	-	-	-	-	-	-	-	-	-	.75	26.0	19.0	CP	CP	-	-	-
2102 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	-	.50	20.0	13.0	.313	.188	.50	24.0	13.0	CP	.250	-	-	-
2202 YZ A	8	.500	7.63	19.00	.375		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	4	18	-	-	-	-	-	-	.50	20.0	12.0	.375	.250	.50	28.0	30.0	.438	.438	-	-	-	-	-	-	.25	-
2302 YZ A	5	.625	4.63	16.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	4	1.00	20.0	17.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	-	.75	16.0	14.0	CP	CP	-	-	-
2402 YZ A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	3	4	-	-	-	-	-	-	.50	24.0	10.0	.250	.188	.50	14.0	14.0	.375	.375	-	-	-	-	-	-	-	-
2502 YZ A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	4	.75	18.0	15.0	.500	.438	.50	20.0	12.0	.250	.188	-	-	-	-	-	-	-	.50	19.0	13.0	CP	.313	-	-	-
2602 YZ A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	4	-	-	-	-	-	-	-	-	-	-	-	.50	11.0	11.0	.375	.313	.50	16.0	13.0	.313	.250	-	-	-
2003 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	-	1.25	19.0	22.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
2803 YZ A	6	.625	4.63	19.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	12	4	-	-	.75	39.0	17.0	.500	.375	.50	30.0	12.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2103 YZ A	3	.375	4.63	21.00	.188	CP	6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	18	6	4	-	-	-	-	-	.50	29.0	30.0	.438	.438	.50	26.0	16.0	.438	.250	.50	21.0	13.0	.500	.313	-	-	-
2203 YZ A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2303 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	.50	17.0	14.0	.313	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2403 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	8	.75	22.0	12.0	.250	.250	-	-	-	-	-	-	-	-	-	-	-	.75	16.0	17.0	CP	CP	-	.25	-
2503 YZ A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	3	3	5	-	.75	13.0	10.0	.250	.250	.75	14.0	11.0	.250	.250	.75	19.0	15.0	CP	CP	-	-	-	-	-	-	-	-
2603 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	-	.75	14.0	14.0	CP	CP	.75	14.0	14.0	CP	CP	-	-	-
2004 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	-	.75	19.0	22.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2804 YZ A	6	.625	4.63	19.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	6	6	5	10	1.25	26.0	16.0	.625	.375	1.25	34.0	15.0	.313	.313	1.25	19.0	15.0	CP	.563	1.25	21.0	17.0	CP	CP	-	.25	-
2104 YZ A	8	.625	7.63	19.00	.375		5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	12	1.25	20.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.75	27.0	18.0	CP	CP	.25	-	-
2204 YZ A	3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	1.00	18.0	15.0	.500	.438	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2304 YZ A	6	.500	4.63	19.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	-	-	-	-	-	-	-	-	-	.50	22.0	19.0	.375	.313	-	-	-	-	-	-	-	-
2404 YZ A	5	.500	4.63	16.00	.313		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	5	10	-	.75	20.0	11.0	.375	.250	.75	25.0	15.0	CP	.438	.50	22.0	19.0	.438	.375	-	-	-	-	-	-	-	-
2504 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	-	-	-	-	-	.75	17.0	15.0	.500	.438	-	-	-	-	-	-	-	.50	16.0	13.0	.313	.250	-	-	-
2604 YZ A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	3	-	-	-	-	-	-	-	-	-	-	.50	25.0	20.0	.438	.375	.50	14.0	11.0	.188	.188	-	-	-
2005 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	5	5	-	-	.75	17.0	23.0	CP	.250	.75	17.0	23.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2805 YZ A	5	.625	4.63	16.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	10	.75	23.0	12.0	.375	.250	-	-	-	-	-	-	.75	14.0	14.0	CP	CP	-	-	-	-	-	-	-	-
2105 YZ A	4	.625	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	6	.75	14.0	16.0	.500	.438	-	-	-	-	-	-	.75	17.0	13.0	CP	.438	.75	23.0	16.0	CP	.375	-	-	-
2205 YZ A	6	.625	4.63	19.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	6	10	10	-	-	-	-	-	1.00	19.0	17.0	CP	.375	1.00	17.0	25.0	CP	CP	1.25	17.0	25.0	CP	CP	.25	.25	-
2305 YZ A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	10	20	6	-	-	-	-	-	1.00	19.0	17.0	CP	.438	1.00	24.0	26.0	CP	CP	1.00	15.0	18.0	.500	.500	.25	-	-
2405 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2505 YZ A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	12	10	4	-	-	-	-	-	.75	21.0	24.0	.438	.438	.75	27.0	21.0	.500	.375	.75	11.0	12.0	CP	.500	.25	-	-
2605 YZ A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2006 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	5	4	-	-	.75	17.0	21.0	CP	.250	.75	15.0	19.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2806 YZ A	3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	6	4	6	5	.75	36.0	15.0	.500	.313	.75	30.0	12.0	.438	.250	.75	22.0	17.0	CP	CP	.75	19.0	15.0	CP	CP	.25	-	-
2106 YZ A	5	.625	4.63	16.00	.313		6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	10	5	4	-	-	-	-	-	1.00	20.0	21.0	CP	CP	.75	24.0	15.0	CP	.500	.50	20.0	13.0	.500	.313	-	.25	-
2206 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	20	12	4	-	-	-	-	-	1.00	29.0	24.0	CP	CP	1.00	22.0	23.0	CP	CP	.50	15.0	16.0	.250	.313	.25	.25	.25
2306 YZ A	5	.500	4.63	16.00	.250		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2406 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	7	.500	7.63	19.00	.313		-	-	-	-	-	-	4	10	-	-	.75	25.0	13.0	.313	.250	.75	35.0	21.0	.438	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2506 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	5	-	-	-	-	-	-	-	-	-	-	-	.50	20.0	15.0	.500	.375	-	-	-	-	-	-	-	-
2007 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	6	-	-	.75	21.0	24.0	CP	.313	.75	20.0	23.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	-
2807 YZ A	7	.500	7.63	19.00	.375		7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	5	5	5	5	.75	33.0	14.0	.500	.313	.75	33.0	14.0	CP	.375	.75	19.0	15.0	CP	CP	.75	20.0	15.0	CP	CP	-	-	-
2107 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	12	5	6	1.00	21.0	22.0	CP	CP	1.00	22.0	23.0	CP	CP	.75	23.0	15.0	CP	.438	.75	26.0	16.0	CP	.500	.25	.25	.50
2207 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2307 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	4	-	-	-	.50	17.0	14.0	.313	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2407 YZ A	3	.375	4.63	21.00	.188	CP	5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	5	5	10	-	1.00	25.0	14.0	.313	.313	1.00	26.0	15.0	.375	.313	1.00	22.0	19.0	CP	CP	-	-	-	-	-	.25	-	-
2507 YZ A	6	.625	4.63	19.00	.313		5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	-	-	-	-	-	-	-	-	-	1.00	18.0	15.0	CP	CP	.50	17.0	13.0	.438	.313	.25	-	-
2008 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	5	-	-	.75	19.0	22.0	CP	.313	.75	17.0	21.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2808 YZ A	6	.625	4.63	19.00	.313		7	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	4	5	5	12	.75	30.0	12.0	.375	.250	.75	33.0	14.0	.500	.313	.75	19.0	15.0	CP	CP	.75	23.0	19.0	CP	CP	-	-	-
2108 YZ A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	5	.75	16.0	13.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	.75	23.0	15.0	CP	.438	-	-	-
2208 YZ A	6	.625	4.63	19.00	.313		7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	10	1.00	22.0	19.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	1.00	21.0	23.0	CP	CP	.25	.25	-
2308 YZ A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	10	10	-	-	-	-	-	-	1.00	23.0	18.0	CP	CP	1.00	18.0	19.0	CP	CP	-	-	-	-	-	.25	-	-
2408 YZ A	4	.500	4.63	13.00	.250		7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	8	4	4	.50	23.0	13.0	.188	.188	1.00	25.0	15.0	CP	CP	.50	14.0	15.0	.375	.375	.50	13.0	14.0	.375	.375	-	-	-
2508 YZ A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	5	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	19.0	15.0	.438	.313	-	-	-
2009 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	5	-	-	-	-	-	-	-	.75	17.0	26.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2809 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	10	.75	13.0	26.0	.500	CP	-	-	-	-	-	-	-	-	-	-	-	.75	21.0	20.0	CP	.500	-	-	-
2109 YZ A	3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	4	1.50	16.0	42.0	.500	CP	-	-	-	-	-	-	-	-	-	-	-	.75	15.0	13.0	CP	.438	.50	-	-
2209 YZ A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	-	10	-	6	-	-	-	-	-	1.00	18.0	19.0	CP	CP	-	-	-	-	-	-	-	.75	13.0	27.0	.313	.375	-	.25	-
2309 YZ A	3	.375	4.63	21.00	.188	CP	6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	3	4	8	10	1.00	10.0	34.0	.313	CP	1.00	15.0	15.0	.500	.375	1.50	19.0	47.0	.625	CP	1.00	18.0	25.0	CP	CP	1.00	-	-
2409 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2509 YZ A	3	.375	4.63	21.00	.188	CP	5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	1.50	12.0	36.0	.313	CP	.50	15.0	16.0	.188	.188	-	-	-
2810 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2110 YZ A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	4	-	-	-	-	-	-	-	-	-	-	.75	17.0	24.0	CP	CP	.50	13.0	25.0	.250	.313	-	-	-
2210 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	8	-	4	1.50	17.0	28.0	CP	CP	1.50	16.0	28.0	CP	CP	-	-	-	-	-	1.50	14.0	37.0	.375	CP	.25	.50	-
2310 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2410 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	3	4	4	3	.50	10.0	19.0	.188	.500	.75	13.0	22.0	.438	CP	.50	11.0	31.0	.188	CP	.75	9.0	27.0	.250	CP	-	-	-
2510 YZ A	6	.500	4.63	19.00	.313		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2011 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	1.00	16.0	20.0	CP	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
2811 YZ A	7	.625	7.63	19.00	.375		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	5	6	14	-	.75	33.0	14.0	.500	.313	.75	17.0	24.0	CP	CP	.50	25.0	21.0	CP	CP	-	-	-	-	-	.25	-	-
2111 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	21.0	13.0	CP	.313	-	-	-	-	-	-	-	-
2211 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2311 YZ A	8	.625	7.63	19.00	.375		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	4	-	8	1.25	23.0	18.0	CP	CP	1.25	12.0	26.0	.313	CP	-	-	-	-	-	1.50	17.0	36.0	.688	CP	.25	.25	.25
2411 YZ A	7	.500	7.63	19.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	6	-	4	-	1.00	23.0	13.0	CP	.500	-	-	-	-	-	.50	13.0	13.0	.375	.375	-	-	-	-	-	-	-	-
2511 YZ A	5	.500	4.63	16.00	.313		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	3	-	-	-	-	-	-	-	-	-	-	.50	28.0	16.0	.313	.250	.50	10.0	22.0	.188	CP	-	-	-
2012 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	5	14	-	-	.75	17.0	21.0	CP	.250	.50	24.0	26.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
2812 YZ A	8	.625	7.63	19.00	.375		7	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	8	4	4	8	1.00	34.0	14.0	CP	.438	1.00	30.0	12.0	.313	.313	1.00	17.0	13.0	CP	.438	1.00	18.0	15.0	CP	CP	.25	.25	.25
2112 YZ A	3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	12	-	-	5	1.00	20.0	21.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.75	23.0	15.0	CP	.438	.25	-	.25
2212 YZ A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2312 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	4	4	-	-	.50	17.0	14.0	.313	.250	.50	16.0	13.0	.313	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2412 YZ A	5	.625	4.63	16.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	8	4	10	1.00	22.0	12.0	.313	.313	1.00	27.0	16.0	.313	.313	1.00	14.0	15.0	CP	CP	1.00	18.0	19.0	CP	CP	-	.25	-
2512 YZ A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	6	-	-	-	-	-	-	-	-	-	-	1.00	17.0	13.0	CP	CP	1.00	15.0	14.0	CP	CP	-	.25	-
2013 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	5	-	-	-	-	-	.75	15.0	19.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2813 YZ A	-	-	-	-	-	-	5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	19.0	15.0	CP	CP	-	-	-
2113 YZ A	7	.500	7.63	19.00	.375		8	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	8	1.00	19.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	22.0	15.0	CP	CP	.25	-	-
2213 YZ A	3	.375	4.63	21.00	.188	CP	8	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	14	-	3	12	1.25	26.0	23.0	CP	CP	-	-	-	-	-	1.00	11.0	13.0	.313	.313	1.00	20.0	22.0	CP	CP	.25	.25	-
2313 YZ A	3

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT	PL	OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEL	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWER	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEU	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWED	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
2319	YZ	A	7	.500	7.63	19.00	.375		6	.500	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-		3	3	8	12	1.25	11.0	14.0	.313	.313	1.25	9.0	23.0	.313	.625	1.25	15.0	26.0	.563	CP	1.25	17.0	31.0	.563	CP	.25	.25	-
2419	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	.50	12.0	22.0	.188	.188	-	-	-	-	-	-	-
2519	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-		-	-	-	-	-		-	-	4	6	-	-	-	-	-	-	-	-	-	-	1.25	15.0	18.0	.313	.313	1.25	13.0	33.0	.375	.625	-	.25	-
2020	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		10	6	-	-	1.00	18.0	29.0	CP	.313	1.00	13.0	39.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2120	YZ	A	7	.500	7.63	19.00	.375		6	.500	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-		-	6	10	4	-	-	-	-	-	1.25	14.0	18.0	CP	CP	1.25	18.0	23.0	CP	CP	1.50	14.0	33.0	.438	CP	.25	.25	-
2220	YZ	A	4	.500	4.63	13.00	.250		6	.625	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-		-	8	-	4	-	-	-	-	-	1.00	16.0	18.0	CP	CP	-	-	-	-	-	-	.75	12.0	28.0	.313	.438	-	.25	-
2320	YZ	A	8	.500	7.63	19.00	.375		7	.500	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-		8	4	-	5	1.25	30.0	14.0	CP	.563	.75	13.0	16.0	.250	.313	-	-	-	-	-	-	.75	15.0	25.0	.375	.500	-	-	.25
2420	YZ	A	6	.625	4.63	19.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		8	4	4	3	1.00	38.0	14.0	CP	.375	.50	15.0	14.0	.438	.375	.50	19.0	13.0	.438	.250	.50	10.0	19.0	.188	.188	-	-	-
2520	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2021	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		12	10	-	-	1.00	20.0	32.0	CP	.313	1.00	18.0	29.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
2121	YZ	A	7	.625	7.63	19.00	.375		4	.500	4.63	13.00	.313		-	-	-	-	-		-	-	-	-	-		4	-	8	10	.75	22.0	13.0	CP	.375	-	-	-	-	-	.75	16.0	21.0	.375	.438	.50	18.0	24.0	.375	.438	-	-	-
2221	YZ	A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	10	-	-	-	-	-	-	-	-	-	-	-	.75	20.0	17.0	CP	CP	-	-	-	-	-	-	-
2321	YZ	C	-	-	-	-	-		-	-	-	-	-		4	.500	4.63	13.00	.250		-	-	-	-	-		4	4	-	-	.50	25.0	12.0	.313	.188	.50	26.0	12.0	.250	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2421	YZ	A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-		7	-	8	8	1.25	42.0	16.0	.438	.313	-	-	-	-	-	1.25	20.0	15.0	CP	CP	1.25	20.0	15.0	CP	CP	.25	.25	-
2521	YZ	A	6	.500	4.63	19.00	.313		6	.500	4.63	19.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	4	8	-	-	-	-	-	-	-	-	-	-	1.25	19.0	12.0	CP	.563	1.00	24.0	15.0	CP	CP	-	-	-
2022	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		8	8	-	-	1.25	14.0	42.0	CP	CP	.75	16.0	27.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2122	YZ	A	7	.500	7.63	19.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		10	10	4	12	1.00	17.0	23.0	CP	CP	1.00	27.0	17.0	.500	.375	1.50	13.0	33.0	.438	CP	1.00	20.0	26.0	CP	CP	.25	.25	-
2222	YZ	A	5	.625	4.63	16.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-		-	-	-	-	-		4	-	4	4	.75	14.0	17.0	CP	CP	-	-	-	-	-	.75	12.0	28.0	.250	.313	.75	16.0	14.0	CP	CP	-	-	-
2322	YZ	A	7	.500	7.63	19.00	.313		5	.500	4.63	16.00	.313		-	-	-	-	-		-	-	-	-	-		4	8	5	-	.75	14.0	17.0	.250	.313	1.00	29.0	14.0	CP	.438	.75	15.0	25.0	.375	.500	-	-	-	-	-	-	-
2422	YZ	A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.250		-	-	-	-	-		-	-	-	-	-		4	4	4	4	.75	14.0	14.0	CP	.500	1.25	33.0	11.0	.500	.313	.50	13.0	23.0	.188	.188	.50	18.0	12.0	.438	.250	-	-	-
2522	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	-	-	7	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	31.0	17.0	CP	.375	.25	.50	-
2023	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		10	4	-	-	1.25	17.0	44.0	CP	.625	1.25	12.0	49.0	.438	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2123	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		6	4	8	8	1.25	18.0	23.0	.563	.500	1.25	13.0	28.0	.438	CP	1.50	15.0	37.0	.688	CP	1.50	20.0	50.0	.625	CP	.25	.75	-
2223	YZ	A	6	.625	4.63	19.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	5	10	10	.75	14.0	16.0	CP	.500	.75	15.0	27.0	CP	CP	.75	17.0	32.0	.500	.500	1.25	16.0	39.0	.563	CP	-	-	-
2323	YZ	A	8	.500	7.63	19.00	.375		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	4	10	4	-	-	-	-	-	.75	14.0	24.0	.313	CP	1.00	18.0	26.0	CP	CP	1.25	13.0	29.0	.563	CP	.25	-	-
2423	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	13.0	29.0	.188	CP	-	-	-
2523	YZ	A	5	.500	4.63	16.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	-	6	4	-	-	-	-	-	-	-	-	-	-	1.00	16.0	17.0	CP	.438	.75	13.0	29.0	.250	.313	-	.25	-
2024	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		10	8	-	-	1.00	17.0	35.0	CP	.313	1.00	15.0	33.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
2124	YZ	A	8	.625	7.63	19.00	.375		7	.500	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-		5	10	10	10	.75	18.0	19.0	.375	.375	.75	20.0	21.0	CP	CP	1.00	16.0	34.0	.500	CP	.75	16.0	34.0	.375	.500	.25	.25	-
2224	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	10	12	6	.75	18.0	15.0	.438	.375	1.00	23.0	19.0	CP	CP	1.00	22.0	23.0	CP	CP	.50	20.0	21.0	.313	.313	.25	.25	.25
2324	YZ	A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	4	4	-	-	-	-	-	-	-	-	-	-	.75	14.0	15.0	CP	CP	.50	13.0	14.0	.438	.438	-	-	-
2424	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		4	6	-	-	1.00	23.0	13.0	.375	.313	1.00	23.0	13.0	CP	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2524	YZ	A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-		-	-	-	-	-		-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	17.0	13.0	.438	.375	-	-	-	-	-	-	-
2025	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		8	10	-	-	1.00	15.0	31.0	CP	.313	1.00	17.0	35.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
2125	YZ	A	3	.375	4.63	21.00	.188	CP	8	.625	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-		4	12	12	10	.75	14.0	14.0	.500	.438	1.00	22.0	23.0	CP	CP	1.25	18.0	30.0	CP	CP	.75	16.0	34.0	.375	.500	.25	.25	.25
2225	YZ	A	6	.625	4.63	19.00	.313		5	.625	4.63	16.00	.313		-	-	-	-	-		-	-	-	-	-		4	4	10	5	.50	18.0	15.0	.313	.250	.75	18.0	15.0	.500	.438	.75	18.0	23.0	CP	CP	.50	18.0	19.0	.375	.375	.25	-	-
2325	YZ	A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		4	-	4	4	.75	17.0	14.0	.313	.250	-	-	-	-	-	.75	17.0	14.0	.313	.250	.75	14.0	15.0	CP	CP	-	-	-
2425	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	4	4	-	-	-	-	-	-	.75	22.0	12.0	.313	.250	.75	14.0	15.0	CP	CP	-	-	-	-	-	-	-
2525	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	16.0	13.0	.500	.375	.50	16.0	13.0	.500	.375	-	-	-
2026	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		8	12	-	-	1.00	16.0	27.0	CP	.313	1.25	19.0	38.0	CP	.438	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
2126	YZ	A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.07	.375		-	-	-	-	-		-	-	-	-	-		4	10	14	8	.75	15.0	14.0	.500	.438	.75	20.0	17.0	CP	CP	1.25	21.0	27.0	CP	CP	.75	15.0	28.0	.438	CP	.25	.25	.25
2226	YZ	A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		4	4	4	4	.50	21.0	13.0	.375	.250	.50	17.0	14.0	.313	.250	.75	15.0	18.0	.313	.375	.75	16.0	17.0	.375	.375	-	-	-
2326	YZ	A	4	.500	4.63	13.00	.313		5	.500	4.63	16.00	.250		-	-	-	-	-		-	-	-	-	-		4	4	4	4	.50	21.0	13.0	.313	.250	.75	18.0	14.0	.438	.375	.50	16.0	13.0	.375	.250	.50	14.0	15.0	.438	.438	-	-	-
2426	YZ	A	4	.625	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		4	4	3	4	1.00	27.0	11.0	.500	.313	.50	23.0	13.0	.375	.250	.50	13.0	11.0	.375	.313	.50	14.0	14.0	.375	.375	-	-	-
2526	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2027	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	14	-	-	-	-	-	-	-	1.25	21.0	41.0	CP	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
2127	YZ	A	-	-	-	-	-		6	.500	4.63	19.07	.313		-	-	-	-	-		-	-	-	-	-		-	4	-	8	-	-	-	-	-	.75	14.0	16.0	CP	CP	-	-	-	-	-	-	.75	15.0	29.0	.500	CP	-	-	-
2227	YZ	A	-	-	-	-	-		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	4	-	4	-	-	-	-	-	.75	14.0	12.0	.438	.250	-	-	-	-	-	-	.75	14.0	23.0	.313	.375	-	-	-
2327	YZ	A	-	-	-	-	-		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	3	-	4	-	-	-	-	-	.50	12.0	10.0	.438	.250	-	-	-	-	-	-	.50	16.0	16.0	.375	.313	-	-	-
2427	YZ	A	-	-	-	-	-		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	16.0	16.0	.313	.250	-	-	-
2527	YZ	A	-	-	-	-	-		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	12.0	12.0	CP	CP	-	-	-
GRID 3																																																						
3301	YZ	B	2	.625	-	-	.438		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3401	YZ	B	3	1.375	-	-	.875		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		32	-	-	-	1.75	64.0	29.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	-	-
3501	YZ	B	2	2.000	-	-	1.375		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	36	-	-	-	-	-	-	-	-	-	-	-	1.25	55.0	31.0	CP	CP	-	-	-	-	-	-	-
3402	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		16	36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	-
3502	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	32	32	-	-	-	-	-	-	-	-	-	-	1.50	52.0	30.0	CP	CP	-	-	-	-	-	-	1.50
3303	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		8	-	-	-	1.00	21.0	17.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3403	YZ	A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-		-	-	-	-	-		12	32	9	-	1.75	43.0	18.0	.750	.750	1.75	52.0	30.0	CP	CP	1.75	48.0	35.0	CP	CP	-	-	-	-	-	.50	1.00	.25
3503	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		3	.500	4.63	10.00	.313		-	-	16	16	-	-	-	-	-	-	-	-	-	-	1.25	31.0	22.0	CP	CP	-	-	-	-	-	-	-
3304	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		4	9	-	-	1.75	18.0	13.0	.375	.375	1.75	40.0	35.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.50
3404	YZ	A	3	.375	4.63	21.00	.188	CP	11	.500	7.63	31.00	.313		-	-	-	-	-		-	-	-	-	-		8	16	32	8	1.00	24.0	15.0	.375	.313	1.25	46.0	22.0	CP	CP	1.25	43.0	30.0	CP	CP	1.00	22.0	16.0	CP	CP	.75	-	-
3504	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	8	12	-	-	-	-	-	-	-	-	-	-	1.25	20.0	15.0	.313	.313	1.25	26.0	19.0	CP	CP	-	-	-
3305	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		9	32	-	-	1.75	43.0	35.0	CP	CP	1.25	39.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	1.00	1.00
3405	YZ	A	7	.500	7.63	19.00	.375		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		10	8	10	4	.50	38.0	17.0	.188	.188	.50	35.0	15.0	.188	.188	.50	23.0	18.0	.313	.313	.50	19.0	14.0	.188	.188	-	.25	-
3505	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		3	.500	4.63	10.00	.250		-	-	8	8	-	-	-	-	-	-	-	-	-	-	.50	21.0	15.0	.438	.438	.50	21.0	15.0	.375	.375	-	-	-
3306	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		-	10	-	-	-	-	-	-	-	.50	24.0	18.0	.250	.250	-	-	-	-	-	-	-	-	-	-	-	-	-
3406	YZ	A	3	.375	4.63	29.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		28	8	-	9	1.75	50.0	28.0	CP	CP	1.75	26.0	15.0	.375	.375	-	-	-	-	-	-	1.75	50.0	35.0	CP	CP	.75	1.00	1.00
3506	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	20	10	-	-	-	-	-	-	-	-	-	-	1.50	33.0	24.0	CP	CP	.50	22.0	17.0	.188	.188	-	-	.50
3307	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3407	YZ	A	3	.375	4.63	32.00	.188	CP	13	.750	7.63	28.00	.438		-	-	-	-	-		-	-	-	-	-		36	20	-	-	1.50	55.0	31.0	CP	CP	1.50	41.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.25	-	-
3507	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	3	28	-	-	-	-	-	-	-	-	-	-	1.50	51.0	33.0	CP	CP	1.50	43.0	28.0	CP	CP	-	-	.75
3308	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		32	-	-	-	1.00	36.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3408	YZ	A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	32.00	.188	CP	-	-	-	-	-		-	-	-	-	-		14	3	3	-	1.00	39.0	20.0	.438	.375	1.50	57.0	33.0	CP	CP	1.50	49.0	29.0	CP	CP	-	-	-	-	-	.75	1.00	1.00
3508	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	20	36	-	-	-	-	-	-	-	-	-	-	1.50	33.0	24.0	CP	CP	1.25	51.0	31.0	CP	CP	-	-	1.00
3309	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		-	3	-	-	-	-	-	-	-	1.50	37.0	29.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3409	YZ	A	3	.375	4.63	32.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	20	3	32	-	-	-	-	-	1.50	38.0	24.0	CP	CP	2.50	34.0	62.0	CP	CP	1.00	48.0	30.0	CP	CP	1.50	.50	1.00
3509	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	3	14	-	-	-	-	-	-	-	-	-	-	1.75	31.0	45.0	CP	CP	.50	26.0	21.0	.500	.500	-	-	-
3310	YZ	A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-		-	-	-	-	-		3	3	-	-	2.00	31.0	56.0	CP	CP	2.50	34.0	61.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	1.00	1.00
3410	YZ	A	4	.500	4.63	13.00	.250		3	.375	4.63	32.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	3	-	-	-	-	-	-	-	1.75	31.0	45.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.25	-
3510	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		3	.500	4.63	10.00	.313		-	-	10	-	-	-	-	-	-	-	-	-	-	-	.50	16.0	39.0	.188	.188	-	-	-	-	-	-	-
3311	YZ	C	-	-	-	-	-		-	-	-	-	-		4	.500	7.63	10.00	.313		-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3411	YZ	A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		3	10	-	3	1.25	45.0	28.0	CP	CP	1.25	17.0	22.0	.313	.375	-	-	-	-	-	-	1.50	31.0	60.0	CP	CP	1.00	1.25	1.00
3511	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	40	-	-	-	-	-	-	-	-	-	-	-	1.75	51.0	33.0	CP	CP	-	-	-	-	-	-	-
3312	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		20	-	-	-	1.50	32.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	-
3412	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	40	32	-	1.00	21.0	11.0	.313	.313	1.75	67.0	33.0	CP	CP	1.00	45.0	30.0	CP	CP	-	-	-	-	-	.50	.25	.25
3512	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	4	3	-	-	-	-	-	-	-	-	-	-	1.00	16.0	13.0	.313	.313	1.25	41.0	28.0	CP	CP	-	-	.50
3313	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		-	32	-	-	-	-	-	-	-	1.00	39.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25	-
3413	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		16	4	-	20	1.50	33.0	22.0	CP	CP	1.50	24.0	12.0	.313	.313	-	-	-	-	-	-	1.50	39.0	24.0	CP	CP	.25	.50	.25
3513	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	16	4	-	-	-	-	-	-	-	-	-	-	1.25	27.0	23.0	CP	CP	.50	13.0	12.0	.375	.375	-	-	.25
3314	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3414	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		28	16	-	-	1.25

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEB	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWEB	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEB	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWEB	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWLU	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWLR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
3322 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	28	-	-	-	-	-	-	-	-	1.25	38.0	28.0	CP	CP	-	-	-	-	-	1.50	42.0	26.0	CP	CP	1.00	.75	.75
3422 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	24	1.50	41.0	29.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3522 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	16	16	-	-	-	-	-	-	-	-	-	-	-	-	1.25	35.0	22.0	CP	CP	1.00	35.0	22.0	CP	CP	-	-	.25
3323 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.25	43.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
3423 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	16	-	-	1.25	37.0	15.0	.313	.313	1.25	34.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	.25
3523 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	8	3	-	-	-	-	-	-	-	-	-	-	-	-	1.00	22.0	15.0	.313	.313	1.50	40.0	29.0	CP	CP	-	-	1.25
3324 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	6	-	-	-	1.75	42.0	35.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
3424 YZ A	14	.625	7.63	31.00	.438		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	12	8	8	36	1.25	56.0	18.0	.625	.563	1.25	25.0	15.0	.313	.313	1.25	22.0	16.0	CP	CP	1.25	46.0	32.0	CP	CP	-	.50	-
3524 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	24	8	-	-	-	-	-	-	-	-	-	-	-	-	.75	37.0	26.0	CP	CP	.50	20.0	15.0	.250	.250	-	-	-
3325 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	1.00	22.0	15.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3425 YZ A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	32	24	-	6	1.75	53.0	30.0	CP	CP	1.75	48.0	26.0	.500	.500	-	-	-	-	-	1.75	48.0	35.0	CP	CP	1.00	1.00	.50
3525 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	20	12	-	-	-	-	-	-	-	-	-	-	-	-	1.25	35.0	24.0	CP	CP	.75	26.0	19.0	CP	CP	-	-	-
3426 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	20	-	-	1.25	58.0	29.0	CP	CP	1.50	46.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00
3526 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	3	32	-	-	-	-	-	-	-	-	-	-	-	-	1.75	49.0	32.0	CP	CP	1.75	43.0	30.0	CP	CP	-	-	1.75
3327 YZ B	-	-	-	-	-	-	4	.625	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3427 YZ B	-	-	-	-	-	-	3	1.125	-	-	.688		-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	-	1.50	67.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3527 YZ B	-	-	-	-	-	-	2	1.125	-	-	.750		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	53.0	29.0	CP	CP	-	-	-
GRID 4																																																							
4301 YZ B	4	.625	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4401 YZ B	3	1.500	-	-	.938		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	-	1.25	63.0	28.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	-	-
4501 YZ B	2	1.750	-	-	1.188		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	52.0	29.0	CP	CP	-	-	-	-	-	-	-	-
4402 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	20	32	-	-	1.50	46.0	24.0	CP	CP	1.25	59.0	29.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25
4502 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	32	3	-	-	-	-	-	-	-	-	-	-	-	-	1.50	42.0	30.0	CP	CP	1.25	44.0	28.0	CP	CP	-	-	1.75
4303 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	6	-	-	-	1.00	20.0	14.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4403 YZ A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	32	40	-	1.75	40.0	16.0	.500	.500	1.75	53.0	30.0	CP	CP	1.75	48.0	35.0	CP	CP	-	-	-	-	-	1.00	1.00	1.00
4503 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	8	20	-	-	-	-	-	-	-	-	-	-	-	-	1.25	21.0	15.0	CP	CP	1.25	35.0	24.0	CP	CP	-	-	-
4304 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	40	-	-	-	-	-	-	-	-	1.75	42.0	35.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
4404 YZ A	3	.375	4.63	31.00	.188	CP	14	.625	7.63	31.00	.438		-	-	-	-	-	-	-	-	-	-	-	-	10	8	36	6	1.00	40.0	16.0	.313	.313	1.00	39.0	15.0	CP	CP	1.00	46.0	33.0	CP	CP	1.00	23.0	14.0	CP	CP	-	-	-
4504 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	10	10	-	-	-	-	-	-	-	-	-	-	-	-	.75	24.0	17.0	.313	.250	.75	24.0	17.0	CP	CP	-	-	-
4305 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	36	-	-	-	-	-	-	-	-	1.00	40.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
4405 YZ A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	24	10	-	-	1.00	47.0	26.0	CP	CP	.50	40.0	16.0	.250	.250	-	-	-	-	-	-	-	-	-	-	.50	-	-
4505 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	24	10	-	-	-	-	-	-	-	-	-	-	-	-	1.25	36.0	26.0	CP	CP	.50	24.0	17.0	.375	.313	-	-	-
4306 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4406 YZ A	3	.375	4.63	24.00	.188	CP	13	.750	7.63	31.00	.438		-	-	-	-	-	-	-	-	-	-	-	-	20	24	-	-	1.50	35.0	24.0	CP	CP	1.50	49.0	26.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4506 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	24	-	-	-	-	-	-	-	-	-	-	-	-	1.50	44.0	28.0	CP	CP	1.00	43.0	26.0	CP	CP	-	-	.25
4307 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	28	-	-	-	1.50	37.0	28.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4407 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	24.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	3	28	-	1.50	24.0	12.0	.313	.313	1.50	43.0	28.0	CP	CP	1.75	42.0	28.0	CP	CP	-	-	-	-	-	.75	1.00	-
4507 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	16	20	-	-	-	-	-	-	-	-	-	-	-	-	1.00	29.0	22.0	CP	CP	1.00	32.0	24.0	CP	CP	-	-	-
4308 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	28	-	-	1.25	41.0	30.0	CP	CP	1.25	39.0	28.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	1.50
4408 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	32	16	20	28	1.50	43.0	30.0	CP	CP	1.50	34.0	22.0	.688	.750	1.50	40.0	24.0	.688	.688	1.50	45.0	28.0	CP	CP	.75	.75	1.00
4508 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	4	-	-	-	-	-	-	-	-	-	-	-	-	1.25	53.0	32.0	CP	CP	.50	19.0	13.0	.250	.188	-	-	1.00
4309 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	20	-	-	-	-	-	-	-	-	.75	35.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-
4409 YZ A	3	.375	4.63	29.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	3	3	32	-	-	-	-	-	-	1.25	42.0	32.0	CP	CP	2.00	34.0	61.0	CP	CP	1.00	43.0	30.0	CP	CP	1.50	1.00	1.25
4509 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	32	-	-	-	-	-	-	-	-	-	-	-	-	1.75	31.0	52.0	CP	CP	1.25	47.0	30.0	CP	CP	-	-	.50
4310 YZ A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	3	-	-	2.50	33.0	61.0	CP	CP	2.50	34.0	60.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	1.00	1.00
4410 YZ A	10	.500	4.63	31.00	.313		3	.375	4.63	29.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	-	1.75	31.0	47.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.50	-
4510 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	22.0	34.0	CP	CP	-	-	-	-	-	-	-	-
4311 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	7.63	10.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4411 YZ A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	32.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	16	-	3	1.25	50.0	29.0	CP	CP	1.25	22.0	24.0	CP	CP	-	-	-	-	-	2.00	32.0	58.0	CP	CP	1.00	1.00	1.00
4511 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	-	-	1.50	47.0	30.0	CP	CP	-	-	-	-	-	-	-	-
4312 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4412 YZ A	3	.375	4.63	21.00	.188	CP	11	.625	7.63	31.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	16	32	-	-	1.50	29.0	22.0	CP	CP	1.50	55.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.25	-	-
4512 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	32	3	-	-	-	-	-	-	-	-	-	-	-	-	1.50	43.0	30.0	CP	CP	1.50	45.0	29.0	CP	CP	-	-	1.00
4313 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	10	-	-	-	.75	23.0	18.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4413 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	32	36	-	1.25	21.0	10.0	.313	.313	1.50	43.0	30.0	CP	CP	1.25	48.0	32.0	CP	CP	-	-	-	-	-	1.00	1.00	1.00
4513 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	16	-	-	-	-	-	-	-	-	-	-	-	-	1.25	15.0	10.0	.313	.313	1.25	30.0	22.0	CP	CP	-	-	-
4314 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	36	-	-	1.25	39.0	32.0	CP	CP	1.25	40.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00
4414 YZ A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	3	3	10	10	.75	22.0	10.0	.250	.250	.75	21.0	10.0	.250	.250	1.00	22.0	18.0	CP	CP	1.00	22.0	18.0	CP	CP	.25	.25	-
4514 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	3	3	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	11.0	.188	.188	.50	15.0	11.0	.188	.188	-	-	-
4315 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	10	-	-	-	-	-	-	-	-	1.00	23.0	18.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
4415 YZ A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	36	3	-	36	1.50	51.0	32.0	CP	CP	1.25	20.0	10.0	.313	.313	-	-	-	-	-	1.25	48.0	32.0	CP	CP	1.00	1.00	1.00
4515 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	16	3	-	-	-	-	-	-	-	-	-	-	-	-	1.25	30.0	22.0	CP	CP	.50	15.0	11.0	.188	.188	-	-	.25
4316 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4416 YZ A	3	.375	4.63	31.00	.188	CP	11	.750	7.63	25.00	.438		-	-	-	-	-	-	-	-	-	-	-	-	36	16	-	-	1.25	57.0	31.0	CP	CP	1.25	35.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
4516 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	36	-	-	-	-	-	-	-	-	-	-	-	-	1.50	51.0	32.0	CP	CP	1.50	48.0	32.0	CP	CP	-	-	1.50
4317 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4417 YZ A	3	.375	4.63	29.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	3	6	-	1.25	23.0	24.0	.313	.313	1.25	56.0	32.0	CP	CP	1.50	33.0	58.0	CP	CP	-	-	-	-	-	1.00	-	1.00
4517 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	54.0	31.0	CP	CP	-	-	-
4318 YZ A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	6	6	-	-	3.00	35.0	62.0	CP	CP	2.25	33.0	60.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.50	1.00	1.00
4418 YZ A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	29.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	12	-	-	-	1.00	20.0	21.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4518 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	19.0	39.0	.188	.188	-	-	-
4319 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4419 YZ A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	6	1.50	55.0	33.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	3.00	35.0	62.0	CP	CP	.75	1.50	1.25
4519 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	36	12	-	-	-	-	-	-	-	-	-	-	-	-	1.75	52.0	31.0	CP	CP	.75	18.0	32.0	CP	CP	-	-	-
4320 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4420 YZ A	3	.375	4.63	29.00	.188	CP	15	.875	7.63	31.00	.500		-	-	-	-	-	-	-	-	-	-	-	-	3	36	-	-	1.75	46.0	28.0	CP	CP	1.75	54.0	31.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.50	-	.25
4520 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	9	3	-	-	-	-	-	-	-	-	-	-	-	-	1.75	50.0	34.0	CP	CP	1.50	50.0	33.0	CP	CP	-	-	1.50
4321 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	10	-	-	-	.75	24.0	18.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4421 YZ A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	29.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	9	40	-	1.75	27.0	14.0	.438	.375	1.75	56.0	34.0	CP	CP	1.75	55.0	34.0	CP	CP	-	-	-	-	-	1.00	1.00	1.00
4521 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	46.0	28.0	CP	CP	-	-	.50
4322 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	4	40	-	-	1.75	19.0	12.0	.375	.375	1.75	42.0	34.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50
4422 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	16	-	32	10	1.25	31.0	22.0	CP	CP	-	-	-	-	-	1.50	45.0	30.0	CP	CP	1.00	23.0	18.0	CP	CP	1.00	-	.25
4522 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	16	4	-	-	-	-	-	-	-	-	-	-	-	-	.75	36.0	22.0	.438	.375	.75	20.0	14.0	.438	.375	-	-	-
4323 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	1.50	41.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.25
4423 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	16	-	4	.75	37.0	17.0	.438	.375	.75	33.0	22.0	.438	.438	-	-	-	-	-	-	-	-	-	-	-	.25	-
4523 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	3	.500	4.63	10.00	.250		-	-	16	16	-	-	-	-	-	-	-	-	-	-	-	-	1.25	30.0	22.0	CP	CP	1.25	30.0	22.0	CP	CP	-	-	-
4324 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4424 YZ A	5	.500	4.63	16.00	.250		8	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	8	16	-	-	1.25	33.0	15.0	.313	.313	1.25	33.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4524 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	24	10	-	-	-	-	-	-	-	-	-	-	-	-	1.25	41.0	26.0	CP	CP	.75	22.0	17.0	CP	CP	-	-	-
4325 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.50	41.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4425 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	24	20	-	1.00	22.0	10.0	.313	.313	1.50	37.0	26.0	CP	CP	1.00	39.0	24.0	CP	CP	.25	.75	-
4525 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	10	8	-	-	-	-	-	-	-	-	-	-	-	-	1.00	21.0	17.0	CP	CP	.50	19.0	16.0	.500	.500	-	-	-
4326 YZ C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	20	-	-	-	-	-	-	-	-	1.00	32.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.25
4426 YZ A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	32	10	3	32	1.50	47.0	29.0	.750	CP	1.50	37.0	17.0	.750	.688	1.50	40.0	29.0	CP	CP	1.50	46.0	30.0	CP	CP	1.25	.75	1.50
4526 YZ C	-	-	-

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT	PL	OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWER	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWER	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWER	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWER	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
5404	YZ	A	3	.375	4.63	21.00	.188	CP	13	.500	7.63	31.00	.438		-	-	-	-	-		-	-	-	-	-		4	10	36	8	1.00	20.0	11.0	.375	.313	1.00	54.0	16.0	CP	.438	1.25	45.0	32.0	CP	CP	1.00	22.0	16.0	CP	CP	.50	-	-
5504	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	.50	18.0	14.0	.188	.188	-	-	-	-	-	-	-
5305	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00
5405	YZ	A	3	.375	4.63	21.00	.188	CP	8	.625	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-		16	4	-	4	.75	33.0	22.0	.438	.438	1.25	43.0	32.0	CP	CP	-	-	-	-	-	.50	18.0	14.0	.188	.188	.25	-	-
5505	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	4	-	-	-	-	-	.50	32.0	13.0	.188	.188	1.25	47.0	28.0	CP	CP	.75	16.0	12.0	.438	.375	-	-	.50
5306	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.75	41.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
5406	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	3	20	-	1.25	28.0	11.0	.375	.313	1.25	40.0	28.0	CP	CP	1.50	38.0	24.0	CP	CP	-	-	-	-	-	.50	.50	-
5506	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.313		-	-	4	16	-	-	-	-	-	-	-	-	-	-	.75	16.0	13.0	.250	.250	.75	34.0	22.0	.438	.375	-	-	-
5307	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
5407	YZ	A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		32	4	-	32	1.75	48.0	30.0	CP	CP	1.25	35.0	24.0	CP	CP	-	-	-	-	-	1.75	44.0	30.0	CP	CP	1.00	.75	1.00
5507	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	3	4	-	-	-	-	-	1.75	23.0	12.0	.375	.375	1.25	48.0	32.0	CP	CP	1.00	13.0	12.0	CP	CP	-	-	.50
5308	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.00	43.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
5408	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-		18	3	3	-	1.00	48.0	24.0	.500	.500	1.25	51.0	32.0	CP	CP	1.25	44.0	29.0	CP	CP	-	-	-	-	-	1.00	1.00	1.00
5508	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	24	32	-	-	-	-	-	-	-	-	-	-	-	1.50	41.0	26.0	CP	CP	1.50	45.0	30.0	CP	CP	-	-	1.50
5309	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
5409	YZ	A	3	.375	4.63	29.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	24	3	36	-	-	-	-	-	1.25	38.0	29.0	CP	CP	2.00	34.0	62.0	CP	CP	1.00	45.0	32.0	CP	CP	1.50	.75	1.00
5509	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	24	18	-	-	-	-	-	1.75	35.0	26.0	CP	CP	1.50	26.0	38.0	CP	CP	.75	30.0	25.0	CP	CP	-	-	-
5310	YZ	A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	3	-	-	2.00	31.0	54.0	CP	CP	2.50	34.0	59.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	1.00	1.00
5410	YZ	A	4	.500	4.63	13.00	.250		3	.375	4.63	29.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	24	-	-	-	-	-	-	-	1.50	26.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.25	-
5510	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	12.0	35.0	.188	.188	-	-	-	-	-	-	-	-
5311	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
5411	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		24	4	-	3	1.75	37.0	26.0	CP	CP	1.25	13.0	17.0	.313	.313	-	-	-	-	-	1.50	31.0	49.0	CP	CP	1.00	1.25	.25
5511	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	36	-	-	-	-	-	-	-	-	-	-	-	1.50	47.0	32.0	CP	CP	-	-	-	-	-	-	-	-
5312	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	12	-	-	-	1.25	26.0	19.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
5412	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	36	18	-	1.00	32.0	13.0	.313	.313	1.50	57.0	32.0	CP	CP	1.00	32.0	26.0	CP	CP	-	-	-	-	-	.50	.25	.50
5512	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	7	24	-	-	-	-	-	-	-	-	-	-	1.50	23.0	19.0	.625	.688	1.75	38.0	26.0	CP	CP	-	.25	1.00
5313	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	8	18	-	-	1.25	22.0	15.0	CP	CP	1.00	33.0	25.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	1.00
5413	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		8	7	16	12	1.25	35.0	15.0	.563	.500	1.25	39.0	18.0	.438	.438	1.25	29.0	24.0	CP	CP	1.25	25.0	20.0	CP	CP	.75	.50	1.00
5513	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.375		-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	16.0	14.0	.375	.375	.50	16.0	14.0	.375	.375	-	-	-
5314	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	18	16	-	-	1.25	34.0	25.0	CP	CP	1.25	31.0	23.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	1.00
5414	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	4	10	8	1.25	31.0	12.0	.313	.313	1.25	32.0	13.0	.313	.313	1.25	22.0	18.0	CP	CP	1.25	20.0	16.0	CP	CP	.50	.25	-
5514	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	10	8	-	-	-	-	-	-	-	-	-	-	1.00	21.0	17.0	CP	CP	1.00	18.0	16.0	CP	CP	-	-	-
5315	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	10	-	-	1.25	41.0	32.0	CP	CP	1.25	25.0	17.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	1.00
5415	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		18	10	16	18	1.25	47.0	24.0	.438	.438	1.25	37.0	17.0	.625	.563	1.50	30.0	22.0	CP	CP	1.25	32.0	26.0	CP	CP	.75	.75	1.00
5515	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.313		-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	17.0	13.0	.250	.250	.50	17.0	13.0	.500	.438	-	-	-
5316	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	16	-	-	1.50	30.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
5416	YZ	A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		40	4	-	36	1.75	54.0	33.0	CP	CP	1.00	23.0	12.0	.313	.313	-	-	-	-	-	1.00	48.0	32.0	CP	CP	.75	.75	1.00
5516	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	18	-	-	-	-	-	-	-	-	-	-	1.25	48.0	32.0	CP	CP	.75	30.0	25.0	CP	CP	-	-	1.00
5317	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
5417	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	3	3	-	1.25	13.0	18.0	.313	.313	1.25	63.0	32.0	CP	CP	1.75	31.0	51.0	CP	CP	-	-	-	-	-	1.50	1.00	.50
5517	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	40	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.75	54.0	33.0	CP	CP	-	-	-
5318	YZ	A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	3	-	-	2.50	34.0	59.0	CP	CP	2.25	33.0	58.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	1.00	1.00
5418	YZ	A	3	.375	4.63	26.00	.188	CP	5	.500	4.63	16.00	.250		-	-	-	-	-		-	-	-	-	-		28	-	-	-	1.50	27.0	37.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	-	-
5518	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	12.0	31.0	.188	.188	-	-	-
5319	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	3	-	-	-	1.50	38.0	29.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
5419	YZ	A	3	.375	4.63	24.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-		24	-	18	3	1.75	38.0	26.0	CP	CP	-	-	-	-	-	1.25	31.0	26.0	CP	CP	2.00	34.0	53.0	CP	CP	.75	1.25	.75
5519	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	18	28	-	-	-	-	-	-	-	-	-	-	1.50	32.0	25.0	CP	CP	1.50	27.0	39.0	CP	CP	-	-	-
5320	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	1.00	33.0	26.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
5420	YZ	A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	24.00	.188	CP	-	-	-	-	-		-	-	-	-	-		3	18	-	3	1.75	51.0	32.0	CP	CP	1.25	52.0	24.0	CP	CP	-	-	-	-	-	1.50	47.0	29.0	CP	CP	1.00	1.25	1.00
5520	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	40	24	-	-	-	-	-	-	-	-	-	-	1.75	50.0	33.0	CP	CP	1.50	41.0	26.0	CP	CP	-	-	1.50
5321	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	10	-	-	-	1.00	24.0	17.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
5421	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-		8	40	36	-	1.75	27.0	15.0	.625	.563	1.75	67.0	33.0	CP	CP	1.75	48.0	32.0	CP	CP	-	-	-	-	-	1.00	1.00	1.00
5521	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	12	3	-	-	-	-	-	-	-	-	-	-	1.00	23.0	19.0	CP	CP	1.50	50.0	32.0	CP	CP	-	-	1.25
5322	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	4	36	-	-	1.75	19.0	13.0	.375	.375	1.75	41.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50
5422	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		20	12	32	10	1.50	36.0	24.0	CP	CP	1.00	38.0	18.0	.438	.375	1.25	46.0	30.0	CP	CP	1.00	22.0	18.0	CP	CP	1.00	.25	.50
5522	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	10	8	-	-	-	-	-	-	-	-	-	-	.75	21.0	17.0	CP	CP	.75	18.0	15.0	CP	CP	-	-	-
5323	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	1.25	41.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	1.00
5423	YZ	A	7	.500	7.63	19.00	.375		8	.625	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-		8	10	-	4	.50	35.0	15.0	.188	.188	.75	37.0	17.0	.375	.313	-	-	-	-	-	.50	19.0	14.0	.188	.188	-	.25	-
5523	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.313		-	-	16	20	-	-	-	-	-	-	-	-	-	-	1.25	31.0	22.0	CP	CP	1.25	33.0	24.0	CP	CP	-	-	-
5324	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	12	-	-	-	2.00	43.0	37.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
5424	YZ	A	10	.625	4.63	31.00	.313		6	.625	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-		20	16	10	-	1.50	47.0	24.0	CP	CP	1.25	29.0	22.0	CP	CP	1.25	24.0	18.0	CP	CP	-	-	-	-	-	-	-	-
5524	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	14	8	-	-	-	-	-	-	-	-	-	-	1.00	27.0	21.0	CP	CP	.50	18.0	15.0	.188	.188	-	-	-
5325	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	10	-	-	-	-	-	-	-	1.00	23.0	18.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
5425	YZ	A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-		-	-	-	-	-		36	14	-	12	2.00	53.0	31.0	.938	.875	2.00	44.0	20.0	.625	.563	-	-	-	-	-	2.00	60.0	37.0	CP	CP	1.00	1.00	1.00
5525	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	20	20	-	-	-	-	-	-	-	-	-	-	1.25	40.0	24.0	CP	CP	1.25	39.0	24.0	CP	CP	-	-	-
5426	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	16	20	-	-	1.50	53.0	21.0	CP	CP	1.50	46.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	1.00
5526	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	3	36	-	-	-	-	-	-	-	-	-	-	1.50	43.0	28.0	CP	CP	1.25	54.0	31.0	CP	CP	-	-	1.50
5327	YZ	B	-	-	-	-	-	-	2	.750	-	-	.500		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
5427	YZ	B	-	-	-	-	-	-	3	1.000	-	-	.625		-	-	-	-	-		-	-	-	-	-		-	3	-	-	-	-	-	-	-	1.50	62.0	28.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
5527	YZ	B	-	-	-	-	-	-	2	1.000	-	-	.625		-	-	-	-	-		-	-	-	-	-		-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	40.0	22.0	CP	CP	-	-	-
GRID 6																																																						
6301	YZ	B	4	.500	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6401	YZ	B	3	.750	-	-	.500		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		16	-	-	-	1.50	57.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6501	YZ	B	13	.750	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6601	YZ	B	3	1.000	-	-	.625		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		16	-	-	-	1.50	38.0	22.0	CP	CP	-	-	-	-	-	.75	53.0	20.0	CP	.500	-	-	-	-	-	-	-	-
6701	YZ	B	4	.625	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		-	-	16	-	-	-	-	-	-	-	-	-	-	-	.75	31.0	27.0	.500	CP	-	-	-	-	-	-	-	-
6402	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	12.00	.188	CP	-	-	-	-	-		10	14	-	-	1.00	44.0	16.0	CP	CP	.75	54.0	19.0	.500	.500	-	-	-	-	-	-	-	-	-	-	-	-	.50
6502	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	12.00	.188	CP	-	-	14	16	-	-	-	-	-	-	-	-	-	-	1.25	43.0	20.0	CP	CP	1.50	52.0	22.0	CP	CP	-	-	.75
6602	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		10	16	-	-	1.25	33.0	18.0	.625	CP	.75	45.0	24.0	.375	.375	-	-	-	-	-	-	-	-	-	-	-	-	.50
6702	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	36	16	-	-	-	-	-	-	-	-	-	-	1.50	32.0	35.0	.688	.750	1.50	25.0	22.0	CP	CP	-	-	1.25
6303	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		4	-	-	-	.50	18.0	15.0	.313	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6403	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	14	36	-	1.00	25.0	12.0	.375	.313	1.00	51.0	20.0	CP	CP	-	-	-	-	-	1.00	48.0	32.0	CP	CP	1.00	.25	-
6503	YZ	A	3	.375	4.63	31.00	.188	CP	13	.500	7.63	31.00	.438		-	-	-	-	-		-	-	-	-	-		10	-	4	10	1.00	29.0	18.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6603	YZ	A	3	.375	4.63	24.00	.188	CP	3	.375	4.63	29.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	36	16	-	-	-	-	-	-	1.00	45.0	33.0	CP	CP	-	-	-	-	-	1.00	31.0	13.0	.313	.313	1.00	37.0	17.0	CP	CP	-	-	-
6703	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	10	-	-	-	-	-	-	-	-	-	-	1.00	28.0	26.0	CP	CP	-	-	-	-	-	.25	.25	-
																																																	1.25	19.0	22.0	CP	CP	-	-	-
6304	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		4	36	-	-	1.00	19.0	14.0	.313	.313	1.00	39.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50
6404	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		4	4	36	4	.50	27.0	13.0	.250	.250	.50	30.0	12.0	.188	.188	1.00	49.0	32.0	CP	CP	.50	19.0	14.0	.375	.438	.50	-	.50
6504	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	16	-	4	-	-	-	-	-	1.00	36.0	25.0	CP	CP	-	-	-	-	-	-	.50	36.0	13.0	.250	.250	-	-	-
6604	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	24.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	-	4	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	17.0	19.0	CP	CP	-	-	-
6704	YZ	C	-	-	-	-	-		-	-	-	-	-		-	-	-	-	-		3	.500	4.63	10.00	.250		-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	17.0	.188	.188	-	-	-	-	-	-	-	-
6305	YZ	C	-	-	-	-	-		-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	-	-		32	36	-	-	1.75	37.0	30.0	CP	CP	1.00	39.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.50	1.00	1.00
6405	YZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-		3	-	4	4	.50	22.0	10.0	.188	.188	-	-	-	-	-	.50	19.0	14.0	.250	.250	.50	19.0	14.0	.313	.313	-	-	-
6505	YZ	A	3	.375	4.63	24.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-		-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	26.0	13.0	.313	.250	.50	22.

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT	PL	OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEB	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWEB	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEB	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWEB	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
6309	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	20	-	-	-	-	-	-	-	-	1.00	32.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
6409	YZ	A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	12	3	10	-	-	-	-	-	1.25	41.0	18.0	CP	CP	1.75	33.0	61.0	CP	CP	.75	23.0	18.0	CP	CP	1.25	-	.50
6509	YZ	A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	18	-	-	-	-	-	-	-	-	-	-	-	1.25	14.0	17.0	CP	CP	.75	43.0	24.0	CP	CP	-	.25	-
6609	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	29.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	20	3	-	1.25	12.0	23.0	.313	.438	1.25	36.0	24.0	CP	CP	1.50	30.0	56.0	CP	CP	-	-	-	-	-	.50	-	1.00
6709	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.313	-	-	-	14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	22.0	27.0	.375	.375	-	-	-
6310	YZ	A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	3	-	-	2.00	32.0	56.0	CP	CP	2.25	33.0	58.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	1.00	1.00
6410	YZ	A	5	.500	4.63	16.00	.313	-	8	.500	4.63	25.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	4	8	-	-	1.00	13.0	21.0	.313	CP	1.00	16.0	16.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.25	-
6510	YZ	A	3	.375	4.63	26.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	1.50	36.0	61.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
6610	YZ	A	3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	10	-	1.00	14.0	24.0	CP	CP	-	-	-	-	-	.75	14.0	43.0	.313	CP	-	-	-	-	-	.25	-	-
6710	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	11.0	31.0	.188	CP	-	-	-
6311	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6411	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	16	-	-	3	1.50	33.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.50	32.0	49.0	CP	CP	.50	1.00	.50
6511	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	10	32	4	1.00	20.0	19.0	.500	CP	1.00	15.0	29.0	.500	CP	1.00	41.0	30.0	CP	CP	.50	14.0	15.0	.250	.313	.75	-	-
6611	YZ	A	3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	10	-	.75	20.0	16.0	.438	.500	-	-	-	-	-	.75	17.0	19.0	.375	CP	-	-	-	-	-	.25	-	-
6711	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	14	8	-	-	-	-	-	-	-	-	-	-	-	1.00	23.0	27.0	.375	.438	1.50	13.0	38.0	.500	CP	-	-	-
6312	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	10	-	-	-	1.25	24.0	18.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	-
6412	YZ	A	3	.375	4.63	21.00	.188	CP	9	.625	7.63	19.00	.438	-	-	-	-	-	-	-	-	-	-	-	-	-	4	32	18	-	.75	33.0	13.0	.250	.250	1.00	44.0	30.0	CP	CP	.75	32.0	26.0	CP	CP	-	-	-	-	-	.25	-	.50
6512	YZ	A	6	.625	4.63	19.00	.313	-	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	10	10	16	-	-	-	-	-	1.25	22.0	19.0	.375	.438	1.25	26.0	17.0	.563	.563	1.50	34.0	22.0	CP	CP	-	.50	.75
6612	YZ	A	5	.625	4.63	16.00	.313	-	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	14	-	8	1.00	21.0	16.0	.438	.500	1.00	27.0	23.0	.375	.438	-	-	-	-	-	1.00	15.0	17.0	CP	CP	-	.25	-
6712	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	8	8	-	-	-	-	-	-	-	-	-	-	-	.75	22.0	27.0	.438	.500	.75	17.0	21.0	.438	.500	-	-	.25
6313	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	8	18	-	-	1.00	22.0	16.0	CP	CP	.75	33.0	26.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50	-
6413	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	10	16	10	1.25	39.0	16.0	.625	.563	1.25	38.0	16.0	.438	.375	1.25	30.0	24.0	CP	CP	1.25	23.0	18.0	CP	CP	.75	.50	1.00
6513	YZ	A	6	.500	4.63	19.00	.250	-	6	.500	4.63	19.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	22.0	13.0	.375	.313	-	-	-
6613	YZ	A	6	.500	4.63	19.00	.313	-	6	.500	4.63	19.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	.50	27.0	23.0	.438	.438	-	-	-	-	-	-	-	-	-	-	-	-	-
6713	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	5	8	-	-	-	-	-	-	-	-	-	-	-	.75	16.0	20.0	.438	.438	.75	17.0	21.0	.500	.500	-	-	.25
6314	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	18	16	-	-	1.25	32.0	26.0	CP	CP	1.25	30.0	24.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	1.00	-
6414	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	-	8	8	.75	23.0	13.0	.250	.250	-	-	-	-	-	1.00	20.0	16.0	CP	CP	1.00	20.0	16.0	CP	CP	.25	.25	-
6514	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	10	-	-	-	-	-	-	-	-	-	-	-	1.25	23.0	15.0	CP	CP	1.00	26.0	17.0	CP	CP	-	-	-
6614	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	5	-	-	.50	19.0	13.0	.250	.313	.75	21.0	16.0	.375	.438	-	-	-	-	-	-	-	-	-	-	-	-	-
6714	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6315	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	16	8	-	-	1.00	30.0	24.0	CP	CP	1.00	21.0	16.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50	-
6415	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	8	12	18	1.00	27.0	13.0	.438	.375	1.25	36.0	15.0	.625	.563	1.00	25.0	20.0	CP	CP	1.00	32.0	26.0	CP	CP	.50	.50	.75
6515	YZ	A	5	.500	4.63	16.00	.313	-	5	.500	4.63	16.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	-	.50	22.0	13.0	.313	.250	.50	23.0	13.0	.188	.188	-	-	-
6615	YZ	A	3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	12	-	-	-	.75	24.0	20.0	.500	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6715	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.375	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	-	.75	14.0	18.0	.313	.375	.50	13.0	17.0	.250	.313	-	-	-
6316	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	12	-	-	-	-	-	-	-	1.00	25.0	20.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25	-
6416	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	36	4	-	16	1.25	47.0	31.0	CP	CP	1.00	33.0	13.0	.313	.313	-	-	-	-	-	1.00	29.0	24.0	CP	CP	.25	.25	.25
6516	YZ	A	3	.375	4.63	24.00	.188	CP	4	.500	4.63	13.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	24	4	-	-	-	-	-	-	-	-	-	-	-	1.50	42.0	26.0	CP	CP	.75	21.0	13.0	.500	.438	.25	-	.50
6616	YZ	A	7	.500	7.63	19.00	.313	-	4	.500	4.63	13.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	10	4	-	-	1.00	22.0	18.0	.500	CP	.75	18.0	14.0	.313	.375	-	-	-	-	-	-	-	-	-	-	-	-	-
6716	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	10	12	-	-	-	-	-	-	-	-	-	-	-	1.00	18.0	23.0	CP	CP	.75	20.0	25.0	.500	CP	.25	.25	.25
6317	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6417	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	24	3	-	1.25	12.0	11.0	.313	.438	1.50	39.0	26.0	CP	CP	1.25	30.0	45.0	CP	CP	-	-	-	-	-	.75	1.00	1.00
6517	YZ	A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	24.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	46.0	31.0	CP	CP	-	.75	-
6617	YZ	A	3	.375	4.63	21.00	.188	CP	8	.500	7.63	19.00	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	3	10	16	-	1.00	10.0	19.0	.313	.313	1.00	22.0	18.0	CP	CP	1.25	22.0	43.0	CP	CP	-	-	-	-	-	.25	.25	-
6717	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	4.63	10.00	.375	-	-	-	-	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	18.0	22.0	CP	CP	-	-	-
6318	YZ	A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	3	-	-	2.25	33.0	58.0	CP	CP	2.00	30.0	54.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	1.00	1.00
6418	YZ	A	3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	20	-	-	-	1.25	24.0	34.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	-	-
6518	YZ	A	3	.375	4.63	31.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	16	16	-	4	1.50	22.0	45.0	.688	CP	1.50	22.0	45.0	.688	CP	-	-	-	-	-	.50	13.0	15.0	.188	.313	-	-	-
6618	YZ	A	4	.500	4.63	13.00	.250	-	4	.500	4.63	13.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6718	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	-	-	-	4	3	-	-	-	-	-	-	-	-	-	-	-	.50	13.0	29.0	.188	CP	.50	9.0	27.0	.188	.250	-	-	-
6319	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	16	-	-	-	1.00	30.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6419	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	-	14	3	1.25	40.0	28.0	CP	CP	-	-	-	-	-	1.25	26.0	22.0	.563	CP	1.75	33.0	57.0	CP	CP	1.00	1.25	1.00
6519	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	31.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	18	20	-	-	-	-	-	-	-	-	-	-	-	1.25	31.0	24.0	CP	CP	1.25	25.0	27.0	CP	CP	.25	.25	-
6619	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	16	4	-	16	1.25	25.0	22.0	CP	CP	1.00	13.0	25.0	.313	CP	-	-	-	-	-	1.25	22.0	43.0	CP	CP	.50	.25	.25
6719	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	18	-	-	-	-	-	-	-	-	-	-	-	-	1.00	26.0	32.0	CP	CP	-	-	-	-	-	-	-	-
6320	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	7.63	10.00	.375	-	-	-	-	-	-	-	-	14	-	-	-	-	-	-	-	.75	28.0	22.0	.500	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6420	YZ	A	7	.500	7.63	19.00	.375	-	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	12	18	-	16	1.00	31.0	21.0	CP	CP	.75	49.0	24.0	.500	.500	-	-	-	-	-	1.00	30.0	22.0	CP	CP	-	.50	.25
6520	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	3	-	4	3	1.25	26.0	34.0	CP	CP	-	-	-	-	-	1.25	22.0	13.0	.563	.438	1.25	41.0	28.0	CP	CP	1.00	.75	1.00
6620	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	18	36	-	1.25	26.0	13.0	.563	.625	1.25	30.0	27.0	CP	CP	1.25	32.0	42.0	CP	CP	-	-	-	-	-	.50	.50	.50
6720	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	14	16	-	-	-	-	-	-	-	-	-	-	-	1.25	27.0	22.0	.438	.375	1.25	22.0	27.0	CP	CP	-	-	.75
6321	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	-	-	-	-	-	-	-	3	-	-	-	.50	14.0	10.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6421	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	4	12	-	.50	26.0	12.0	.188	.188	.75	25.0	12.0	.500	.438	.50	25.0	20.0	.438	.500	-	-	-	-	-	-	-	-
6521	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	12	36	16	12	1.00	20.0	25.0	CP	CP	1.25	34.0	33.0	CP	CP	1.00	33.0	23.0	CP	CP	1.00	32.0	21.0	CP	CP	.50	1.00	1.00
6621	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	12	14	16	3	1.25	34.0	19.0	CP	CP	1.00	27.0	25.0	.438	.500	1.00	24.0	30.0	CP	CP	1.25	28.0	24.0	CP	CP	.75	1.00	1.25
6721	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	4	8	-	-	-	-	-	-	-	-	-	-	-	1.00	14.0	14.0	.313	.313	1.00	17.0	20.0	CP	CP	-	-	.25
6322	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250	-	-	-	-	-	-	-	-	12	-	-	-	-	-	-	-	.50	25.0	20.0	.375	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6422	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	16	18	3	.75	33.0	13.0	.250	.250	.75	45.0	22.0	.500	.500	1.00	32.0	26.0	CP	CP	.50	17.0	11.0	.188	.188	.25	.25	.25
6522	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	16	12	4	.75	13.0	11.0	.375	CP	1.00	23.0	29.0	CP	CP	1.00	35.0	22.0	CP	CP	.50	22.0	13.0	.188	.188	.50	.25	.25
6622	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	12	4	12	12	1.00	28.0	20.0	CP	CP	.75	27.0	13.0	.250	.250	.75	20.0	24.0	.438	CP	1.00	21.0	25.0	CP	CP	.25	.25	.50
6722	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	12	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	20.0	20.0	CP	CP	-	-	.50
6323	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	1.00	32.0	26.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
6423	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	14	12	-	-	1.00	42.0	20.0	CP	CP	1.00	34.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.50	.50	-
6523	YZ	A	5	.500	4.63	16.00	.250	-	7	.625	7.63	19.00	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	6	12	4	4	.50	15.0	20.0	.250	.313	.75	19.0	26.0	.438	CP	.75	21.0	13.0	.438	.375	.75	21.0	13.0	.438	.375	-	-	-
6623	YZ	A	6	.500	4.63	19.00	.313	-	6	.500	4.63	19.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	4	.75	27.0	13.0	.250	.250	-	-	-	-	-	.75	15.0	18.0	.250	.313	.75	11.0	15.0	.250	CP	-	-	-
6723	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	12.00	.188	CP	-	-	4	12	-	-	-	-	-	-	-	-	-	-	-	1.00	14.0	14.0	.313	.313	1.00	26.0	20.0	CP	CP	-	-	.25
6324	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.25	37.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6424	YZ	A	7	.500	7.63	19.00	.375	-	5	.500	4.63	16.00	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	3	4	4	-	1.00	21.0	10.0	.313	.313	1.00	33.0	13.0	.313	.313	1.00	15.0	12.0	CP	CP	-	-	-	-	-	-	-	-
6524	YZ	A	7	.500	7.63	19.00	.375	-	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	14	1.00	13.0	16.0	.313	.313	1.00	14.0	18.0	.313	.313	1.00	21.0	13.0	.438	.375	1.00	30.0	21.0	CP	CP	-	.25	-
6624	YZ	A	5	.500	4.63	16.00	.313	-	6	.500	4.63	19.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	6	-	-	-	-	-	-	.50	27.0	13.0	.188	.188	.50	15.0	18.0	.188	.188	.50	15.0	17.0	.250	.375	-	-	-
6724	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	12.00	.188	CP	-	-	12	4	-	-	-	-	-	-	-	-	-	-	-	1.25	26.0	20.0	CP	CP	.50	14.0	14.0	.188	.188	-	-	.50
6325	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.313	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	1.00	17.0	12.0	.438	.375	-	-	-	-	-	-	-	-	-	-	-	-	-
6425	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	12	4	-	32	1.25	36.0	22.0	CP	CP	1.25	26.0	12.0	.375	.375	-	-	-	-	-	1.25	47.0	30.0	CP	CP	.50	1.00	1.00
6525	YZ	A	3	.375	4.63	21.00	.188	CP	5	.500	4.63	16.00	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	14	4	4	3	1.00	21.0	27.0	CP	CP	.50	15.0	17.0	.188	.188	.50	22.0	13.0	.188	.188	.50	19.0	10.0	.188	.188	.25	-	.25
6625	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	4	12	12	4	.50	27.0	12.0	.188	.188	1.25	28.0	20.0	CP	CP	1.00	20.0	25.0	CP	CP	.50	14.0	16.0	.188	.250	.25	.25	.50
6725	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	14	-	-	-	-	-	-	-	-	-	-	-	-	1.25	28.0	22.0	CP	CP	-	-	-	-	-	-	-	.50
6426	YZ	C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	16	4	-	-	1.00	53.0	21.0	.500	.438	.50	27.0	12.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-
6526	YZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	12	10	12	1.00	20.0	19.0	.500	CP	1.00	20.0	25.0	CP	CP	1.00	29.0

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL OT	LEFT BEAM SHEAR PLATE NBL SPTL SPWL SPLL WLDL BWEL	RIGHT BEAM SHEAR PLATE NBR SPTR SPWR SPLR WLDR BWER	UPPER COLUMN SHEAR PLATE NBU SPTU SPWU SPLU WLDU CWEU	LOWER COLUMN SHEAR PLATE NBD SPTD SPWD SPLD WLDD CWED	BRACE BOLTS BUL BUR BLL BLR	UP-LEFT GUSSET PLATE GTUL GWUL GLUL GWCUL GWBUL	UP-RIGHT GUSSET PLATE GTUR GWUR GLUR GWCUR GWBUR	DN-LEFT GUSSET PLATE GTLL GWLL GLLL GWCLL GWBLL	DN-RIGHT GUSSET PLATE GTLR GWLR GLLR GWCLR GWBLR	DOUBLER PLATES GBRL GBRR DBRC
7309 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7409 YZ A	3 .375 4.63 26.00 .188 CP	3 .375 4.63 32.00 .188 CP	- - - - - -	- - - - - -	- 3 6 -	- - - - -	1.25 51.0 28.0 CP CP	1.75 33.0 52.0 CP CP	- - - - -	1.25 1.00 .75
7509 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 24 6	- - - - -	- - - - -	1.75 26.0 44.0 CP CP	1.75 54.0 34.0 CP CP	- - 1.25
7310 YZ A	2 .375 4.63 11.00 .188 CP	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - - - -	3 6 - -	2.00 30.0 54.0 CP CP	2.25 33.0 58.0 CP CP	- - - - -	- - - - -	1.00 1.00 1.00
7410 YZ A	3 .375 4.63 29.00 .188 CP	3 .375 4.63 26.00 .188 CP	- - - - - -	- - - - - -	- 24 - -	- - - - -	1.50 26.0 40.0 CP CP	- - - - -	- - - - -	- 1.00 -
7510 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - 3 -	- - - - -	- - - - -	.50 10.0 23.0 .188 .188	- - - - -	- - -
7311 YZ C	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7411 YZ A	3 .375 4.63 31.00 .188 CP	3 .375 4.63 29.00 .188 CP	- - - - - -	- - - - - -	3 3 - 3	1.75 51.0 28.0 CP CP	1.25 15.0 11.0 .313 .313	- - - - -	1.75 30.0 51.0 CP CP	.75 .75 1.00
7511 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 28 -	- - - - -	- - - - -	1.50 39.0 28.0 CP CP	- - - - -	- - .25
7312 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7412 YZ A	11 .750 7.63 25.00 .438	13 .750 7.63 31.00 .438	- - - - - -	- - - - - -	12 28 - -	1.50 35.0 18.0 CP .750	1.50 51.0 28.0 CP CP	- - - - -	- - - - -	- - -
7512 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 28 3	- - - - -	- - - - -	1.75 42.0 28.0 CP CP	1.75 45.0 28.0 CP CP	- - 1.00
7313 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	12 - - -	.75 26.0 20.0 CP CP	- - - - -	- - - - -	- - - - -	- - -
7413 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 26.00 .188 CP	- - - - - -	- - - - - -	- 28 32 -	- - - - -	1.75 46.0 28.0 CP CP	1.50 45.0 30.0 CP CP	- - - - -	1.00 1.00 1.00
7513 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 4 12	- - - - -	- - - - -	.75 17.0 13.0 .250 .250	.75 25.0 19.0 CP CP	- - -
7314 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	36 32 - -	1.50 40.0 32.0 CP CP	1.50 37.0 30.0 CP CP	- - - - -	- - - - -	- .25 1.00
7414 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	3 4 10 12	.75 22.0 10.0 .250 .250	.75 24.0 12.0 .250 .250	1.00 22.0 18.0 CP CP	1.00 25.0 20.0 CP CP	.25 .25 -
7514 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7315 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- 10 - -	- - - - -	1.00 23.0 18.0 CP CP	- - - - -	- - - - -	- - .25
7415 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	36 - - 36	1.50 56.0 32.0 CP CP	- - - - -	- - - - -	1.25 49.0 32.0 CP CP	1.00 1.00 1.00
7515 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 10 3	- - - - -	- - - - -	1.25 22.0 17.0 CP CP	.50 15.0 11.0 .313 .250	- - .25
7316 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7416 YZ A	3 .375 4.63 31.00 .188 CP	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	32 10 - -	1.25 55.0 30.0 CP CP	1.25 28.0 16.0 CP CP	- - - - -	- - - - -	- - .25
7516 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 3 36	- - - - -	- - - - -	1.50 50.0 32.0 CP CP	1.50 47.0 32.0 CP CP	- - 1.25
7317 YZ C	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7417 YZ A	3 .375 4.63 31.00 .188 CP	3 .375 4.63 31.00 .188 CP	- - - - - -	- - - - - -	3 3 3 -	1.00 18.0 11.0 .313 .313	1.25 55.0 32.0 CP CP	1.25 30.0 46.0 CP CP	- - - - -	.25 1.00 .75
7517 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - 32	- - - - -	- - - - -	- - - - -	1.25 42.0 30.0 CP CP	- - .25
7318 YZ A	2 .375 4.63 11.00 .188 CP	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - - - -	3 3 - -	3.00 34.0 61.0 CP CP	1.75 30.0 54.0 CP CP	- - - - -	- - - - -	1.50 1.00 1.50
7418 YZ A	3 .375 4.63 31.00 .188 CP	3 .375 4.63 31.00 .188 CP	- - - - - -	- - - - - -	10 - - -	1.25 19.0 21.0 CP CP	- - - - -	- - - - -	- - - - -	- - -
7518 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - 3	- - - - -	- - - - -	- - - - -	.50 10.0 22.0 .188 .188	- - -
7319 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7419 YZ A	3 .375 4.63 31.00 .188 CP	3 .375 4.63 31.00 .188 CP	- - - - - -	- - - - - -	3 - - 3	1.50 55.0 33.0 CP CP	- - - - -	- - - - -	2.75 34.0 60.0 CP CP	.75 1.25 1.25
7519 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 36 10	- - - - -	- - - - -	1.75 54.0 31.0 CP CP	1.25 16.0 28.0 CP CP	- - -
7320 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7420 YZ A	3 .375 4.63 31.00 .188 CP	3 .375 4.63 31.00 .188 CP	- - - - - -	- - - - - -	3 36 - -	1.75 53.0 32.0 CP CP	1.75 54.0 31.0 CP CP	- - - - -	- - - - -	.50 - -
7520 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 9 3	- - - - -	- - - - -	1.75 52.0 33.0 CP CP	1.50 52.0 33.0 CP CP	- - 1.00
7321 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	10 - - -	1.00 24.0 17.0 CP CP	- - - - -	- - - - -	- - - - -	- - -
7421 YZ A	3 .375 4.63 26.00 .188 CP	3 .375 4.63 31.00 .188 CP	- - - - - -	- - - - - -	4 9 9 -	1.75 29.0 13.0 .438 .375	1.75 58.0 33.0 CP CP	1.75 56.0 37.0 CP CP	- - - - -	1.00 1.00 1.00
7521 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 3 3	- - - - -	- - - - -	1.00 16.0 10.0 .313 .313	1.25 54.0 32.0 CP CP	- - .50
7322 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- 9 - -	- - - - -	1.75 45.0 37.0 CP CP	- - - - -	- - - - -	- - .25
7422 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 26.00 .188 CP	- - - - - -	- - - - - -	16 3 36 10	1.50 31.0 22.0 CP CP	1.25 28.0 10.0 .313 .313	1.25 46.0 32.0 CP CP	1.25 23.0 18.0 CP CP	1.00 - .25
7522 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 4 4	- - - - -	- - - - -	.75 18.0 13.0 .375 .313	.75 19.0 13.0 CP .438	- - -
7323 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- 36 - -	- - - - -	1.25 44.0 32.0 CP CP	- - - - -	- - - - -	- - 1.00
7423 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	12 4 - -	.75 40.0 18.0 .313 .313	.50 25.0 12.0 .313 .313	- - - - -	- - - - -	- - -
7523 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - 16 16	- - - - -	- - - - -	1.50 31.0 22.0 CP CP	1.50 31.0 22.0 CP CP	- - -
7324 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
7424 YZ A	4 .500 4.63 13.00 .250	9 .750 7.63 19.00 .500	- - - - - -	- - - - - -	4 16 - -	1.50 29.0 12.0 .313 .313	1.50 33.0 22.0 CP CP	- - - - -	- - - - -	- - -
7524 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 28 12	- - - - -	- - - - -	1.50 45.0 28.0 CP CP	.75 24.0 19.0 .500 .500	- - -
7325 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	36 - - -	1.50 44.0 32.0 CP CP	- - - - -	- - - - -	- - - - -	- - -
7425 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	3 28 20 -	1.50 21.0 10.0 .313 .313	1.75 40.0 28.0 CP CP	1.50 38.0 24.0 CP CP	- - - - -	.50 1.00 -
7525 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 16 4	- - - - -	- - - - -	1.25 30.0 22.0 CP CP	.50 18.0 13.0 .188 .188	- - -
7326 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- 20 - -	- - - - -	1.50 32.0 24.0 CP CP	- - - - -	- - - - -	- - .50
7426 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	14 16 3 36	1.50 49.0 19.0 .625 .563	1.50 33.0 22.0 CP CP	1.50 45.0 32.0 CP CP	1.50 48.0 32.0 CP CP	1.25 1.00 1.75
7526 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 20 3	- - - - -	- - - - -	1.25 33.0 24.0 CP CP	.50 13.0 11.0 .188 .188	- - .50
7327 YZ B	- - - - - -	2 1.750 - - 1.188	- - - - - -	- - - - - -	- 3 - -	- - - - -	1.50 47.0 32.0 CP CP	- - - - -	- - - - -	- - -
7427 YZ B	- - - - - -	3 .750 - - .500	- - - - - -	- - - - - -	- 20 - -	- - - - -	1.25 45.0 24.0 CP CP	- - - - -	- - - - -	- - -
7527 YZ B	- - - - - -	2 .625 - - .438	- - - - - -	- - - - - -	- - - 14	- - - - -	- - - - -	- - - - -	1.00 34.0 20.0 CP CP	- - -
GRID 8										
8301 YZ B	3 .500 - - -	- - - - - -	- - - - - -	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
8401 YZ B	5 .500 - - -	- - - - - -	- - - - - -	- - - - - -	6 - - -	1.00 37.0 13.0 CP .438	- - - - -	- - - - -	- - - - -	- - -
8501 YZ B	3 1.000 - - .688	- - - - - -	- - - - - -	- - - - - -	16 - 10 -	1.25 32.0 22.0 CP CP	- - - - -	.50 41.0 19.0 .438 .375	- - - - -	.50 - -
8601 YZ B	3 .875 - - .563	- - - - - -	- - - - - -	- - - - - -	8 - 8 -	1.00 30.0 15.0 CP CP	- - - - -	.75 30.0 23.0 .500 CP	- - - - -	.50 - -
8701 YZ B	3 .500 - - -	- - - - - -	- - - - - -	- - - - - -	- - 4 -	- - - - -	- - - - -	.75 21.0 16.0 .438 .438	- - - - -	- - -
8402 YZ C	- - - - - -	- - - - - -	4 .500 7.63 10.00 .313	- - - - - -	4 10 - -	.50 27.0 12.0 .188 .188	.50 35.0 19.0 .375 .375	- - - - -	- - - - -	- - -
8502 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	8 8 18 6	1.00 20.0 16.0 .500 CP	1.00 30.0 22.0 .438 .500	1.00 35.0 25.0 CP CP	1.00 24.0 13.0 CP .500	.50 .25 .50
8602 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	8 4 18 16	1.00 21.0 16.0 CP CP	1.00 24.0 13.0 .313 .313	1.00 29.0 28.0 .500 CP	1.25 29.0 22.0 CP CP	.50 .50 .75
8702 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 12.00 .188 CP	- - 4 8	- - - - -	- - - - -	1.00 14.0 17.0 .375 .375	1.00 19.0 18.0 CP CP	- - .50
8303 YZ C	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - - - -	4 - - -	.50 19.0 13.0 .188 .188	- - - - -	- - - - -	- - - - -	- - -
8403 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	3 18 9 -	.50 22.0 10.0 .188 .188	.75 49.0 24.0 .500 .500	.75 31.0 24.0 CP CP	- - - - -	.50 - .50
8503 YZ A	6 .500 4.63 19.00 .313	8 .500 7.63 19.00 .375	- - - - - -	- - - - - -	18 6 4 -	- - - - -	.75 29.0 28.0 .500 CP	.75 25.0 15.0 CP .500	.50 22.0 13.0 .188 .188	- .25
8603 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	10 4 4 8	.75 23.0 18.0 CP CP	.75 19.0 14.0 .313 .375	.75 13.0 14.0 .250 .438	.75 15.0 17.0 CP CP	.25 .25 .25
8703 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 12.00 .188 CP	- - 4 8	- - - - -	- - - - -	- - - - -	1.00 17.0 20.0 CP CP	- - .25
8304 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	4 9 - -	.50 19.0 13.0 .188 .188	.75 30.0 23.0 CP CP	- - - - -	- - - - -	- - .25
8404 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	3 6 36 4	.50 22.0 10.0 .188 .188	.75 25.0 15.0 CP .500	.75 50.0 32.0 CP CP	.50 19.0 14.0 .188 .188	.25 .25 .25
8504 YZ A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	3 4 4 3	.50 12.0 13.0 .188 .188	.50 17.0 13.0 .250 .250	.50 22.0 13.0 .375 .313	.50 19.0 10.0 .188 .188	- - -
8604 YZ A	4 .500 4.63 13.32 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
8704 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .500 4.63 10.00 .313	- - 4 10	- - - - -	- - - - -	.75 14.0 14.0 .250 .250	.75 19.0 22.0 CP CP	- - .25
8305 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- 36 - -	- - - - -	.75 40.0 32.0 CP CP	- - - - -	- - - - -	- - .75
8405 YZ A	5 .500 4.63 16.00 .313	5 .500 4.63 16.00 .250	- - - - - -	- - - - - -	4 4 - 4	.50 23.0 13.0 .313 .250	.50 25.0 12.0 .375 .313	- - - - -	.50 19.0 14.0 .188 .188	- - -
8505 YZ A	3 .375 4.63 21.00 .188 CP	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	12 - 6 3	1.25 20.0 24.0 CP CP	- - - - -	1.00 21.0 14.0 CP CP	.50 19.0 10.0 .188 .188	.25 - -
8605 YZ A	6 .500 4.63 19.00 .313	4 .500 4.63 13.32 .313	- - - - - -	- - - - - -	4 4 10 3	.50 27.0 13.0 .188 .188	.50 19.0 15.0 .188 .188	1.00 19.0 23.0 .500 CP	.50 11.0 11.0 .188 .188	.25 - .25
8705 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .375 4.63 10.00 .250	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
8306 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	32 - - -	1.00 38.0 30.0 CP CP	- - - - -	- - - - -	- - - - -	- - .25
8406 YZ A	5 .625 4.63 16.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	3 6 8 -	.75 21.0 10.0 .250 .250	1.00 33.0 13.0 .438 .375	.75 20.0 16.0 CP CP	- - - - -	.25 - -
8506 YZ A	5 .500 4.63 16.00 .313	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	3 10 4 4	- - - - -	.75 18.0 23.0 CP CP	.50 22.0 13.0 .250 .188	.50 23.0 13.0 .313 .250	- .25 -
8606 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	5 - - 12	1.25 29.0 15.0 .438 .438	- - - - -	- - - - -	1.25 20.0 24.0 CP CP	.25 .25 .25
8706 YZ C	- - - - - -	- - - - - -	- - - - - -	4 .625 4.63 10.00 .375	- - 8 4	- - - - -	- - - - -	1.00 16.0 16.0 CP CP	.50 14.0 14.0 .188 .188	- - .25
8307 YZ C	- - - - - -	- - - - - -	4 .625 7.63 10.00 .375	- - - - - -	- 8 - -	- - - - -	.75 22.0 16.0 CP .500	- - - - -	- - - - -	- - -
8407 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	28 4 - 32	1.25 41.0 28.0 CP CP	1.00 33.0 13.0 .313 .313	- - - - -	1.00 47.0 30.0 CP CP	.50 .25 .25
8507 YZ A	6 .625 4.63 19.00 .313	4 .500 4.63 13.00 .313	- - - - - -	- - - - - -	- - 10 3	- - - - -	- - - - -	1.00 26.0 17.0 CP CP	.50 19.0 10.0 .188 .188	- - -
8607 YZ A	6 .500 4.63 19.00 .313	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	10 8 - -	1.00 32.0 17.0 .375 .375	1.00 29.0 15.0 CP CP	- - - - -	- - - - -	- - .25
8707 YZ C	- - - - - -	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - 20 5	- - - - -	- - - - -	1.25 29.0 24.0 CP CP	.75 20.0 16.0 CP .500	.25 .25 .75
8308 YZ C	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
8408 YZ A	6 .500 4.63 19.00 .313	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	4 10 - -	1.00 27.0 13.0 .313 .313	1.00 39.0 16.0 .500 .438	- - - - -	- - - - -	- - .25
8508 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	32 - 4 28	1.25 29.0 37.0 CP CP	- - - - -	1.25 19.0 12.0 .625 .500	1.25 41.0 28.0 CP CP	.50 .75 1.00
8608 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- 20 3 -	- - - - -	1.25 40.0 24.0 CP CP	1.25 26.0 34.0 CP CP	- - - - -	.75 .50 1.00
8708 YZ C	- - - - - -	- - - - - -	- - - - - -	4 .500 4.63 10.00 .375	- - 4 10	- - - - -	- - - - -	.75 14.0 14.0 .250 .250	.75 18.0 19.0 CP CP	- - -
8309 YZ C	- - - - - -	- - - - - -	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
8409 YZ A	3 .375 4.63 21.00 .188 CP	9 .500 7.63 19.00 .438	- - - - - -	- - - - - -	- 4 3 -	- - - - -	1.00 30.0 11.0 .375 .313	1.50 30.0 46.0 CP CP	- - - - -	1.00 .25 .50
8509 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- 3 20 4	- - - - -	1.25 26.0 36.0 CP CP	1.50 24.0 37.0 CP CP	.50 22.0 13.0 .313 .250	.50 .75 -
8609 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- 4 16 32	- - - - -	1.25 27.0 13.0 .313 .313	1.25 22.0 49.0 CP CP	1.25 29.0 37.0 CP CP	.25 .25 1.00
8709 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - 4 -	- - - - -	- - - - -	.50 11.0 30.0 .188 .188	- - - - -	- - -
8310 YZ A	2 .375 4.63 11.00 .188 CP	2 .375 4.63 11.00 .188 CP	- - - - - -	- - - - - -	3 3 - -	1.75 27.0 47.0 CP CP	2.00 30.0 53.0 CP CP	- - - - -	- - - - -	1.00 1.00 1.00
8410 YZ A	4 .500 4.63 13.00 .250	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	4 20 - -	1.25 12.0 14.0 .563 CP	1.50 24.0 33.0 CP CP	- - - - -	- - - - -	- .25 -
8510 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	4 16 - -	1.50 13.0 36.0 .313 CP	1.50 23.0 57.0 .625 CP	- - - - -	- - - - -	.25 .75 -
8610 YZ A	7 .500 7.63 19.00 .375	7 .500 7.63 19.00 .313	- - - - - -	- - - - - -	4 4 - -	.50 12.0 16.0 .188 .313	.50 12.0 16.0 .188 .313	- - - - -	- - - - -	- - -
8710 YZ C	- - - - - -	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
8311 YZ C	- - - - - -	- - - - - -	3 .500 4.63 10.00 .250	- - - - - -	- - - -	- - - - -	- - - - -	- - - - -	- - - - -	- - -
8411 YZ A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	16 - - 3	1.25 34.0 22.0 CP CP	- - - - -	- - - - -	1.25 27.0 39.0 CP CP	.25 .75 .25
8511 YZ A	3 .375 4.63 21.00 .188 CP	6 .500 4.63 19.00 .313	- - - - - -	- - - - - -	16 - 10 4	1.00 22.0 30.0 CP CP	- - - - -	1.00 25.0 17.0 .438 .438	1.00 12.0 18.0 .313 .500	.50 - -
8611 YZ A	3 .375 4.63 21.00 .188 CP	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	4 - 8 4	.75 27.0 13.0 .250 .250	- - - - -	.75 22.0 27.0 .375 .500	.75 10.0 45.0 .250 .375	- - -
8711 YZ C	- - - - - -	- - - - - -	- - - - - -	4 .500 4.63 10.00 .375	- - 16 4	- - - - -	- - - - -	.50 29.0 25.0 .438 .438	.50 11.0 31.0 .188 .188	- - -

Thu Nov 30 13:57:11 1995 jkhoeber frsc_6.dgn DMS pl1

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

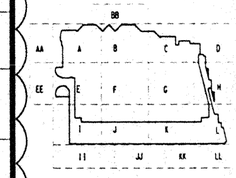
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 623-1221

LICENSED PROFESSIONAL ENGINEER NO. 21775 WASHINGTON, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision/observation as defined in Title 18 Chapter 18 Section 1.3 (a) of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	6-19-95		

Sheet Title

CONNECTION SCHEDULE

Designed BJM
Drawn DMS
Checked JDM
Date 5 / 8 / 95
Job No. 93036.02.9
Sheet No.

S6.10

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT	PL	OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEBL	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWEBR	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEBU	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWEBD	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
8312 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8412 YZ	A		4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-	-	4	10	-	-	.50	33.0	13.0	.250	.188	.50	39.0	16.0	.313	.250	-	-	-	-	-	-	-	-	-	-	-	-	.25	.50	.75
8512 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	8	8	6	16	1.00	17.0	16.0	.500	CP	1.00	21.0	26.0	.438	CP	1.00	25.0	15.0	CP	.438	1.25	34.0	22.0	CP	CP	.25	.50	.75
8612 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	4	16	16	16	1.00	27.0	13.0	.313	.313	1.00	38.0	23.0	.375	.375	1.00	24.0	30.0	CP	CP	1.00	22.0	28.0	CP	CP	.25	.50	.75
8712 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		3	.500	4.63	10.00	.250		-	-	4	4	-	-	-	-	-	-	-	-	-	-	-	-	.50	16.0	13.0	.313	.250	.50	14.0	15.0	.188	.250	-	-	-
8313 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	4	-	-	-	.50	17.0	13.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8413 YZ	A		3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	4	6	18	-	.75	33.0	13.0	.375	.313	.75	25.0	15.0	CP	.438	.75	32.0	26.0	.438	CP	-	-	-	-	-	.25	-	.25
8513 YZ	A		9	.625	7.63	19.00	.438		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	-	16	18	4	-	-	-	-	-	1.00	24.0	30.0	CP	CP	.75	36.0	25.0	CP	CP	.50	22.0	13.0	.375	.313	-	.25	.50
8613 YZ	A		4	.500	4.63	13.00	.250		6	.625	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	4	4	-	8	.75	27.0	13.0	.250	.250	.75	26.0	12.0	.250	.250	-	-	-	-	-	.75	17.0	20.0	.375	CP	-	-	-
8713 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	12.00	.188	CP	-	-	4	4	-	-	-	-	-	-	-	-	-	-	-	-	.75	16.0	13.0	.250	.250	.75	14.0	15.0	.500	CP	-	-	.25
8314 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	.75	31.0	27.0	.438	.438	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8414 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	36	18	-	4	.75	56.0	32.0	CP	CP	.75	45.0	24.0	CP	.500	-	-	-	-	-	.50	18.0	13.0	.188	.188	.75	.50	-
8514 YZ	A		9	.500	7.63	19.00	.438		7	.500	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-	-	-	-	8	4	-	-	-	-	-	-	-	-	-	-	-	1.00	23.0	15.0	CP	CP	.75	21.0	13.0	CP	.500	-	-	-
8614 YZ	A		4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-	-	12	4	-	-	.50	34.0	19.0	.313	.313	.50	26.0	12.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8714 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		3	.500	4.63	10.00	.313		-	-	8	4	-	-	-	-	-	-	-	-	-	-	-	1.00	16.0	17.0	CP	CP	.50	14.0	15.0	.375	.375	.25	-	-
8315 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8415 YZ	A		4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-		-	-	-	-	-	-	3	8	-	-	1.00	23.0	10.0	.313	.313	1.00	36.0	15.0	.500	.438	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8515 YZ	A		3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	18	-	3	36	1.25	25.0	32.0	CP	CP	-	-	-	-	-	.75	19.0	11.0	.250	.250	.75	46.0	32.0	CP	CP	.25	-	.25
8615 YZ	A		3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	4	8	20		1.25	27.0	13.0	.313	.313	1.25	29.0	15.0	.625	.563	1.25	24.0	30.0	CP	CP	-	-	-	-	-	.50	-	.50
8715 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		3	.500	4.63	10.00	.250		-	-	3	12	-	-	-	-	-	-	-	-	-	-	-	.50	14.0	11.0	.188	.188	.50	24.0	20.0	.375	.375	-	-	-
8316 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8416 YZ	A		7	.625	4.63	22.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-	-	10	3	-	-	.75	47.0	16.0	.375	.313	.50	21.0	10.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8516 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	-	20	3	3	-	-	-	-	-	1.25	24.0	31.0	CP	CP	1.25	42.0	28.0	CP	CP	.50	19.0	11.0	.188	.188	.50	.25	.75
8616 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	18	3	-	18	1.25	40.0	25.0	CP	CP	1.25	19.0	10.0	.313	.313	-	-	-	-	-	1.25	25.0	32.0	CP	CP	.25	.25	.25
8716 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		4	.625	4.63	10.00	.375		-	-	8	4	-	-	-	-	-	-	-	-	-	-	-	.75	16.0	17.0	CP	CP	.50	14.0	14.0	.188	.188	-	-	.25
8317 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8417 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	4	3	3	-	1.00	13.0	19.0	.313	.500	1.25	47.0	28.0	CP	CP	1.50	30.0	44.0	CP	CP	-	-	-	-	-	.75	1.00	1.00
8517 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	-	-	-	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	26.0	17.0	CP	CP	-	-	-
8617 YZ	A		4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-		-	-	-	-	-	-	-	8	4	-	-	-	-	-	-	1.00	29.0	15.0	.500	.438	.75	10.0	45.0	.250	.438	-	-	-	-	-	-	-	-
8717 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	4	18	-	-	-	-	-	-	-	-	-	-	-	1.00	12.0	30.0	.313	.313	1.00	32.0	26.0	CP	CP	-	-	.50
8318 YZ	A		2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	3	3	-	-	2.25	32.0	58.0	CP	CP	2.00	30.0	55.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.50	1.00	1.00
8418 YZ	A		3	.375	4.63	26.00	.188	CP	7	.500	7.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	10	-	-	-	1.00	17.0	19.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	-	-
8518 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	20	4	-	4	1.50	30.0	62.0	CP	CP	1.50	15.0	39.0	.313	CP	-	-	-	-	-	.75	12.0	26.0	.250	.438	1.00	-	-
8618 YZ	A		4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-		-	-	-	-	-	-	4	4	-	-	.50	12.0	17.0	.313	CP	.75	12.0	17.0	.375	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8718 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8319 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8419 YZ	A		3	.375	4.63	29.00	.188	CP	3	.375	4.63	26.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	3	-	-	3	1.25	54.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.75	32.0	57.0	CP	CP	1.00	1.25	1.00
8519 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	-	-	14	10	-	-	-	-	-	-	-	-	-	-	1.00	30.0	21.0	CP	CP	1.25	16.0	25.0	CP	CP	.50	-	-
8619 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	16	-	-	20	1.25	29.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.50	24.0	53.0	CP	CP	.50	.25	1.00
8719 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	36	4	-	-	-	-	-	-	-	-	-	-	-	1.25	39.0	32.0	CP	CP	.50	12.0	31.0	.188	.313	-	-	.50
8320 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8420 YZ	A		6	.500	4.63	19.00	.313		6	.625	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	8	14	-	-	1.00	27.0	15.0	CP	.500	1.00	57.0	20.0	.500	.500	-	-	-	-	-	-	-	-	-	-	-	-	.25	-
8520 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	3	-	8	3	1.25	26.0	33.0	CP	CP	-	-	-	-	-	1.25	24.0	15.0	.375	.313	1.50	43.0	29.0	CP	CP	.75	1.00	1.25
8620 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	4	36	36	-	1.50	27.0	13.0	.313	.313	1.50	49.0	32.0	CP	CP	1.50	31.0	40.0	CP	CP	-	-	-	-	-	.75	.75	.75
8720 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	8	16	-	-	-	-	-	-	-	-	-	-	-	1.25	23.0	18.0	.375	.375	1.25	28.0	22.0	CP	CP	-	-	.75
8321 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313		-	-	-	-	-	-	4	-	-	-	1.25	18.0	11.0	.563	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8421 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	3	8	16	-	1.00	22.0	10.0	.313	.313	1.00	28.0	14.0	.313	.313	1.00	29.0	24.0	CP	CP	-	-	-	-	-	.25	-	.25
8521 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	12	36	4	8	1.00	20.0	25.0	CP	CP	1.50	31.0	40.0	CP	CP	.75	21.0	13.0	.250	.250	.75	24.0	15.0	CP	CP	.25	.75	1.00
8621 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	12	8	14	3	1.25	28.0	20.0	CP	CP	1.00	25.0	18.0	.375	.375	1.00	22.0	27.0	CP	CP	1.25	26.0	33.0	CP	CP	.75	1.00	1.25
8721 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		3	.500	4.63	10.00	.313		-	-	4	4	-	-	-	-	-	-	-	-	-	-	-	.50	14.0	14.0	.313	.313	.50	14.0	14.0	.375	.438	-	-	.25
8322 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	16	-	-	-	-	-	-	-	1.00	31.0	23.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25	
8422 YZ	A		8	.625	7.63	19.00	.375		6	.625	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	8	4	-	4	1.25	36.0	15.0	.625	.563	1.25	25.0	12.0	.313	.313	-	-	-	-	-	1.25	16.0	12.0	CP	CP	-	.25	-
8522 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	8	14	36	3	.50	17.0	16.0	.375	.500	1.00	21.0	27.0	CP	CP	.75	46.0	32.0	CP	CP	.50	19.0	10.0	.188	.188	.50	.25	.25
8622 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	12	4	14	12	1.00	28.0	20.0	CP	CP	.75	27.0	13.0	.250	.250	1.00	22.0	27.0	CP	CP	1.00	20.0	25.0	CP	CP	.50	.25	.75
8722 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	-	12	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	26.0	20.0	CP	CP	-	-	.50
8323 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.00	37.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00
8423 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	6	36	-	-	.75	25.0	15.0	.438	.313	.75	45.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.75	-
8523 YZ	A		5	.500	4.63	16.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	6	14	3	8	1.25	17.0	23.0	.375	.500	1.25	21.0	28.0	.563	CP	1.25	19.0	10.0	.313	.313	1.25	22.0	15.0	CP	CP	.50	-	-
8623 YZ	A		5	.500	4.63	16.00	.313		6	.500	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	4	-	8	8	.50	27.0	13.0	.188	.188	-	-	-	-	-	.50	15.0	18.0	.250	.375	.50	17.0	20.0	.250	.375	-	-	-
8723 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	12.00	.188	CP	-	-	3	12	-	-	-	-	-	-	-	-	-	-	-	1.00	12.0	11.0	.313	.313	1.00	27.0	20.0	CP	CP	-	-	.25
8324 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8424 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	32	3	-	32	1.00	44.0	30.0	CP	CP	1.00	23.0	10.0	.313	.313	-	-	-	-	-	1.00	48.0	30.0	CP	CP	.50	.50	.50
8524 YZ	A		5	.500	4.63	16.00	.313		6	.500	4.63	19.00	.313		-	-	-	-	-		-	-	-	-	-	-	4	6	6	-						.50	15.0	16.0	.250	.375	1.00	21.0	14.0	CP	CP	.50	26.0	15.0	.375	.313	-	-	.25
8624 YZ	A		4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.250		-	-	-	-	-		-	-	-	-	-	-	3	3	-	6	.50	24.0	10.0	.188	.188	.50	19.0	10.0	.188	.188	-	-	-	-	-	.50	19.0	22.0	.375	.500	-	-	-
8724 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	16	4	-	-	-	-	-	-	-	-	-	-	-	1.25	29.0	22.0	CP	CP	.50	14.0	14.0	.188	.188	-	-	.50
8325 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	18	-	-	-	1.00	33.0	25.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	
8425 YZ	A		7	.500	7.63	19.00	.375		5	.500	4.63	16.00	.313		-	-	-	-	-		-	-	-	-	-	-	8	6	4	-	1.00	38.0	15.0	CP	.500	1.00	34.0	13.0	.375	.313	.50	19.0	14.0	.375	.375	-	-	-	-	-	-	-	-
8525 YZ	A		9	.625	7.63	19.00	.438		3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	32	-	5	32	1.50	29.0	37.0	CP	CP	-	-	-	-	-	1.00	26.0	15.0	CP	.500	1.00	43.0	30.0	CP	CP	-	.50	.75
8625 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	4	16	3	-	1.25	27.0	13.0	.313	.313	1.25	30.0	22.0	CP	CP	1.25	26.0	34.0	CP	CP	-	-	-	-	-	.50	.25	1.00
8725 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		3	.500	4.63	10.00	.250		-	-	3	3	-	-	-	-	-	-	-	-	-	-	-	.50	12.0	11.0	.188	.188	.50	12.0	11.0	.188	.250	-	-	-
8326 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	4	.625	7.63	10.00	.375		-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	.50	19.0	14.0	.313	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8426 YZ	A		3	.375	4.63	29.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	12	5	-	18	1.25	50.0	17.0	CP	CP	1.00	27.0	14.0	CP	.500	-	-	-	-	-	1.00	42.0	26.0	CP	CP	.25	1.00	.25
8526 YZ	A		3	.375	4.63	29.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	-	3	24	8	-	-	-	-	-	1.25	26.0	34.0	CP	CP	1.75	54.0	26.0	CP	CP	1.00	21.0	15.0	CP	CP	1.00	.75	.75
8626 YZ	A		3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-		-	-	-	-	-	-	18	3	-	32	1.50	48.0	24.0	.625	.563	1.50	19.0	10.0	.313	.313	-	-	-	-	-	1.50	29.0	38.0	CP	CP	.50	1.00	1.00
8726 YZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		2	.375	4.63	11.00	.188	CP	-	-	16	4	-	-	-	-	-	-	-	-	-	-	-	1.25	33.0	22.0	CP	CP	.50	14.0	14.0	.375	.438	-	-	-
8327 YZ	B		-	-	-	-	-	-	4	.625	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8427 YZ	B		-	-	-	-	-	-	3	.875	-	-	-	.563	-	-	-	-	-		-	-	-	-	-	-	-	24	-	-	-	-	-	-	-	1.75	57.0	26.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8527 YZ	B		-	-	-	-	-	-	6	.500	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-	-	12	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	46.0	18.0	CP	CP	-	-	-
8627 YZ	B		-	-	-	-	-	-	8	.625	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	16	-	-	-	-	-	-	-	1.25	40.0	22.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8727 YZ	B		-	-	-	-	-	-	2	.750	-	-	-	.500	-	-	-	-	-		-	-	-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	35.0	25.0	CP	CP	-	-	-
GRID 10																																																									
10004 ZZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	12	-	-	-	1.00	20.0	28.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	
10104 ZZ	A		7	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	10	-	4	-	.75	23.0	19.0	.500	.438	-	-	-	-	-	.75	17.0	15.0	.438	.375	-	-	-	-	-	-	-	-
10204 ZZ	A		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10005 ZZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	.75	14.0	23.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10105 ZZ	A		4	.500	4.63	13.00	.250		7	.500	7.63	19.00	.375		-	-	-	-	-		-	-	-	-	-	-	4	-	-	12	1.00	12.0	24.0	.375	CP	-	-	-	-	-	-	-	-	-	-	1.00	23.0	22.0	CP	CP	-	.25	-
10205 ZZ	A		5	.625	4.63	16.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-		-	-	-	-	-	-	6	-	4	10	.75	17.0	21.0	.375	.438	-	-	-	-	-	.75	12.0	36.0	.250	.375	.75	18.0	19.0	CP	CP	-	-	-
10305 ZZ	A		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-	5	-	-	-	-	-	-	-	-	-	-	-	-	.75	15.0	26.0	.375	.500	-	-	-	-	-	-	-	-
10006 ZZ	C		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	16	-	-	-	1.25	21.0	25.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	
10106 ZZ	A		3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.250		-	-	-	-	-		-	-	-	-	-	-	12	4	10	-	1.00	51.0	16.0	CP	.375	1.00	13.0	23.0	.500	CP	1.00	30.0	16.0	CP	CP	-	-	-	-	-	.25	-	-
10206 ZZ	A		5	.625	4.63	16.00	.313		5	.500	4.63	16.00	.250		-	-	-	-	-		-

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT	PL	OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEB	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWEB	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEB	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWEB	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
10017	ZZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	16	-	-	1.00	16.0	26.0	CP	.313	1.00	21.0	25.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50
10117	ZZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	10	10	16	1.00	17.0	19.0	CP	CP	1.25	38.0	15.0	CP	.625	1.00	19.0	23.0	.500	.500	1.00	41.0	22.0	CP	CP	.25	.25	.25
10217	ZZ	A	3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	12	-	-	12	1.00	35.0	23.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.75	34.0	17.0	CP	.438	.25	-	.25
10317	ZZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10019	ZZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	10	-	-	.75	18.0	29.0	CP	.250	.75	18.0	29.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10119	ZZ	A	5	.500	4.63	16.00	.313		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	8	-	-	-	-	-	-	-	-	-	-	-	1.00	17.0	21.0	CP	CP	1.00	17.0	21.0	.500	CP	.25	.25	-
10219	ZZ	A	7	.500	7.63	19.00	.375		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	8	-	-	-	-	-	-	-	-	-	-	-	1.00	17.0	20.0	CP	CP	1.00	17.0	19.0	CP	CP	.25	.25	-
10319	ZZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	20	12	-	-	-	-	-	-	-	-	-	-	-	1.00	36.0	24.0	CP	CP	1.00	34.0	23.0	CP	CP	.25	-	.25
10020	ZZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	8	-	-	.75	21.0	22.0	CP	.375	1.00	16.0	26.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	.25
10120	ZZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	14	8	10	10	.75	41.0	19.0	CP	.438	1.00	18.0	15.0	CP	CP	1.25	24.0	17.0	CP	CP	.75	19.0	23.0	.500	.500	.25	.25	.25
10220	ZZ	A	6	.500	4.63	19.00	.313		7	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	-	20	14	-	-	-	-	-	-	-	1.00	37.0	24.0	CP	CP	.75	30.0	20.0	CP	CP	-	-	-	-	-	-	-	-
10320	ZZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10022	ZZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	10	-	-	1.00	16.0	26.0	CP	.313	1.25	19.0	20.0	CP	.563	-	-	-	-	-	-	-	-	-	-	-	-	.50
10122	ZZ	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	14	10	12	.75	20.0	22.0	.500	.500	.75	41.0	19.0	CP	.438	.75	19.0	23.0	CP	CP	.75	26.0	19.0	CP	CP	.25	.25	-
10222	ZZ	A	8	.625	7.63	19.00	.375		7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	14	1.00	33.0	21.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.75	30.0	20.0	CP	CP	.25	-	-
10322	ZZ	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10023	ZZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	10	-	-	.75	13.0	37.0	.250	.313	.75	18.0	29.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10123	ZZ	A	3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	8	-	-	-	-	-	-	-	-	-	-	1.50	16.0	34.0	.688	CP	.75	17.0	21.0	.500	.500	.25	-	-
10223	ZZ	A	3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	10	-	-	-	-	-	-	-	-	-	-	1.50	12.0	30.0	.500	CP	.75	19.0	22.0	.500	.500	.25	-	-
10323	ZZ	A	5	.500	4.63	16.00	.313		7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	10	-	-	-	-	-	-	-	-	-	-	1.00	13.0	24.0	.438	CP	1.00	32.0	21.0	CP	CP	.25	-	-
10024	ZZ	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	1.25	15.0	38.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10124	ZZ	A	-	-	-	-	-	-	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	4	-	-	-	-	-	1.25	13.0	23.0	CP	CP	-	-	-	-	-	.75	12.0	38.0	.250	.250	-	.25	-
10224	ZZ	A	-	-	-	-	-	-	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	-	-	-	-	1.00	14.0	16.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	.25	-
10324	ZZ	A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
GRID E																																																							
11001	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	-	-	-	-	-	-	-	1.00	22.0	21.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
11801	XY	A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	23.0	15.0	.500	.313	-	-	-
11901	XY	A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11201	XY	A	-	-	-	-	-	-	5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	22.0	17.0	CP	CP	-	-	-
11301	XY	A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	-	-	-	-	.75	24.0	13.0	.438	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11501	XY	A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	1.00	22.0	11.0	.500	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11601	XY	A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	17.0	14.0	CP	.438	-	-	-
11002	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	10	-	-	1.00	17.0	17.0	CP	.313	1.00	19.0	22.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	-	.25
11802	XY	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	12	12	8	-	-	-	-	-	1.00	30.0	18.0	CP	CP	1.00	28.0	18.0	CP	CP	.75	19.0	15.0	CP	CP	-	.25	-
11902	XY	A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	8	.75	31.0	16.0	.438	.313	-	-	-	-	-	-	-	-	-	-	.50	18.0	15.0	CP	.500	-	-	-
11202	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	.75	19.0	14.0	.375	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11302	XY	A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	8	-	-	-	-	-	.50	19.0	14.0	.188	.188	-	-	-	-	-	.50	16.0	16.0	.500	.500	-	-	-
11402	XY	A	-	-	-	-	-	-	5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	6	4	-	-	-	-	-	.75	24.0	13.0	.438	.313	-	-	-	-	-	.75	11.0	12.0	CP	CP	-	-	-
11502	XY	A	4	.500	4.63	13.00	.313		5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	6	4	-	4	.75	24.0	13.0	.375	.250	.50	18.0	13.0	.250	.188	-	-	-	-	-	.50	17.0	13.0	.438	.313	-	-	-
11602	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	1.00	14.0	12.0	CP	CP	.50	14.0	14.0	.375	.375	-	-	-
11003	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	.75	16.0	20.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11803	XY	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	10	10	-	1.00	25.0	15.0	.375	.313	1.00	28.0	17.0	CP	.438	1.00	21.0	17.0	CP	CP	-	-	-	-	-	.25	-	-
11903	XY	A	7	.500	7.63	19.00	.375		5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	12	-	-	-	-	-	-	-	-	-	-	1.00	22.0	15.0	.500	.375	1.00	23.0	19.0	CP	CP	-	-	-
11203	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	-	.50	21.0	16.0	.375	.313	.50	18.0	13.0	.313	.250	-	-	-	-	-	-	-	-	-	-	-	-	-
11303	XY	A	5	.500	4.63	16.00	.313		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	.75	17.0	13.0	.375	.313	.50	18.0	14.0	.313	.250	.75	14.0	14.0	.500	.500	.50	14.0	14.0	.438	.438	-	-	-
11403	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	6	4	.50	23.0	12.0	.313	.188	.50	28.0	16.0	.313	.188	.50	14.0	14.0	.313	.313	.75	14.0	14.0	CP	.500	-	-	-
11503	XY	A	6	.500	4.63	19.00	.250		6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	6	.50	18.0	13.0	.250	.250	.50	18.0	14.0	.313	.250	.75	18.0	13.0	CP	.500	.50	22.0	17.0	.500	.375	-	-	-
11603	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	14.0	14.0	.313	.313	.50	14.0	14.0	.500	.500	-	-	-
11804	XY	A	5	.500	4.63	16.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	1.00	25.0	15.0	CP	.500	-	-	-	-	-	-	-	-	-	-	-	-	-
11904	XY	A	6	.500	4.63	19.00	.313		5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	10	-	-	-	-	-	-	.75	20.0	13.0	.438	.313	.75	20.0	17.0	CP	CP	-	-	-	-	-	-	-	-
11204	XY	A	7	.500	7.63	19.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	8	4	.50	18.0	13.0	.375	.313	-	-	-	-	-	.75	16.0	16.0	CP	CP	.50	15.0	13.0	.438	.375	-	-	-
11304	XY	A	6	.625	4.63	19.00	.313		5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	-	.75	19.0	14.0	.375	.313	.50	18.0	13.0	.313	.250	.50	14.0	14.0	.438	.375	-	-	-	-	-	-	-	-
11404	XY	A	5	.625	4.63	16.00	.313		6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	6	5	4	7	.75	29.0	16.0	.375	.250	.75	26.0	14.0	.375	.250	.75	14.0	14.0	.438	.438	.75	25.0	20.0	CP	.500	-	-	-
11504	XY	A	6	.625	4.63	19.00	.313		4	.625	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	4	6	4	.50	19.0	14.0	.375	.313	.50	18.0	13.0	.250	.250	.50	22.0	17.0	.375	.313	.50	17.0	13.0	.500	.375	-	-	-
11604	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	14.0	14.0	.438	.375	.50	14.0	14.0	.438	.438	-	-	-
2801	XY	A	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	-	-
2901	XY	A	8	.500	7.63	19.00	.375		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	4	8	-	1.00	15.0	16.0	.313	.313	1.25	12.0	13.0	CP	CP	1.00	16.0	16.0	CP	CP	-	-	-	-	-	-	-	-
2201	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	3	-	-	-	-	-	.50	13.0	17.0	.250	.250	.75	16.0	16.0	.438	.313	.50	10.0	19.0	.188	.188	.25	-	-
2301	XY	A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	7	-	-	5	.75	22.0	22.0	CP	.500	-	-	-	-	-	-	-	-	-	-	.75	15.0	25.0	.375	.438	-	.25	-
2401	XY	A	4	.625	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	-	.50	19.0	13.0	.375	.250	-	-	-	-	-	.50	15.0	16.0	.313	.250	-	-	-	-	-	-	-	-
2501	XY	A	5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	5	-	.50	16.0	13.0	.500	.313	-	-	-	-	-	.50	17.0	15.0	CP	.438	-	-	-	-	-	-	-	-
2601	XY	A	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	16.0	.250	.250	-	-	-	-	-	-	-	-
11130	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	3	4	-	-	.50	12.0	12.0	.188	.188	.50	21.0	12.0	.250	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11230	XY	A	7	.500	7.63	19.00	.313		6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	5	4	4	5	.75	17.0	20.0	.438	.500	.75	20.0	13.0	.250	.250	1.25	11.0	15.0	CP	CP	.75	19.0	15.0	CP	CP	.25	-	-
11330	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	14.0	16.0	.188	.188	.50	15.0	14.0	CP	.500	-	-	-
11131	XY	A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	5	4	-	-	.75	20.0	17.0	.500	.375	.75	16.0	13.0	.375	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11231	XY	A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	3	.75	17.0	16.0	.375	.313	.50	16.0	15.0	.188	.188	.50	16.0	13.0	.250	.188	.50	14.0	12.0	.188	.188	-	-	-
11331	XY	A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	17.0	15.0	.250	.188	.50	16.0	15.0	.250	.188	-	-	-
11132	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	3	3	-	-	.50	19.0	10.0	.188	.188	.50	19.0	10.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11232	XY	A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	4	1.00	20.0	13.0	.313	.313	1.00	19.0	12.0	.313	.313	1.00	13.0	12.0	CP	CP	1.00	13.0	12.0	CP	CP	-	-	-
11332	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	15.0	13.0	.188	.188	.50	15.0	13.0	.313	.250	-	-	-
11133	XY	A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	10	-	-	1.00	14.0	11.0	CP	.500	.75	21.0	20.0	CP	.438	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11233	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	3	3	.50	14.0	13.0	.375	.250	.50	15.0	13.0	.500	.313	.50	11.0	13.0	.188	.188	.50	12.0	13.0	.250	.188	-	-	-
11333	XY	A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	14.0	15.0	.188	.188	.50	15.0	16.0	.250	.250	-	-	-
11134	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	3	3	-	-	.50	19.0	10.0	.250	.188	.50	19.0	10.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11234	XY	A	6	.625	4.63	19.00	.313		8	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	4	10	10	4	1.25	20.0	13.0	.313	.313	1.25	25.0	17.0	.625	.500	1.25	20.0	17.0	CP	CP	1.25	13.0	12.0	CP	CP	-	.25	.25
11334	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	14.0	.375	.375	-	-	-	-	-	-	-	-
3001	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	-	-	-	-	-	.75	17.0	34.0	CP	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3101	XY	A	7	.500	7.63	19.00	.375		7	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	8	1.25	21.0	11.0	.625	.438	.75	23.0	12.0	.250	.250	-	-	-	-	-	.75	14.0	29.0	.375	CP	-	-	-
3201	XY	A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	4	-	-	-	-	-	-	-	-	-	-	.75	14.0	11.0	.250	.250	.75	17.0	14.0	CP	CP	-	-	-
3301	XY	A	7	.500	7.63	19.00	.375		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	12	-	-	-	-	-	-	-	-	-	-	.75	20.0	18.0	CP	CP	1.00	30.0	20.0	CP	CP	.25	.25	-
3501	XY	A	-	-	-	-	-	-	4	.625	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11005	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	-	1.00	15.0	33.0	CP	.438	1.25	12.0	28.0	CP	.563	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11105	XY	A	3	.375	4.63	21.00	.188	CP	8	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	4	4	10	16	1.00	22.0	13.0	.500	.375	1.00	21.0	12.0	.313	.313	1.00	16.0	34.0	.500	CP	1.25	22.0	33.0	CP	CP	.25	.25	.25
11205	XY	A	8	.500	7.63	19.00	.375		8	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	12	-	4	10	1.00	28.0	20.0	CP	CP	-	-	-	-	-	.75	16.0	13.0	.250	.250	.75	20.0	17.0	CP	CP	-	.25	-
11305	XY	A	4	.500	4.63	13.00	.250		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	16.0	16.0	CP	CP	-	-	-
11006	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	16	4	-	-	1.25	22.0	49.0	CP	.500	1.25	12.0	33.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11106	XY	A	7	.500	7.63	19.00	.375		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	10	-	4	16	1.25	22.0	19.0	CP	.500	-	-	-	-	-	1.50	11.0	28.0	.500	CP	1.25	22.0	37.0	CP	CP	-	.25	-
11206	XY	A	4	.500	4.63	13.00	.313		8	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	8	3	4	10	1.25	19.0	18.0	.500	.375	1.25	12.0	12.0	.313	.313	1.25	17.0	14.0	.313	.313	1.25	22.0	18.0	CP	CP	-	.25	-
11306	XY	A	4	.500	4.63	13.00	.250		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	24.0	21.0	CP	CP	-	.25	-
11506	XY	A	4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11007	XY	C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	16	8	-	-	1.25	22.0	44.0	CP	.313	1.00	15.0	33.0	C

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL QT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEL	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWER	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEU	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWED	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
11013 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	1.25	16.0	29.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
11113 XY A	6	.500	4.63	19.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	8	.75	18.0	15.0	CP	CP	1.00	13.0	11.0	CP	CP	-	-	-	-	-	.75	15.0	24.0	.438	CP	-	.25	-
11213 XY A	4	.500	4.63	13.00	.250		5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	7	4	4	-	-	-	-	-	.50	27.0	19.0	.438	.375	.50	15.0	16.0	.188	.188	.50	15.0	17.0	.250	.313	-	-	-
11313 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	14.0	.500	.438	-	-	-
11014 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	8	-	-	.75	16.0	29.0	CP	.250	.75	16.0	29.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11114 XY A	7	.500	7.63	19.00	.375		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	8	8	4	1.00	15.0	17.0	.313	.313	1.00	18.0	15.0	.438	.375	1.25	15.0	23.0	.625	CP	1.00	11.0	19.0	.313	.438	.25	-	-
11214 XY A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	10	4	4	.50	20.0	13.0	.313	.250	.50	24.0	17.0	.313	.250	1.00	14.0	15.0	.375	.438	.50	15.0	16.0	.188	.188	-	-	-
11314 XY A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	18	7	-	-	-	-	-	-	.75	34.0	25.0	CP	.500	.50	22.0	19.0	CP	.500	-	-	-	-	-	.50	-	.75
11414 XY A	-	-	-	-	-	-	8	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	-	7	-	5	-	-	-	-	-	.75	34.0	18.0	CP	.438	-	-	-	-	-	.75	18.0	16.0	CP	CP	-	.25	-
11514 XY A	-	-	-	-	-	-	4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	25.0	17.0	CP	.438	-	-	-
5001 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	1.00	12.0	22.0	CP	.313	.50	14.0	26.0	.375	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-
5101 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	-	8	8	.75	16.0	15.0	.250	.250	-	-	-	-	-	.75	16.0	21.0	.313	.375	.75	16.0	21.0	.438	.500	-	-	-
5201 XY A	4	.500	4.63	13.00	.250		4	.625	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	12	-	-	-	-	-	-	-	-	-	-	.50	15.0	18.0	.375	.438	.75	21.0	20.0	.500	.500	-	-	-
5301 XY A	4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	5	-	10	-	.75	25.0	15.0	.500	.375	-	-	-	-	-	.50	20.0	18.0	.375	.375	-	-	-	-	-	-	-	-
5401 XY A	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	18	-	.75	35.0	16.0	.438	.313	-	-	-	-	-	.75	28.0	25.0	CP	CP	-	-	-	-	-	.25	-	-
5501 XY A	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	7	-	-	-	-	-	-	-	-	-	-	-	.75	27.0	18.0	CP	CP	-	-	-	-	-	.25	-	-
11015 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	-	.75	16.0	29.0	CP	.250	.50	14.0	27.0	.438	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11115 XY A	5	.500	4.63	16.00	.313		5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	12	4	4	8	1.00	20.0	22.0	.438	.500	1.00	15.0	16.0	.313	.313	1.00	13.0	21.0	.313	.313	1.00	15.0	23.0	.500	CP	-	.25	-
11215 XY A	4	.500	4.63	13.00	.250		8	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	18.0	19.0	CP	CP	-	.25	-
11016 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	1.00	16.0	29.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11116 XY A	6	.625	4.63	19.00	.313		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	8	4	4	10	1.00	17.0	15.0	CP	CP	.75	15.0	17.0	.250	.250	.75	13.0	21.0	.250	.250	.75	17.0	26.0	.500	CP	.25	.25	-
11216 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	16.0	.188	.188	-	-	-	-	-	-	-	-
11017 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	4	-	-	1.00	18.0	31.0	CP	.313	1.00	12.0	24.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11117 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11217 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	15.0	16.0	.188	.188	.50	16.0	17.0	.188	.188	-	-	-
11018 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	.75	16.0	29.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11118 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	4	.50	15.0	17.0	.188	.188	.50	15.0	17.0	.188	.188	1.00	11.0	19.0	.375	.500	.50	13.0	21.0	.250	.313	-	-	-
11218 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	17.0	.188	.188	-	-	-
11019 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	12	-	-	1.00	14.0	28.0	CP	.313	1.00	19.0	34.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	.25	-
11119 XY A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	12	8	4	1.00	15.0	17.0	.313	.313	1.00	20.0	22.0	.313	.313	1.00	15.0	23.0	.438	CP	1.25	11.0	19.0	.438	.563	-	-	-
11219 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	10	4	-	-	-	-	-	-	.50	24.0	17.0	.375	.313	.50	15.0	17.0	.188	.188	-	-	-	-	-	-	-	-
11319 XY A	-	-	-	-	-	-	4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6001 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	1.25	12.0	21.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
6101 XY A	3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	8	12	-	-	-	-	-	-	.75	20.0	16.0	.438	.438	.75	21.0	24.0	CP	CP	-	-	-	-	-	.25	-	-
6201 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	3	-	-	-	-	-	-	-	-	-	-	.50	24.0	20.0	.313	.313	.50	12.0	11.0	.188	.188	-	-	-
6301 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	4	-	-	-	-	-	-	-	-	-	-	.50	19.0	18.0	.438	.438	.50	12.0	12.0	.438	.438	-	-	-
11120 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	3	3	-	-	.50	12.0	13.0	.188	.188	.50	12.0	13.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-
11220 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	4	8	4	.75	18.0	11.0	.250	.250	.50	20.0	13.0	.188	.188	.75	18.0	19.0	.375	.438	.50	16.0	17.0	.313	.313	-	-	-
11320 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	14.0	.188	.188	-	-	-
11121 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	.50	15.0	14.0	.438	.375	.50	15.0	17.0	.250	.250	-	-	-	-	-	-	-	-	-	-	-	-	-
11221 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	-	3	4	.50	20.0	13.0	.188	.188	-	-	-	-	-	.50	11.0	13.0	.188	.188	.50	15.0	17.0	.188	.188	-	-	-
11321 XY A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	14.0	.313	.250	-	-	-	-	-	-	-	-
11122 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	4	3	-	-	.50	16.0	17.0	.188	.188	.50	12.0	13.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-
11222 XY A	4	.500	4.63	13.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	8	-	-	-	-	-	.50	20.0	13.0	.313	.250	.50	16.0	17.0	.250	.250	.50	18.0	19.0	.313	.313	-	-	-
11322 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	14.0	.188	.188	-	-	-
11123 XY A	4	.500	4.63	13.00	.313		6	.500	4.63	19.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	12	.75	17.0	15.0	.500	.438	1.00	13.0	11.0	CP	CP	-	-	-	-	-	.75	20.0	31.0	.375	.500	-	-	-
11223 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	3	4	.50	20.0	13.0	.188	.188	.50	19.0	13.0	.250	.188	.50	11.0	13.0	.188	.188	.50	15.0	17.0	.188	.188	-	-	-
11323 XY A	7	.500	7.63	19.00	.313		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	5	4	3	-	-	-	-	-	.75	22.0	15.0	CP	.438	.50	15.0	13.0	.438	.375	.50	13.0	11.0	.188	.188	-	-	-
11423 XY A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	13.0	.375	.313	-	-	-
11523 XY A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	-	-	-	-	-	.50	13.0	17.0	.188	.188	-	-	-	-	-	.75	19.0	13.0	CP	.375	-	-	-
11623 XY A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11723 XY A	-	-	-	-	-	-	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	16.0	.188	.188	-	-	-
11024 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	12	8	-	-	1.00	20.0	37.0	CP	.313	1.00	16.0	29.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11124 XY A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	8	.75	15.0	14.0	.250	.250	-	-	-	-	-	-	-	-	-	-	.75	15.0	24.0	.438	CP	-	-	-
11224 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	3	4	4	-	.75	16.0	10.0	.250	.250	.75	20.0	13.0	.313	.250	1.00	13.0	15.0	.375	.375	-	-	-	-	-	-	.25	-
11324 XY A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	8	4	3	.50	20.0	13.0	.250	.188	1.00	22.0	15.0	CP	.500	.50	15.0	13.0	.375	.313	.50	13.0	11.0	.188	.188	-	-	-
11424 XY A	7	.500	7.63	19.00	.375		5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	14	5	-	.75	27.0	12.0	.375	.250	1.00	37.0	20.0	CP	.500	.75	18.0	16.0	CP	CP	-	-	-	-	-	-	-	-
11524 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	.50	14.0	14.0	.375	.313	-	-	-	-	-	-	-	-	-	-	-	-	-
11624 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	-	.50	15.0	16.0	.188	.188	.50	14.0	15.0	.188	.188	.50	13.0	17.0	.188	.188	-	-	-	-	-	-	-	-
11724 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	12.0	13.0	.188	.188	-	-	-
7001 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	.75	16.0	25.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
7101 XY A	6	.500	4.63	19.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	8	-	-	-	-	-	-	1.00	18.0	14.0	.313	.313	1.00	18.0	19.0	CP	CP	-	-	-	-	-	.25	-	-
7201 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	4	.50	23.0	10.0	.188	.188	-	-	-	-	-	-	-	-	-	-	.50	14.0	14.0	.375	.500	-	-	-
7301 XY A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	6	-	-	-	-	-	-	-	-	-	-	1.00	17.0	14.0	.438	.375	1.00	15.0	14.0	CP	CP	-	-	-
7401 XY A	6	.500	4.63	19.00	.313		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	10	-	-	-	-	-	-	-	-	-	-	1.00	18.0	16.0	CP	CP	1.00	20.0	18.0	CP	CP	-	.25	-
7501 XY A	3	.375	4.63	21.00	.188	CP	4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	14	-	-	-	-	-	-	.75	15.0	15.0	.313	.313	.75	29.0	20.0	CP	CP	-	-	-	-	-	-	-	-
7601 XY A	4	.500	4.63	13.00	.250		6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	3	3	4	8	.75	20.0	11.0	.250	.250	.75	20.0	11.0	.250	.250	.75	15.0	14.0	.250	.438	.75	16.0	16.0	CP	CP	-	.25	.25
7701 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	-	-	-	-	.50	16.0	14.0	.188	.250	.50	16.0	14.0	.313	.375	-	-	-
11125 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	.50	15.0	14.0	.438	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11225 XY A	6	.625	4.63	19.00	.313		5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	6	-	4	8	1.00	20.0	14.0	CP	.438	-	-	-	-	-	.75	15.0	17.0	.250	.250	.75	18.0	19.0	.500	.500	-	-	-
11325 XY A	3	.375	4.63	21.00	.188	CP	7	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	8	1.25	24.0	17.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	16.0	16.0	CP	CP	.25	.25	-
11425 XY A	3	.375	4.63	21.00	.188	CP	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	8	-	-	-	-	-	.75	26.0	12.0	.250	.250	-	-	-	-	-	.75	17.0	16.0	CP	CP	-	-	-
11525 XY A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	6	.75	16.0	15.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.75	24.0	17.0	CP	CP	.25	-	-
11625 XY A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	3	4	-	.50	15.0	16.0	.250	.250	.50	12.0	12.0	.188	.250	.50	13.0	17.0	.313	.313	-	-	-	-	-	-	-	-
11725 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	4	-	-	-	-	-	-	-	-	-	-	.50	12.0	12.0	.188	.188	.75	16.0	16.0	.375	.375	-	-	-
11026 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	-	-	-	-	1.00	14.0	26.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11126 XY A	6	.500	4.63	19.00	.313		7	.500	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	8	1.25	18.0	15.0	CP	.625	-	-	-	-	-	-	-	-	-	-	1.25	15.0	23.0	CP	CP	-	.25	-
11226 XY A	6	.625	4.63	19.00	.313		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	8	1.00	23.0	15.0	CP	CP	-	-	-	-	-	-	-	-	-	-	1.00	18.0	20.0	CP	CP	-	.25	-
11326 XY A	5	.625	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	.75	23.0	15.0	.438	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11426 XY A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	6	-	-	-	.75	32.0	16.0	CP	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11526 XY A	4	.500	4.63	13.00	.250		5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	5	-	-	-	-	-	.75	13.0	14.0	.313	.313	.75	19.0	12.0	.375	.250	.75	21.0	15.0	CP	.500	-	-	-
11626 XY A	4	.500	4.63	13.00	.250		4	.625	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	5	.75	16.0	17.0	.375	.375	.75	15.0	16.0	.250	.250	-	-	-	-	-	.75	16.0	21.0	.438	.500	-	-	-
11726 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	-	-	-	-	.50	12.0	13.0	.188	.250	-	-	-	-	-	-	-	-
11027 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	-	1.25	16.0	29.0	CP	.313	.75	14.0	28.0	CP	.250	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
11127 XY A	8	.625	7.63	19.00	.375		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	8	4	6	6	1.00	18.0	15.0	CP	CP	1.00	15.0	17.0	.313	.313	1.00	13.0	21.0	.438	CP	1.00	13.0	21.0	.500	CP	.25	.25	-
11227 XY A	4	.500	4.63	13.00	.250		4	.625	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	-	-	-	-	-	.50	19.0	13.0	.250	.188	-	-	-	-	-	.75	15.0	17.0	.313	.375	.25	.25	-
11327 XY A	4	.500	4.63	13.00	.250		5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	4	-	-	-	-	-	.50	20.0	13.0	.375	.250	-	-	-	-	-	.50	15.0	14.0	CP	.500	-	-	-
11427 XY A	5	.500	4.63	16.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	5	4	-	5	.75	29.0	14.0	.500	.313	.50	26.0	12.0	.313	.188	-	-	-	-	-	.50	18.0	16.0	CP	.500	-	-	-
11527 XY A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	5	-	-	-	.75	16.0	16.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11627 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	4	4	-	-	-	-	-	-	.50	15.0	16.0	.250	.250	.50	13.0	17.0	.188	.250	-	-	-	-	-	-	-	-
11727 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	16.0	.188	.188	-	-	-	-	-	-	-	-
11028 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	6	4	-	-	1.25	14.0	26.0	CP	.313	1.25	12.0	24.0	CP	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-
11128 XY A	5	.500	4.63	16.00	.313		4	.500	4.63	13.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	4	4	4	6	1.00	15.0	17.0	.375	.375	1.00	15.

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL OT	LEFT BEAM SHEAR PLATE NBL SPTL SPWL SPLL WLDL BWER	RIGHT BEAM SHEAR PLATE NBR SPTR SPWR SPLR WLDR BWER	UPPER COLUMN SHEAR PLATE NBU SPTU SPWU SPLU WLDU CWER	LOWER COLUMN SHEAR PLATE NBD SPTD SPWD SPLD WLDD CWER	BRACE BOLTS BUL BUR BLL BLR	UP-LEFT GUSSET PLATE GTUL GWUL GLUL GWCUL GWBUL	UP-RIGHT GUSSET PLATE GTUR GWUR GLUR GWCUR GWBUR	DN-LEFT GUSSET PLATE GTLL GWLL GLLL GWCLL GWBLL	DN-RIGHT GUSSET PLATE GTLR GWLR GLLR GWCLR GWBLR	DOUBLER PLATES GBRL GBRR DBRC
GRID H										
3027 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- 16 - -	- - - - - -	1.25 22.0 37.0 CP .313	- - - - - -	- - - - - -	- - -
3127 XY A	- - - - - -	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- 10 - 10	- - - - - -	.75 19.0 22.0 .500 CP	- - - - - -	.50 18.0 23.0 CP CP	- .25 -
3227 XY A	- - - - - -	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- 3 - 10	- - - - - -	1.00 18.0 10.0 .313 .313	- - - - - -	1.00 19.0 22.0 CP CP	- .25 -
3327 XY A	- - - - - -	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- - - 10	- - - - - -	- - - - - -	- - - - - -	1.00 19.0 18.0 CP CP	- .25 -
3427 XY A	- - - - - -	6 .625 4.63 19.00 .313	- - - - - -	- - - - - -	- 4 - 12	- - - - - -	.75 29.0 12.0 .313 .250	- - - - - -	1.00 29.0 20.0 CP .500	- - -
3527 XY A	- - - - - -	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- - - 12	- - - - - -	- - - - - -	- - - - - -	.75 26.0 19.0 CP CP	- .75 -
17001 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	10 10 - -	1.00 18.0 34.0 CP .313	1.00 18.0 34.0 CP .313	- - - - - -	- - - - - -	- - -
17101 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	10 8 16 10	1.25 18.0 21.0 CP CP	1.25 16.0 19.0 CP CP	1.25 22.0 37.0 CP CP	1.25 17.0 30.0 CP CP	.50 .25 .25
17201 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	10 8 10 12	1.00 21.0 20.0 CP CP	1.00 19.0 18.0 CP CP	1.00 17.0 29.0 .500 CP	1.00 19.0 31.0 CP CP	.25 .25 -
17301 XY A	8 .625 7.63 19.00 .375	6 .625 4.63 19.00 .313	- - - - - -	- - - - - -	12 4 3 6	1.00 22.0 20.0 CP CP	.75 17.0 16.0 .313 .250	.75 13.0 12.0 .250 .250	.75 21.0 19.0 CP .500	.25 - -
17401 XY A	6 .625 4.63 16.00 .313	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	12 - - 4	1.00 29.0 18.0 CP .500	- - - - - -	- - - - - -	.75 16.0 15.0 .438 .313	- - -
17501 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- - 4 4	- - - - - -	- - - - - -	.50 16.0 13.0 .500 .313	.50 16.0 13.0 CP .375	.75 .50 -
17002 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	10 8 - -	1.25 18.0 30.0 CP .313	.75 16.0 28.0 CP .250	- - - - - -	- - - - - -	- - -
17102 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	12 8 10 12	1.00 20.0 20.0 CP CP	1.00 16.0 15.0 CP CP	1.00 17.0 24.0 CP CP	1.25 19.0 27.0 CP CP	.25 .25 -
17202 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	6 - 8 12	.75 24.0 17.0 CP .500	- - - - - -	1.25 16.0 21.0 CP CP	.75 20.0 26.0 .375 .438	.25 - -
17302 XY A	8 .625 7.63 19.00 .375	6 .625 4.63 19.00 .313	- - - - - -	- - - - - -	4 12 8 -	1.00 20.0 13.0 .438 .313	1.25 27.0 19.0 CP CP	1.00 16.0 16.0 CP CP	- - - - - -	.25 - -
17402 XY A	6 .500 4.63 19.00 .313	6 .625 4.63 19.00 .313	- - - - - -	- - - - - -	4 4 4 3	.50 25.0 12.0 .375 .188	.75 26.0 12.0 .438 .313	.50 15.0 14.0 .438 .375	.75 17.0 16.0 CP CP	- - -
17502 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- - - 6	- - - - - -	- - - - - -	- - - - - -	.75 24.0 17.0 CP .438	.50 .50 -
17003 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	12 12 - -	1.25 20.0 36.0 CP .313	1.00 20.0 39.0 CP .313	- - - - - -	- - - - - -	- - .25
17103 XY A	6 .625 4.63 19.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	12 - 8 -	.75 20.0 23.0 CP CP	- - - - - -	.75 15.0 28.0 .375 .500	- - - - - -	- - -
17203 XY A	5 .625 4.63 16.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- 4 8 -	- - - - - -	1.00 17.0 16.0 .500 .438	1.00 15.0 26.0 CP CP	- - - - - -	.25 - -
17303 XY A	7 .500 7.63 19.00 .375	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	5 - - 14	.75 19.0 17.0 .500 .375	- - - - - -	- - - - - -	.75 26.0 24.0 CP CP	- .25 -
17403 XY A	3 .375 4.63 21.00 .188 CP	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	6 8 12 -	1.25 26.0 16.0 .563 .375	1.25 25.0 14.0 .625 .375	1.25 23.0 21.0 CP CP	- - - - - -	.50 - -
17503 XY A	5 .500 4.63 16.00 .313	5 .500 4.63 16.00 .250	- - - - - -	- - - - - -	- - 4 -	- - - - - -	- - - - - -	.75 16.0 13.0 CP .500	- - - - - -	- - -
17104 XY A	7 .500 7.63 19.00 .375	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- 10 12 8	- - - - - -	1.00 21.0 20.0 CP CP	1.00 20.0 27.0 CP CP	1.00 18.0 19.0 CP CP	.25 .25 .25
17204 XY A	8 .500 7.63 19.00 .375	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	14 - - 12	.75 30.0 21.0 CP CP	- - - - - -	- - - - - -	.75 23.0 22.0 .500 .500	- - -
17304 XY A	5 .500 4.63 16.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 4 -	- - - - - -	- - - - - -	.75 15.0 14.0 CP CP	- - - - - -	- - -
17404 XY C	- - - - - -	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	- 5 - -	- - - - - -	.50 37.0 13.0 .313 .188	- - - - - -	- - - - - -	- - -
17504 XY C	- - - - - -	- - - - - -	- - - - - -	6 .625 4.63 19.00 .313	- - 8 6	- - - - - -	- - - - - -	1.00 21.0 15.0 CP CP	1.00 22.0 13.0 CP CP	- - .25
17005 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	8 20 - -	1.25 16.0 30.0 CP .313	1.50 24.0 50.0 CP .563	- - - - - -	- - - - - -	- - .25
17105 XY A	5 .500 4.63 16.00 .250	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	12 12 - 8	1.25 20.0 21.0 CP .625	1.25 21.0 33.0 CP CP	- - - - - -	1.50 15.0 32.0 CP CP	- .25 -
17205 XY A	6 .625 4.63 19.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- 10 4	- - - - - -	- - - - - -	1.00 18.0 27.0 CP CP	1.25 11.0 26.0 .500 CP	.25 - -
17305 XY A	4 .500 4.63 13.00 .250	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -
17405 XY A	6 .500 4.63 19.00 .313	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	6 8 - -	1.25 25.0 12.0 CP .438	1.00 18.0 15.0 CP CP	- - - - - -	- - - - - -	- - -
17505 XY A	5 .500 4.63 16.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 5 4	- - - - - -	- - - - - -	.50 21.0 14.0 CP .313	.50 15.0 16.0 .375 .313	- - -
4027 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	8 - - -	1.25 15.0 29.0 CP .313	- - - - - -	- - - - - -	- - - - - -	- - -
4127 XY A	3 .375 4.63 21.00 .188 CP	8 .625 7.63 19.00 .375	- - - - - -	- - - - - -	4 12 20 -	1.25 12.0 12.0 CP CP	1.25 20.0 22.0 CP CP	1.25 24.0 37.0 CP CP	- - - - - -	.50 .25 -
4227 XY A	6 .625 4.63 19.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 12 -	- - - - - -	- - - - - -	1.00 21.0 30.0 .500 CP	- - - - - -	.25 - -
4327 XY A	4 .500 4.63 13.00 .250	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	- - - 8	- - - - - -	- - - - - -	- - - - - -	.75 29.0 19.0 CP .500	- - -
4427 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	4 6 - 10	1.00 23.0 12.0 .313 .313	1.00 24.0 15.0 .375 .313	- - - - - -	1.00 20.0 18.0 CP CP	.25 .25 -
4527 XY A	4 .500 4.63 13.00 .313	7 .500 7.63 19.00 .313	- - - - - -	- - - - - -	- - 8 6	- - - - - -	- - - - - -	1.25 18.0 16.0 CP CP	1.25 19.0 14.0 CP CP	.25 .25 -
17106 XY C	- - - - - -	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	- 3 - -	- - - - - -	.50 11.0 11.0 .188 .188	- - - - - -	- - - - - -	- - -
17206 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	8 4 12 16	1.25 27.0 19.0 .625 .500	1.25 20.0 13.0 .500 .375	1.25 20.0 25.0 CP CP	1.25 22.0 27.0 CP CP	.25 .25 .25
17306 XY A	3 .375 4.63 21.00 .188 CP	8 .625 7.63 19.00 .375	- - - - - -	- - - - - -	10 - - 10	1.00 25.0 17.0 CP CP	- - - - - -	- - - - - -	1.00 19.0 18.0 CP CP	.25 .25 -
17406 XY A	6 .500 4.63 19.00 .313	4 .500 4.63 13.00 .313	- - - - - -	- - - - - -	6 - - 8	1.25 26.0 13.0 CP .500	- - - - - -	- - - - - -	.75 17.0 16.0 CP CP	- - -
17506 XY A	4 .500 4.63 13.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 6 4	- - - - - -	- - - - - -	.50 22.0 15.0 .438 .250	.50 20.0 13.0 .313 .188	- - -
17007 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- 4 - -	- - - - - -	.75 14.0 27.0 CP .250	- - - - - -	- - - - - -	- - -
17107 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	16 - - 16	1.25 22.0 28.0 CP CP	- - - - - -	- - - - - -	1.50 22.0 30.0 CP CP	.25 .25 -
17207 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	10 4 3 12	1.00 25.0 17.0 CP CP	1.00 20.0 13.0 .438 .313	1.00 11.0 15.0 .313 .313	1.00 20.0 25.0 CP CP	.25 .25 -
17307 XY A	7 .500 7.63 19.00 .375	6 .500 4.63 19.00 .250	- - - - - -	- - - - - -	8 4 4 5	.75 22.0 15.0 CP .438	.75 20.0 13.0 .375 .250	.75 15.0 14.0 CP CP	.75 17.0 16.0 CP CP	- - -
17407 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	4 4 - 4	.50 20.0 13.0 .313 .188	.50 26.0 12.0 .188 .188	- - - - - -	.50 15.0 13.0 .375 .313	- - -
17507 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - 3	- - - - - -	- - - - - -	- - - - - -	.50 16.0 11.0 .313 .188	- - -
17008 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	16 - - -	1.50 22.0 38.0 CP .313	- - - - - -	- - - - - -	- - - - - -	- - .25
17108 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	12 3 4 16	1.00 20.0 26.0 CP CP	1.00 11.0 12.0 .313 .313	1.00 13.0 20.0 .313 .438	1.25 22.0 30.0 CP CP	.25 .25 -
17208 XY A	6 .625 4.63 19.00 .313	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	5 - - 12	1.25 22.0 15.0 .563 .438	- - - - - -	- - - - - -	1.25 20.0 25.0 CP CP	- .25 -
17308 XY A	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	- - - - - -	4 - 4 -	.75 19.0 13.0 .250 .250	- - - - - -	.75 15.0 14.0 CP .500	- - - - - -	- - -
17408 XY A	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - - - -	3 - 4 -	.50 22.0 10.0 .188 .188	- - - - - -	.50 15.0 14.0 .500 .438	- - - - - -	- - -
17508 XY A	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - - - -	- - 4 -	- - - - - -	- - - - - -	.50 18.0 13.0 .313 .188	- - - - - -	- - -
17009 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	16 6 - -	1.25 22.0 38.0 CP .313	.75 14.0 25.0 CP .250	- - - - - -	- - - - - -	- - .50
17109 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	12 - - 8	1.50 20.0 20.0 CP CP	- - - - - -	- - - - - -	1.25 16.0 22.0 CP CP	.25 .25 .25
17209 XY A	6 .500 4.63 19.00 .313	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	- - 3 -	- - - - - -	- - - - - -	.50 11.0 15.0 .188 .188	- - - - - -	- - -
17010 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	8 10 - -	1.25 16.0 28.0 CP .313	1.00 18.0 30.0 CP .313	- - - - - -	- - - - - -	- - .25
17110 XY A	4 .500 4.63 13.00 .250	7 .500 7.63 19.00 .313	- - - - - -	- - - - - -	- 12 6 4	- - - - - -	1.00 20.0 25.0 CP CP	1.00 14.0 19.0 .438 .500	1.00 12.0 17.0 .375 .500	- .25 -
17210 XY A	5 .500 4.63 16.00 .313	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -
5027 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	4 - - -	1.00 12.0 22.0 CP .313	- - - - - -	- - - - - -	- - - - - -	- - -
5127 XY A	6 .500 4.63 19.00 .313	- - - - - -	- - - - - -	- - - - - -	- - 10 -	- - - - - -	- - - - - -	.50 18.0 23.0 CP CP	- - - - - -	.25 - -
5227 XY A	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	- - - - - -	- - 12 -	- - - - - -	- - - - - -	1.00 20.0 23.0 CP CP	- - - - - -	.25 - -
17011 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- 20 - -	- - - - - -	1.50 24.0 40.0 CP .375	- - - - - -	- - - - - -	- - .75
17111 XY A	- - - - - -	6 .500 4.63 19.00 .313	- - - - - -	- - - - - -	- 6 - -	- - - - - -	1.25 14.0 13.0 CP CP	- - - - - -	- - - - - -	- .25 .25
17211 XY A	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - 4	- - - - - -	- - - - - -	- - - - - -	.50 14.0 18.0 .188 .188	- - -
17112 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	4 12 20 -	1.50 14.0 14.0 .313 .313	1.50 20.0 20.0 CP CP	1.50 24.0 32.0 CP CP	- - - - - -	.50 .25 .25
17212 XY A	5 .500 4.63 16.00 .313	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	- 5 6 4	- - - - - -	.75 22.0 15.0 CP .438	1.25 14.0 19.0 .625 CP	.50 13.0 18.0 .188 .188	.25 - -
17312 XY A	- - - - - -	5 .625 4.63 16.00 .313	- - - - - -	- - - - - -	- 4 - 4	- - - - - -	.75 19.0 12.0 .250 .250	- - - - - -	.75 15.0 14.0 CP .500	- - -
17412 XY A	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - 4	- - - - - -	- - - - - -	- - - - - -	.75 15.0 14.0 CP .500	- - -
17512 XY A	- - - - - -	5 .625 4.63 16.00 .313	- - - - - -	- - - - - -	- 4 - 4	- - - - - -	.75 13.0 17.0 .375 CP	- - - - - -	.75 21.0 14.0 CP .375	- - -
17612 XY A	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -
17113 XY C	- - - - - -	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	4 3 - -	.50 14.0 13.0 .250 .250	.50 11.0 11.0 .313 .313	- - - - - -	- - - - - -	- - -
17213 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	4 7 12 8	1.25 20.0 13.0 .438 .313	1.25 27.0 19.0 .375 .313	1.25 20.0 25.0 CP CP	1.25 17.0 21.0 CP CP	.25 .25 .25
17313 XY A	7 .500 7.63 19.00 .375	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	4 5 5 6	1.00 20.0 13.0 .375 .313	1.00 22.0 14.0 .313 .313	1.00 18.0 16.0 CP CP	1.25 14.0 14.0 CP CP	- .25 -
17413 XY A	5 .500 4.63 16.00 .313	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	4 4 4 4	.75 21.0 14.0 CP .375	.75 26.0 12.0 .250 .250	.75 15.0 13.0 .313 .250	.75 15.0 14.0 CP CP	- - -
17513 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -
17613 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .313	- - - - - -	- - - - - -	- - 4 10	- - - - - -	- - - - - -	.50 12.0 23.0 .188 .313	.75 17.0 29.0 .313 .438	- - -
17014 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- 10 - -	- - - - - -	1.25 18.0 30.0 CP .313	- - - - - -	- - - - - -	- - -
17114 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	8 6 - 10	1.25 14.0 13.0 CP CP	1.25 14.0 13.0 CP CP	- - - - - -	1.25 17.0 24.0 CP CP	.25 .25 -
17214 XY A	7 .500 7.63 19.00 .313	7 .500 7.63 19.00 .313	- - - - - -	- - - - - -	6 6 3 8	1.25 20.0 14.0 CP .563	1.00 19.0 14.0 CP .500	.75 10.0 15.0 .250 .250	.75 16.0 22.0 .438 .500	- - -
17314 XY A	8 .625 7.63 19.00 .375	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	4 10 7 12	.75 20.0 13.0 CP .438	.75 25.0 17.0 CP .438	.50 22.0 19.0 .438 .438	.75 21.0 19.0 CP CP	- .25 -
17414 XY A	4 .500 4.63 19.00 .313	6 .500 4.63 19.00 .313	- - - - - -	- - - - - -	- 5 5 4	- - - - - -	.75 29.0 14.0 .500 .313	.50 17.0 15.0 .375 .375	.50 15.0 13.0 .438 .375	- - -
17514 XY A	6 .625 4.63 19.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	10 - 4 4	.50 17.0 21.0 CP CP	- - - - - -	.50 19.0 13.0 .438 .250	.50 19.0 13.0 .375 .250	- - -
17614 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -
7027 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	10 6 - -	1.00 18.0 29.0 CP .313	1.00 14.0 27.0 CP .375	- - - - - -	- - - - - -	- - -
7127 XY A	3 .375 4.63 21.00 .188 CP	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	8 4 10 12	.75 18.0 15.0 CP CP	.75 14.0 15.0 CP CP	1.00 19.0 22.0 CP CP	.75 18.0 29.0 CP CP	.25 .25 -
7227 XY A	8 .625 7.63 19.00 .375	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	12 - 6 10	.75 29.0 19.0 .500 .438	- - - - - -	1.00 16.0 17.0 .500 CP	.75 17.0 25.0 CP CP	.25 .25 -
7327 XY A	7 .500 7.63 19.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	4 - 6 -	1.00 22.0 12.0 .313 .313	- - - - - -	1.00 15.0 14.0 CP CP	- - - - - -	.25 - -
7427 XY A	7 .500 7.63 19.00 .375	3 .375 4.63 21.00 .188 CP	- - - - - -	- - - - - -	4 4 10 -	.75 29.0 12.0 .250 .250	.75 22.0 12.0 .438 .313	.75 20.0 18.0 CP CP	- - - - - -	- .25 -
7527 XY A	5 .500 4.63 16.00 .313	8 .500 7.63 19.00 .375	- - - - - -	- - - - - -	- - 5 8	- - - - - -	- - - - - -	1.00 22.0 15.0 CP .500	1.00 18.0 16.0 CP CP	- .25 -
7627 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- 3 - -	- - - - - -	.50 18.0 10.0 .188 .188	- - - - - -	- - - - - -	- - -
7727 XY A	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - 4	- - - - - -	- - - - - -	- - - - - -	.50 18.0 14.0 .188 .188	- - -
17015 XY C	- - - - - -	4 .500 4.63 13.00 .313	- - - - - -	- - - - - -	12 10 - -	1.25 19.0 40.0 CP .625	.75 18.0 30.0 CP .250	- - - - - -	- - - - - -	- - .25
17115 XY A	3 .375 4.63 21.00 .188 CP	5 .625 4.63 16.00 .313	- - - - - -	- - - - - -	10 12 6 -	1.25 17.0 25.0 CP CP	1.00 20.0 20.0 CP CP	1.50 13.0 33.0 .375 .563	- - - - - -	.25 - -
17215 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 4 8	- - - - - -	- - - - - -	1.00 13.0 31.0 .313 .438	1.00 16.0 21.0 CP CP	- .25 -
17315 XY A	3 .375 4.63 21.00 .188 CP	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -
17415 XY A	6 .625 4.63 19.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	8 4 - -	1.00 19.0 18.0 CP CP	.50 28.0 12.0 .375 .188	- - - - - -	- - - - - -	.25 - -
17515 XY A	5 .500 4.63 16.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 4 4	- - - - - -	- - - - - -	.50 16.0 14.0 .438 .375	.50 19.0 12.0 CP .375	- - -
17615 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	4 4 - -	.50 14.0 13.0 .250 .188	.50 20.0 12.0 .188 .188	- - - - - -	- - - - - -	- - -
17715 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 3 6	- - - - - -	- - - - - -	.50 11.0 14.0 .188 .188	.50 21.0 17.0 .500 .375	- - -
17016 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -
17116 XY A	3 .375 4.63 21.00 .188 CP	7 .500 7.63 19.00 .313	- - - - - -	- - - - - -	8 - 10 10	1.00 19.0 18.0 CP CP	- - - - - -	.75 20.0 21.0 .438 .438	.75 21.0 27.0 CP CP	.25 .25 -
17216 XY A	3 .375 4.63 21.00 .188 CP	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	- 7 12 -	- - - - - -	.75 27.0 19.0 CP .500	.75 23.0 22.0 .500 .500	- - - - - -	- - -
17316 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .313	- - - - - -	- - - - - -	- - - 6	- - - - - -	- - - - - -	- - - - - -	.50 20.0 18.0 CP CP	- - -
17416 XY C	- - - - - -	- - - - - -	4 .500 4.63 13.00 .250	- - - - - -	4 - - -	.50 26.0 11.0 CP .313	- - - - - -	- - - - - -	- - - - - -	- - -
17516 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- 5 4 4	- - - - - -	.50 15.0 20.0 .313 .500	.50 23.0 13.0 CP .313	.50 20.0 13.0 .250 .188	- - -
17616 XY A	5 .500 4.63 16.00 .250	6 .500 4.63 19.00 .313	- - - - - -	- - - - - -	6 - - 10	1.00 32.0 16.0 .313 .313	- - - - - -	- - - - - -	1.00 17.0 29.0 .500 CP	- .25 -
17716 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - 4 3	- - - - - -	- - - - - -	.50 19.0 13.0 .313 .188	.50 13.0 11.0 .188 .188	- - -
17017 XY C	- - - - - -	- - - - - -	- - - - - -	- - - - - -	10 8 - -	1.25 21.0 36.0 CP .313	1.25 16.0 28.0 CP .313	- - - - - -	- - - - - -	- - .25
17117 XY A	4 .500 4.63 13.00 .250	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	- 10 - 4	- - - - - -	1.25 18.0 18.0 CP .625	- - - - - -	1.25 12.0 17.0 .500 .625	- .25 -
17217 XY A	5 .500 4.63 16.00 .313	6 .625 4.63 19.00 .313	- - - - - -	- - - - - -	6 - - 6	1.25 25.0 17.0 .438 .375	- - - - - -	- - - - - -	1.25 14.0 19.0 .625 CP	- .25 -
17317 XY A	6 .500 4.63 19.00 .313	5 .500 4.63 16.00 .313	- - - - - -	- - - - - -	- 7 7 -	- - - - - -	.75 27.0 19.0 .375 .313	.75 22.0 19.0 CP CP	- - - - - -	- - -
17417 XY A	4 .500 4.63 13.00 .250	7 .500 7.63 19.00 .375	- - - - - -	- - - - - -	4 3 - 10	.75 20.0 13.0 .250 .250	.75 23.0 10.0 .313 .250	- - - - - -	1.00 19.0 18.0 CP CP	- .25 -
17517 XY A	7 .500 7.63 19.00 .313	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	10 - - 4	1.00 17.0 21.0 CP CP	- - - - - -	- - - - - -	.75 19.0 13.0 CP .500	.25 - -
17617 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	3 3 5 -	.50 18.0 10.0 .188 .188	.50 18.0 10.0 .188 .188	.50 14.0 26.0 .188 .250	- - - - - -	- - -
17717 XY A	4 .500 4.63 13.00 .250	4 .500 4.63 13.00 .250	- - - - - -	- - - - - -	- - - -	- - - - - -	- - - - - -	- - - - - -	- - - - - -	- - -

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

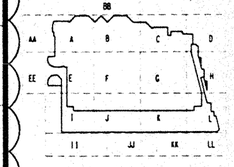
Consultants

Skilling Ward Magnusson Barkshire Inc.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 623-7201

LICENSED PROFESSIONAL ENGINEER NO. 7177S HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervising engineer in Title III, Chapter 16 Section 1.24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	6-19-95		

Sheet Title

CONNECTION SCHEDULE

Designed BJM
Drawn DMS
Checked JDM
Date 5 / 8 / 95
Job No. 93036.02.9
Sheet No.

S6.14

DWG: pit jjxhccbrfrsc10.dgn Thu Nov 30 14:08:07 1995

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL OT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEL	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWER	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEU	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWED	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE GTLR	GWLR	GLLR	GWCLR	GWBLR	DOUBLER PLATES GBRL	GBRR	DBRC
17018 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	8	-	-	1.25	12.0	23.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17118 XY A	3	.375	4.63	21.00	.188	CP	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	6	10	8	8	1.25	14.0	13.0	CP	CP	.75	18.0	18.0	CP	CP	1.25	16.0	22.0	CP	CP	1.00	16.0	22.0	.500	CP	.25	.25	.25
17218 XY A	4	.625	4.63	13.00	.313		8	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	-	5	10	8	-	-	-	-	-	.75	22.0	15.0	.500	.375	.75	19.0	23.0	.438	.500	.75	16.0	21.0	.500	CP	-	.25	-
17318 XY A	7	.625	7.63	19.00	.375		6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	10	4	-	5	1.00	25.0	17.0	CP	CP	.75	20.0	13.0	.375	.250	-	-	-	-	-	.75	17.0	16.0	CP	CP	-	.25	-
17418 XY A	7	.500	7.63	19.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	4	7	4	.75	27.0	12.0	.500	.313	.75	27.0	12.0	.438	.250	.50	22.0	19.0	.438	.438	.50	15.0	13.0	.438	.375	-	-	-
17518 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	10	3	-	-	-	-	-	-	-	.50	17.0	21.0	CP	CP	.50	16.0	11.0	.500	.250	-	-	-	-	-	-	-
17618 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17718 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	3	-	-	-	-	-	-	-	-	-	-	.50	13.0	11.0	.313	.250	.50	13.0	11.0	.188	.188	-	-	-
17019 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	8	-	-	1.00	16.0	28.0	CP	.313	1.00	16.0	28.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-
17119 XY A	8	.625	7.63	19.00	.375		3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	8	8	8	16	1.00	16.0	15.0	CP	CP	1.00	16.0	15.0	CP	CP	1.00	15.0	22.0	CP	CP	1.25	22.0	30.0	CP	CP	.25	.25	-
17219 XY A	3	.375	4.63	21.00	.188	CP	6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	5	8	10	4	.75	22.0	15.0	CP	.438	1.00	22.0	16.0	CP	.500	.75	18.0	23.0	CP	CP	.75	14.0	18.0	.438	.500	.25	-	-
17319 XY A	6	.625	4.63	19.00	.313		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	4	8	5	-	.75	19.0	13.0	.313	.250	.75	22.0	15.0	.500	.375	.75	17.0	16.0	CP	CP	-	-	-	-	-	-	-	-
17419 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	-	-	-	-	-	.75	30.0	14.0	.438	.313	.50	15.0	14.0	.500	.438	-	-	-	-	-	-	-	-
17519 XY A	4	.500	4.63	13.00	.250		5	.500	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	8	4	-	-	-	-	-	-	1.00	15.0	19.0	CP	CP	.75	19.0	13.0	CP	.438	-	-	-	-	-	-	.25	-
17619 XY A	5	.500	4.63	16.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	3	4	10	3	.50	18.0	10.0	.188	.188	.50	21.0	13.0	.313	.188	.50	17.0	29.0	.313	.375	.50	10.0	22.0	.188	.250	-	-	-
17719 XY A	4	.500	4.63	13.00	.250		4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	13.0	11.0	.313	.250	-	-	-
8027 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	16	-	-	-	1.25	22.0	35.0	CP	.313	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
8127 XY A	3	.375	4.63	21.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	8	-	1.00	16.0	16.0	.438	.500	-	-	-	-	-	1.00	18.0	19.0	CP	CP	-	-	-	-	-	.25	-	-
8227 XY A	6	.625	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	1.00	18.0	19.0	CP	CP	-	-	-	-	-	.25	-	-
8327 XY A	6	.500	4.63	19.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	1.00	17.0	16.0	CP	CP	-	-	-	-	-	.25	-	-
8427 XY A	5	.500	4.63	16.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	.75	17.0	16.0	CP	CP	-	-	-	-	-	-	-	-
8527 XY A	5	.625	4.63	16.00	.313		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	8	-	.75	11.0	14.0	.313	.500	-	-	-	-	-	.75	22.0	15.0	CP	.500	-	-	-	-	-	-	-	-
8627 XY A	7	.625	7.63	19.00	.375		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	-	8	-	1.00	23.0	10.0	.313	.313	-	-	-	-	-	1.00	16.0	22.0	CP	CP	-	-	-	-	-	.25	-	-
8727 XY A	4	.500	4.63	13.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	18.0	14.0	.375	.313	-	-	-	-	-	-	-	-
TRUSS T1																																																					
12210 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.25	32.0	44.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	-	.50
12211 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.00	32.0	42.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
12212 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	16	-	-	-	1.25	23.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
12213 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	12.00	.188	CP	-	-	-	-	-	-	12	4	-	-	.75	23.0	29.0	.500	.500	.75	14.0	18.0	.313	.375	-	-	-	-	-	-	-	-	-	-	-	-	.25
12214 XY C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	5	-	-	-	-	-	-	-	.75	15.0	20.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
12215 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	7.63	10.00	.375		-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	1.00	25.0	33.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
12216 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	36	-	-	-	-	-	-	-	1.00	32.0	42.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
12217 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	36	-	-	-	-	-	-	-	1.50	32.0	42.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50
12310 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	32.0	42.0	CP	CP	-	-	.25
12311 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	23.0	31.0	CP	CP	-	-	.25
12312 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.313		-	-	-	12	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	23.0	29.0	.438	.500	-	-	-
12313 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12314 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	4	-	-	-	-	-	-	-	-	-	-	-	.75	14.0	18.0	.313	.375	-	-	-	-	-	-	-	-
12315 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	5	-	-	-	-	-	-	-	-	-	-	-	.75	15.0	20.0	.375	.438	-	-	-	-	-	-	-	-
12316 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	4.63	10.00	.375		-	-	18	-	-	-	-	-	-	-	-	-	-	-	1.00	25.0	33.0	CP	CP	-	-	-	-	-	-	-	-
12317 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	36	-	-	-	-	-	-	-	-	-	-	-	1.00	32.0	42.0	CP	CP	-	-	-	-	-	-	-	-
TRUSS T2																																																					
12228 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.50	32.0	42.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25	.25
12229 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.25	30.0	40.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25	.25
12230 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	18	-	-	-	1.25	26.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
12231 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313		-	-	-	-	-	-	16	-	-	-	.75	21.0	28.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12232 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	7.63	10.00	.375		-	-	-	-	-	-	-	9	-	-	-	-	-	-	-	.75	21.0	37.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
12233 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	1.25	26.0	32.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
12234 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	1.25	30.0	39.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
12235 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	1.50	30.0	40.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.50
12328 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	30.0	39.0	CP	CP	-	-	.25
12329 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	18	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	25.0	33.0	CP	CP	-	-	.25
12330 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.313		-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	21.0	34.0	.438	CP	-	-	-
12331 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12332 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12333 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	4.63	10.00	.375		-	-	9	-	-	-	-	-	-	-	-	-	-	-	.75	20.0	34.0	CP	CP	-	-	-	-	-	-	-	-
12334 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	18	-	-	-	-	-	-	-	-	-	-	-	1.25	25.0	33.0	CP	CP	-	-	-	-	-	-	-	.25
12335 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	32	-	-	-	-	-	-	-	-	-	-	-	1.25	30.0	39.0	CP	CP	-	-	-	-	-	-	-	.25
TRUSS T3																																																					
6205 XY A	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12237 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.25	32.0	43.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
12238 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.00	30.0	40.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
12239 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	12	-	-	-	1.25	19.0	25.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
12240 XY C	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	6	4	-	-	.50	15.0	19.0	.375	.438	.50	14.0	14.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-
12241 XY C	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	-	5	-	-	-	-	-	-	-	.75	15.0	20.0	.375	.438	-	-	-	-	-	-	-	-	-	-	-	-	-
12242 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	7.63	10.00	.375		-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	.75	25.0	33.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
12243 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	1.00	30.0	40.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
12244 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	36	-	-	-	-	-	-	-	1.25	32.0	42.0	CP	CP	-	-	-	-	-	-	-	-	-	-	.25	.50
12337 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	30.0	39.0	CP	CP	-	-	.25
12338 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	12	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.25	19.0	26.0	CP	CP	-	-	.25
12339 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	15.0	19.0	.375	.438	-	-	-
12340 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12341 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	4	-	-	-	-	-	-	-	-	-	-	-	.50	14.0	19.0	.188	.188	-	-	-	-	-	-	-	-
12342 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	5	-	-	-	-	-	-	-	-	-	-	-	.75	15.0	20.0	.375	.438	-	-	-	-	-	-	-	-
12343 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	4.63	10.00	.375		-	-	18	-	-	-	-	-	-	-	-	-	-	-	.75	25.0	33.0	CP	CP	-	-	-	-	-	-	-	-
12344 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	32	-	-	-	-	-	-	-	-	-	-	-	1.00	30.0	39.0	CP	CP	-	-	-	-	-	-	-	.25
TRUSS T4																																																					
12255 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	10	-	-	-	1.00	17.0	19.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
12257 XY C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	16	-	-	-	-	-	-	-	.50	25.0	29.0	.438	.438	-	-	-	-	-	-	-	-	-	-	-	-	-
10205 XY B	3	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12355 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12357 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
10305 XY B	2	1.000	-	-	.625		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	.50	29.0	28.0	.438	.500	-	-	-	-	-	-	-	-
TRUSS T5																																																					
13255 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	16	-	-	-	1.00	23.0	31.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
13256 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	16	-	-	-	.75	23.0	31.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
13257 XY C	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	8	6	-	-	.50	17.0	23.0	.313	.313	.75	17.0	19.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-
13258 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	1.00	41.0	30.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
10209 XY B	3	.500	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
13355 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	4.63	10.00	.375		-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	23.0	31.0	CP	CP	-	-	-
13356 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	17.0	23.0	.313	.313	-	-	-
13357 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
13358 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	6	-	-	-	-	-	-	-	-	-	-	-	.75	17.0	24.0	.438	.500	-	-	-	-	-	-	-	-
10309 XY B	2	1.125	-	-	.750		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	1.00	43.0	30.0	CP	CP	-	-	-	-	-	-	-	-
TRUSS T6																																																					
14255 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.00	32.0	42.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
14256 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	18	-	-	-	1.00	25.0	33.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
14257 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313		-	-	-	-	-	-	16	-	-	-	.75	23.0	31.0	.500	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14258 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	12.00	.188	CP	-	-	-	-	-	-	4	16	-	-	.75	14.0	14.0	.375	.438	.75	23.0	31.0	.500	.500	-	-	-	-	-	-	-	-	-	-	-	-	.25
14259 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	18	-	-	-	-	-	-	-	1.00	25.0	33.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
14260 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	1.00	29.0	45.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	.25
14355 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	18	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	25.0	33.0	CP	CP	-	-	.25
14356 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.313		-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.75	23.0	31.0	.500	CP	-	-	-
14357 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	14.0	19.0	.250	.313	-	-	-
14358 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
14359 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	4.63	10.00	.313		-	-	16	-	-	-	-	-	-	-	-	-	-	-	.75	23.0	31.0	.500	.500	-	-	-	-	-	-	-	-
14360 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	18	-	-	-	-	-	-	-	-	-	-	-	1.00	25.0	33.0	CP	CP	-	-	-	-	-	-	-	.25
10314 XY B	2	1.000	-	-	.688		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	1.00	36.0	37.0	CP	CP	-	-	-	-	-	-	-	-
TRUSS T7																								

BRACED FRAME AND TRUSS CONNECTION SCHEDULE

JOINT PL QT	LEFT BEAM SHEAR PLATE NBL	SPTL	SPWL	SPLL	WLDL	BWEL	RIGHT BEAM SHEAR PLATE NBR	SPTR	SPWR	SPLR	WLDR	BWER	UPPER COLUMN SHEAR PLATE NBU	SPTU	SPWU	SPLU	WLDU	CWEU	LOWER COLUMN SHEAR PLATE NBD	SPTD	SPWD	SPLD	WLDD	CWED	BRACE BOLTS BUL	BUR	BLL	BLR	UP-LEFT GUSSET PLATE GTUL	GWUL	GLUL	GWCUL	GWBUL	UP-RIGHT GUSSET PLATE GTUR	GWUR	GLUR	GWCUR	GWBUR	DN-LEFT GUSSET PLATE GTLL	GWLL	GLLL	GWCLL	GWBLL	DN-RIGHT GUSSET PLATE					DOUBLER PLATES		
15355 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	32.0	41.0	CP	CP	-	-	.25
15356 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	24.0	31.0	CP	CP	-	-	.25
15357 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	14.0	18.0	.188	.188	-	-	-
15358 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
15359 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	4	-	-	-	-	-	-	-	-	-	-	-	-	.50	13.0	17.0	.250	.313	-	-	-	-	-	-	-	-
15360 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	4	.625	4.63	10.00	.375		-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	.75	23.0	31.0	CP	CP	-	-	-	-	-	-	-	-
15361 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	18	-	-	-	-	-	-	-	-	-	-	-	-	1.25	25.0	33.0	CP	CP	-	-	-	-	-	-	-	.25
10319 XY B	2	1.625	-	-	1.125		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	-	1.50	39.0	31.0	CP	CP	-	-	-	-	-	.25	-	-
TRUSS T8																																																						
16255 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.50	30.0	40.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
16256 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	32	-	-	-	1.50	30.0	40.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50
16257 XY C	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	36	-	-	-	1.00	32.0	40.0	CP	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
16258 XY C	-	-	-	-	-	-	-	-	-	-	-	-	4	.500	7.63	10.00	.313		-	-	-	-	-	-	16	4	-	-	.50	24.0	30.0	.375	.438	.50	14.0	14.0	.188	.188	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.25
16259 XY C	-	-	-	-	-	-	-	-	-	-	-	-	3	.375	4.63	10.00	.250		-	-	-	-	-	-	-	6	-	-	-	-	-	-	-	.50	18.0	22.0	.313	.375	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
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10223 XY B	3	.500	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
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16356 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2	.375	4.63	11.00	.188	CP	-	-	-	36	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.00	32.0	40.0	CP	CP	-	-	.25
16357 XY C	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	3	.500	4.63	10.00	.250		-	-	-	16	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	.50	24.0	30.0	.375	.438	-	-	-
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TRUSS T9																																																						
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16506 XY A	2	.375	4.63	11.00	.188	CP	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	10	-	-	-	-	-	-	-	-	-	-	-	-	.50	18.0	38.0	.375	CP	-	-	-	-	-	-	-	-
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3523 XY A	2	.375	4.63	11.00	.188	CP	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	32	-	-	-	-	-	-	-	-	-	-	-	-	1.25	27.0	55.0	CP	CP	-	-	-	-	-	.25	-	.25

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.

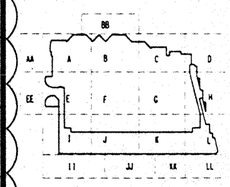
Consulting Structural and Civil Engineers

1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 623-1201

JOHN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11775
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision is exercised in Title 16, Chapter 18 Section 1-24 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	6-19-95		

Sheet Title

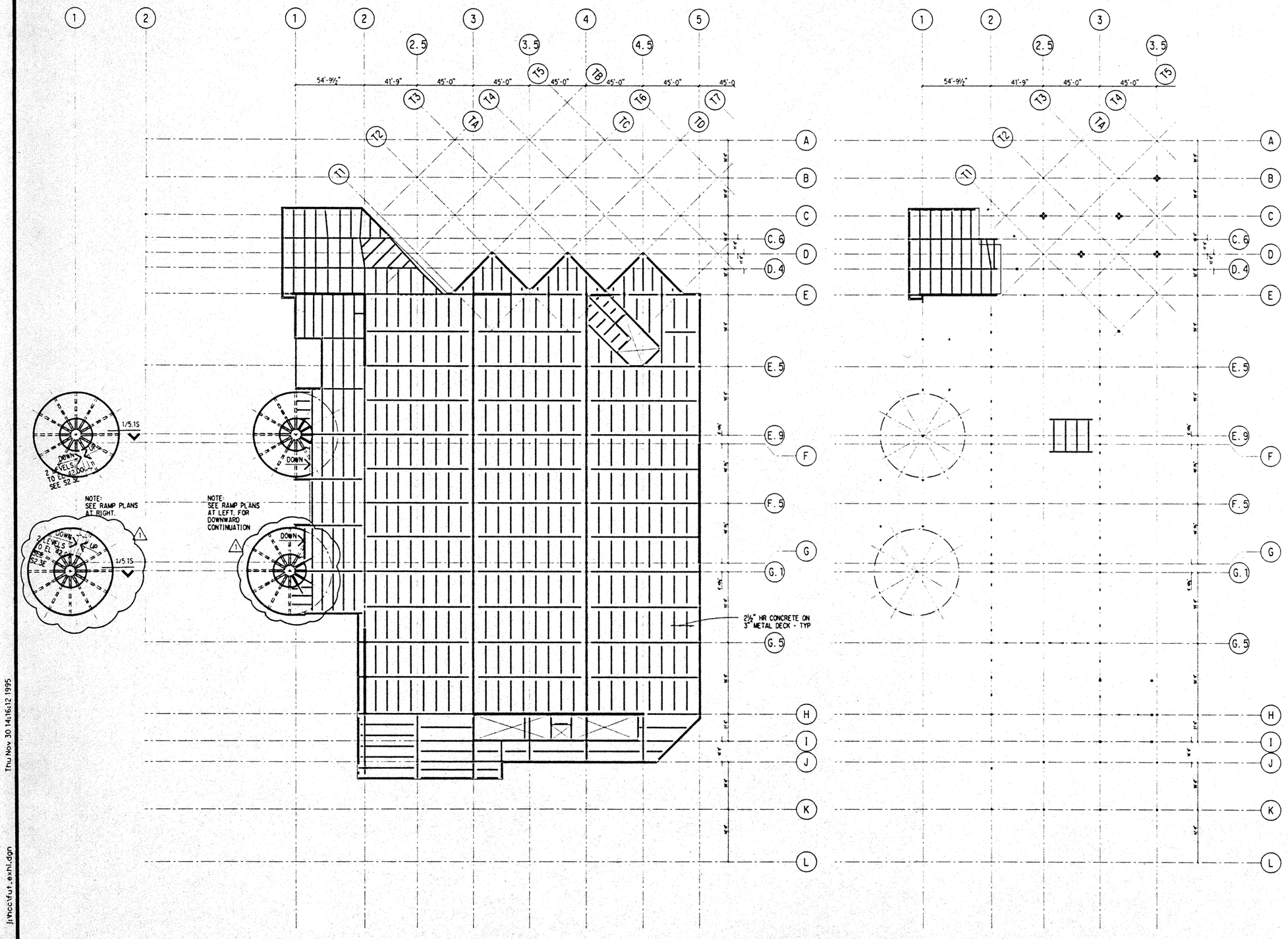
CONNECTION SCHEDULE

Designed BJM
Drawn DMS
Checked JDM
Date 5 / 8 / 95
Job No. 93036.02.9

Sheet No.

S6.16

Thu Nov 30 14:13:32 1995
j:\hacbr\frsc12.dgn
DMS pit



FUTURE EXHIBITION HALL FRAMING PLAN (NIC) EL 100'-6"

0 16' 32' 64' 96'

FUTURE MEETING FRAMING PLAN (NIC) EL 77'-6"

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii
in association with
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Seattle, Washington

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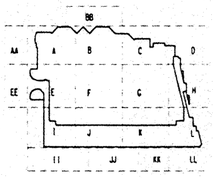
(206) 292-1200 FAX: (206) 623-1201

JON D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 11773
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervisor(s) defined in Title 16 Chapter 18 Section 1.3 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET
1	8-4-95		

Sheet Title

FUTURE EXHIBIT HALL FRAMING PLAN (NIC)

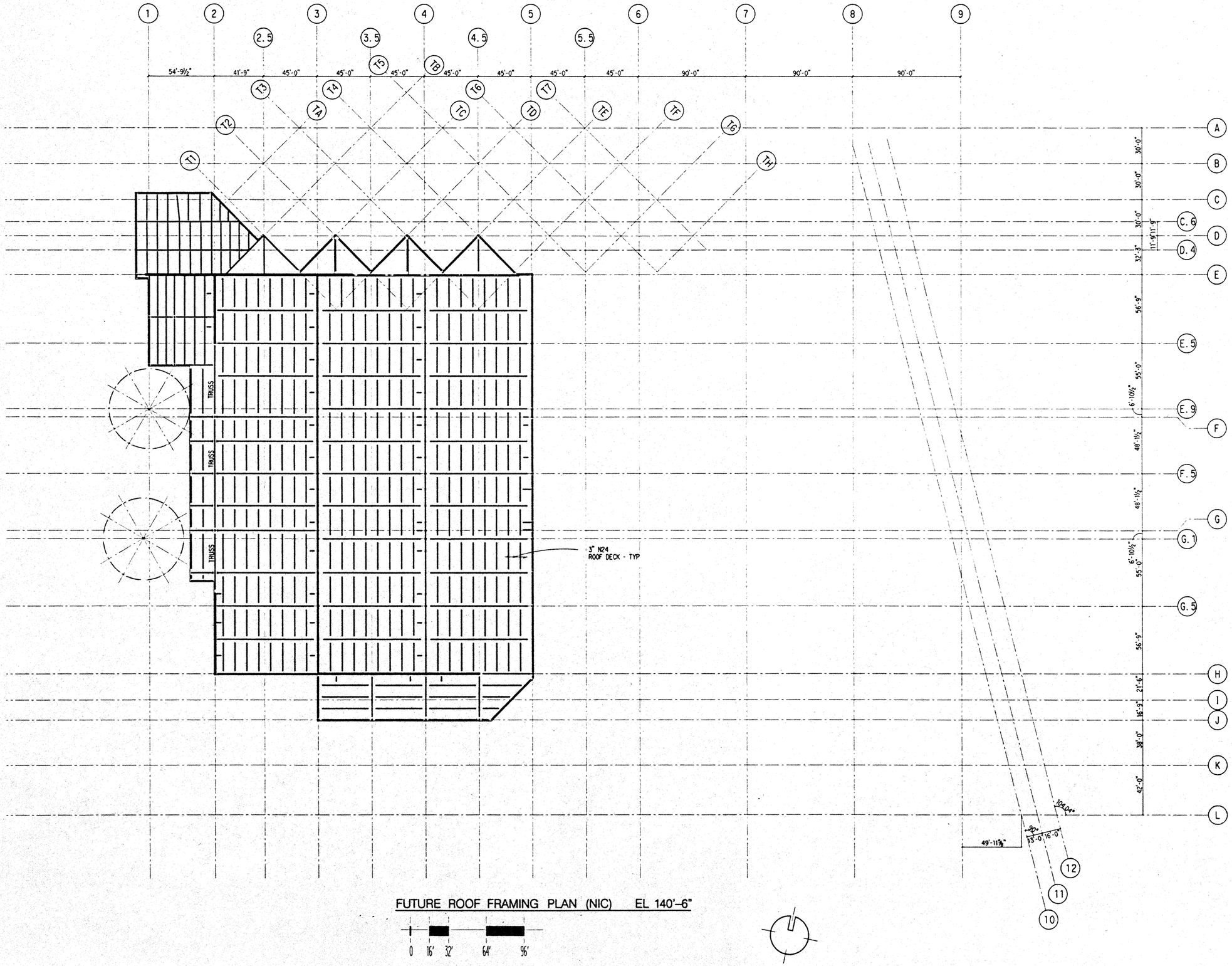
Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S9.1

Thu Nov 30 14:16:12 1995

j:\hccfut_exhi.dgn

DWG plt



FUTURE ROOF FRAMING PLAN (NIC) EL 140'-6"

0 16' 32' 64' 96'

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

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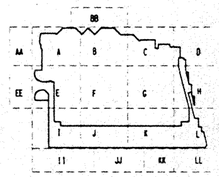
SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX: (206) 623-1201

DAN D. MAGNUSSON
LICENSED PROFESSIONAL ENGINEER
NO. 7177S
HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervision Verified as Title or Owner's Designee 1.3.4 of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

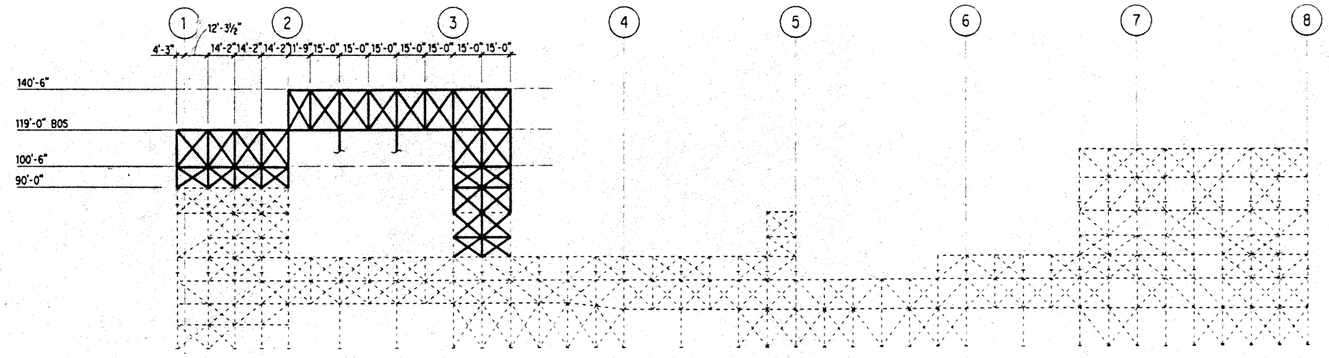
FUTURE ROOF FRAMING PLAN (NIC)

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9

Sheet No.

S9.2

j:\hccvfut_roof.dgn Thu Nov 30 14:18:27 1995 DMS plt



FRAME ON GRID E

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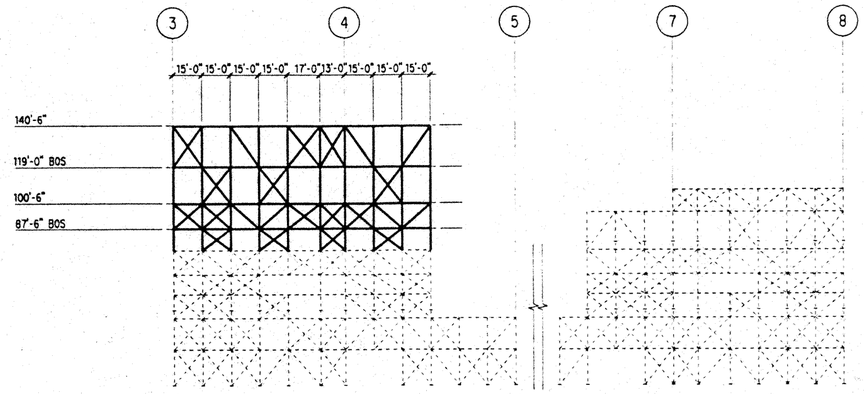

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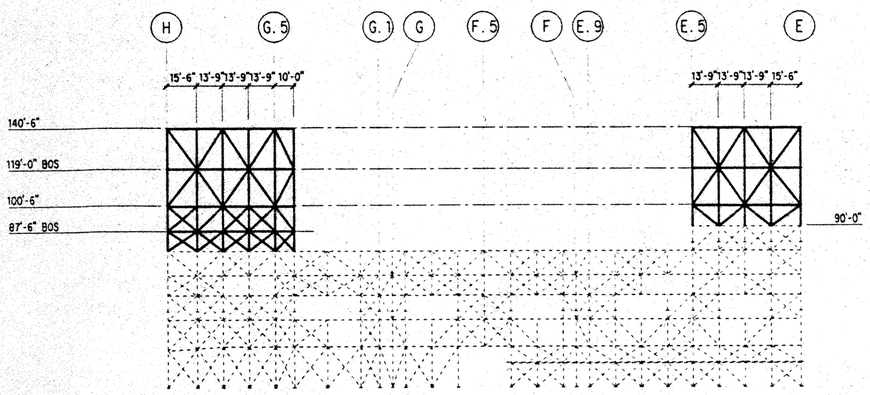

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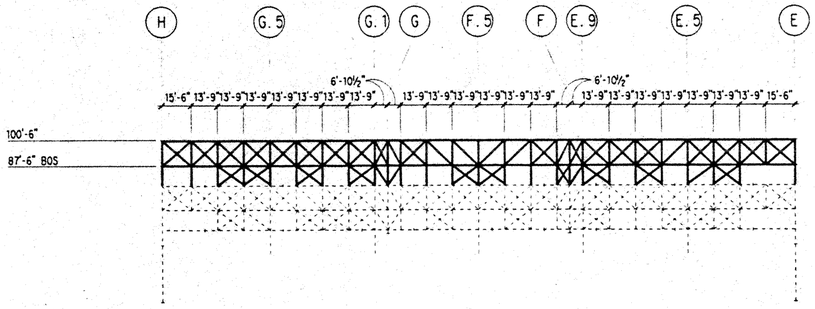
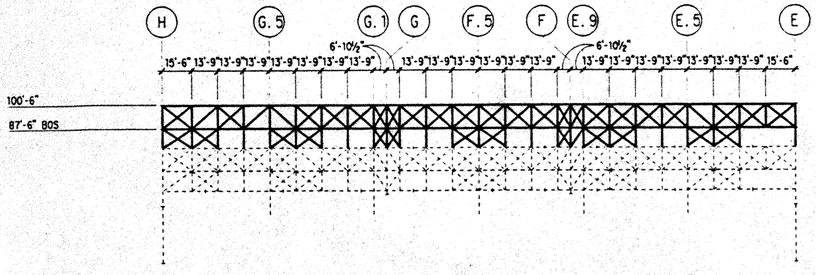


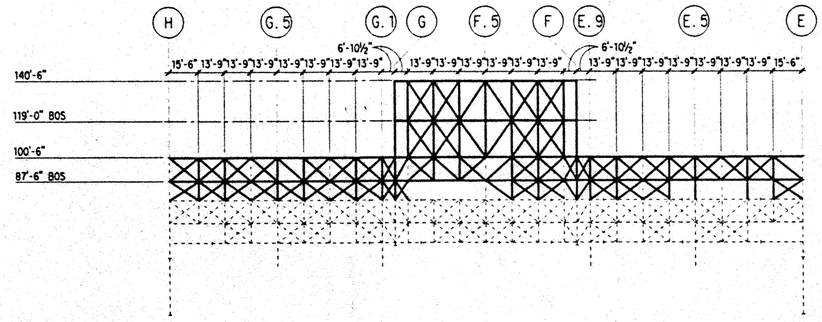
TRUSS ON GRID 3

SCALE: 1/32" = 1'-0"



TRUSS ON GRID 4

SCALE: 1/32" = 1'-0"



TRUSS ON GRID 5

SCALE: 1/32" = 1'-0"

Hawai'i Convention Center

Convention Center Authority
Honolulu, Hawaii

Wimberly Allison Tong & Goo, Inc.
Honolulu, Hawaii

in association with

Loschky Marquardt & Nesholm
Seattle, Washington

NORDIC /PCL
A Joint Venture
Honolulu, HI

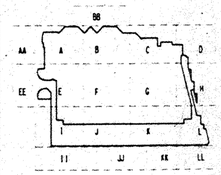
Consultants

SKILLING WARD MAGNUSSON BARKSHIRE INC.
Consulting Structural and Civil Engineers
1301 Fifth Avenue, Suite 3200 Seattle WA 98101-2699
(206) 292-1200 FAX (206) 292-1201

LICENSED PROFESSIONAL ENGINEER NO. 11773 HAWAII, U.S.A.

This work was prepared by me or under my supervision. Construction of this project will be under my observation.

Supervisor as defined in Title 16 Chapter 18 Subpart 1.3-H of the Rules and Regulations of the Board of Professional Engineers, Architects, and Surveyors of the State of Hawaii.



Revisions

No.	Date	By	Description
	12-15-95		FINAL DOCUMENTS
	9-15-95		BUILDING PERMIT SET

Sheet Title

FUTURE TRUSS & BRACED FRAME ELEVATIONS (NIC) – GRIDS 2, 3, 4, 5, E, & H

Designed BJM
Drawn DMS
Checked JDM
Date 4 / 12 / 95
Job No. 93036.02.9
Sheet No.

S9.3

Sat Dec 2 09:41:06 1995

j:\hccfut_trus.dgn